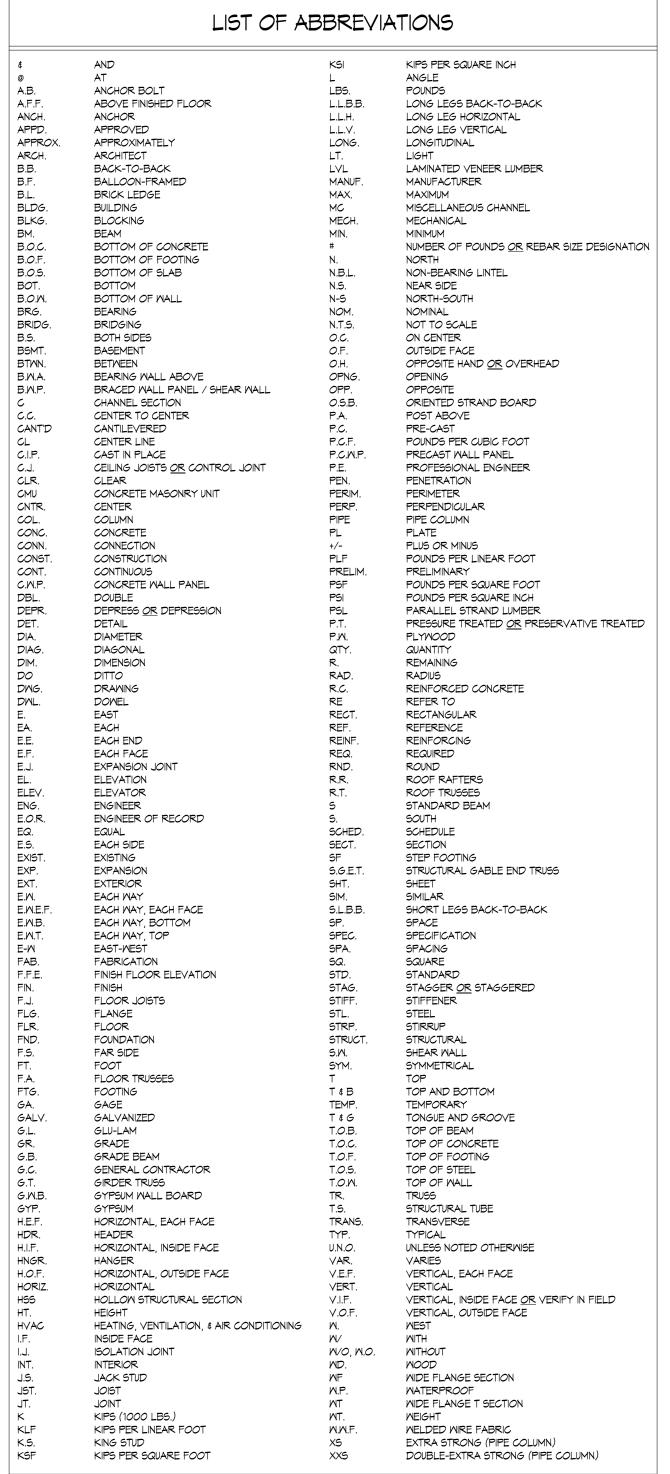
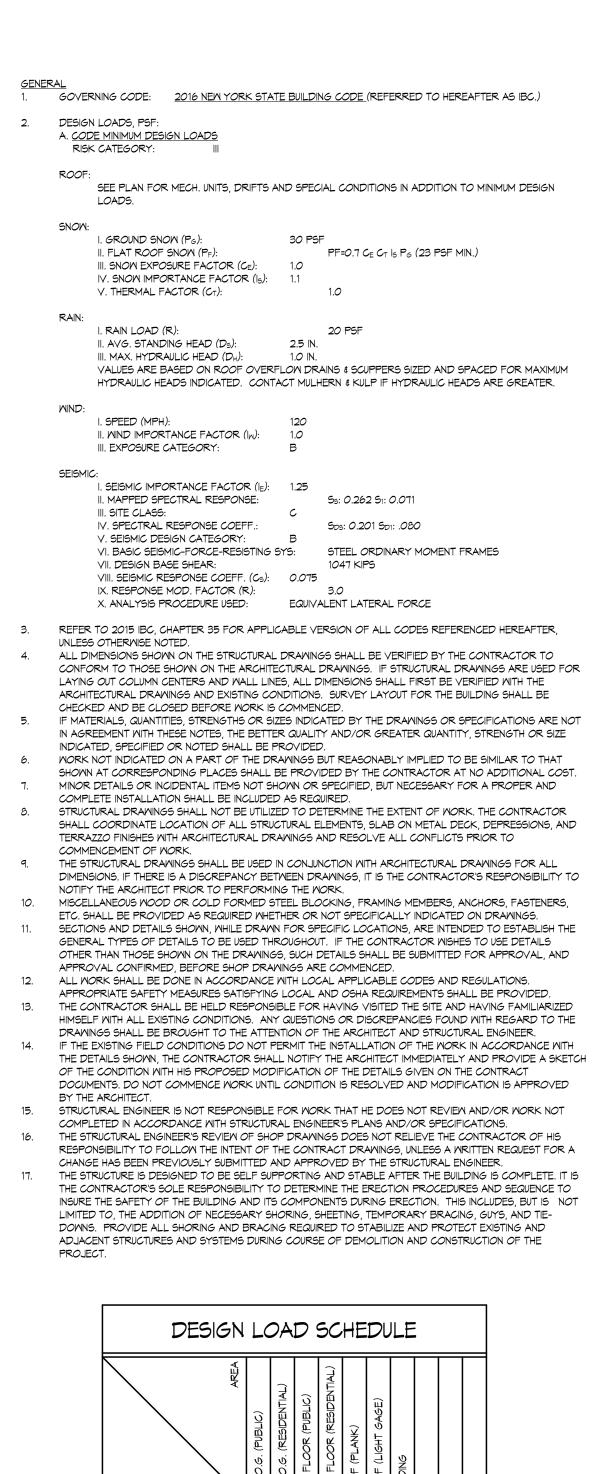
General Notes & Specifications

BRAEMAR AT MONTEBELLO ASSISTED LIVING FACILITY

ROCKLAND COUNTY, NEW YORK



SSM	A MEMBER N	NOMENCLAT	ΓURE	
ME	EMBER DEP	TH SCHEDU	LE	
MEMBER DEP	ΓΗ (WEB SIZE)	SSMA DES	SIGNATION	
2-1	2"	2	50	
3-9	; " 8	3	62	
6	"	6	00	
8	"	8	00	
10)"	10	000	
12)" -	12	200	
M	EMBER TYP	E SCHEDUL	.E	
MEMBER TYPE	SSMA DESIGNA	TION FI	FLANGE WIDTH	
	S137		1-38"	
	S162		1-5/8"	
	S200		2"	
STUDS	S250		2-1/2"	
	S300		3"	
	S350		3-1/2"	
TRACKS	T125		1-1/4"	
	T200		2"	
MA	TERIAL THI	CKNESS/GA	GE	
MILLS	GAGE	DESIGN THK.	MIN THK.	
33	20	0.0346"	0.0329"	
43	18	0.0451"	0.0428"	
54	16	0.0566"	0.0538"	
68	14	0.0713"	0.0677"	
07	10	0.4047"	0.0060"	



AREA	" S.O.G. (PUBLIC)	" S.O.G. (RESIDENTIAL)	TYP. FLOOR (PUBLIC)	TYP. FLOOR (RESIDENTIAL)	ROOF (PLANK)	ROOF (LIGHT GAGE)	-ANDING	
COMPONENT CONCRETE SLAB	50 - 4	50	_	_	<u>α</u>	ı⊻ —		
E8 PLANK (ROOF)	50	50			54			
H10 PLANK (FLOOR)			71	71	24			
8" SOLID PLANK				- 11			100	
PARTITIONS		10		10				
FLOOR FINISH	2	2	2	2			2	
CEILING FINISH			2	2	2		2	
GYPCRETE			10	10	_		_	
ROOFING / INSULATION					გ			
MECH. / ELEC. / PLUMBING			3	3	5			
MISC.			3	3	5			
LG ROOF						25		
TOTAL DEAD LOAD	52	62	91	101	74	25	104	
TOTAL LIVE LOAD	100*	40*	100*	40*	20	20	100*	
TOTAL LOAD	152	102	191	141	94	45	204	

FOUNDATIONS SHALL BEAR ON UNDISTURBED VIRGIN SOIL AND/OR CONTROLLED COMPACTED FILL MATERIAL PROVIDING A BEARING PRESSURE OF 4000 PSF MINIMUM, BASED ON A SUBSURFACE EXPLORATION PROGRAM CARRIED OUT BY SESI CONSULTING ENGINEERS AND DESCRIBED IN REPORT NO. 9403, DATED 5/16/16. ALL EARTHWORK AND SUBGRADE PREPARATION SHALL BE EXECUTED AS PER THE RECOMMENDATIONS DESCRIBED IN THIS REPORT. THE SLAB-ON-GRADE DESIGN WAS BASED ON ACHIEVING A WESTERGAARD MODULUS OF SUBGRADE REACTION, K, EQUAL TO OR BETTER THAN 100 PCI. IN ADDITION, ALL FOUNDATION WALLS BELOW GRADE WERE DESIGNED FOR A SOIL FRICTION ANGLE OF 34 DEGREES. ALL REQUIREMENTS FOR SITE PREPARATION AND SOIL COMPACTION SPECIFIED IN THE SOILS REPORT SHALL BE FOLLOWED UNLESS ADDITIONAL MORE STRINGENT REQUIREMENTS ARE SPECIFIED. THE SERVICES OF A GEOTECHNICAL ENGINEER OR APPROVED TESTING AGENCY SHALL BE RENDERED TO VERIFY THAT THE SUBSURFACE SITE CONDITIONS MEET THE DESIGN PARAMETERS NOTED ABOVE. NOTIFY ARCHITECT OR STRUCTURAL ENGINEER IF FOUNDATION CONDITIONS ENCOUNTERED DIFFER FROM SOILS EXPLORATION INFORMATION MADE AVAILABLE TO THE CONTRACTOR. CONDITIONS THAT DO NOT MEET THE MINIMUM STANDARDS CITED ABOVE WILL RENDER THIS FOUNDATION AND SLAB DESIGN VOID, IN WHICH CASE THE STRUCTURAL ENGINEER SHALL BE CONTACTED TO PROVIDE NEW FOUNDATION DESIGN. FOOTINGS ARE TO BEAR AT LOWEST OF FOLLOWING REQUIREMENTS: ELEVATIONS NOTED ON DRAWINGS. SOIL SUITABLE FOR DESIGN BEARING PRESSURE, AS DETERMINED BY GEOTECHNICAL ENGINEER. FROST DEPTH (AS DETERMINED BY LOCAL BUILDING DEPARTMENT) WITH RESPECT TO FINISH GRADE. SLOPE OF 1 VERTICAL TO 2 HORIZONTAL FROM NEAREST ADJACENT FOUNDATION. PROTECT ALL EXISTING UNDERGROUND UTILITIES WITHIN WORK AREAS. CONSULT EXISTING MECHANICAL DRAWINGS RELEVANT TO SUCH UTILITIES. BACKFILL SHALL BE GRANULAR MATERIAL APPROVED BY GEOTECHNICAL ENGINEER, DEPOSITED AND MACHINE COMPACTED IN 8-INCH MAXIMUM LAYERS. COMPACTION SHALL HAVE A MINIMUM OF 95% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT, IN ACCORDANCE WITH ASTM D698 (STANDARD PROCTOR) EXCAVATE ALL FOUNDATIONS TO REASONABLY EXACT OUTLINE AND DEPTH, AVOIDING OVER-EXCAVATION AND CAVE-IN OF SURROUNDING MATERIALS AFTER SLAB SUBGRADE WORK IS COMPLETE. BOTTOMS OF ALL FOUNDATIONS SHALL BE DRY AND LEVEL PRIOR TO POURING. PROTECT SUBGRADE UNDER ALL FOOTINGS AND SLABS ON GRADE FROM FREEZING DURING CONSTRUCTION. NO FILL OR BACKFILL SHALL BE PLACED AGAINST RETAINING OR FOUNDATION WALLS UNTIL GROUT OR CONCRETE HAS ATTAINED DESIGN STRENGTH AND SUPPORTING MEMBERS ARE IN PLACE, UNLESS PRIOR WRITTEN APPROVAL IS OBTAINED FROM STRUCTURAL ENGINEER. ALL CONCRETE WORK SHALL BE IN CONFORMANCE WITH ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", AND ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE". CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315, "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT", UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS. ALL CONCRETE SHALL BE READY MIX AND DESIGNED IN ACCORDANCE WITH ACI 301. DESIGN MIXES AND ADMIXTURES SHALL BE SUBMITTED FOR APPROVAL. CONCRETE SHALL ATTAIN THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: FOOTINGS AND GRADE BEAMS: FOUNDATION WALLS: SLABS ON GRADE: CAST-IN-PLACE STRUCTURAL SLABS: 4000 PSI CAST-IN-PLACE BEAMS, COLUMNS, AND PIERS: 4000 PSI ALL EXPOSED CONCRETE: ALL OTHER CONCRETE, U.O.N.: ALL CONCRETE SHALL HAVE: A SLUMP OF 4" (PLUS OR MINUS 1"), 2 TO 4 PERCENT AIR ENTRAINMENT, AND A MAX. WATER/CEMENT RATIO OF 0.50. PROVIDE 4-6 PERCENT AIR ENTRAINMENT FOR ALL EXPOSED CONCRETE. SUBMIT SHOP DRAWINGS TO STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL INCLUDING FULI INFORMATION FOR PLACING ALL REINFORCING, WITHOUT REFERENCE TO THE DESIGN DRAWINGS. ALL CONCRETE REINFORGING BARS SHALL BE FROM BILLET STEEL IN ACCORDANCE WITH ASTM A615 GRADE 60. ALL WELDED WIRE FABRIC SHALL BE ASTM A185. WWF SHALL BE LAPPED AT LEAST 8 INCHES AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 8 INCHES. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. CONCRETE EXPOSED TO EARTH OR WEATHER, #6 THROUGH #18 BARS. CONCRETE EXPOSED TO EARTH OR WEATHER. #5 BAR AND SMALLE 1 1/2" CONCRETE NOT EXPOSED TO MEATHER OR IN CONTACT WITH EARTH - FOR THE PRIMARY REINFORCEMENT, TIES, STIRRUPS, AND SPIRALS IN BEAMS AND COLUMNS 3/4" CONCRETE NOT EXPOSED TO WEATHER NOR IN CONTACT WITH EARTH - FOR SLABS, WALLS, AND JOISTS, #11 BAR AND SMALLER. PROVIDE CORNER BARS TO MATCH SIZE AND SPACING OF HORIZONTAL REINFORCING AT CORNERS OF ALL CONCRETE WALL, FOOTING AND GRADE BEAM CONSTRUCTION. CORNER BARS SHALL LAP HORIZONTAL REINFORCEMENT A MINIMUM OF 48 BAR DIAMETERS, U.N.O. CONTRACTOR SHALL PROVIDE SPACERS, CHAIRS, BOLSTERS, ETC. AS NECESSARY TO SUPPORT REINFORCING STEEL. SUPPORT ITEMS WHICH BEAR ON EXPOSED CONCRETE SURFACES SHALL HAVE ENDS WHICH ARE PLASTIC TIPPED OR STAINLESS STEEL. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS AND AT SLABS EDGES. MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE 48 BAR DIAMETERS, EXCEPT WHERE OTHERWISE NOTED ON THE DRAWINGS. FOR BEAMS AND ELEVATED SLABS, LAP BOTTOM STEEL AT THE SUPPORT AND TOP STEEL OVER THE MIDSPAN, UNLESS OTHERWISE NOTED. REFER TO TYPICAL DETAILS FOR SPECIFICATIONS ON CONTROL JOINTS, CONSTRUCTION JOINTS, AND HORIZONTAL KEYWAYS IN CONSTRUCTION JOINTS SHALL BE PROVIDED IN BEAMS, SUPPORTED SLABS, AND WALL FOOTINGS WITH A DEPTH OF 1 1/2" AND HEIGHT EQUAL TO ONE THIRD OF THE MEMBER'S DEPTH. REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS UNLESS OTHERWISE NOTED ON THE DRAWINGS. CONSTRUCTION JOINTS MAY BE USED ONLY AT LOCATIONS SHOWN ON THE DRAWINGS OR AT OTHER LOCATIONS APPROVED BY THE STRUCTURAL ENGINEER. CONSTRUCTION JOINTS IN GRADE BEAMS SHALL BE LOCATED WITHIN MIDDLE THIRD OF SPANS WITH ALL REINFORCEMENT PASSING THROUGH JOINTS. JOINTS SHALL BE BULKHEADED AND PROVIDED WITH HORIZONTAL SHEAR KEYS AT 1/3 POINTS. ROUGHEN CONSTRUCTION JOINT SURFACES OF CONCRETE TO STRUCTURALLY SUPPORTED SLABS ON GRADE BEAMS SHALL HAVE CONSTRUCTION JOINTS LOCATED WITHIN MIDDLE THIRD OF SPANS WITH ALL REINFORCEMENT PASSING THROUGH JOINTS. PROVIDE JOINTS WITH BULKHEADS HAVING CONTINUOUS CHAMFERED SHEAR KEYS. 17. COMPOSITE CONCRETE DECKS SHALL BE LIMITED TO POUR AREAS NOT TO EXCEED 3600 SQUARE FEET. CONSTRUCTION JOINTS SHALL BE LOCATED AT 1/3 POINTS OF GIRDERS AND AT MIDSPACING OF BEAMS WITH MELDED WIRE FABRIC REINFORCING BARS PASSING THROUGH THE BULKHEADS. SUBMIT LOCATIONS TO STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO PLACING CONCRETE. 18. ALL CONCRETE, INCLUDING FOUNDATION WORK, IS TO BE VIBRATED. VIBRATORS SHALL NOT BE USED TO TRANSPORT CONCRETE. 19. CONCRETE SHALL BE PLACED IN ACCORDANCE WITH THE RECOMMENDATIONS OF ACI COMMITTEE 304. CONCRETE SHALL NOT BE SUBJECT TO DROPS IN EXCESS OF 5 FEET. CONDUITS, PIPES AND SLEEVES SHALL NOT BE LARGER THAN 1/3 OVERALL THICKNESS OF SLAB, WALL OR BEAM IN WHICH THEY ARE EMBEDDED UNLESS OTHERWISE NOTED ON DRAWING OR APPROVED BY STRUCTURAL ENGINEER. INSERTS SHALL NOT BE PLACED CLOSER THAN 3 DIAMETERS OR WIDTHS ON CENTER. REFER TO ACI 318 AND PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. ALL INSERTS ARE TO BE REVIEWED BY ENGINEER PRIOR TO INSTALLATION AND PLACEMENT OF CONCRETE. 21. CONTRACTOR SHALL REVIEW ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF OPENINGS, INSERTS, EMBEDDED ITEMS, SLEEVES, SLAB DEPRESSIONS, SLOPES, ETC., AS REQUIRED BY OTHER TRADES. THESE ITEMS SHALL BE FURNISHED AND INSTALLED PRIOR TO PLACEMENT OF CONCRETE. ALL ANCHOR BOLTS SHALL BE IN PLACE PRIOR TO POURING CONCRETE CONTRACTOR SHALL PROVIDE 3/4 INCH CHAMFER ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS, AND WALLS UNLESS OTHERWISE INDICATED ON THE ARCHITECTURAL DRAWINGS. SLABS ON GRADE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE TYPICAL SLAB DETAILS INDICATED ON THE CONSTRUCTION DOCUMENTS: 25. PROVIDE 6 INCHES CRUSHED STONE UNDER ALL SLAB-ON-GRADE LOCATIONS UNLESS OTHERWISE NOTED IN 26. POROUS FILL FOR SLABS SHALL BE A UNIFORMLY GRADED MEDIUM COURSE STONE AGGREGATE TO PROVIDE, WHEN COMPACTED, A LEVEL, STABLE AND WELL DRAINING SUB-BASE FOR THE SLAB. USE #57 CRUSHED NATURAL STONE OR EQUAL PRIOR TO POURING FLOOR SLABS, REFER TO THE CONSTRUCTION DOCUMENTS FOR ADDITIONAL WORK TO 28. AFTER ALL UNDER-SLAB WORK HAS BEEN INSTALLED, CONTRACTOR SHALL FIELD CONFIRM THE DENSITY OF THE SOIL. ANY SOFT, PUMPING, OR OTHERWISE UNSTABLE OR UNSUITABLE SUBGRADE SOIL THUS DETECTED SHALL BE UNDERCUT AND REPLACED WITH SUITABLE FILL PLACED AND COMPACTED AS DIRECTED BY GEOTECHNICAL ENGINEER. ANY AREAS WHERE THE COMPACTED SUB-GRADE IS DEPRESSED BY MORE THAN 2" SHALL BE FILLED WITH SUITABLE MATERIAL AND RE-COMPACTED 29. PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND AT OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP. PROVIDE 1/2 INCH PREFORMED EXPANSION JOINTS IN SLABS WHERE INDICATED. REFER TO TYPICAL SLAB WHERE INTERIOR SLABS ABUT MASONRY OR CONCRETE WALLS, THE SLAB SHALL BE THICKENED TO δ^{\shortparallel} MINIMUM ADJACENT TO WALL. PROVIDE 1/2 INCH THICK PRE-MOLDED JOINT FILLER ALL AROUND SLABS-ON-GRADE WHERE ABUTTING CONCRETE/MASONRY WALLS, UNLESS OTHERWISE NOTED. COLUMNS SHALL BE ISOLATED FROM THE FLOOR SLAB WITH FULL CONSTRUCTION JOINTS AND COMPRESSIBLE

