#### General Notes:

- 1 The purpose of these drawings is to show the structural work associated with the alterations and addition to the School of the Holy Child, in Rye, New York.
- 2 The work shown on these drawings has been designed in accordance with the structural requirements of the 2020 edition of the Building Code of New York State which is part of the New York State Uniform Fire Prevention and Building Code and is based upon the 2018 International Building Code.
- 3 The structural components have been designed for the following loads:

#### A. Uniform live load:

Assembly areas 100 psf Lobbies 100 psf Movable seats Balconies and decks 1.5 times the live load of the area served, not greater than 100 psf Corridors 100 psf

First floor Schools Corridors above first floor 80 psf First-floor corridors 100 psf 40 psf + 15 psf partitions Classrooms 100 psf Stairs and exits

Live load has been reduced on girders, columns and footings in accordance with the building

#### B. Roof loads:

Snow:	
Ground snow load, pg	30 psf
Flat roof snow load, p <sub>f</sub>	30 psf*
Exposure factor, C <sub>e</sub>	1.00
Importance factor, I <sub>s</sub>	1.10
Thermal factor, C <sub>t</sub>	1.00
Drift surcharge load, p <sub>d</sub>	See plan.
Width of snow drift, W	See plan.
Rain loads: in accordance with Section 1611	
Roof live load:	20 psf min
Rainfall intensity (15-min. duration/100 year avg. interval) *Note: the flat roof snow load shall be no less than 30 psf.	6.00 in./hr.
transaction and transaction and an arranged the transaction and bear	

#### C. Wind design data:

Negative Zone 4

Wind loads have been determined based on Section 1609.1.1 in accordance with ASCE 7-16, Chapters 26, 27, 29 and 30, Directional Procedure

Risk category	Ш
Basic wind speed (3-second gust):	
Ultimate design wind speed, Vult	126 mph
Nominal design wind speed, Vasd	97.6 mph
Exposure	В
Internal pressure coefficient	0.18
"a" dimension for use with components and cladding	6.2 ft

-18.6

Design wind pressure (Nominal design wind pressures) for components and cladding on building walls (use Zone 4 generally; use Zone 5 within "a" of building wall corners): Surface pressure (psf)

-17.8

Design wind pressure (Nominal design wind pressures) for components and cladding on				
building roofs (for locations of zones 1, 2 and 3, refer to building code):				
Surface pressure (psf)				

•	Sur	face pressure (p	sf)	
Area	10 sq.ft	20 sq. ft.	50 sq. ft.	100 sq.ft.
Negative Zone 1, 2e &2r	-28.8	-24.4	-18.6	-14.2
Negative Zone 2n, & 3r	-31.7	-28.3	-23.9	-20.5
Negative Zone 3e	-38.8	-34.4	-28.6	-24.1
Positive all zones	15.7	13.9	11.6	10.0
Overhang Zones 1, 2e & 2r	-37.8	-33.4	-27.6	-23.2
Overhang Zone 2n & 3r	-40.7	-37.3	-32.9	-29.5
Overhang Zone 3e	-47.8	-43.4	-37.6	-33.1

### D. Earthquake design data:

Risk category	III
Seismic importance factor, I <sub>e</sub> :	1.25
Mapped short period spectral response accelerations, S <sub>s</sub> :	0.285g
Mapped 1 second period spectral response accelerations, S <sub>1</sub> :	0.060g
Site class:	D
Design short period spectral response accelerations, S <sub>DS</sub> :	0.299g
Design 1 second period spectral response accelerations, S <sub>D1</sub> :	0.096g
Seismic design category:	В
Seismic force resisting system: intermediate reinforced masonry	shear walls and

wall system with flat strap bracing

Design base shear:	50 KIPS
Seismic response coefficient, C <sub>s</sub> :	0.093
Response modification factor, R:	4.0
Deflection amplification factor, C <sub>d</sub>	4.0
Analysis procedure: Equivalent Lateral Force	

#### E. Other loads: Concentrated loads

	All floors except as noted (on 2-1/2 feet square) Stair treads and catwalks (on 4 inches square) Elevator machine room grating (on 4 inches square) All other roof members Impact loads	1000 lbs 300 lb 300 lbs 300 lbs]
	Loads increased as follows: Elevator machinery	100%
F.	Special loads: Retaining walls Lateral equivalent fluid pressure Seismic load (h = height of wall) Vertical live load surcharge	35 pcf 5.5 h^2 100 psf
G.	Guardrails (load applied in any direction):	

Top rail concentrated

Top rail uniform load

Intermediate rail concentrated load

H. Existing buildings: For existing structural elements carrying gravity loads, the proposed addition does not increase the stress in any structural element by more than 5 percent nor does it decrease the strength of

200 lbs

50 plf

50 lbs

This structure has been designed to be self-supporting and stable after the work shown on these drawings has been completed. The stability of the structure prior to completion is solely the responsibility of the contractor. This responsibility extends to all related aspects of the construction activity including, but not limited to, erection methods, erection sequence, temporary bracing, forms, shoring, use of equipment, and similar construction procedures. Review of the construction by the engineer is for conformance with design aspects only, not to review the contractor's construction procedures. Lack of comment on the part of the engineer with regard to construction procedures is

any structural element to less than required by the building code for new structures.