MATERIAL. SLAB BLOCK-OUTS AROUND COLUMNS SHALL BE DIAMOND OR CIRCULAR IN SHAPE, AND OF A

33. RAMPS, SLOPING SLABS, STEPS, AND SLABS EXPOSED TO MEATHER SHALL RECEIVE A LIGHT BROOMED

34. LOCATE WELDED WIRE FABRIC 1-1/2 INCHES BELOW TOP OF SLAB.

ALL PRECAST/PRESTRESSED WORK SHALL BE IN CONFORMANCE WITH PRECAST/PRESTRESSED CONCRETE INSTITUTE AND THE REQUIREMENTS OF ACI 318 AND THE PROJECT STANDARDS AND SPECIFICATIONS. PRECAST CONCRETE PLANK SHALL BE PRETENSIONED, HOLLOW CORE, FLAT SLABS BY A RECOGNIZED MANUFACTURER, DESIGNED FOR SUPERIMPOSED LOADS LISTED INCLUDING PARTITIONS. REFER TO THE ARCHITECTURAL DRAWINGS FOR NON-BEARING PARTITIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS REFER TO STRUCTURAL DRAWINGS FOR SIZE AND SPANNING DIRECTION OF PRECAST MEMBERS. PROVIDE SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PROPOSED CONSTRUCTION WITH FULL INFORMATION FOR PLACING ALL REINFORCING WITHOUT REFERENCE TO DESIGN DRAWINGS ALL OPENINGS IN PRECAST CONCRETE MUST BE PROVIDED BY OR APPROVED IN WRITING BY THE PRECAST MANUFACTURER. NO REINFORCEMENT IN PRECAST CONCRETE IS TO BE CUT WITHOUT PRIOR APPROVAL OF PRECAST MANUFACTURER THE PRECAST MANUFACTURER SHALL COORDINATE ALL OPENINGS IN PRECAST MEMBERS WITH THE ARCHITECTURAL AND MECHANICAL DRAWINGS. THE MANUFACTURER SHALL SUPPLY HEADERS, HANGERS, INSERTS ATTACHMENTS AND APPURTENANCES AS REQUIRED AT OPENINGS. GENERAL CONTRACTOR IS TO PROVIDE OPENINGS FOR ALL DUCTS AND PIPES PENETRATING PLANK. ALL GROUT KEYS SHALL BE PROPERLY FILLED WITH A MINIMUM 3000 PSI GROUT FOR FULL LENGTH AND PROPERLY TIED INTO BEARING POINTS AS DETAILED IN DRAWINGS. FABRICATE PLANK TO A LENGTH TOLERANCE OF \pm ½ INCH. NO FIELD CUTTING OF PLANK FOR OPENINGS WILL BE PERMITTED. FOR OPENINGS UP TO 8 INCHES IN DIAMETER FIELD DRILLING OR CORING MAY BE ALLOWED PROVIDED THAT ALL SUCH OPENINGS BE MARKED AND APPROVED BY THE PRECAST CONTRACTOR PRIOR TO ANY DRILLING OR CORING. ALL OPENINGS OVER 8 INCHES IN DIAMETER MUST BE SHOP-FORMED OR FRAMED. (SEE NOTES 5, 6 & 11) PRECAST MANUFACTURER TO SELECT PLANK REINFORCEMENT TO SUPPORT ALL DEAD AND LIVE LOADS WITH PRECAST MEMBERS SHALL BE DESIGNED BY THE MANUFACTURER FOR COMPOSITE ACTION TO SUPPORT SUPERIMPOSED LIVE LOADS AS GIVEN IN THE NOTES PLUS THE DEAD LOAD OF PRECAST AND TOPPING AND A SUPERIMPOSED DEAD LOAD OF 10 PSF AT FLOOR SLABS. TOPPING SLAB OVER PRECAST MEMBERS SHALL BE AS NOTED ON THE DRAWINGS ANCHOR DOWELS AND SPECIAL REINFORCING SHALL BE PLACED BY THE CONTRACTOR IN STRICT ACCORDANCE WITH THE DRAWINGS. PRECAST UNITS SHALL HAVE 3" MINIMUM BEARING AT ENDS. BEARING WIDTHS AT THE SIDES OF THE UNITS SHALL BE 3" MINIMUM, UNLESS A GREATER SIDE BEARING AREA IS REQUIRED BY THE PRECAST 16. GENERAL CONTRACTOR SHALL PROVIDE A LEVEL AND ADEQUATE BEARING SURFACE FOR ALL PRECAST UNITS. PROVIDE SHIMS AS REQUIRED. SHIMS MUST BE CONTINUOUS FOR THE FULL WIDTH OF PLANK. NO POINT SHIMMING IS ALLOWED. USE KOROLATH SHIMS OR APPROVED EQUAL. STRUCTURAL STEEL ALL STRUCTURAL STEEL WORK SHALL BE IN CONFORMANCE WITH "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", AISC 360 (STEEL CONSTRUCTION MANUAL, 14TH EDITION MATERIALS STANDARDS (UNLESS NOTED OTHERWISE ON DRAWINGS OR IN PROJECT SPECIFICATIONS): ALL STEEL WF BEAMS SHALL BE ASTM A992, 50,000 PSI YIELD. ALL ANGLES CHANNELS AND PLATES SHALL BE ASTM A36 36 000 PSI YIELD PIPE SHAPES: ASTM A53, GRADE B. 35,000 PSI YIELD HSS RECTANGULAR SHAPES: ASTM A500, GRADE B, 46,000 PSI YIELD. HSS ROUND SHAPES: ASTM A500, GRADE B, 42,000 PSI YIELD. ANCHOR BOLTS: ASTM F1554 GRADE 36 THREADED RODS: ASTM A36, UNLESS OTHERWISE NOTEI ALL OTHER SHAPES SHALL BE ASTM A36, 36,000 PSI YIELD, UNLESS OTHERWISE NOTED SHOP CONNECTIONS SHALL BE HIGH-STRENGTH BOLTED OR WELDED. MINIMUM BOLT SHALL BE 3/4" DIAMETER. ASTM A325N. U.N.O. MINIMUM SIZE WELD, UNLESS OTHERWISE NOTED, IS TO BE 3/16 INCH FILLET, ETOXX ELECTRODES. ELECTRODES SHALL BE SUITED TO STEEL GRADE. FIELD CONNECTIONS SHALL BE HIGH-STRENGTH BOLTED, 3/4" DIAMETER, ASTM A325N, U.N.O. BEAM AND SHEAR CONNECTIONS WITH HIGH-STRENGTH BOLTS ARE TO BE BEARING TYPE, UNLESS NOTED OTHERWISE. WHERE FIELD-WELDING IS NOTED, IT SHOULD BE PERFORMED BY CERTIFIED WELDERS ONLY. MELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING BOLTS AND BOLTED CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" AS APPROVED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC). ALL CONNECTIONS SHALL BE FULL DEPTH CONNECTIONS ON ALL GIRDER AND BEAM CONNECTIONS TO WF COLUMNS: DOUBLE ANGLE SHEAR CONNECTION. BOLTS SHALL BE AT 3 INCH O.C. VERT., U.N.O. HSS AND PIPE COLUMNS: 3/8" THICK (MIN.), FULL DEPTH THRU-PLATE. BOLTS SHALL BE AT 3" O.C. BEAM TO GIRDER: FULL DEPTH, SINGLE ANGLE SHEAR CONNECTION TO BE SUBMITTED FOR REVIEW DESIGN STANDARD CONNECTIONS FOR THE LARGER OF EITHER THE SHEAR SHOWN ON THE DRAWINGS (INDICATED AS "V= K" AT THE MEMBER ENDS) OR 55% OF THE TOTAL LOAD CAPACITY, DERIVED FROM THE "MAXIMUM TOTAL UNIFORM LOAD TABLES" IN PART 3 OF THE AISC MANUAL, 14TH EDITION. IN NO CASE SHALL THE ANGLE SIZE AND MINIMUM NUMBER OF ROWS OF BOLTS FOR THE GIVEN BEAM SIZE BE LESS THAN THAT SHOWN IN TABLE 1, PART 10 OF THE AISC MANUAL, 14TH EDITION. WELD HEADED STUDS TO EMBEDDED BEARING PLATES TO DEVELOP THE FULL TENSION CAPACITY OF THE FIELD CONNECTIONS BY CUTTING OR BURNING ARE PROHIBITED, EXCEPT BY SPECIFIC APPROVAL OF THE WHERE COLUMN STIFFENERS ARE NOTED, PROVIDE MINIMUM SIZE WELDS ON BOTH SIDES OF STIFFENER PLATES, UNLESS NOTED OTHERWISE FIELD WELDING OF SOME COLUMN STIFFENER PLATES IS ACCEPTABLE TO FACILITATE THE ERECTION STEEL FRAMING SHALL BE PROPERLY BRACED UNTIL AFTER FINAL CONNECTIONS ARE MADE. STRUCTURAL AND MISCELLANEOUS STEEL FABRICATORS SHALL BE RESPONSIBLE FOR OBTAINING AND VERIFYING ALL FIELD DIMENSIONS NECESSARY FOR THE COMPLETION OF THEIR WORK. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL. FABRICATION SHALL NOT COMMENCE UNTIL SHOP DRAWINGS ARE APPROVED. IF THE FABRICATOR PROPOSES USING DETAILS OTHER THAN THOSE SHOWN, SUCH DETAILS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL BEFORE DETAILED SHOP DRAWINGS HAVE BEEN SUBMITTED. COORDINATE ALL DETAILING TO INCLUDE STRUCTURAL STEEL INFORMATION SHOWN ON THE ARCHITECTURAL DRAWINGS. SHOP DRAWINGS SHALL INCLUDE THE EXACT LOCATION AND SIZE OF ALL ROOF AND FLOOR OPENINGS FOR MECHANICAL EQUIPMENT. SEE TYPICAL DETAIL FOR FRAMING AROUND OPENINGS. STEEL SHAPES, PLATES, ETC. WHICH ARE EXPOSED TO WEATHER SHALL BE GALVANIZED OR PAINTED WITH A ALL STEEL BEAMS SHALL BE THOROUGHLY CLEANED IN ACCORDANCE WITH SSPC-SP2 OR BETTER. PROVIDE ONE COAT OF STANDARD SHOP PAINT ON ALL UNGALVANIZED PIECES EXCEPT AT AREAS TO BE DELETE PAINT ON ALL STEEL TO RECEIVE SPRAYED-ON FIREPROOFING OR CONCRETE ENCASEMENT. TOUCH UP FIELD MELDS AND ANY DAMAGED AREAS OF PAINT IN FIELD AFTER MELDING WITH A ZINC RICH ALL STEEL LINTELS AND SHELF ANGLES SHALL BE PRIMED AND HAVE TWO (2) FINISH COATS OF APPROVED RUST INHIBITIVE PAINT OR BE HOT DIPPED GALVANIZED ALL STEEL BEAMS SHALL BE FABRICATED AND ERECTED WITH THE NATURAL CAMBER (WITHIN THE MILL TOLERANCE) LOCATED ABOVE THE HORIZONTAL CENTERLINE BETWEEN THE END CONNECTIONS. FABRICATOR SHALL SUPPLY LOOSE ANGLES OVER ALL MASONRY OPENINGS AND RECESSES UNLESS NOTED OTHERWISE. LINTELS NOT SCHEDULED ON DRAWINGS SHALL CONSIST OF A SINGLE ANGLE WITH A 3 1/2 INCH LEG HORIZONTAL FOR EACH 4 INCHES OF WALL THICKNESS. ANGLES SHALL BE AS FOLLOWS: MASONRY OPENING ANGLE SIZE BEARING EACH END