- not to be interpreted as approval of those procedures. This structure utilizes masonry shear walls and bracing elements, to provide lateral stability. Therefore, temporary bracing, guys, etc., must be maintained until all masonry shear walls have been erected, and bracing has been installed.
- Shoring note: The contractor is responsible for designing, providing and installing all temporary shoring that is required to support instabilities of existing structure during construction and due to the removal of existing supporting walls and existing framing members for installation of new framing and foundations. Shoring shall be fully installed and stable prior to removal of existing structural
- Jobsite safety and construction procedures are solely the responsibility of the contractor. Review of the construction by the engineer is for conformance with design aspects only, not to review the

- contractor's provisions for job site safety. Lack of comment by the engineer is not to be interpreted as approval of those aspects of work.
- PDF digital files of all erection and detail shop drawings for steel reinforcing bars (concrete and concrete -masonry construction), structural steel, cold-formed metal framing, wood trusses, glu-lam, and timber framing indicating the fabricator, manufacturer, finish, layout, and all accessories, must be submitted to and be checked by the contractor and subcontractor and bear the checker's initials
- before submission to the architect for review prior to fabrication. Fabrication and/or delivery to the site of components prior to receiving approved shop drawings shall be at the fabricator's own risk. Deferred submittals: Deferred submittals are those portions of the design that are not submitted at the time of application and are to be submitted to the building official within a specified period. The
- following deferred submittals shall be submitted to the engineer for approval.
  - A. Cold-formed metal framing
  - B. Structural glued laminated (glu-lam) timber framing

 C. Heavy timber framing. After approval by the engineer, the deferred submittal documents shall be submitted to the building official for approval. Deferred submittal items shall not be installed until the deferred submittal documents have been approved by the building official.

- Testing and inspection of concrete, steel reinforcing bars (concrete and concrete masonry construction), concrete masonry, structural steel, cold-formed metal framing, wood framing, wood trusses, wood sheathing, glu-lam framing, timber framing, and other work are described in the project "Statement of Special Inspections". The contractor shall review the "Statement of Special Inspections" and coordinate the scheduling of inspections with the special inspector. Uninspected work that required inspections may be rejected solely on that basis.
- 11 If faulty construction procedures, or material, result in defective work that requires additional engineering time to devise corrective measures, professional fees may be charged to the contractor at the standard hourly rate of additional services. Such fees may be withheld from the general
- 12 Loads, openings and structure in any way related to requirements of other (non-structural) disciplines are shown for bidding purposes only. However, these plans do not show the full scope of openings, in roofs, floors and walls. For size and location of all openings, see architectural and mechanical drawings. Do not scale openings. The contractor shall obtain from the heating and ventilating, electrical, plumbing and other trades the final approved size and location of all openings, equipment and work to be provided for their trade for roofs, floors and walls, whether shown or not shown on structural drawings. Excess cost related to variation in requirements or equipment are not
- 13 The contractor shall verify all dimensions, elevations and angles with architectural drawings and existing conditions before proceeding with any work.
- 14 The contractor shall field verify existing conditions before proceeding with any work. The contractor shall field verify all dimensions noted "±" that are indicated on the drawings.
- 15 The contractor and subcontractors shall obtain the latest copies of approved plans and surveys and they shall familiarize themselves thoroughly with these plans before commencing any work.
- 16 These drawings are supplemented by a detailed technical specification. The notes shown under certain categories of work are intended to summarize basic requirements.
- 17 Work shown as "Typical Details" apply throughout the project as required. Work shown as "Sections" shall be considered to apply for the same and similar conditions in the building.
- 18 Some details of the work are shown on the architectural drawings. A careful review and study of these details are necessary before the full scope of the work can be comprehended.
- 19 Do not scale drawings.

#### Codes and Standards References

#### 1 Concrete:

Concrete work shall conform to the requirements of: ACI 301-10, "Specifications for Structural Concrete in Buildings" and ACI 318-14, "Building Code Requirements for Structural Concrete".

Grades for Structural Glued Laminated Timber

- Concrete masonry work shall conform to the requirements of TMS 402-16, "Building Code Requirements for Masonry Structures" and TMS 602-16, "Specifications for Masonry Structures".
- Design, fabrication and erection of structural steel shall conform to the "Specification for Structural Steel for Buildings" as adopted on July 7, 2016, by the American Institute of Steel Construction (AISC) and the 15th Edition of the AISC Steel Construction Manual.
- 2 Cold formed metal framing: Design, fabrication and erection of cold-formed metal framing shall conform to the American Iron and Steel Institute's "Specification for the Design of Cold-formed Steel Structural Members", 2016.
- Wood construction shall conform to the 2018 ANSI/AWC NDS-2018. "National Design Specification for Wood Construction" with the 2018 supplement.
- Structural glued laminated timber: Glued laminated timber shall be manufactured in conformance with AITC 117-10, "Standard Specification of Structural Glued Laminated Timber of Softwood Species" ppearance of glued laminated timbers shall conform to ALLC 110-01, "Standard Appearance