4'-0" OR LESS

6'-0" TO 8'-0

5 X 3 1/2 X 1/4"

5 X 3 1/2 X 1/4"

5 X 3 1/2 X 3/8"

COLD FORMED LIGHT GAGE STEEL FRAMING PRODUCTS AND INSTALLATION SHALL MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS, UNLESS MORE STRINGENT REQUIREMENTS ARE DESCRIBED HEREIN: AMERICAN IRON AND STEEL INSTITUTE (AISI) \$100, "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS AMERICAN WELDING SOCIETY (AWS) D1.3, "STRUCTURAL WELDING CODE - SHEET STEEL" ASTM INTERNATIONAL SPECIFICATIONS: A653, "STANDARD SPECIFICATION FOR STEEL SHEET, ZING-COATED (GALVANIZED) OR ZING-IRON ALLOY-COATED (GALVANNEALED) BY THE HOT-DIP PROCESS. A1003, "STANDARD SPECIFICATION FOR STEEL SHEET, CARBON, METALLIC- AND NONMETALLIC-COATED FOR COLD-FORMED FRAMING MEMBERS. C955, "STANDARD SPECIFICATION FOR LOAD BEARING (TRANSVERSE AND AXIAL) STEEL STUDS, RUNNERS (TRACK), AND BRACING OR BRIDGING, FOR SCREW APPLICATION OF GYPSUM PANEL PRODUCTS AND METAL PLASTER BASES C1007, "STANDARD SPECIFICATION FOR THE INSTALLATION OF LOAD BEARING (TRANSVERSE AND AXIAL) STEEL STUDS AND RELATED ACCESSORIES." LIGHT GAGE METAL STUD DESIGNATION SHOWN ON STRUCTURAL DRAWINGS ASSUME [MARINO/WARE] [SSMA] AS A DESIGN BASIS. THE FOLLOWING ITEMS SHALL BE FURNISHED BY THE CONTRACTOR PRIOR TO FABRICATION: SUBMIT SHOP DRAWINGS FOR REVIEW WHICH INCLUDE: SUBMIT STATEMENTS FROM THE FRAMING MANUFACTURER CERTIFYING CONFORMANCE WITH APPLICABLE STANDARDS SECTIONS, PLANS AND/OR ELEVATIONS SHOWING COMPONENT TYPES AND LOCATIONS FOR EACH UNIQUE FRAMING APPLICATION. CONNECTION DETAILS SHOWING FASTENER TYPE, QUANTITY, LOCATIONS AND OTHER INFORMATION TO ASSURE PROPER INSTALLATION. CONTRACTORS ELECTING TO INSTALL PREFABRICATED/PREFINISHED PANELS SHALL SUBMIT DRAWINGS SHOWING CONFIGURATIONS, DIMENSIONS, COMPONENTS, LOCATIONS AND END-BEARING AND BRIDGING DETAILS. WHERE CONTRACTOR CHOOSES TO USE FRAMING OR CONNECTIONS DIFFERENT FROM THOSE SHOWN ON THE STRUCTURAL DRAWINGS, SUBMIT DETAILS FOR REVIEW ALONG WITH SIGNED AND SEALED CALCULATIONS, BY AN ENGINEER LICENSED IN THE STATE OF THE PROPOSED CONSTRUCTION WHICH STRUCTURAL ANALYSIS FOR EACH UNIQUE FRAMING APPLICATION. SELECTION OF FRAMING COMPONENTS AND ACCESSORIES AND VERIFICATION OF STRUCTURAL ENGINEER OF RECORD MAINTAINS THE RIGHT TO CHANGE, MODIFY OR REJECT SUBMITTALS THAT ARE DIFFERENT FROM THOSE SHOWN ON THE CONTRACT DOCUMENTS. ROOF TRUSSES ARE TO BE DESIGNED FOR THE METAL STUD SUPPLIER BY A PROFESSIONAL ENGINEER AND SIGNED AND SEALED CALCULATIONS AND SHOP DRAWINGS ARE TO BE SUBMITTED FOR REVIEW, INDICATING CAPACITY OF MEMBERS, FRAMING DETAILS, CONSTRUCTION AND PERMANENT BRACING, BRIDGING AND ALL OTHER APPURTENANCES OF MEMBERS TO CONFORM TO LOAD CRITERIA. ALL METAL STUD HEADERS INDICATED ON DRAWINGS ARE TO BE PROVIDED BY THE STUD INSPECTIONS SHALL BE PERFORMED BY AN INDEPENDENT AUTHORITY OR STRUCTURAL ENGINEER TO CHECK CONFORMANCE WITH THE PROVISIONS OF THE CONTRACT DOCUMENTS AND APPROVED ERECTION DRAWINGS. THE CONTRACTOR ASSUMES ALL RESPONSIBILITY FOR QUALITY CONTROL FOR THE SCOPE OF WORK SHOWN LIGHT GAGE METAL STUDS SHALL HAVE THE FOLLOWING STRENGTHS: 16, 14 AND 12 GAGE FRAMING COMPONENTS SHALL BE FORMED FROM STEEL CONFORMING TO THE MINIMUM REQUIREMENTS OF ASTM A653, GRADE 50, POSSESSING A MINIMUM YIELD OF 50,000 PSI. 18 AND 20 GAGE FRAMING COMPONENTS SHALL BE FORMED FROM STEEL CONFORMING TO THE MINIMUM REQUIREMENTS OF ASTM A653, GRADE 33, POSSESSING A MINIMUM YIELD OF 33,000 PS ALL METAL STUDS, PLATES, ETC. SHOULD BE GALVANIZED FRAMING PRODUCTS COATED IN ACCORDANCE MATERIALS SHALL BE STORED FLAT AND IN A MANNER TO PREVENT DISTORTION. PREVENT EXPOSURE TO WEATHER BY IMPERVIOUS COVER OR SHELTER. CONNECTIONS (WIRE TYING OF FRAMING COMPONENTS SHALL NOT BE PERMITTED): WELDS SHALL BE OF THE TYPE, SIZE AND LOCATION SHOWN IN THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS. MELDED CONNECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE AMERICAN MELDING - SOCIETY (AMS) D1.3. "STRUCTURAL MELDING CODE - SHEET STEEL". MELDERS. MELD OPERATIONS AND WELDING PROCEDURES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS CONSULT APPLICABLE AWS SPECIFICATIONS FOR INFORMATION REGARDING SAFE WELDING ALL WELDS SHALL BE CLEANED AND COATED WITH PAINT CONFORMING TO SSPC-PA 1, SEC. WELD SIZE, INDICATED AS "t" ON DRAWINGS, SHALL BE THE GREATER OF 1/16" OR THE THICKNESS OF THE THINNEST MATERIAL JOINED. SCREMS SHALL BE OF THE TYPE, SIZE AND LOCATION SHOWN IN THE CONTRACT DOCUMENTS SCREW PENETRATION THROUGH JOINED MATERIALS SHALL NOT BE LESS THAN THREE EXPOSED SCREW THREADS. SCREMS SHALL BE ITM BUILDEX TEKS SELF-DRILLING FASTENERS, HILTI KMIK-PRO SELF-DRILLING SCREMS, SIMPSON STRONG TIE QUIK-DRIVE SCREMS, OR APPROVED EQUAL. CONTRACTOR SHALL REFER TO INSTALLATION INSTRUCTIONS PUBLISHED BY THE SCREW MANUFACTURER AND ASTM C955 AND ASTM C954 FOR MINIMUM SPACING AND EDGE DISTANCE REQUIREMENTS AND TORQUE REQUIREMENTS. TYPES: ANCHOR BOLTS, EPOXY BOLTS, WEDGE EXPANSION BOLTS, SCREW TYPE CONCRETE FASTENERS, POWDER-ACTUATED FASTENERS, SHEAR AND TENSION CAPACITIES OF THE FASTENERS MUST BE VERIFIED FOR THE APPLICATION IN QUESTION. BEARING CAPACITY OF THE SUPPORTED ELEMENT SHOULD BE CHECKED IN ACCORDANCE WITH THE AISI CONCRETE ANCHORS SHALL NOT BE INSTALLED UNTIL FULL COMPRESSIVE STRENGTH OF THE CONCRETE IS OBTAINED. CONTRACTOR SHALL REFER TO INSTRUCTIONS PUBLISHED BY THE ANCHOR MANUFACTURER FOR MINIMUM SPACING, EDGE DISTANCE, AND CONCRETE EMBEDMENT AND ADDITIONAL INSTALLATION REQUIREMENTS: PREFABRICATED PANELS SHALL BE SQUARE, WITH COMPONENTS ATTACHED IN A MANNER TO PREVENT RACKING DURING FABRICATION, TRANSPORTATION AND LIFTING. PROVISIONS TO LIFT THE PANELS SHALL BE NCLUDED IN THE SHOP DRAWINGS. FIELD CUTTING OF STEEL FRAMING MEMBERS SHALL BE BY SAW OR SHEAR. TORCH CUTTING WILL NOT BE PERMITTED EXCEPT BY WRITTEN APPROVAL OF THE ENGINEER OF RECORD. TEMPORARY BRACING SHALL BE PROVIDED AND REMAIN IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED.

SPLICING OF FRAMING COMPONENTS, OTHER THAN TRACK, IS NOT PERMITTE

ARCH'L SPECIFICATIONS.

MANUFACTURER OR FABRICATOR

CONTRACT DOCUMENTS SHALL BE FLAGGED UPON REVIEW.

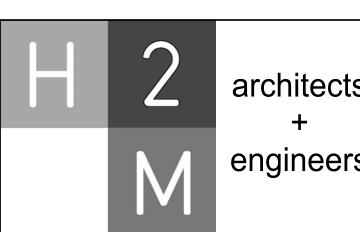
TEMS ARE CONSTRUCTED TO CONTRACT DOCUMENTS.

THE START OF FABRICATION OR COMMENCEMENT OF WORK.

10. REVIEW PERIOD SHALL BE A MINIMUM OF TWO (2) WEEKS.

RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR.

WHERE SPLICING OF TRACK IS NECESSARY BETWEEN STUD SPACING, A PIECE OF STUD SHALL BE PLACED BETWEEN ADJACENT TRACKS AND FASTENED BY WELD OR SCREW TO EACH OF THE TRACK, EACH END. SEALANT SHALL BE APPLIED TO CONCRETE OR MASONRY SURFACES PRIOR TO ANCHORING TRACKS. BOTTOM TRACKS OF STUD WALLS SHALL BE SECURED TO THE FOUNDATION WITH .157 DIA. HILTI X-U PINS @ 16" PROVIDE WEB STIFFENERS AT ALL SUPPORT POINTS OF HORIZONTAL JOISTS AND AS SPECIFIED ON THE LOAD BEARING STUD WALLS SHALL HAVE BRIDGING AT A MAXIMUM SPACING OF 4'-O" O.C. NON-LOAD BEARING STUD WALLS SHALL HAVE BRIDGING AT A MAXIMUM SPACING OF 5'-0" O.C PROVIDE BRIDGING BETWEEN JOISTS 8'-O" OR MORE IN LENGTH AT A MAXIMUM SPACING OF 8'-O" O.C WALL STUDS SHALL BE SPACED AS SHOWN IN THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS OR AS REQUIRED TO MEET THE DESIGN REQUIREMENTS AND LIMITATIONS OF THE COLLATERAL MATERIALS. STANDARD WEB PUNCH-OUTS ARE ONLY PERMITTED IN FRAMING MEMBERS USED AS TYPICAL WALL STUDS, JOISTS, RAFTERS, OR IF INDICATED IN STANDARD DETAILS. STANDARD WEB PUNCH-OUTS USED IN ALL OTHER FRAMING MEMBERS INCLUDING, BUT NOT LIMITED TO, HEADERS, BEAMS, GIRDERS, JAMB POSTS, & SHEAR WALL POSTS, ARE NOT PERMITTED WITHOUT THE APPROVAL OF THE COMPONENT ENGINEER. DESIGN FRAMING SYSTEMS TO WITHSTAND DEFLECTION LIMITS PER THE REQUIREMENTS OF THE FINISH MATERIALS OR AS SPECIFIED BY THE ARCHITECT OR ENGINEER OF RECORD, BUT NOT TO EXCEED THE FLOOR TRUSSES: VERTICAL DEFLECTION OF L/480 FOR LIVE LOADS AND L/360 FOR TOTAL LOADS ROOF TRUSSES: VERTICAL DEFLECTION OF L/360 FOR ROOF/SNOW LOADS AND L/240 FOR TOTAL SCISSOR ROOF TRUSSES: HORIZONTAL DEFLECTION OF 1 1/4" AT SUPPORTS. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED BY THE CONTRACTOR SHALL REVIEW ALL DRAWINGS PRIOR TO SUBMITTAL. ITEMS NOT IN ACCORDANCE WITH ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM CONTRACT DOCUMENTS SHALL BE CLOUDED BY REPRODUCTION OF ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR RESUBMITTAL AS SHOP DRAWINGS IS PROHIBITED. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND THE SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT ARE NOT TO BE CONSIDERED CHANGES TO CONTRACT DOCUMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAKE SURE THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY OTHERS RESTS WITH THE DESIGNING REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. SHOP DRAWINGS FOR ALL STRUCTURAL MATERIALS TO BE SUBMITTED TO ARCHITECT FOR REVIEW PRIOR TO



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MONTEBELLO REALTY GROUP LLC

Braemar at Montebello Assisted Living Residence

MONTEBELLO CROSSING 250 LAFAYETTE AVENUE (NYS ROUTE 59) VILLAGE OF MONTEBELLO

CONTRACT	

REGULATORY REVIEW

GENERAL NOTES

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Special Inspection Requirements

BRAEMAR AT MONTEBELLO ASSISTED LIVING FACILITY ROCKLAND COUNTY, NEW YORK

STRUCTURAL OBSERVATIONS 1. PRIOR TO INITIATING CONSTRUCTION, THE GENERAL CONTRACTOR, A REPRESENTATIVE OF THE BUILDING OWNER, AND PROJECT ARCHITECT SHALL MEET WITH A REPRESENTATIVE FROM M&K TO DISCUSS

OWNER, AND PROJECT ARCHITECT SHALL MEET WITH A REPRESENTATIVE FROM M&K TO DISCUSS CONSTRUCTION PRACTICES AND PROCEDURES OF THE MAJOR STRUCTURAL SYSTEMS, ESTABLISH PROCEDURES AND GUIDELINES FOR REQUESTING INFORMATION FROM M&K, AND TO REVIEW THE STRUCTURAL OBSERVATIONS, SPECIAL INSPECTIONS, AND TESTING REQUIREMENTS OUTLINED IN THE CONSTRUCTION

- DOCUMENTS.

 2. M&K <\$\text{SHALL} OR MAY} \times VISIT THE PROJECT AT APPROPRIATE INTERVALS DURING CONSTRUCTION TO BECOME GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE CONTRACTORS' WORK AND TO DETERMINE IF THE WORK IS PROCEEDING IN GENERAL ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE CLIENT HAS NOT RETAINED M&K TO MAKE DETAILED INSPECTIONS NOR TO PROVIDE EXHAUSTIVE OR CONTINUOUS PROJECT REVIEW AND OBSERVATION SERVICES. M&K DOES NOT GUARANTEE THE PERFORMANCE OF, AND SHALL HAVE NO RESPONSIBILITY FOR, FURNISHING MATERIALS OR PERFORMING ANY WORK ON THE PROJECT. IF THE CLIENT DESIRES MORE EXTENSIVE PROJECT OBSERVATION OR FULL-TIME PROJECT REPRESENTATION, THE
- CLIENT SHALL REQUEST SUCH SERVICES BE PROVIDED BY M&K AS ADDITIONAL SERVICES.

 3. M&K DOES NOT GUARANTEE THE PERFORMANCE OF, AND HAS NO RESPONSIBILITY FOR, THE ACTS OR OMISSIONS OF ANY CONTRACTOR, SUBCONTRACTOR, SUPPLIER OR ANY OTHER ENTITY FURNISHING MATERIALS OR PERFORMING ANY WORK ON THE PROJECT.
- MATERIALS OR PERFORMING ANY WORK ON THE PROJECT.

 4. STRUCTURAL OBSERVATIONS PERFORMED BY M#K SHALL NOT BE CONSIDERED A SUBSTITUTION FOR THE QUALITY CONTROL PROGRAMS AND PROCEDURES OF ANY CONTRACTOR, SUBCONTRACTOR, SUPPLIER OR ANY OTHER ENTITY FURNISHING MATERIALS OR PERFORMING ANY WORK ON THE PROJECT.
- STRUCTURAL OBSERVATIONS PERFORMED BY M&K SHALL NOT BE CONSIDERED A SUBSTITUTION FOR THE REQUIRED SPECIAL INSPECTIONS LISTED.
 M&K SHALL PROVIDE STRUCTURAL OBSERVATION/SITE VISIT REPORTS TO THE CLIENT FOLLOWING EACH VISIT TO THE JOB SITE. DEFICIENCIES OR DEVIATIONS LISTED IN THE REPORT MUST BE RESOLVED TO THE SATISFACTION OF M&K. M&K SHALL NOTIFY THE CODE OR BUILDING OFFICIAL IN WRITING OF ANY OUTSTANDING OR UNRESOLVED STRUCTURAL DEFICIENCIES OR DEVIATIONS PRIOR TO THE COMPLETION OF

SPECIAL INSPECTION GENERAL:

- SPECIAL INSPECTIONS AND TESTS SHALL BE PERFORMED FOR THIS PROJECT FOR ALL STRUCTURAL SYSTEMS
 OF THE CONSTRUCTION TYPES LISTED HEREIN. THE BUILDING OWNER OR A REGISTERED DESIGN
 PROFESSIONAL IN RESPONSIBLE CHARGE ACTING AS THE BUILDING OWNER'S AGENT SHALL EMPLOY ONE OR
 MORE SPECIAL INSPECTORS TO PROVIDE THE SPECIAL INSPECTIONS AND TESTS.
 THE QUALIFICATIONS OF ALL PERSONNEL PERFORMING SPECIAL INSPECTIONS AND TESTING ACTIVITIES SHALL
 BE SUBMITTED TO THE BUILDING OFFICIAL, AND ARE SUBJECT TO APPROVAL OF THE BUILDING OFFICIAL
 AND/OR THE STRUCTURAL ENGINEER. QUALIFIED SPECIAL INSPECTORS SHALL DEMONSTRATE COMPETENCE
- AND RELATED EXPERIENCE OR TRAINING FOR INSPECTION OF THE PARTICULAR CONSTRUCTION TYPES REQUIRING SPECIAL INSPECTIONS.

 3. THE CREDENTIALS OF ALL INSPECTORS AND TESTING TECHNICIANS SHALL BE PROVIDED TO M&K IF REQUIRESTED.
- THE CONSTRUCTION OR WORK FOR WHICH SPECIAL INSPECTION OR TESTING IS REQUIRED SHALL REMAIN
 ACCESSIBLE AND EXPOSED FOR SPECIAL INSPECTION OR TESTING PURPOSES UNTIL COMPLETION OF THE
 REQUIRED SPECIAL INSPECTIONS OR TESTS.
 SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS AND TESTS. THE SPECIAL INSPECTOR SHALL
- FURNISH INSPECTION AND TESTING REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE [,AND STRUCTURAL ENGINEER OF RECORD]. REPORTS SHALL INDICATE THAT WORK INSPECTED OR TESTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS.

 6. DISCREPANCIES BETWEEN THE INSPECTED CONSTRUCTION AND THE CONSTRUCTION DOCUMENTS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE BUILDING CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE [AND STRUCTURAL BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE [AND STRUCTURAL BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE [AND STRUCTURAL BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE [AND STRUCTURAL BUILDING OFFICIAL AND STRUCTURAL BUILDING OFFICIAL BUILDING OFFICI
- ENGINEER OF RECORD], PRIOR TO COMPLETION OF THE PHASE OF CONSTRUCTION IN QUESTION.

 7. SPECIAL INSPECTORS SHALL SUBMIT A FINAL REPORT DOCUMENTING ALL REQUIRED SPECIAL INSPECTIONS AND TESTS DIRECTLY TO THE STRUCTURAL ENGINEER OF RECORD AND TO THE BUILDING OFFICIAL. THE REPORT SHALL DOCUMENT THE REQUIRED SPECIAL INSPECTIONS AND TESTS, THE CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTION RECORDS, AND SHALL INDICATE THAT THE FINAL INSPECTED CONSTRUCTION IS IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS.

 8. SPECIAL INSPECTIONS SHALL BE PERFORMED ON A PERIODIC OR CONTINUOUS BASIS IF INDICATED HEREAFTER
- AS (PERIODIC) OR (CONTINUOUS). PERIODIC AND CONTINUOUS SPECIAL INSPECTIONS SHALL BE DEFINED AS FOLLOWS:

 A. CONTINUOUS SPECIAL INSPECTIONS:

 THE FULL-TIME OBSERVATION AND/OR TESTING OF WORK REQUIRING SPECIAL INSPECTION BY A QUALIFIED SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS
- QUALIFIED SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.

 B. PERIODIC SPECIAL INSPECTIONS:
 THE PART-TIME OR INTERMITTENT OBSERVATION AND/OR TESTING OF WORK REQUIRING SPECIAL
- INSPECTION BY A QUALIFIED SPECIAL INSPECTOR WHO IS PRESENT WHERE THE WORK TO BE
 INSPECTED HAS BEEN OR IS BEING PERFORMED, AND AT THE COMPLETION OF THE WORK.

 9. A STATEMENT OF SPECIAL INSPECTIONS SHALL BE PREPARED BY THE REGISTERED DESIGN PROFESSIONAL IN
 RESPONSIBLE CHARGE PER THE REQUIREMENTS OF IBC SECTION 1704.3 FOR ALL SYSTEMS IDENTIFIED HEREIN
 AS REQUIRING SPECIAL INSPECTIONS.
- EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF MAIN WIND/SEISMIC FORCE RESISTING SYSTEMS, DESIGNATED SEISMIC SYSTEMS, OR A WIND/SEISMIC RESISTING COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS, SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND BUILDING OWNER PRIOR TO THE COMMENCEMENT OF WORK FOR THE SYSTEM OR COMPONENT FOR WHICH HE IS RESPONSIBLE. THE STATEMENT SHALL CONTAIN ACKNOWLEDGEMENT OF THE SPECIAL INSPECTIONS REQUIREMENTS LISTED HEREIN OR IN THE STATEMENT OF SPECIAL INSPECTIONS FOR THE SYSTEMS OR COMPONENTS FOR WHICH THE CONTRACTOR IS RESPONSIBLE.
- 11. IMPLEMENTATION OF THE SPECIAL INSPECTIONS PROGRAM SHALL NOT BE CONSIDERED A SUBSTITUTION FOR THE QUALITY CONTROL PROGRAMS AND PROCEDURES OF ANY CONTRACTOR, SUBCONTRACTOR, SUPPLIER OR ANY OTHER ENTITY FURNISHING MATERIALS OR PERFORMING ANY WORK ON THE PROJECT.ON IS IN CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS, SHALL BE SUBMITTED TO THE BUILDING OFFICIAL [AND STRUCTURAL ENGINEER].