- 1 The site preparation and earthwork within the perimeter of the proposed new structure shall include, as a minimum, the complete removal of all topsoil, organic and unsuitable fill materials. Proof compact the top of the remaining excavated surface. The removed soil shall be replaced with
- compacted structural fill where required for the support of foundations and slabs on grade. 2 The foundations have been designed to rest on inorganic, undisturbed soil or compacted granular fill having a presumptive bearing value of 3000 psf. Such bearing strata are anticipated at the bottom of footing elevations noted on the foundation plan. All bearing strata shall be reviewed by the engineer prior to placing concrete in order to verify the presumptive bearing value.
- 3 If ledge rock is encountered above the proposed bottom of footing elevations indicated, then it shall be removed to the extent necessary to provide a minimum of 1'-0" soil cover above the top of the
- 4 Footings supported on ledge shall rest on broom clean solid rock. If the slope of the rock surface exceeds 1 on 6, then the footing shall be dowelled to the ledge with 3/4" steel rods drilled 10 inches into the rock surface at 2 feet on center.
- 5 If ledge rock and soil both occur at the bottom of footing bearing elevation, then a minimum of one foot of ledge rock below planned bottom of footing elevation shall be removed and replaced with a "cushion" layer of crushed stone to minimize differential settlement. The crushed stone "cushion" layer shall extend beyond the end of the ledge rock for a minimum of 10 feet along the length of the supported footing.
- 6 In areas requiring structural fill, the fill material shall be a uniformly graded mixture of sand and gravel weighing no less than 120 pcf dry density after compaction in place. This mixture shall be uniformly graded having no stone greater than 3 inches in any one dimension, with no more than 90 percent by weight passing a 1-1/2-inch sieve, and with less than 12 percent by weight, passing a no. 200 sieve. A soils testing lab, hired by the owner, shall test each on-site or borrow soil material proposed for backfill for classification according to ASTM D 2487 and for laboratory compaction curve according to ASTM D 1557. Uniformly moisten or aerate subgrade and each backfill layer before compaction to within 2 percent of optimum moisture content. The fill material shall be placed in maximum lifts of 8 inches in loose depth for material compacted by heavy compaction equipment, and in maximum lifts of 4 inches loose depth for material compacted by hand-operated tampers. Each lift shall be compacted with appropriate equipment to a minimum of 95 percent of its maximum density at or near optimum moisture. No lifts shall be placed when weather conditions are such that the moisture content of the fill cannot be properly controlled. In placing and compacting fill and backfill material, do not damage nor displace concrete work already in place by contact from compaction machinery, by subjecting it to overturning from heavy compacting loadings, or any other cause. Place fill against such concrete work at the same rate as the remainder of fill, compacting uniformly on both sides using hand - operated tampers. A soils testing lab, hired by the owner, shall test compaction of soils in place according to ASTM D 1556, ASTM D 2167, ASTM D 2922, and ASTM D 2937 as applicable. When test reports indicate that backfills have not achieved the degree of compaction specified, scarify and moisten or aerate, or remove and replace soil to depth required; recompact and retest until specified compaction is obtained.
- The slab-on-grade sub-base shall be a crusher run stone free from soft disintegrated pieces, mud, dirt, or other injurious material. The material shall have no stone greater than 2 inches in any one dimension and with less than 10 percent by weight passing a No.100 sieve
- The bottom of exterior footings not on solid rock shall be at least 3' 6" below finished grade. All soil surrounding and under footings shall be protected from freezing and frost action during the course of construction.
- 10 Step footings where elevations change at a maximum slope of one vertical on two horizontal and place lower footings first. Basement wall foundation walls shall not be backfilled before they are temporarily braced or before the concrete has attained its specified compressive strength and the framed floor system at the top
- of the wall is in place. 12 Frost wall foundation walls shall be backfilled by placing fill on both sides simultaneously and to the same level.

15 Existing utilities: locate existing underground utilities in areas of excavation work. Provide adequate

- 13 Keep foundation excavations free of water at all times. Use crushed stone backfill or controlled compacted fill or lean concrete (fc=1500 psi) for overexcavation of footings.
- means of support and protection during earthwork operations. 16 Where footings are in close proximity to sub-surface piping bottom of footings shall be at least 8" below elevation of piping unless otherwise shown on the drawings.
- 17 Submittals to the engineer are required for structural fill.

#### Concrete Notes:

- All concrete work shall conform to all the requirements of ACI 301, "Specifications for Structural Concrete in Buildings" and ACI 318 "Building Code Requirements for Structural Concrete", as specified in the code reference section of these general notes.
- Concrete shall be the specified weight and develop a minimum compressive strength in 28 days as

Location	<u>Weight</u>	Minimum Strength	W/C Ratio (or slur where indicated)
Footings	Normal	3,000 psi	0.55
Walls and piers:			
Interior	Normal	3,000 psi	0.55
Exterior	Normal	4,000 psi	0.45
Exterior exposed	Normal	5,000 psi	0.40
Slabs-on-grade -interior	Normal	4,000 psi	0.45