- SHOP FABRICATION

 1. THE SPECIAL INSPECTION REQUIREMENTS LISTED HEREIN SHALL APPLY TO THE FABRICATION OF STRUCTURAL, LOAD-BEARING, AND LATERAL LOAD-RESISTING MEMBERS OR ASSEMBLIES THAT IS PERFORMED ON THE PREMISES OF A FABRICATION SHOP. THE SPECIAL INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY-CONTROL PROCEDURES AND SHALL REVIEW THESE PROCEDURES TO CONFIRM THAT THEY ARE SUFFICIENT FOR THE FABRICATION TO CONFORM TO THE
- CONSTRUCTION DOCUMENTS.

 THE SPECIAL INSPECTION REQUIREMENTS LISTED HEREIN FOR FABRICATION OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES ARE NOT REQUIRED WHEN THAT WORK IS PERFORMED ON THE PREMISES OF A FABRICATION SHOP THAT IS REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION BY A QUALIFIED AND APPROVED SPECIAL INSPECTIONS AGENCY. FOR SHOP FABRICATION TO QUALIFY FOR THIS EXEMPTION, ONE COPY OF BOTH THE DOCUMENTATION OF THE SHOP'S APPROVAL STATUS, AND OF THE CERTIFICATE OF COMPLIANCE FROM THE FABRICATOR STATING THAT THE FABRICATION IS IN CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS, SHALL BE SUBMITTED TO THE BUILDING OFFICIAL [AND STRUCTURAL ENGINEER].

STEEL CONSTRUCTION STRUCTURAL STEEL: SPECIAL INSPECTIONS AND TESTING FOR STRUCTURE SPECIAL INSPECTIONS AND TESTING FOR STRUCTURE STEEL STRUCTURE STEEL CONSTRUCTIONS AND TESTING FOR STRUCTURE STEEL CONSTRUCTIONS AND TESTING FOR STRUCTURE STEEL CONSTRUCTIONS STEEL CONSTRUCTIONS STRUCTURE STRUCTURE

- SPECIAL INSPECTIONS AND TESTING FOR STRUCTURAL STEEL, INCLUDING ALL STRUCTURAL STEEL MEMBERS AND THEIR CONNECTIONS, SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360.

 COLD FORMED STEEL TRUSSES:
- COLD FORMED STEEL TRUSSES:

 FOR COLD FORMED STEEL TRUSSES SPANNING 60 FEET OR MORE, SPECIAL INSPECTOR SHALL VERIFY THAT

 TEMPORARY INSTALLATION RESTRAINT/BRACING AND PERMANENT INDIVIDUAL TRUSS MEMBER

 RESTRAINT/BRACING IS INSTALLED IN ACCORDANCE WITH APPROVED TRUSS SUBMITTAL PACKAGE.

CONCRETE CONSTRUCTION 1. CONCRETE CONSTRUCTION, INCLUDING REINFORCING STEEL AND FORMWORK, SHALL BE INSPECTED IN ACCORDANCE WITH THE REQUIREMENTS IN IBC SECTION 1705.3.

- COLD-FORMED STEEL LIGHT FRAMED CONSTRUCTION

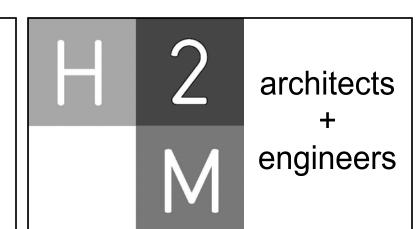
 1. ALL PREFABRICATED LIGHT GAGE CONSTRUCTION SHALL BE INSPECTED IN ACCORDANCE WITH THE
- REQUIREMENTS OF IBC SECTION 1704.2.5.

 COLD FORMED STEEL BRACED FRAMES, LATERAL BRACES, COLLECTORS, DRAG STRUTS, HOLD DOWNS, AND THEIR CONNECTIONS, SHALL BE INSPECTED IN ACCORDANCE WITH THE REQUIREMENTS OF IBC SECTIONS 1705.11.2 AND 1705.12.3.
- 3. THE SPECIAL INSPECTOR SHALL VERIFY THAT INSTALLATION AND ATTACHMENT OF ALL HOLD DOWNS AND ANCHORAGE OF SHEAR WALLS AND BRACED WALL PANELS TO THE SUPPORTING STRUCTURE AND FOUNDATIONS ARE IN CONFORMANCE WITH THE STRUCTURAL DRAWINGS, INCLUDING:

 A. WELDING OF POSTS TO EMBEDMENT FLATES IN THE FLOROR OR FOUNDATION.
- A. WELDING OF POSTS TO EMBEDMENT PLATES IN THE FLOOR OR FOUNDATION.
 B. CONSTRUCTION AND ATTACHMENT/EMBEDMENT OF EMBEDMENTS TO THE SUPPORTING STRUCTURE OR FOUNDATION.
 C. WELDING AND/OR BOLTING OF HOLD DOWNS AND ANCHORAGE ELEMENTS TO SHEAR WALL AND
- BRACED WALL PANEL POSTS AND/OR TRACK.

 D. INSTALLATION OF ALL SCREWS, BOLTS, AND ANCHORS CONNECTING SHEAR WALLS AND BRACED WALL PANELS TO SUPPORTING STRUCTURE AND FOUNDATIONS.
- WALL PANELS TO SUPPORTING STRUCTURE AND FOUNDATIONS.

 E. CONNECTION OF FLOOR AND ROOF DIAPHRAGM ELEMENTS TO SHEAR WALLS AND BRACED WALL PANELS AND TOP OF WALL CONNECTION DETAILS.



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MONTEBELLO REALTY GROUP LLC

Braemar at Montebello Assisted Living Residence

MONTEBELLO CROSSING 250 LAFAYETTE AVENUE (NYS ROUTE 59) VILLAGE OF MONTEBELLO

CONTRACT

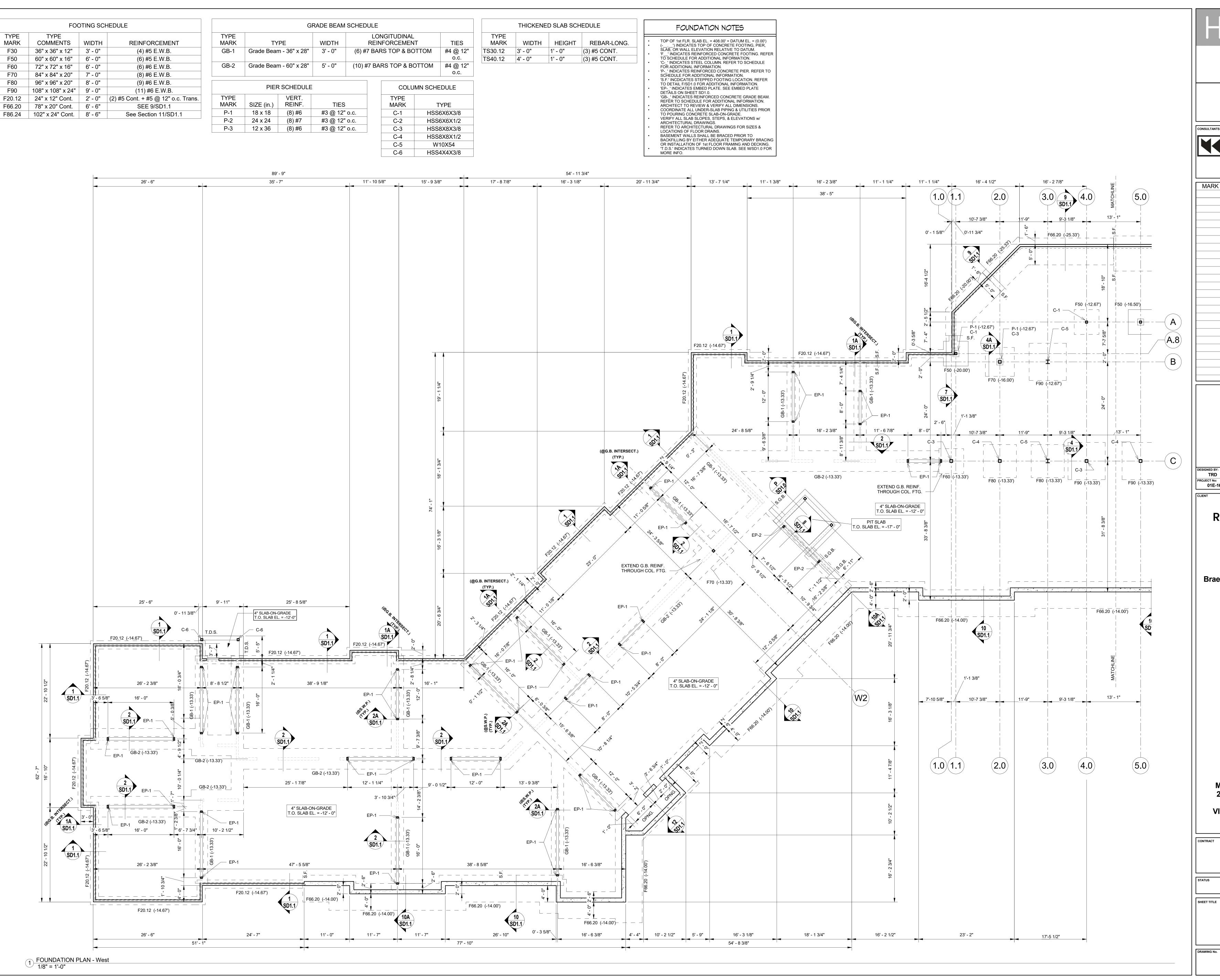
REGULATORY REVIEW

SHEET TITLE

SPECIAL INSPECTION NOTES

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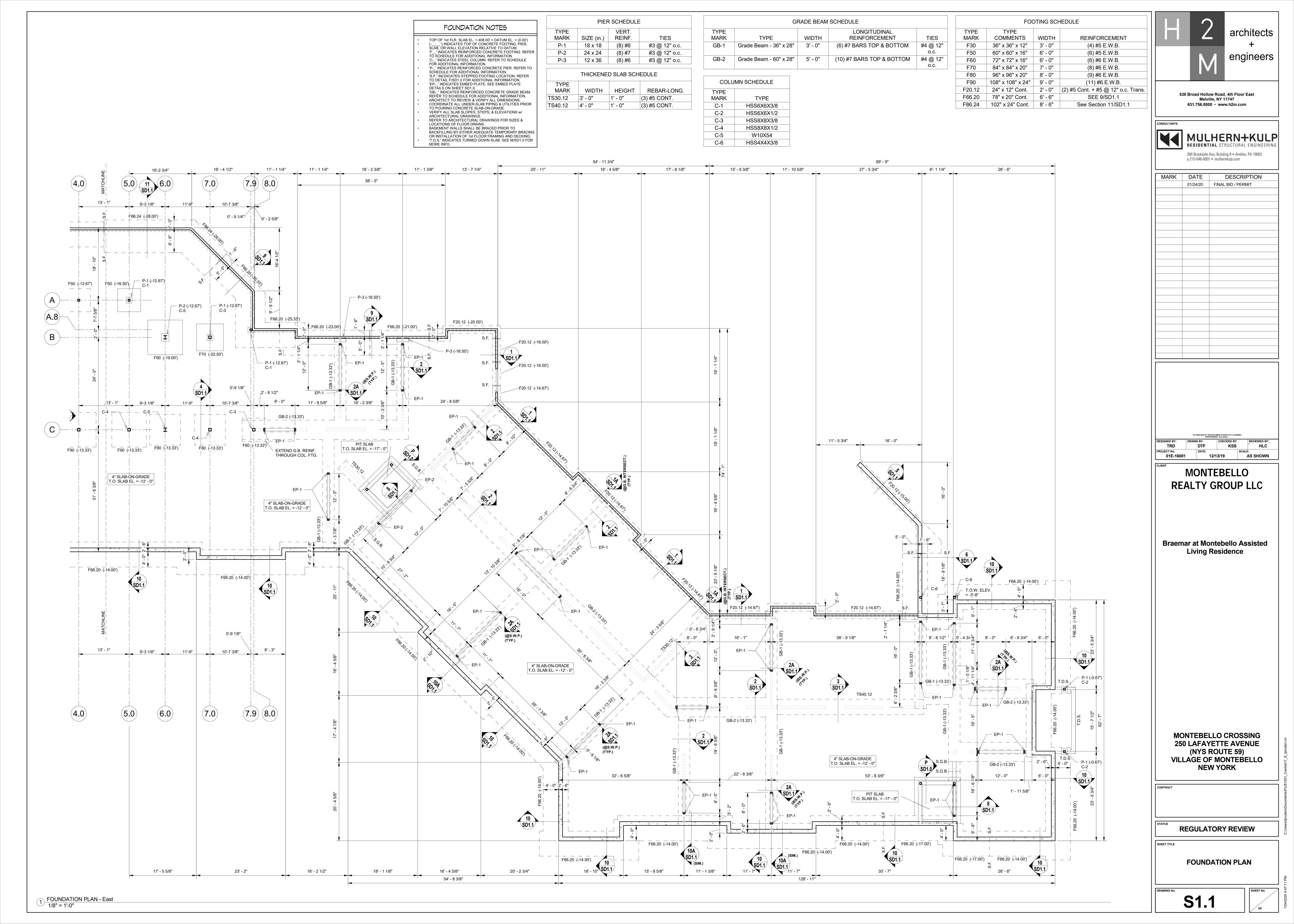
CONTRACT