- All detailing fabrication, and erection of reinforcing bars, unless otherwise noted, must follow the latest ACI code and the latest ACI "Manual of Standard Practice for Detailing Reinforced Concrete
- Concrete design mix will be submitted to the engineer for review, together with laboratory reports attesting that the mixes can attain the minimum strength required in accordance with ACI 301 indicated above.
  - Portland cement shall be Type I or Type II and conform to ASTM C 150.
- Other cementitious material such as flyash or ground granulated blast- furnace slag may be blended with cement for use in the concrete mix. Flyash shall conform to ASTM C 618 and may replace cement if the following ranges for the 2 classes of flyash; Class C, 20 to 35%; Class F, 15 to 25%. Ground granulated blast- furnace slag shall conform to ASTM C 989 and may not exceed 50% of total weight of cementitious materials.
- For normal weight concrete: coarse aggregate shall be 3/4" and conform to ASTM C 33. Fine aggregate shall be manufactured or natural sand from the same source for the entire project and shall conform to ASTM C 33.
- No admixtures are permitted without the engineer's written permission other than entrained air. Concrete exposed to the weather, such as that used in foundation walls, shall contain 5% +/- 1 1/2% entrained air. Concrete exposed to the weather and to de-icing compounds shall contain 6% +/- 1
- 1/2% entrained air. Do not use air entrainment admixture for interior normal weight concrete slabs. Limit water-soluble, chloride-ion content in hardened concrete to the following percent by weight of cement: 1.00 for reinforced concrete that will be dry and protected from moisture, 0.30 for reinforced concrete that will exposed to moisture but not exposed to chlorides, 0.15 for reinforced concrete exposed to moisture and chlorides from deicing chemicals and salt/seawater, and 0.06 for prestressed (post-tensioned) concrete.
- 10 Reinforcing steel shall conform to ASTM A 615, Grade 60.
- 11 Welded wire fabric shall conform to ASTM A 1064 with a minimum yield strength of 65 ksi. Lap one
- mesh size at sides and ends, and wire together. 12 Vapor retarder shall be Stego Wrap (15 mil) vapor retarder by Stego Industries LLC and shall have a water vapor permeance after conditioning (ASTM E 1745, Paragraphs 7.1.2 – 7.1.5) that is less than 0.01 perms and meets the requirements of Class A. Place as per ASTM E 1643 and manufacturer's
- 13 The following concrete cover shall be provided for reinforcement:

Location	Cover (inches)
Concrete cast against and	
Permanently exposed to earth	3
Concrete exposed to earth or weather:	
#6 through #18 bars	2
#5 bar and smaller	1 1/2
Concrete not exposed to weather or in contact with ground: Slabs, walls, joists:	
#14 and #18 bars	1 1/2
#11 bar and smaller	3/4
Beams, columns	
Primary reinforcement, ties, stirrups, spirals	1 1/2

- 14 The conveyance, placement and protection of the concrete shall conform to the requirements of ACI 318, indicated above, and ACI 304R, "Guide for Measuring, Mixing, Transporting and Placing Concrete". Mechanical vibrators are to be used to consolidate the freshly cast concrete around the reinforcing and against form surfaces and to prevent the formation of air or stone pockets. honeycombing, pitting or planes of weakness. However, care must be used to avoid over vibration that can lead to aggregate segregation.
- 15 No welding of reinforcing will be permitted.
- 16 All lap splices shall be Class B, in accordance with ACI 318 indicated above.
- 17 Concrete piers: Place concrete piers and walls together. Set pier reinforcing and set wall reinforcing through pier vertical bars. Provide dowels with standard hook from footing at all piers. Size and quantity of dowels to match vertical pier reinforcing (Class "B" splice).
- 18 The contractor shall be responsible for limiting pours to minimize shrinkage cracking. In general, walls shall not be poured in continuous lengths exceeding 30 feet without providing construction joints or control joints. The location and configuration of joints exposed to view shall be coordinated with the architect.
- 19 The installation of slabs shall conform to the requirements of ACI 302.1R, "Guide to Concrete Floor and Slab Construction". Interior finish slab surfaces are to have a steel trowel finish. <Use the following where surfaces of slabs form the substrate for mud jobs.> [Surfaces of slabs forming the substrate for mud jobs are to have a clean textured (scratched) surface.] Exterior slab surfaces are to have a broom finish unless specified on the architectural drawings.
- 20 Expansion and isolation-joints: Filler strips: ASTM D 1751, asphalt-saturated cellulosic fiber or ASTM D 1752, cork or selfexpanding cork
- Sealant at top of joint: Sika's Sikaflex 2c SL poly urethane elastomeric sealant. Provide cap to separate sealant from filler. 21 Waterstops: Flexible polyvinyl chloride (PVC) waterstop conforming to Corp of Engineers
- Specification CRD-C 572, with flat dumbbell and center bulb.
- 22 The curing and protection of concrete shall conform to the requirements of ACI 318 and ACI 308R. "Guide to Curing Concrete". Concrete slabs shall be protected from loss of surface moisture for not less than 7 days using a curing compound conforming to ASTM C 309 or constantly wetted burlap. Curing compounds shall be compatible with any intended flooring overlay. Do not install finish flooring until slab has adequately dried per the flooring manufacturer's specifications.
- 23 Cold weather concrete placement: If cold weather concreting conditions exist as defined by a period of more than three days when the average outdoor temperature, (high + low)/2, is less than 40 deg. F. the procedures outlined in ACI 306.1, "Standard Specification for Cold Weather Concreting" and ACI 306R, "Guide to Cold Weather Concreting" shall be utilized.
- 24 Hot weather concrete placement: Maintain concrete temperature below 90 deg. F. at time of placement and comply with ACI 301 and ACI 305R, "Guide to Hot Weather Concreting".
- 25 Accurately position, support, and secure reinforcement and anchors against displacement. Locate and support reinforcement with bar supports to maintain minimum concrete cover. Do not tack weld crossing reinforcing bars. Provide bar supports as follows:
- A. Bolsters, chairs, spacers, and other devices for spacing, supporting, and fastening reinforcing bars and welded wire reinforcement in place. Do not "wet stick" reinforcement or anchors. Manufacture bar supports from steel wire, plastic, or precast concrete according to CRSI's
- "Manual of Standard Practice," of greater compressive strength than concrete. 26 Sizes and locations of all required embedded items, such as anchor bolts, piping sleeves, etc., for all trades shall be coordinated by the general contractor with other trades.
- 27 Submittals to the engineer are required for concrete mix designs, cement, reinforcing bars, admixtures, and aggregates.

#### Concrete Masonry Notes:

- 1 All concrete masonry work shall conform to the requirements of the "Building Code Requirements for Masonry Structures (ACI 530/ ASCE 5/ TMS 402)" and the "Specifications for Masonry Structures (ACI 530.1/ASCE 6/ TMS 602, as specified in the code reference section of these general notes. The compressive masonry strength, f'm, shall be 2,000 psi minimum. System components have
- been selected based on the unit strength method. 3 Concrete block shall be lightweight hollow load bearing masonry units conforming to ASTM C 90, with a minimum ultimate compressive strength of 2,000 psi on the net area of the units. Units shall be protected from moisture absorption.
- 4 Portland cement used in the mortar and grout shall conform to ASTM C 150. Masonry cement or mortar cement shall not be used. Mortar shall be Type S conforming to the volumetric proportions set forth in ASTM C 270. Use 1 part Portland cement; 0.25 to 0.5 parts hydrated lime or lime putty; and aggregate proportioned to

lime). Provide aggregate in loose, damp condition. Add water to produce a workable mix.

2.25 to 3 times the sum of the separate volumes of cementitious materials (i.e. Portland cement plus

Coarse grout used in pilasters and walls shall conform to the volumetric proportions set forth in ASTM C 476. Use one part Portland cement, 2.25 to 3 parts damp, loose sand, 1 to 2 parts 3/8" pea gravel. Add water to produce a flowable mix with an 8 to 11 inch slump. Alternatively, fine grout may be used that conforms to the volumetric proportions set forth in ASTM C 476 using one part Portland cement, 2.25 to 3 parts damp loose sand and adding water to produce

a flowable mix with an 8 to 11 inch slump.

- 7 Steel reinforcing bars shall conform to ASTM A 615, Grade 60. Reinforcing bars to be lapped 48 bar diameters at splices. Reinforcement to be secured against displacement at spacing not exceeding 200 bar diameters.
- Joint (horizontal) reinforcement shall be hot-dipped galvanized W1.7 (9 gage) steel wire, ASTM A 951 with ASTM A 153 Class B-2 coating, Ladder type, by Hohmann & Barnard, Inc., or an approved equal. Place joint reinforcing in every second course (16"o.c.). Joint reinforcement shall be lapped 6 inches at splices.
- 9 Place units while mortar is soft and plastic. Remove and relay in fresh mortar any unit disturbed to
- the extent that initial bond is broken after initial positioning.
- 10 Fully bed units in all shearwalls including cross webs.
- 11 All cells with reinforcing bars or bolts shall be grouted solid.
- 12 Vertical cells to be grouted solid shall have a minimum clear opening of 3"x2-1/2". The entire
- perimeter of the cell shall be fully bedded with mortar. 13 Consolidate grout pours exceeding 12 inches in height by mechanical vibration and reconsolidate by mechanical vibration after initial water absorption and settlement has occurred. Grout pours exceeding 5.33 feet are high lift pours and shall require cleanouts. High lift grouting shall not be
- used unless high lift grout procedures are submitted to the engineer for review and approved by the 14 Cover the tops of all masonry construction to protect against precipitation.
- 15 Masonry shall not be constructed in temperatures below 40 F. Provide a heat source and protection as required to maintain temperature above 40 F in accordance with ACI 530.1.
- 16 Hot weather construction techniques shall be in accordance with ACI 530.1 and shall be implemented when the ambient air temperature exceeds 100 F, or 90 F if the wind speed exceeds 8
- 17 Unless otherwise shown on architectural or structural drawings provide vertical control joints through concrete masonry unit walls for full wall height as follows:
- A Distance between joints should not exceed the lesser of length to height ratio of 1.5, or 25 feet At changes in wall height
- At changes in wall thickness including pipe and duct chases and pilasters
- D At and above expansion joints in foundations and floors
- At and below expansion joints in roofs and floors that bear on the wall For openings, do not locate control joints within 32 inches of openings
- G Adjacent to corners of walls or at wall intersections within a distance equal to half the control
- Submittals to the engineer are required for certificates of compliance for block grade and strength grout, mortar, and reinforcing bars prior to delivery to the site.