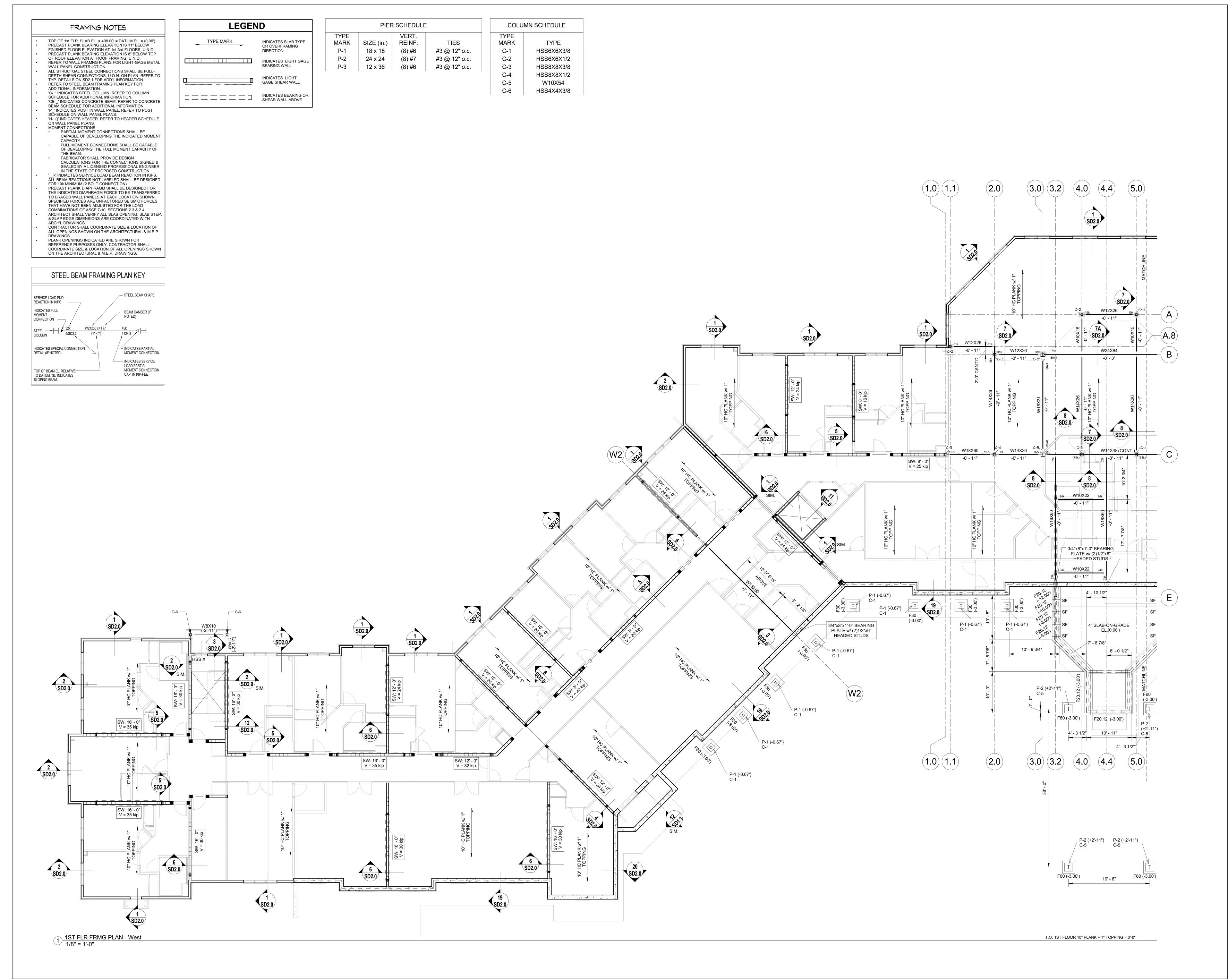
REGULATORY REVIEW

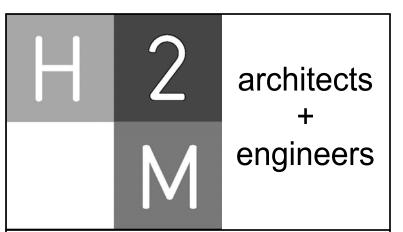
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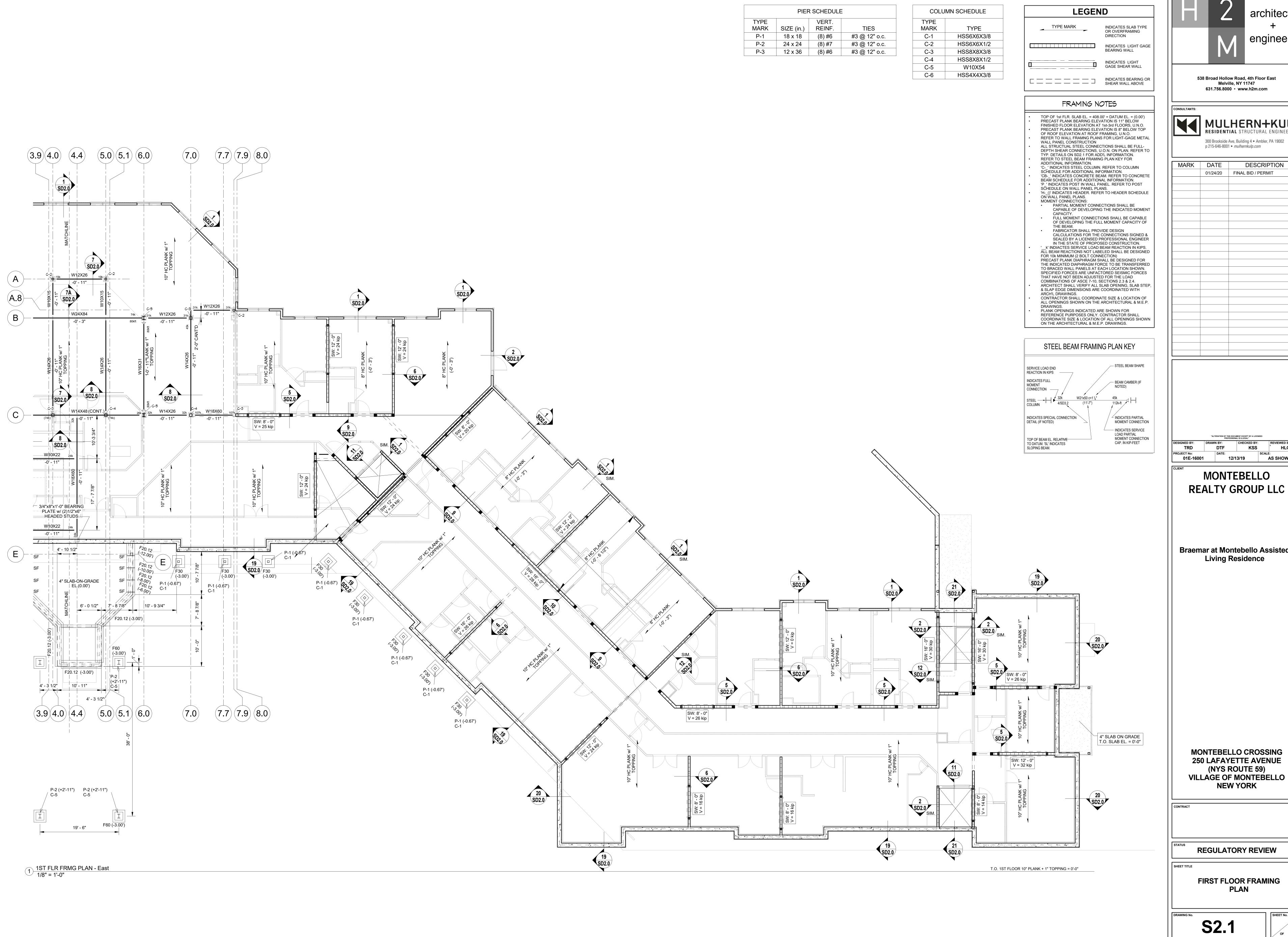
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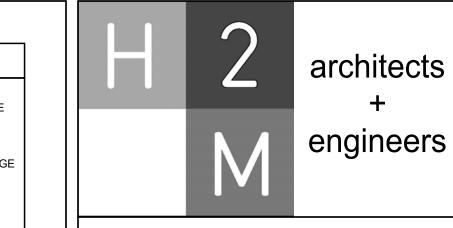
REGULATORY REVIEW

EET TITLE

FIRST FLOOR FRAMING PLAN

S2.0





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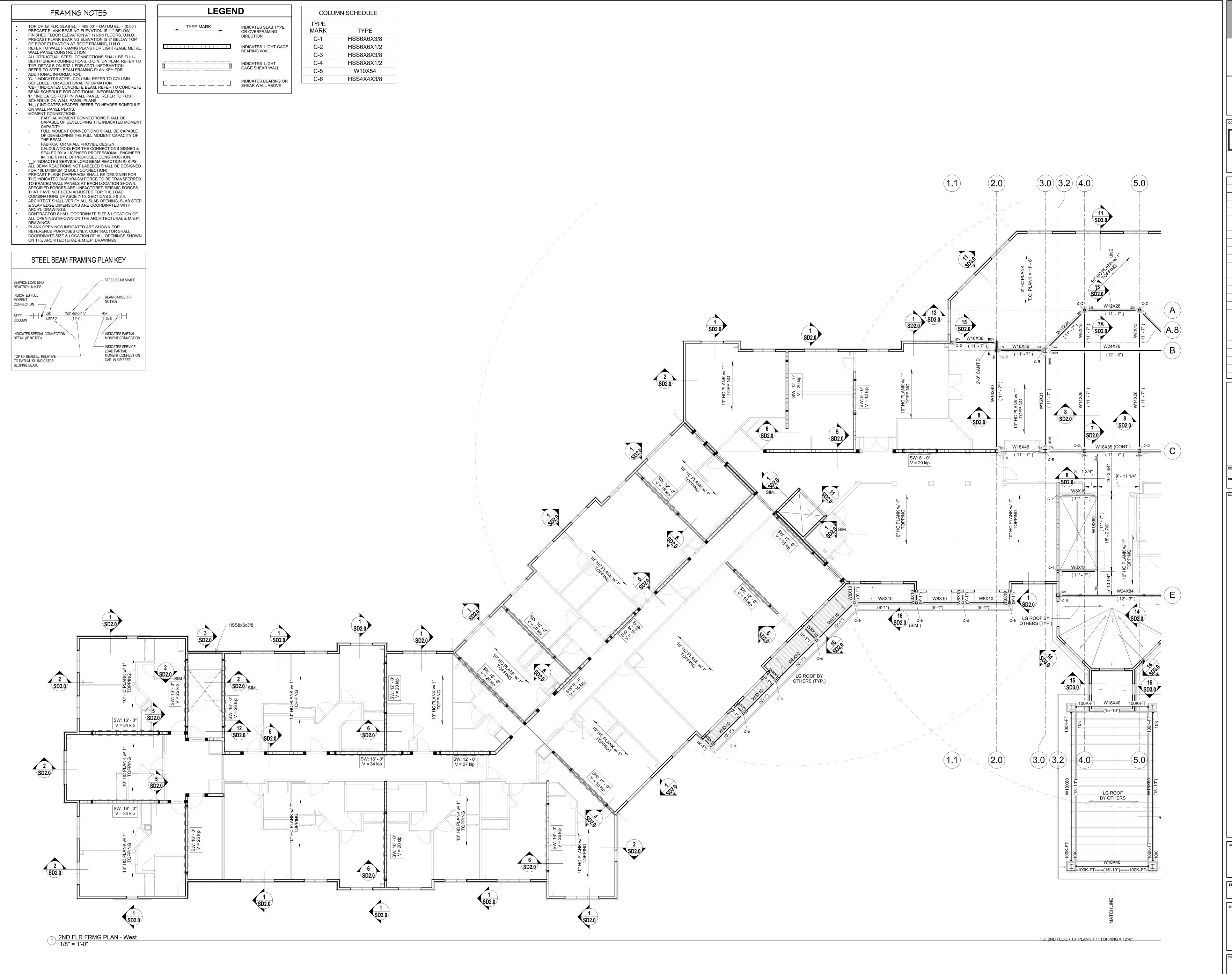
KSS **MONTEBELLO**

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VILLAGE OF MONTEBELLO **NEW YORK**

REGULATORY REVIEW

FIRST FLOOR FRAMING **PLAN**



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PROJECT No: 01E-16001 DATE: 12/13/19 SCALE: AS SHOWN SLIENT MONTEBELLO

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MONTEBELLO CROSSING 250 LAFAYETTE AVENUE (NYS ROUTE 59) VILLAGE OF MONTEBELLO NEW YORK

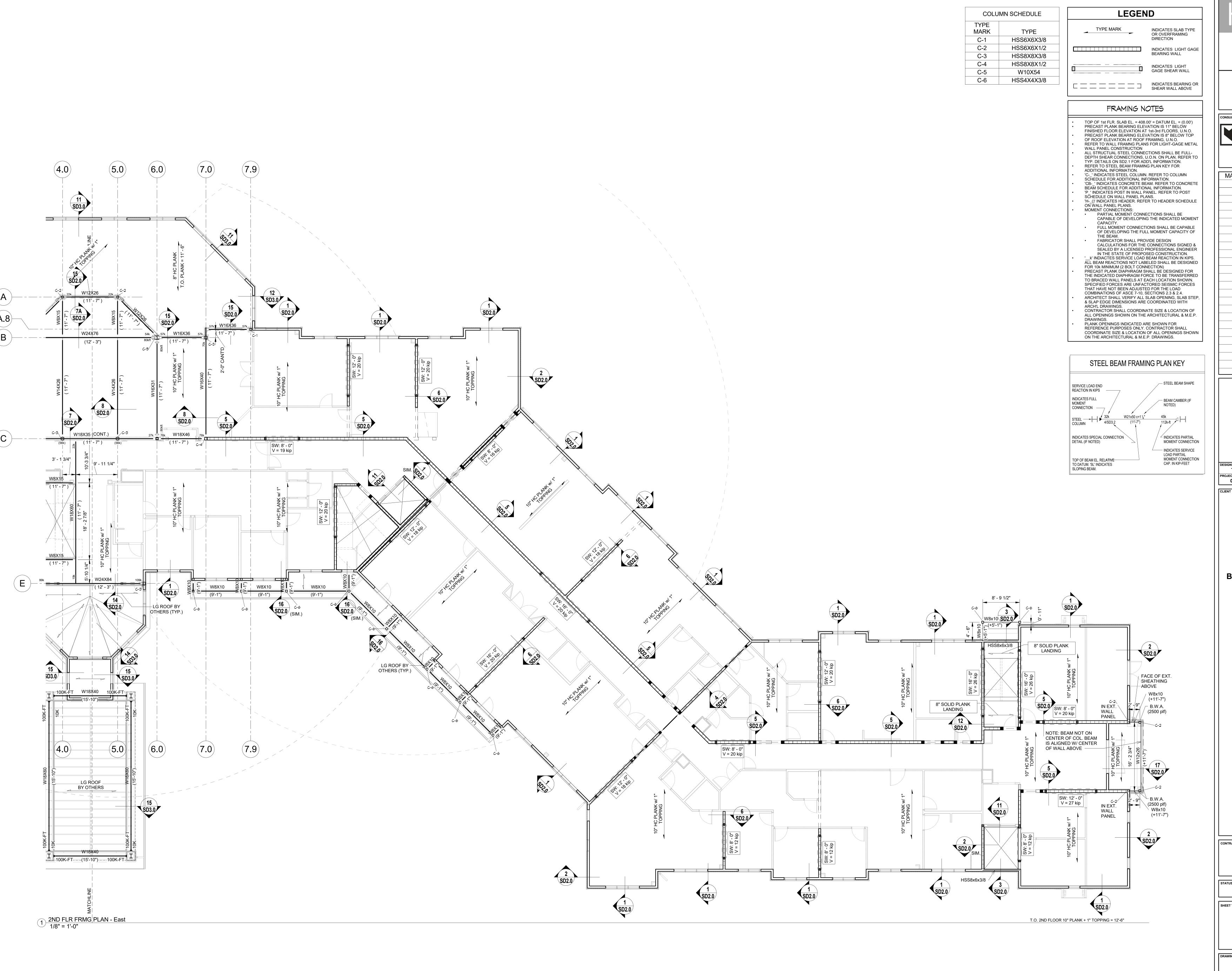
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REGULATORY REVIEW

HEET TITLE

SECOND FLOOR FRAMING PLAN

S3.0



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TRD DATE: SCALE:

01E-16001

DESIGNED BY: DATE: 12/13/19

DATE: AS SHOWN

MONTEBELLO
REALTY GROUP LLC

Braemar at Montebello Assisted Living Residence

MONTEBELLO CROSSING 250 LAFAYETTE AVENUE (NYS ROUTE 59) VILLAGE OF MONTEBELLO NEW YORK

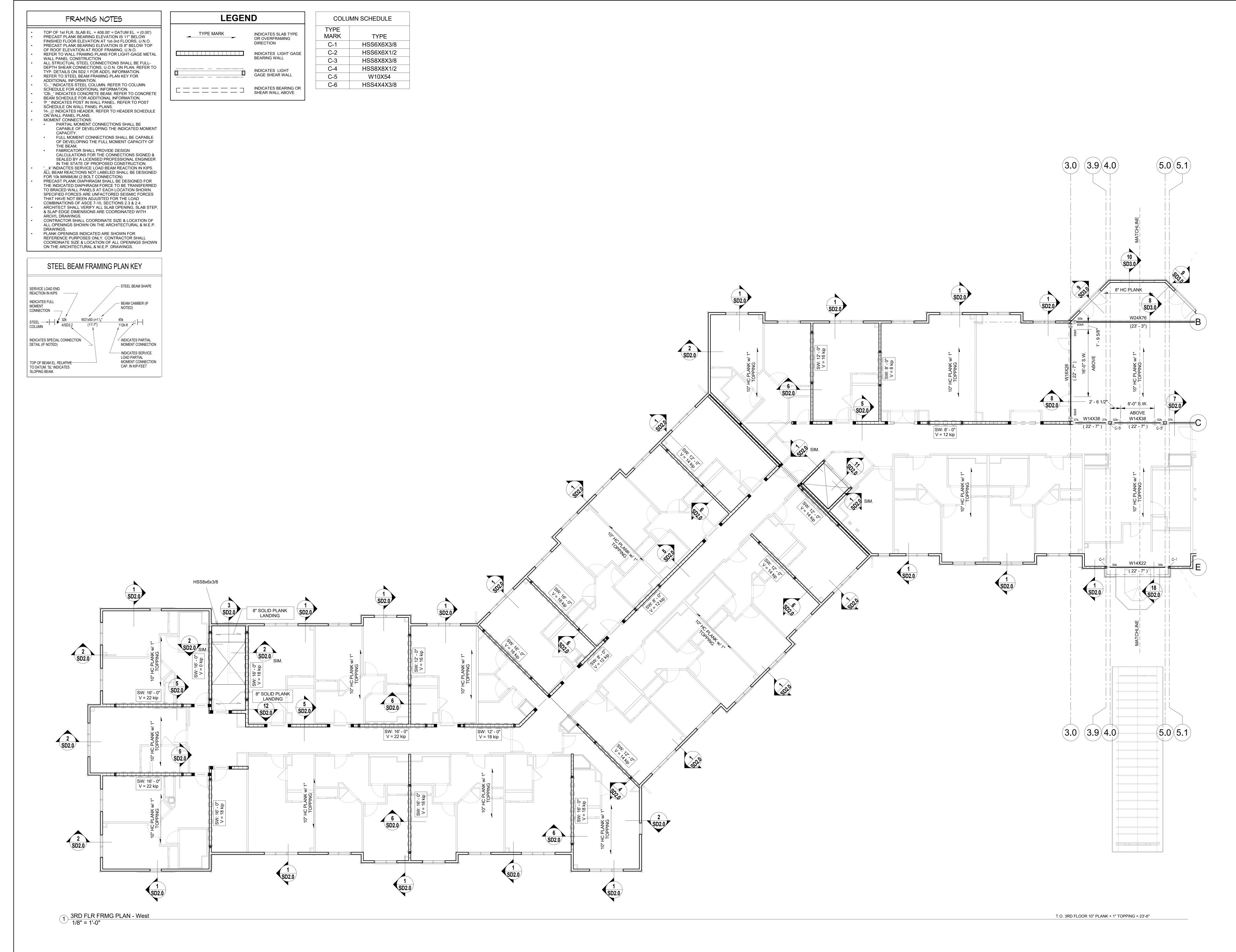
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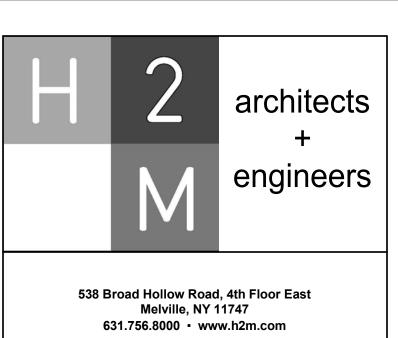
REGULATORY REVIEW

PLAN

SECOND FLOOR FRAMING

C2 4





CONSULTANTS:

MULHERN+KULP

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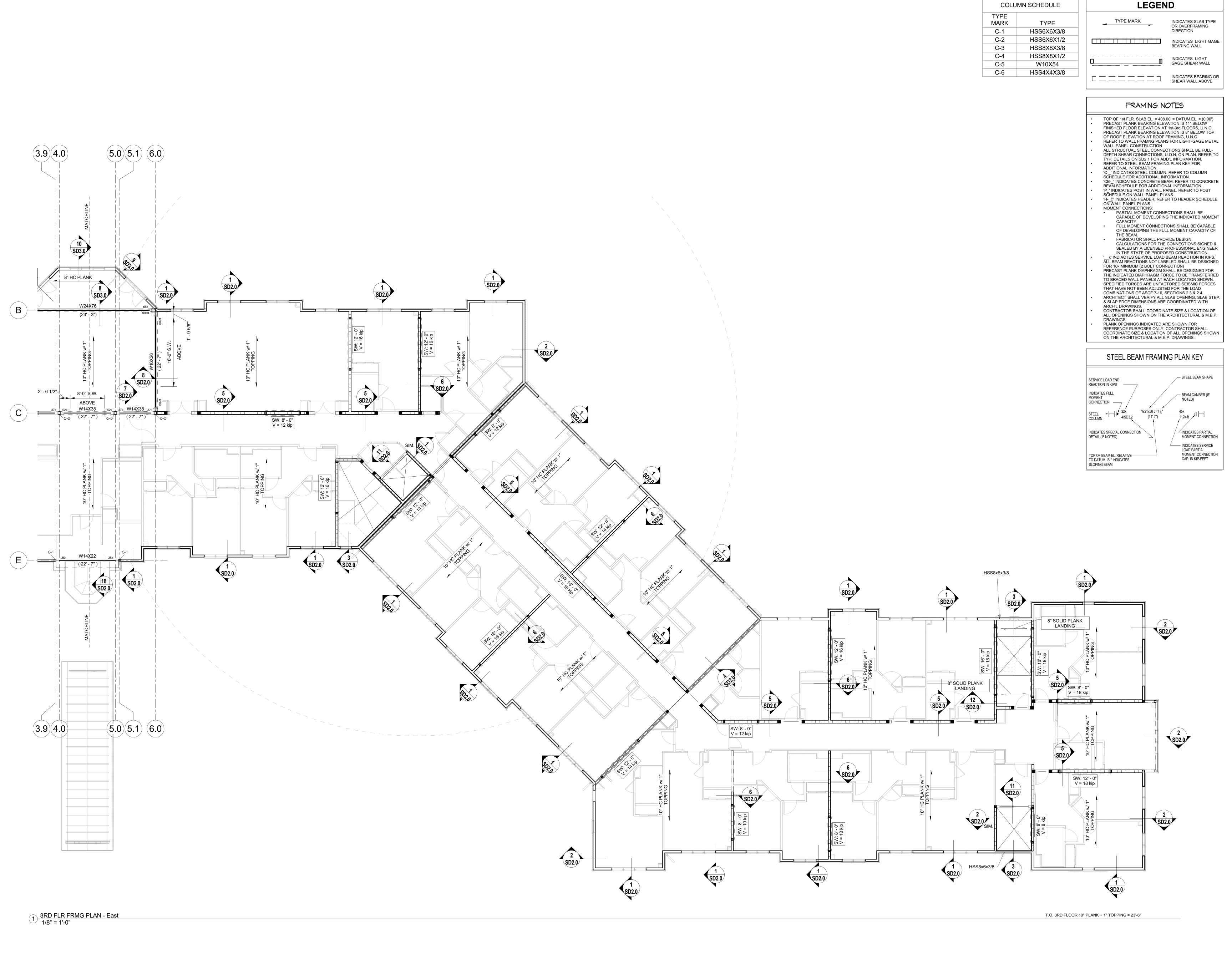
REGULATORY REVIEW