#### Connections to Existing Masonry or Hardened Concrete:

- All proprietary anchoring systems (expansion, adhesive anchoring systems, etc.) to be installed into hardened concrete and masonry elements are to be installed in strict accordance with the manufacturer's instructions for drilling and preparation of holes, for spacing and edge distance requirements, and for the utilization of supplemental components for the anchoring systems such as screen tubes, doweling adhesives, etc.
- Connections to hardened concrete shall be made with anchors conforming to ACI 318, as specified in the code reference section of these general notes, for cracked concrete, and Chapter 19 of the state building code indicated at the beginning of these general notes.
- A. Mechanical anchors shall be either
- Hilti "Kwik Bolt TZ" expansion anchor. Hilti "Kwik HUS-EZ" screw anchor (use only in permanently dry, interior non-corrosive
- Simpson "Strong Bolt 2" expansion anchor

dry, interior non-corrosive environments)

- Simpson "Titen HD" screw ancho, zinc-plated or galvanized (use only in permanently dry, interior non-corrosive environments)
- Dewalt "Power-Stud + SD2" expansion anchor. Dewalt "Power-Stud + SD4/SD6", Type 304/316 SS expansion anchor Dewalt "Screw-Bolt+" screw anchor, zinc-plated or galvanized (use only in permanently
- Size, embedment, spacing and edge distance of anchors shall be as indicated on the
- B. Adhesive anchor rods or reinforcing bars shall be installed in rotary hammered drilled holes with carbide drill bits using one of the following adhesive anchoring systems: Hilti "HIT-HY 200" adhesive anchoring system with Hilt "HAS" ASTM F 1554, Grade 36
- anchor rods. Simpson "AT-XP" adhesive anchoring system for base material temperatures between 14 degrees and 80 degrees or Simpson "Set-3G" adhesive anchoring system for temperatures above 40 degrees, with Simpson "RFB" ASTM F 1554 Grade 36 anchor
- Dewalt "Pure110+" epoxy adhesive, standard cure or Dewalt "AC200+" two part
  - adhesive, cold temperature cure with ASTM F 1554. Grade 36 anchor rods. Reinforcing bars shall conform to the requirements of the Concrete General Notes Adhesive for reinforcing bars and anchors shall have been tested in accordance with ACI 355.4 "Qualification of Post-Installed Adhesive Anchors in Concrete" and ICC-ES
  - Concrete Elements" (AC308) for cracked concrete and seismic applications. Adhesive bond design strength is based upon concrete that has cured at least 21 days with a minimum compressive strength of 2,500 psi and an in-service temperature in

(ICC Evaluation Service) "Acceptance Criteria for Post-Installed Adhesive Anchors in

- accordance with ACI 355.4 Temperature Category B. Installation method shall be in accordance with the Manufacturer's Printed Installation
- Instructions (MPII Installation of adhesive anchors horizontally or upwardly inclined to support sustained tension loads shall be performed by personnel certified by the ACI/CRSI Adhesive
- Anchor Installer Certification program or equal. Unless otherwise noted on the drawings, embed anchor rods and reinforcing bars into drilled holes a minimum of 9 anchor diameters, with a minimum edge distance of 4 inches, measured from the edge of the concrete to the centerline of the
- anchor/reinforcing bar. Increased embedment depths or edge distances may be required at certain locations, see plans and details. Connections to grout filled concrete masonry shall be made with either 1) Hilti "HAS" ASTM F 1554, Grade 36 anchor rods using Hilti "HIT HY270" masonry adhesive anchoring systems or 2) Simpson "RFB" ASTM F 1554 Grade 36 anchor rods using Simpson "Set-XP" masonry adhesive anchoring system or 3) ASTM F1554 Grade 36 or ASTM A 193 Grade B7 high strength anchor rods using
- Dewalt AC100+ Gold masonry adhesive anchoring system. A. Unless otherwise noted on the drawings, embed anchor rods into drilled holes a minimum of 9 anchor diameters, with a minimum edge distance of 4 measured from the edge of the masonry to the centerline of the anchor. Increased embedment depths or edge distances may be
- required at certain locations, see plans and details. Connections to hollow concrete or clay brick masonry shall be made with either 1) Hilti standard "HAS-E" ISO 898 Class 5.8 anchor rods (minimum yield strength = 58 ksi and minimum ultimate strength = 72.5 ksi) using Hilti "HIT HY270" masonry adhesive anchoring system with "HIT-SC" composite screen tubes or 2) Simpson "RFB" ASTM F 1554 Grade 36 anchor rods using Simpson "SET-XP" masonry adhesive anchoring system with Simpson "Opti-mesh" plastic screen tubes or 3) ASTM F1554 Grade 36 or ASTM A 193 Grade B7 high strength anchor rods using Dewalt AC100+
- Gold masonry adhesive anchoring system with composite screen tubes. A. For anchors in hollow concrete masonry, embed anchor rods into drilled holes a minimum of 2 inches, with a minimum edge distance of 4 inches, unless otherwise noted, measured from the edge of the masonry to the centerline of the anchor. Increased embedment depths or edge
- distances may be required at certain locations, see plans and details. B. For anchors in hollow clay brick masonry, embed anchor rods into drilled holes a minimum of 3 1/2 inches, with a minimum edge distance of 4 inches, unless otherwise noted, measured from the edge of the masonry to the centerline of the anchor. Increased embedment depths or edge distances may be required at certain locations, see plans and details.