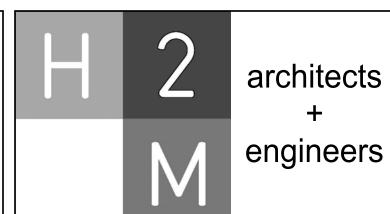
HEET TITLE

THIRD FLOOR FRAMING PLAN

S4.0

OF





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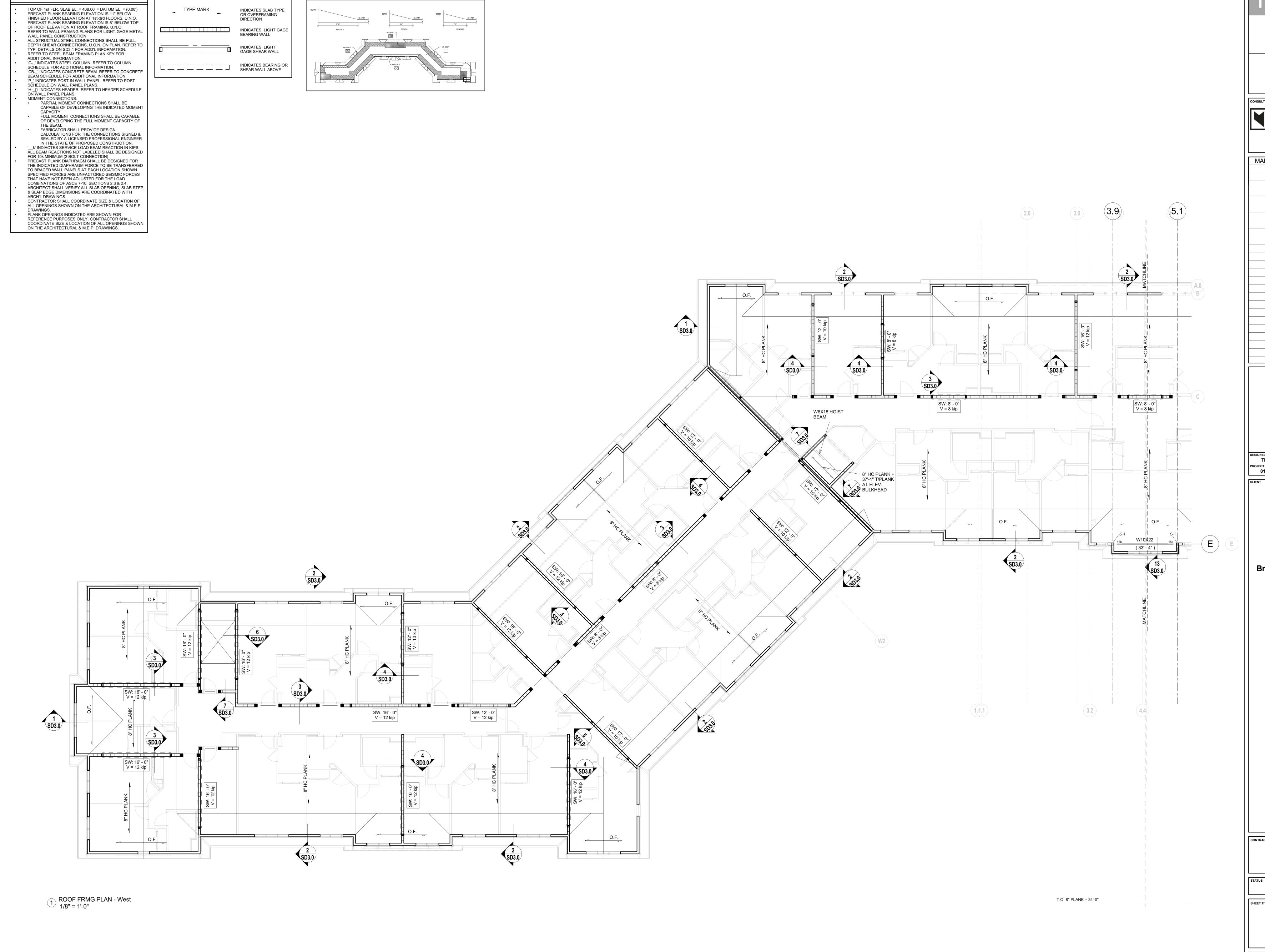
CONTRACT			

REGULATORY REVIEW

THIRD FLO

THIRD FLOOR FRAMING PLAN

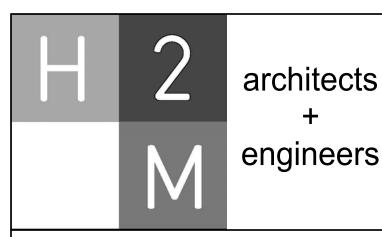
S4.1



LEGEND

SNOW DRIFT DIAGRAM

FRAMING NOTES



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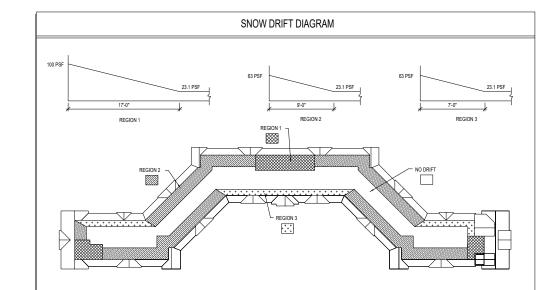
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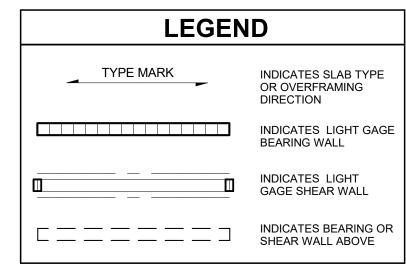
REGULATORY REVIEW

HEET TITLE

ROOF FRAMING PLAN

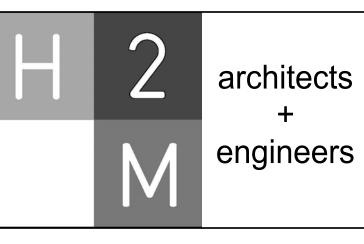
S5.0





FRAMING NOTES

TOP OF 1st FLR. SLAB EL. = 408.00' = DATUM EL. = (0.00') PRECAST PLANK BEARING ELEVATION IS 11" BELOW FINISHED FLOOR ELEVATION AT 1st-3rd FLOORS, U.N.O. PRECAST PLANK BEARING ELEVATION IS 8" BELOW TOP OF ROOF ELEVATION AT ROOF FRAMING, U.N.O. REFER TO WALL FRAMNG PLANS FOR LIGHT-GAGE METAL WALL PANEL CONSTRUCTION ALL STRUCTUAL STEEL CONNECTIONS SHALL BE FULL-DEPTH SHEAR CONNECTIONS, U.O.N. ON PLAN. REFER TO TYP. DETAILS ON SD2.1 FOR ADD'L INFORMATION. REFER TO STEEL BEAM FRAMING PLAN KEY FOR ADDITIONAL INFORMATION. 'C-_' INDICATES STEEL COLUMN. REFER TO COLUMN SCHEDULE FOR ADDITIONAL INFORMATION. BEAM SCHEDULE FOR ADDITIONAL INFORMATION. 'P_' INDICATES POST IN WALL PANEL. REFER TO POST SCHEDULE ON WALL PANEL PLANS. 'H-_()' INDICATES HEADER. REFER TO HEADER SCHEDULE MOMENT CONNECTIONS: PARTIAL MOMENT CONNECTIONS SHALL BE FULL MOMENT CONNECTIONS SHALL BE CAPABLE OF DEVELOPING THE FULL MOMENT CAPACITY OF FABRICATOR SHALL PROVIDE DESIGN CALCULATIONS FOR THE CONNECTIONS SIGNED & IN THE STATE OF PROPOSED CONSTRUCTION. '_k' INDIACTES SERVICE LOAD BEAM REACTION IN KIPS. ALL BEAM REACTIONS NOT LABELED SHALL BE DESIGNED FOR 10k MINIMUM (2 BOLT CONNECTION) PRECAST PLANK DIAPHRAGM SHALL BE DESIGNED FOR



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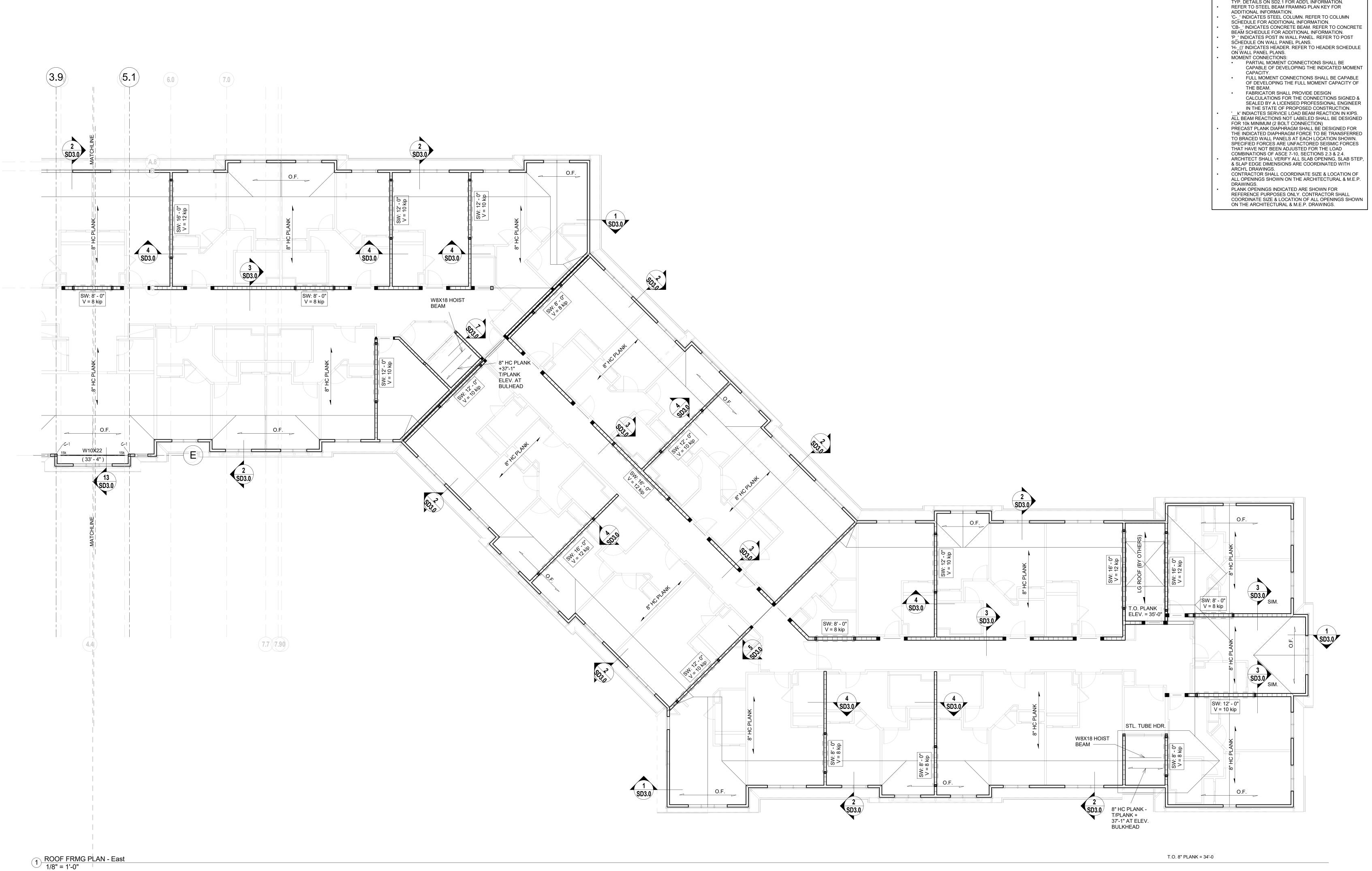
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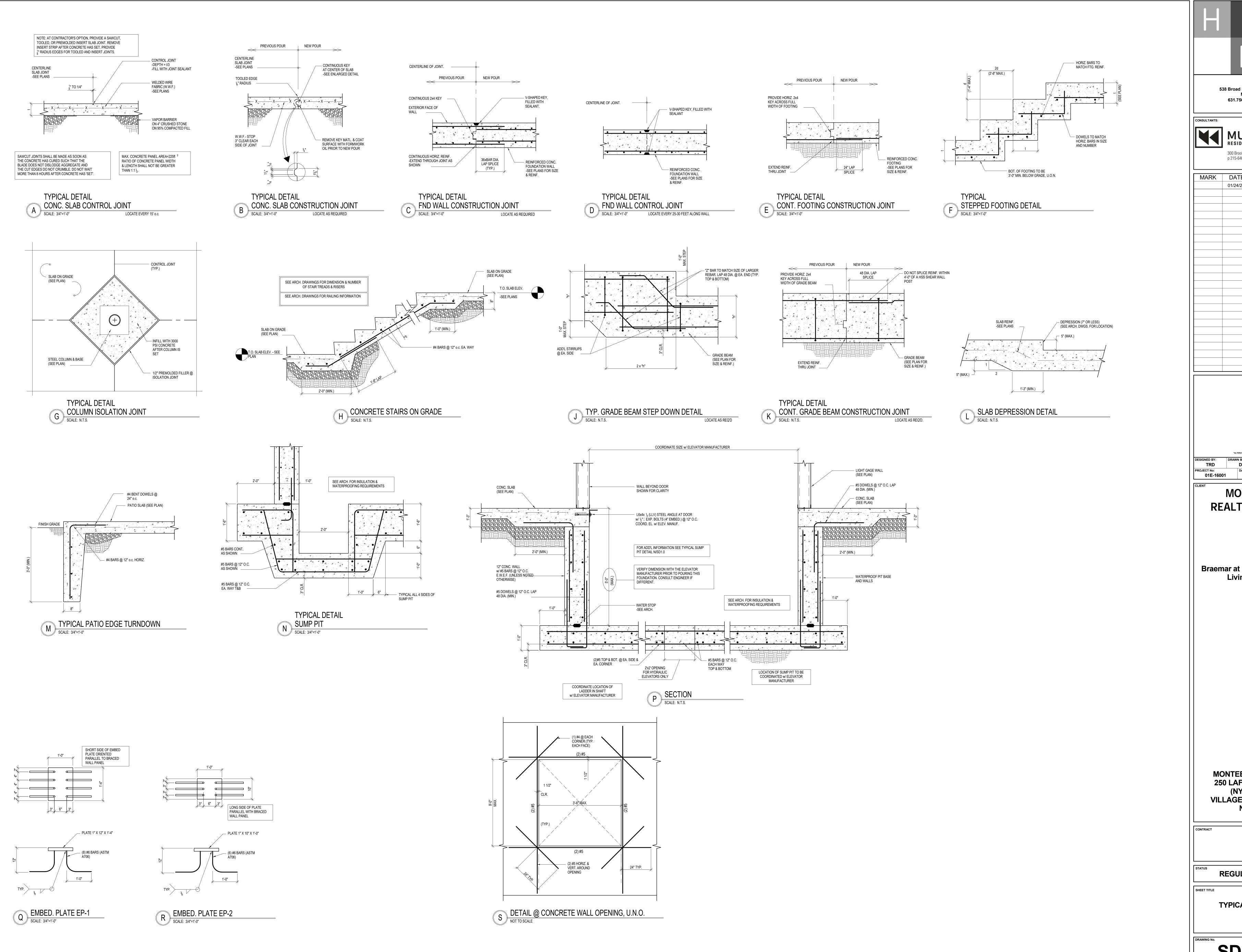
MONTEBELLO CROSSING 250 LAFAYETTE AVENUE (NYS ROUTE 59) VILLAGE OF MONTEBELLO NEW YORK

REGULATORY REVIEW

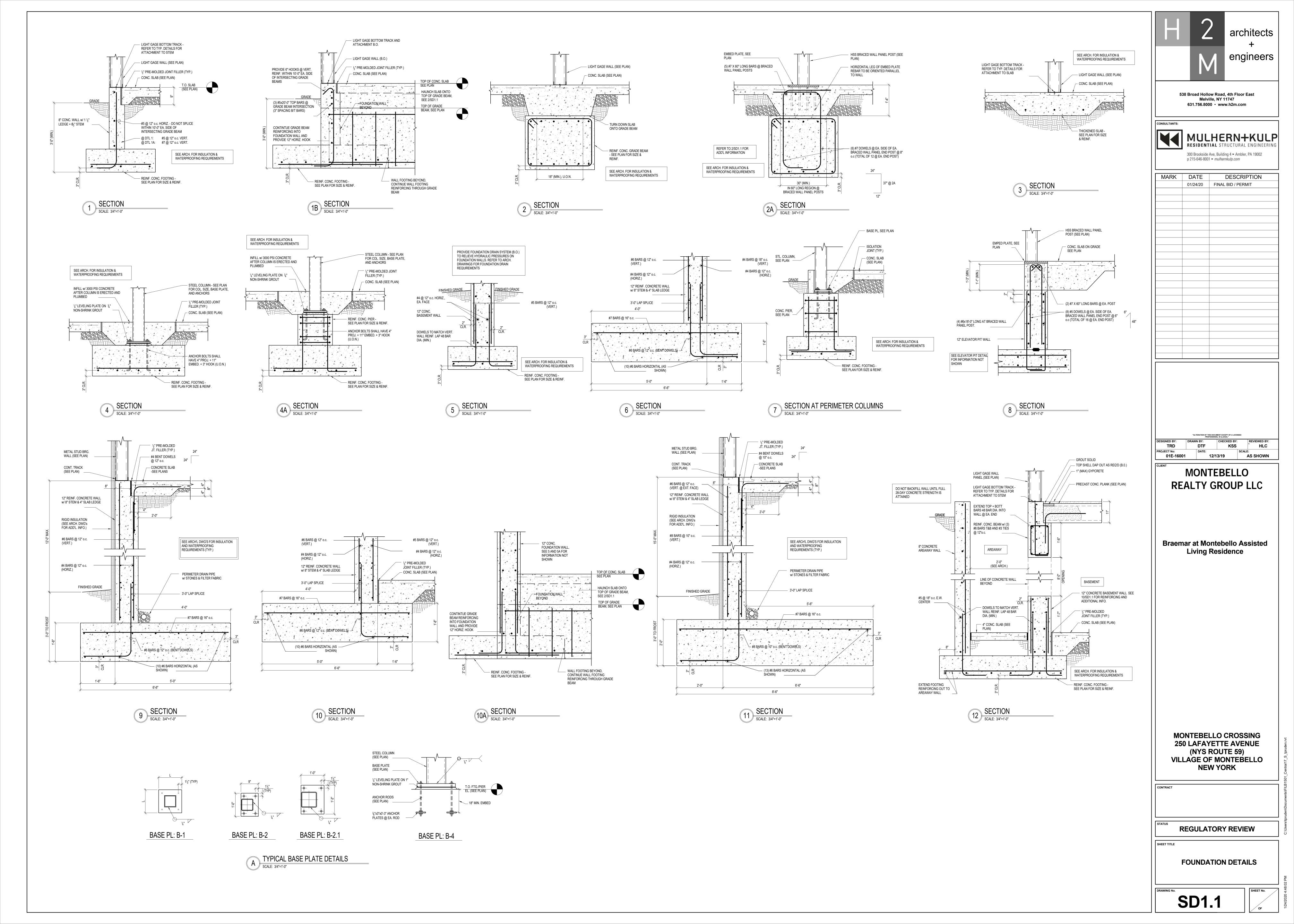
ROOF FRAMING PLAN

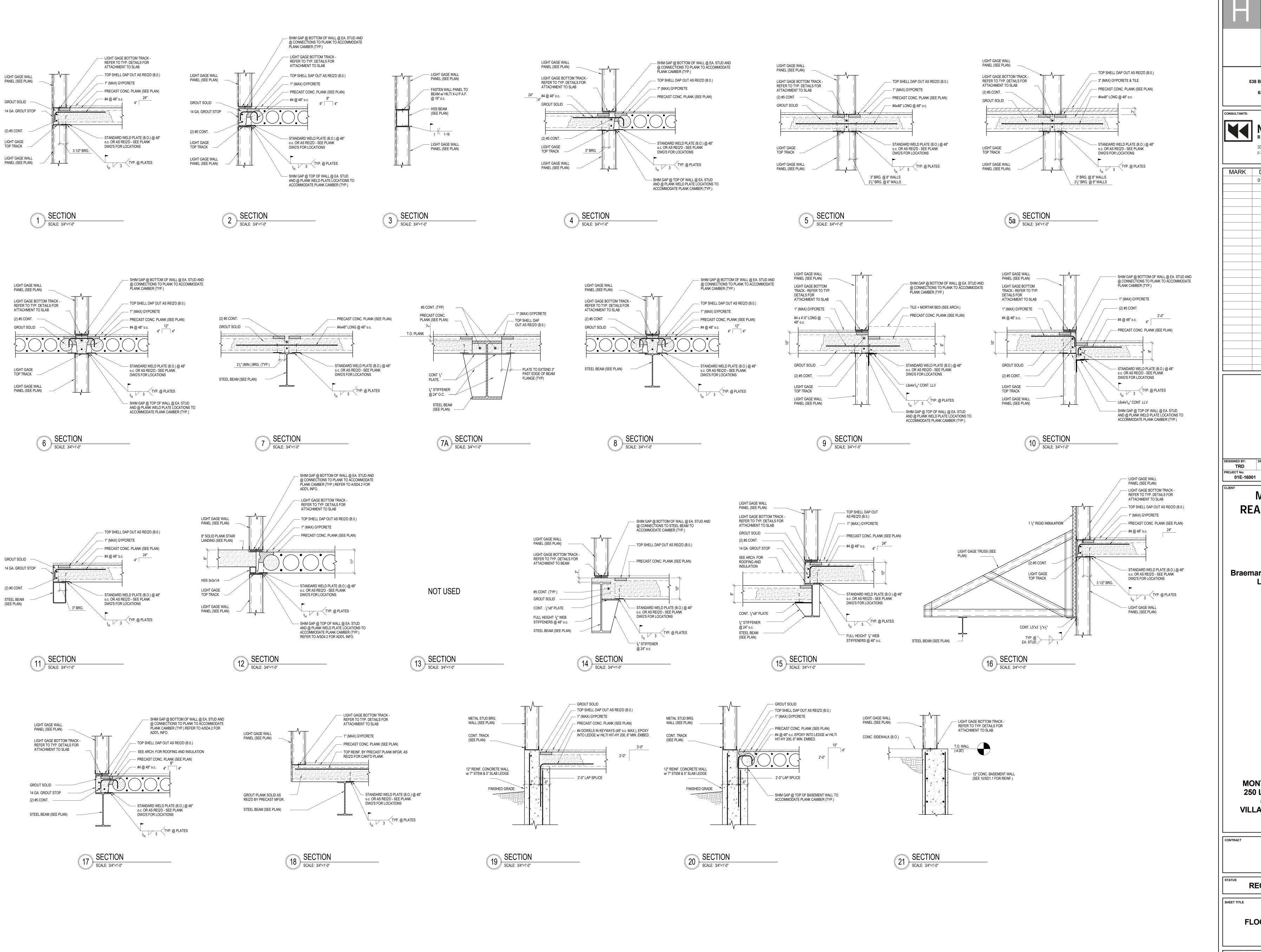
S5.1

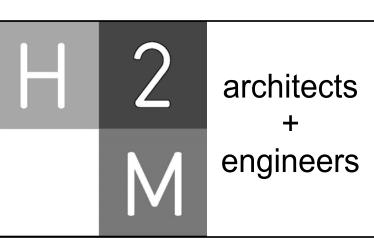




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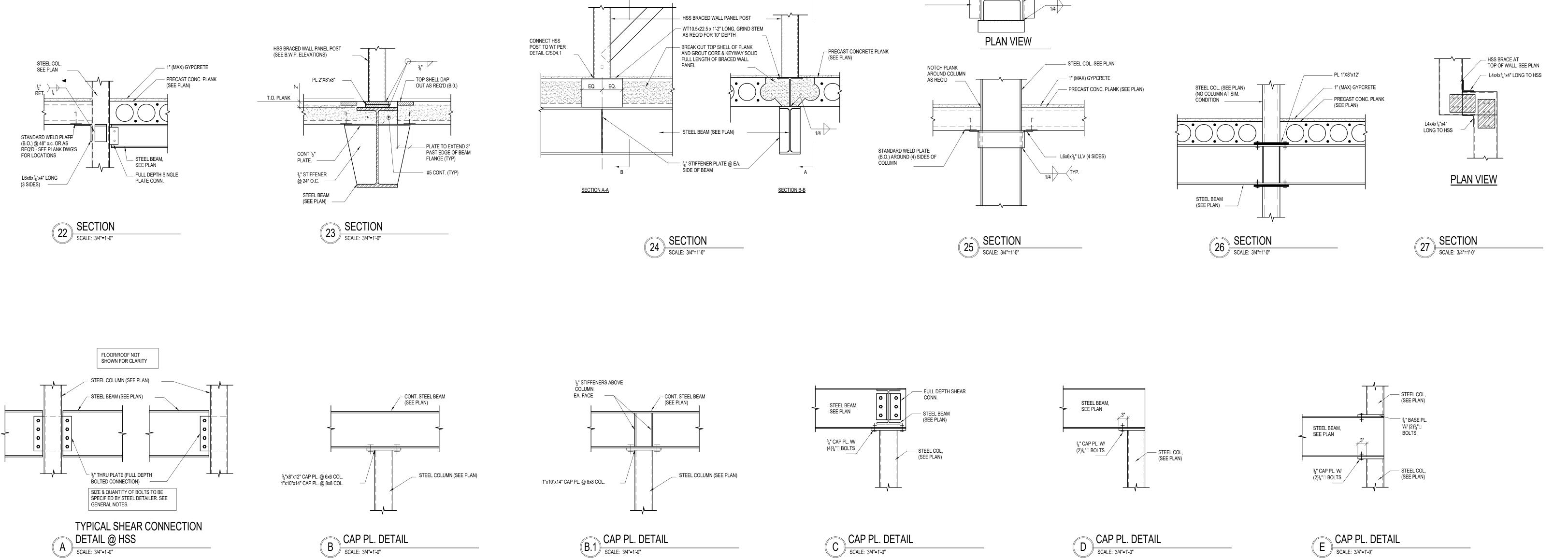
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MONTEBELLO CROSSING 250 LAFAYETTE AVENUE (NYS ROUTE 59) VILLAGE OF MONTEBELLO NEW YORK

REGULATORY REVIEW

FLOOR FRAMING DETAILS

SD2.0



CAP PL. DETAIL

MOMENT CONN. @ HSS COL.

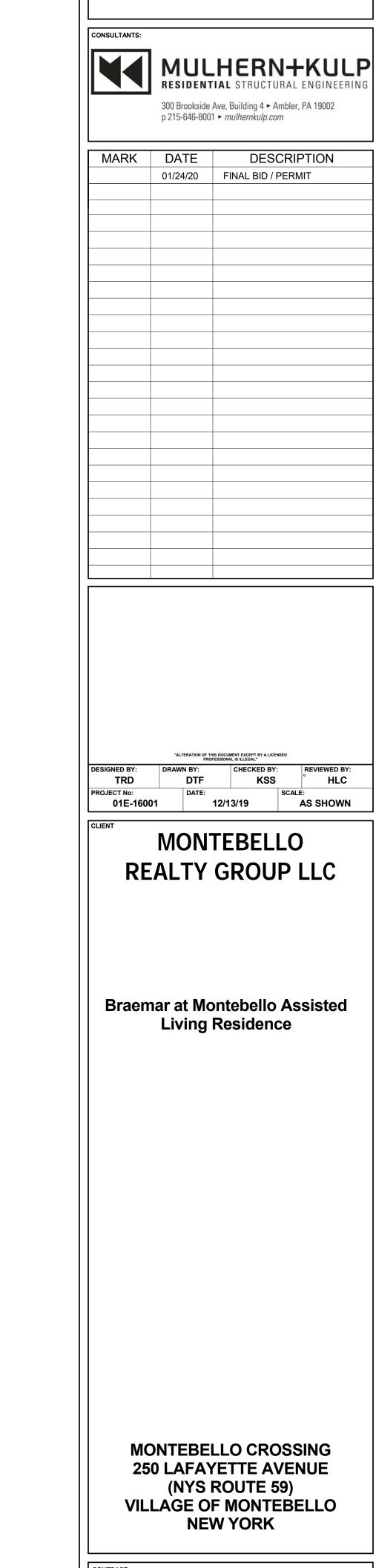
SINGLE PL. CONNECTION

1₂ BOT. PLATE

L6x4x38" LLV (4 SIDES)

ANGLES ONLY OCCUR AT COL. FLANGE AT SIM. CONDITION (2 SIDES)

__ STEEL COL. SEE PLAN



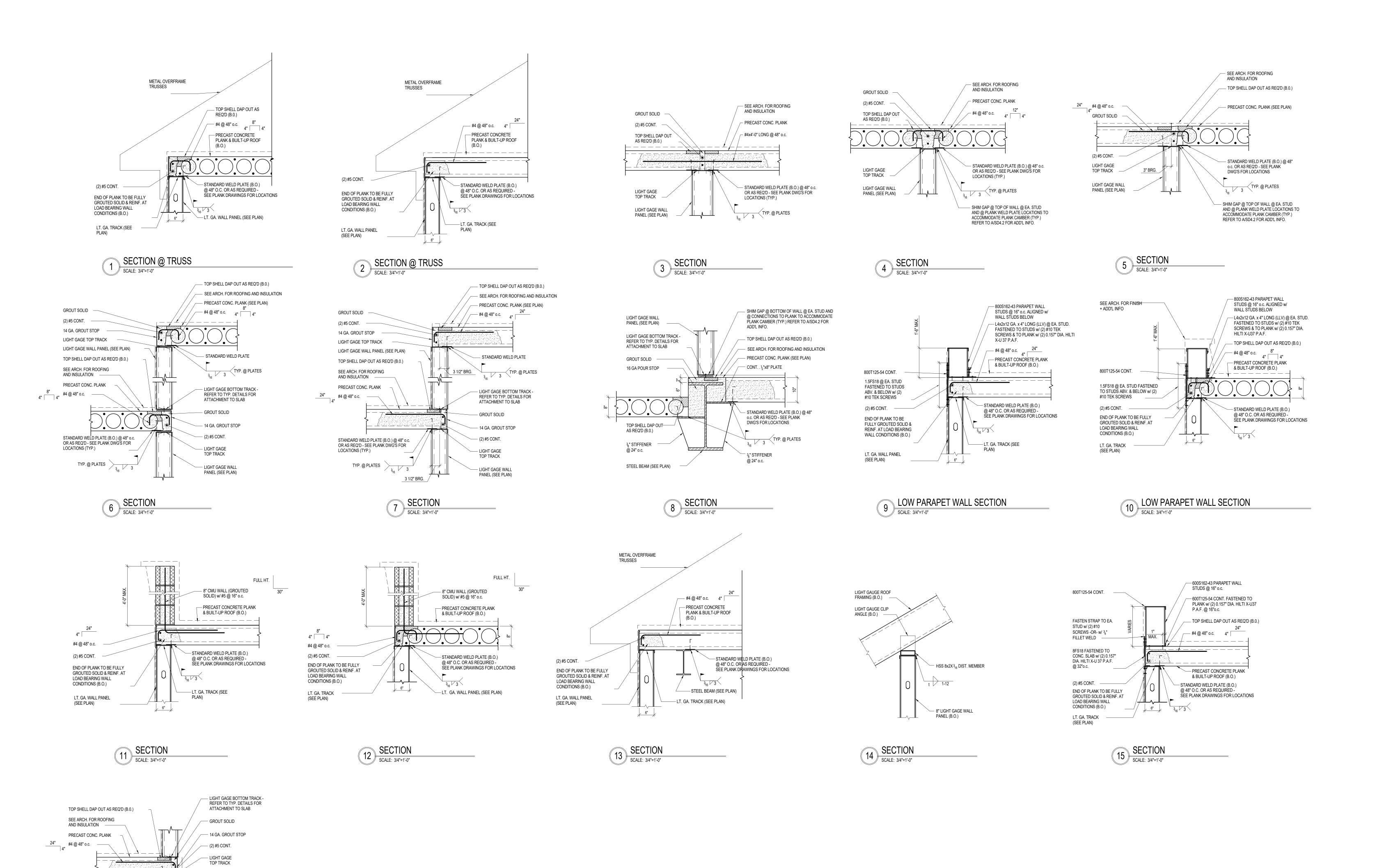
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SD2.1

FLOOR FRAMING DETAILS

REGULATORY REVIEW



- PRECAST CONCRETE PLANK & BUILT-UP ROOF (B.O.)

L6x4x58" (LLV) (LENGTH

STANDARD WELD PLATE (B.O.) @ 48" o.c. OR AS REQ'D - SEE PLANK DWG'S FOR LOCATIONS (TYP.)

16 SECTION SCALE: 3/4"=1'-0"

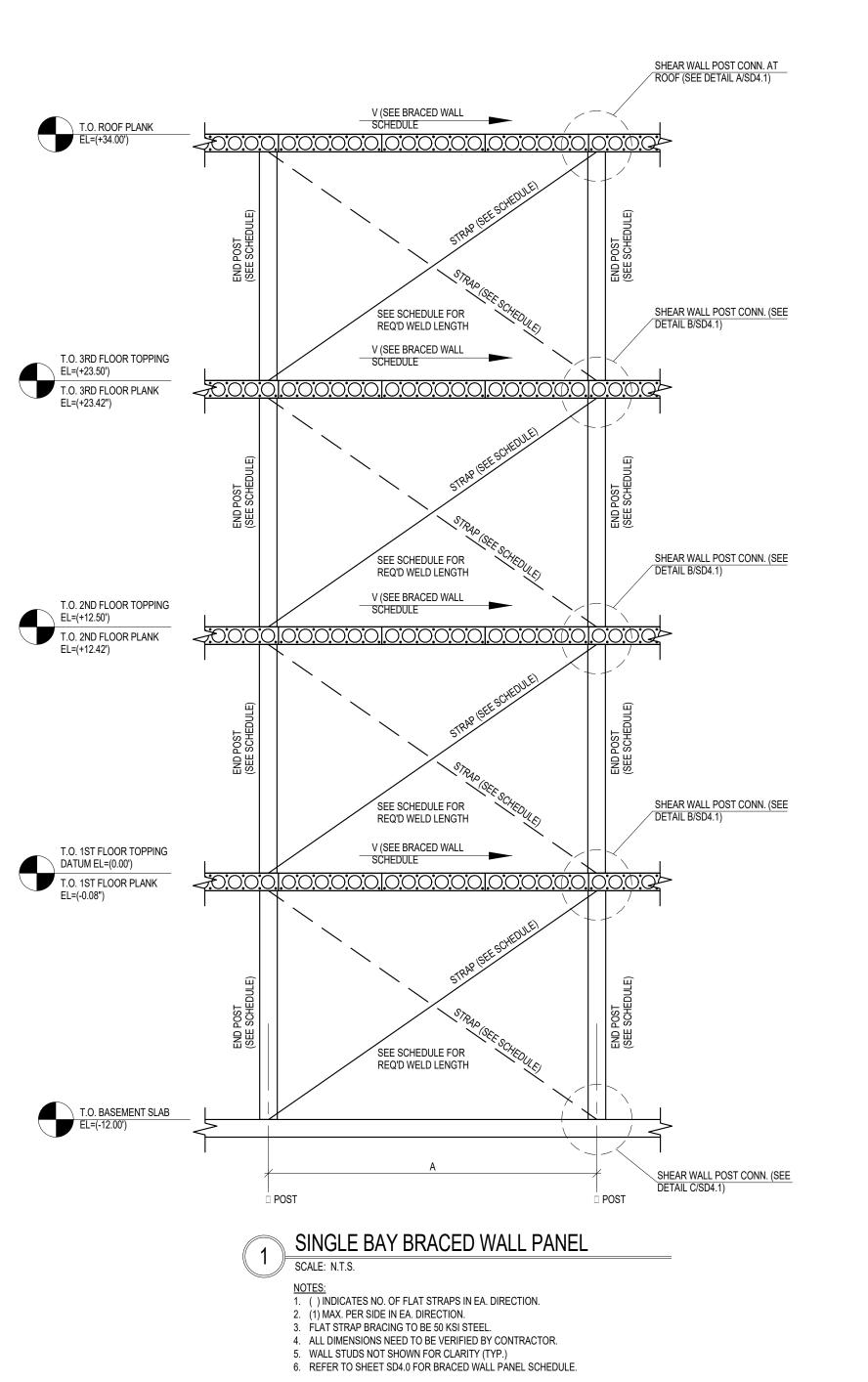


MONTEBELLO CROSSING 250 LAFAYETTE AVENUE (NYS ROUTE 59) **VILLAGE OF MONTEBELLO NEW YORK**

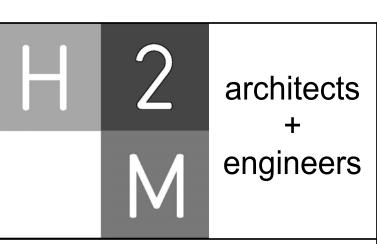
REGULATORY REVIEW

ROOF FRAMING DETAILS

SD3.0



S	shearwall Schedu	le
Level	SW Post Size	SW Strap Size
Basement Floor	HSS6X4X1/4	(2)10FS12
Basement Floor	HSS8X4X1/4	(2)10FS12
First Floor	HSS6X4X1/4	(2)10FS12
First Floor	HSS8X4X1/4	(2)10FS12
Second Floor	HSS6X4X1/4	(2)8FS12
Second Floor	HSS8X4X1/4	(2)8FS12
Third Floor	HSS6X4X1/4	(2)8FS14
Third Floor	HSS8X4X1/4	(2)8FS14