#### STREAM Center for Experiential Learning

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No. Date

4 08/12/2021 ISSUED FOR CONSTRUCTION 3 06/25/2021 ISSUED FOR PERMIT 2 | 05/08/2020 | 90% CONSTRUCTION

DOCUMENTS

1 02/21/2020 DESIGN DEVELOPMENT

Sheet Title GENERAL NOTES AND MATERIAL SPECIFICATIONS

11/26/2019 19126.00 Scale Drawn / Checked AS NOTED **Author Checker Sheet Number** 

#### Structural Steel Notes:

Anchor rods

Welding electrode

Shear connectors

- Design fabrication and erection of structural steel shall conform to the American Institute of Steel Construction's "Specification for Structural Steel for Buildings", as specified in the code reference section of these general notes.
- Materials:

American standard shapes, angles, Plates and bars:

Structural steel tubing, Rectangular and square ASTM A 36

- ASTM A 500, Grade C (Fy=50 ksi) ASTM F3125, Grade A 325 ASTM F 1554, Grade 36 ASTM E 70xx, low hydrogen 3/4" diam. ASTM A 108
- All welding shall conform to American Welding Society's AWS D1.1 "Structural Welding Code-Steel" code for arc and gas welding and be performed by a certified welder in accordance with A.W.S.
- All moment connections utilizing full or partial penetration groove welds, shall be ultrasonic tested, as indicated in the Statement of Special Inspections, and shall be detailed to allow for such ultrasonic testing.
- Structural steel shall be cleaned in accordance with the Steel Structures Painting Council Specification SP 3 for Power Tool Cleaning (except for steel exposed to weather) and painted to a minimum dry film thickness of 2 mils with a shop coat of Tnemec #10-99 alkyd rust inhibitive primer as manufactured by Tnemec Company, Inc. of Kansas City, MO, or an approved equal.
- All steel members and bolting exposed to weather shall be cleaned in accordance with the Steel Structures Painting Council Specification SP 6 for Commercial Blast Cleaned and hot-dipped galvanized in accordance with ASTM A 123 and ASTM A 153. Minimum acceptable zinc coating weight shall be 2 oz./sq. Ft. See architectural specifications for finished paint if required. Clean
- areas where galvanizing is damaged or missing and repair galvanizing to comply with ASTM A 780. Bent beams are defined as beams having its full section properties throughout its length, including at
- the bent. Provide full penetration welds all around at the bent. Provide 1/4" closure plates with seal weld at ends of all HSS members.
- 9 For miscellaneous steel, see architectural drawings.
- 10 Submittals to the engineer are required for certificates of compliance for structural steel, bolts, nuts, washers, and weld filler material prior to the fabrication of any steel
- 11 At the completion of fabrication, the fabricator shall submit a certificate of compliance stating that the work was performed in accordance with the approved contract documents, as required by Section 1704.2 of the building code indicated at the beginning of these General Notes

#### Steel Deck Notes:

- Steel deck shall be designed, fabricated, and erected in accordance with the current specification of the Steel Deck Institute.
- 2 Formed steel roof deck to be 1-1/2" deep, 20 gage (uncoated steel thickness = 0.0358"), galvanized (G60° coating), wide rib, United Steel Deck "B" Deck profile, as manufactured by Canam Steel Deck, Incorporated or an approved equal.
- End closures, roof sumps, closures at penetrations, and all other accessories necessary for a complete installation are required.
- 4 Formed steel roof deck shall be welded to supporting steel with 5/8" diameter puddle welds at all edge ribs plus a sufficient number of interior ribs to limit the spacing between adjacent points of attachment to 12" on center. Intermediate side connections shall be made with No.10 self-tapping screws at mid-span or 3'-0" on center, whichever is smaller. End laps of sheets shall be a minimum of 2" and shall occur over supports.

In lieu of puddle welds, pneumatically applied fasteners having the same capacity as the specified puddle welds, may be used. Fasteners shall be manufactured by Pneutek, Inc. or an approved equal. Shop drawings shall be submitted for approval to the engineer indicating fastener data including size vs. steel substrate material, spacings, capacities, including diaphragm shear capacities, method of installation and program for quality assurance of installation.

Steel deck must be protected before and after erection and all debris cleaned from its surface where concrete will be poured or roofing is to be placed.