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REALTY GROUP LLC

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CONTRACT

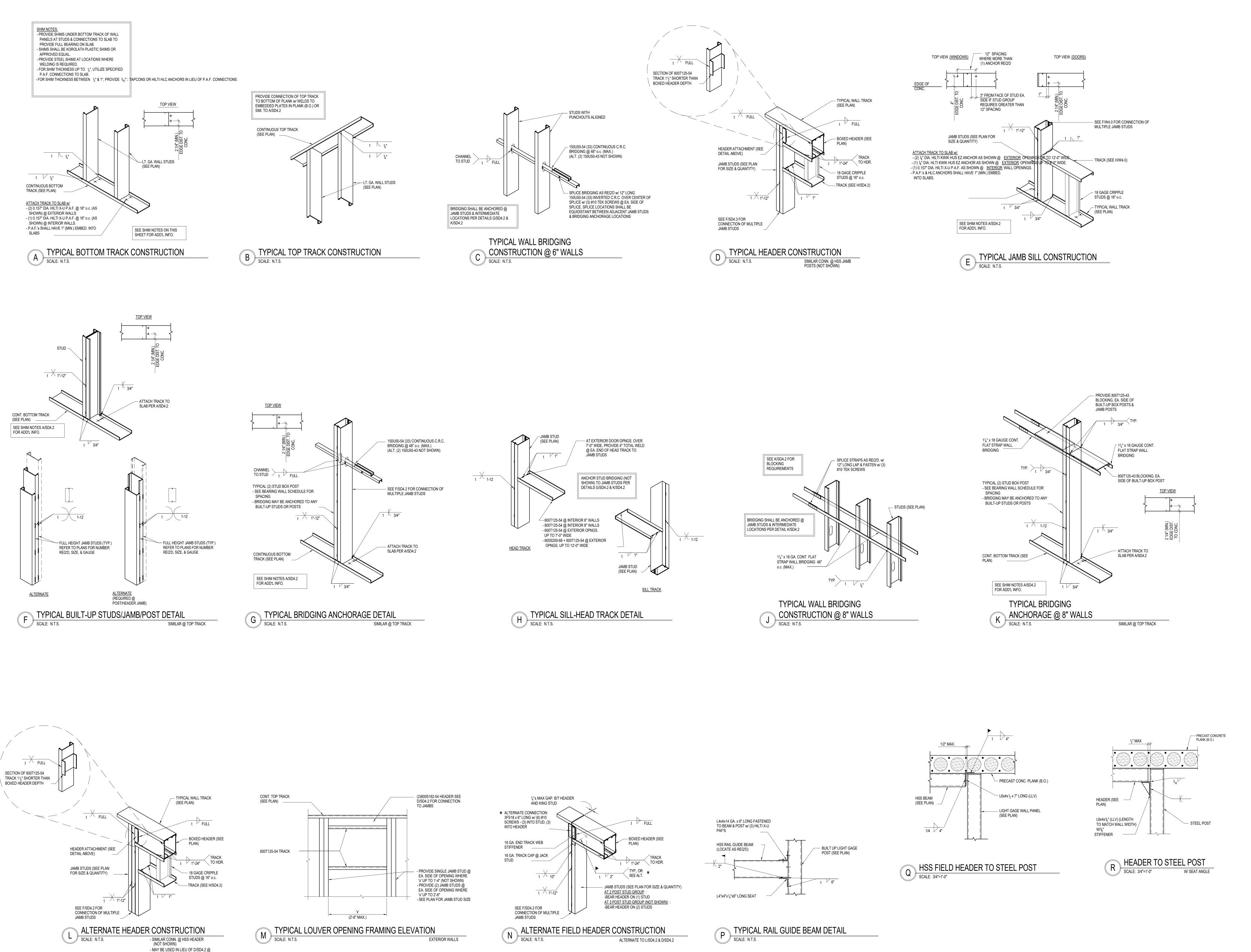
REGULATORY REVIEW

SHEET TITL

BRACED WALL PANEL ELEVATIONS

SD4.0

OF



PANEL FABRICATOR'S PREFERENCE

H 2 architects + engineers

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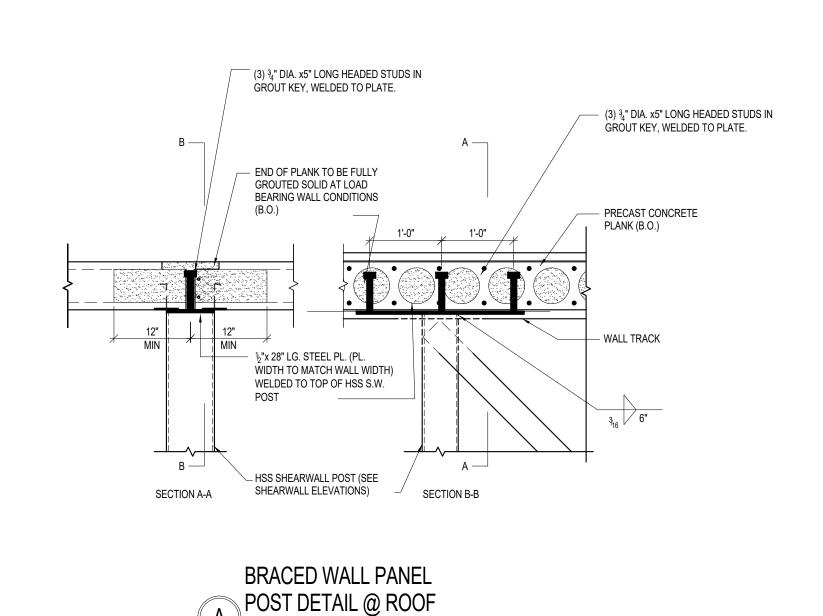
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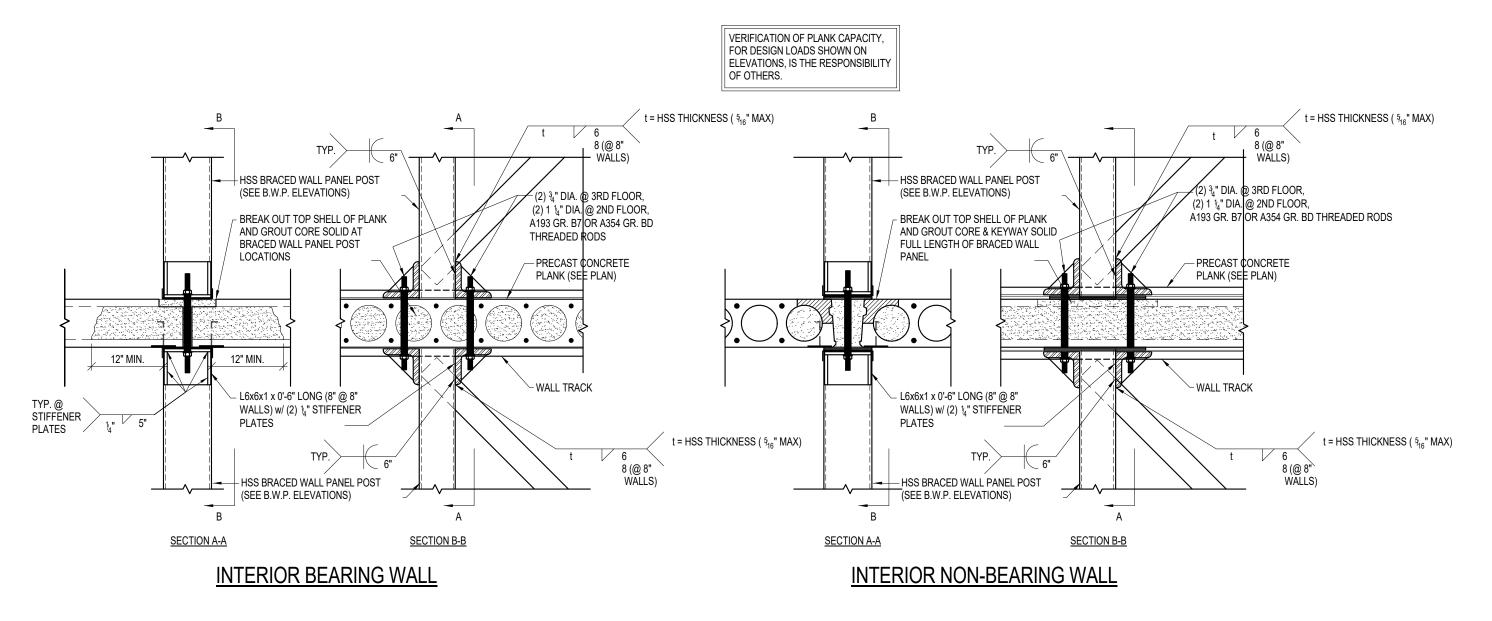
CONTRACT

REGULATORY REVIEW

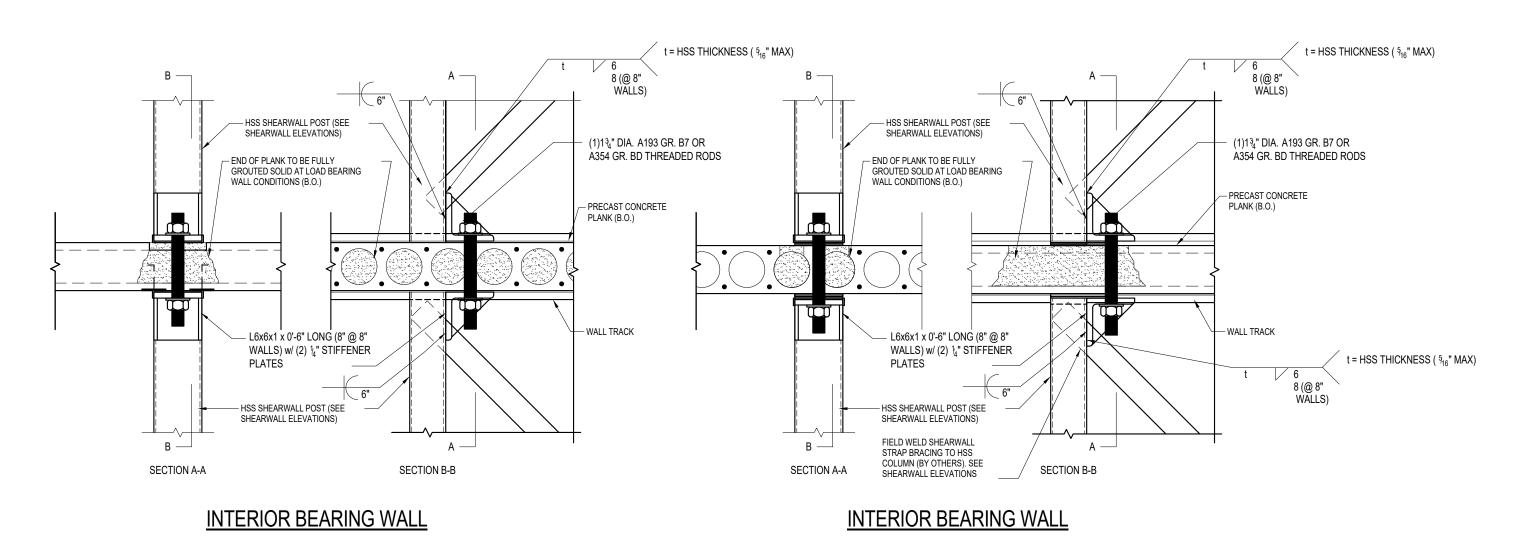
TYPICAL LIGHT GAGE DETAILS

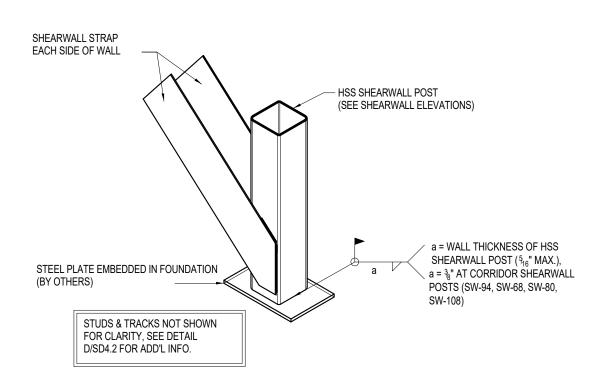
SD4 1

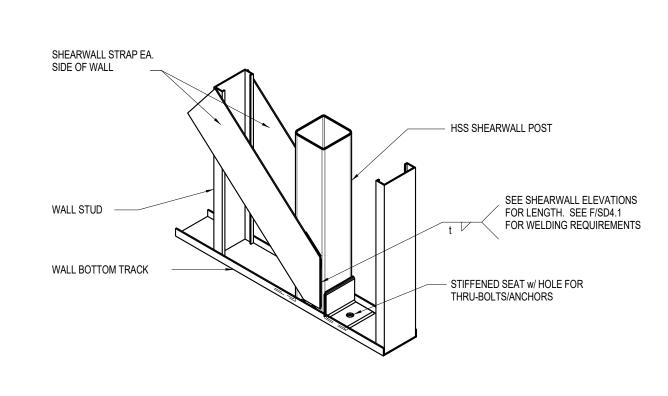




- REFER TO 'D.1' & 'E' FOR ADD'L INFO. - TYP. @ POSTS ALIGNING BETWEEN FLOORS



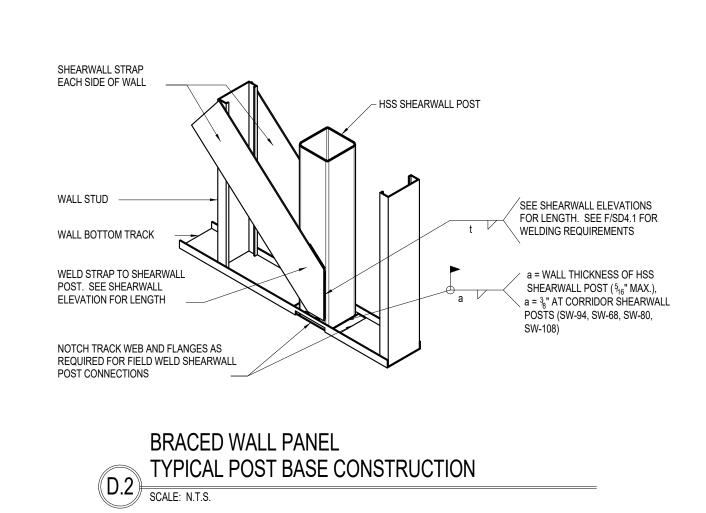


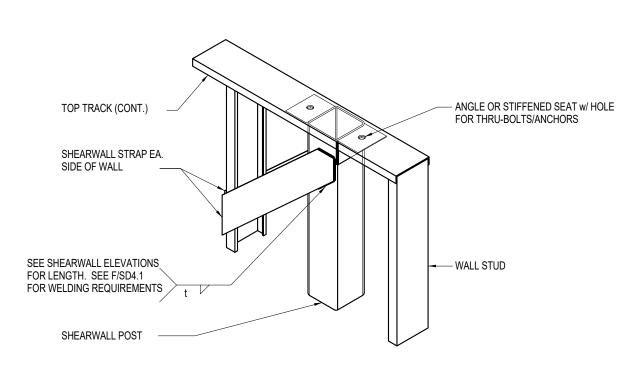


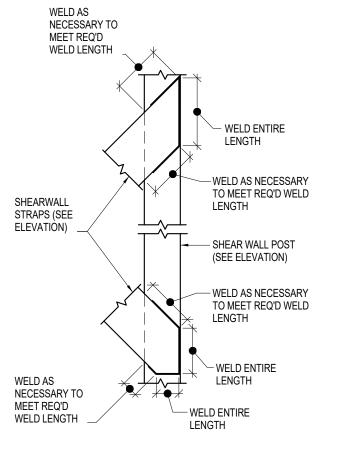
BRACED WALL PANEL POST DETAIL SINGLE SIDED CONNECTION ADJACENT TO DOORS

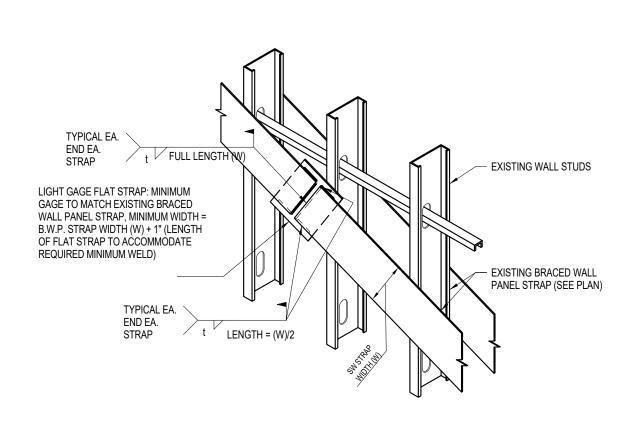
BRACED WALL PANEL TYPICAL POST BASE DETAIL @ FOUNDATION - SHEARWALL POSTS AT EMBEDDED PLATES - REFER TO 'D.2' FOR ADD'L INFO











BRACED WALL PANEL
TYPICAL POST TOP CONSTRUCTION

SCALE: N.T.S.

BRACED WALL PANEL TYPICAL STRAP WELDING

BRACED WALL PANEL TYPICAL STRAP SPLICE DETAIL

SCALE: N.T.S.

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Braemar at Montebello Assisted

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REGULATORY REVIEW

BRACED WALL PANEL DETAILS

SD5.0