#### <u>Lintel Notes:</u>

3 1/2" x 3/8" flat plate

3 1/2" x 4" x 5/16" (LLV)

3 1/2" x 5" x 5/16" (LLV)

Unless otherwise noted, for lintels over doors, windows, ducts, and miscellaneous openings in nonbearing 4", 8" and 12" CMU walls and brick walls use for each 4" of masonry: Angle size Max. M.O. <u>Bearing each end</u>

2'-0"

3'-6"

5'-0"

3 1/2" x 6" x 5/16" (LLV)	6'-0"	8"
3 1/2" x 6" x 3/8" (LLV)	8'-0"	8"
		els or lintel sizes are indicated shall
have 8" bearing at each end a	and shall have the following size	s:
Max. M.O.	Wall thickness	Lintel size
9'-0"	8"	W8x24

9'-0" 9"-13" W8x31 + 5/16" plate 12'-0" W8x28 12'-0" 9"-13" W8x35 + 5/16" plate Plates indicated in above lintels shall have a width 1" less than the wall thickness and a length 1"

Provide 7 1/2" x 5/8" x 0'-7 1/2" bearing plates on 3/4" grout bed with (2)-5/8" diameter x 6" long welded anchor studs at 3" o.c. unless otherwise noted. Field weld bottom flange of beam to bearing plate with 1/4" weld at each side of flange.

- For 6" masonry partitions use WT 7x13 for spans up to 7'-0" with 6" bearing each end.
- For 10" masonry partitions use WT 7x21.5 for spans up to 7'-0" with 8" bearing each end.
- Fill the first two courses directly under bearing with grout for 16" length.
- See architectural and mechanical drawings for size and location of openings
- When openings occur in bearing walls or the height of masonry above the lintel is less than the opening width or when a control joint is located directly above or within 16" of the jamb opening and drawings do not otherwise indicate a specific lintel design, consult with the architect to confirm lintel
- 8 Lintels over adjacent openings with piers between less than 2'-8" wide shall be continuous over piers. Masonry units of such piers shall be filled with grout for full story height.
- 9 Lintels supporting exterior masonry shall be hot-dipped galvanized. See structural steel notes. 10 Contractor shall design and provide shoring system as indicated in shoring note in general notes. 11 Lintels for openings in walls: These plans do not show the full scope of steel lintels required for new wall openings for doors, windows, ducts, louvers, etc.. For masonry opening size and location of all

wall openings, see architectural and mechanical drawings. Do not scale openings. For steel lintel

size for corresponding masonry opening size, see notes, above, unless otherwise noted on plans.

#### Cold-Formed Metal Framing Notes:

- 1. Design, fabrication and erection of cold-formed metal framing shall conform to the American Iron and Steel Institute's "Specification for the Design of Cold-formed Steel Structural Members", current edition as referenced by the applicable building code, and the specifications of the manufacturer of the cold-formed metal framing.
- 2. All studs and/or joists and accessories shall be of the type, size, steel thickness and spacing shown on the drawings. Studs, tracks, bracing and bridging shall be manufactured per ASTM C 955.
- 3. All studs, joists and accessories shall be formed from steel that conforms to the requirements of ASTM A 1003 with a yield as follows:
  - 16 ga.(0.0598") or heavier 50 ksi
- 18 ga (0.0474") or lighter 33 ksi
- 4. All studs, joists and accessories shall be galvanized with a minimum G-60 coating. 5. Touchup paint: Immediately after fabrication and erection, clean welds, fasteners, and damaged galvanized surfaces. Touchup and repair surfaces with galvanized repair paint in accordance with ASTM A 780, applied by brush or spray to provide minimum dry film thickness of 2.0 mils
- 6. [Where framing systems are not shown on the drawings, provide delegated design of framing systems for the design loads indicated in the contract documents. Provide for movement of framing members without damage or overstressing, sheathing failure, connection failure, undue strain on fasteners and anchors, or other detrimental effects when subject to a maximum ambient temperature of 120 deg F. Design framing system to maintain clearances at openings, to allow for construction tolerances, and to accommodate live load deflection of primary building structure for an upward and downward movement of L/360 for floors, L/240 for roofs.
- Connections shall be accomplished with self-driving screws or welding so that the connection meets or exceeds the design loads required at that connection.
- 8. All connections shall be made using a minimum of four (4) No. 10-16 screws, unless otherwise shown on drawings.
- 9. Screw spacing and edge distance shall not be less than 1". 10. Minimum connection angle thickness shall be 16 gage, but no thinner than the material of the
- members that are being connected, 11. Welding shall conform to Structural Welding Code D1.1 and Specification for Welding Sheet in Structures E1.3 of the American Welding Society and be performed by a certified welder in
- accordance with AWS standards. 12. Temporary bracing shall be provided and left in place until work is permanently stabilized. 13. Joists shall be located directly over bearing studs or a load distribution member shall be provided to

- All built-up framing members shall be of welded construction, unless otherwise noted. 15. Avoid holes at ends of members. However, should holes occur, provide additional reinforcing at the
- ends of the member where holes occur, unless otherwise noted. 16. Provide lateral blocking, bridging, and web stiffeners for vertical and horizontal framing members, and other framing members as required and in accordance with manufacturer's specifications or
- recommendations, unless indicated otherwise on the drawings. Track: 16 gage minimum, securely anchored to adjacent structure or member.
- 18. All framing components: cut squarely or at an angle to fit squarely against abutting members. All members: held firmly in position until properly fastened, erect member level, plumb, and true to line and to dimensions and elevations indicated.
- 19. Studs: Seated squarely in the track with the stud web and flanges abutting the track web, and securely attached to the flanges or web of both tracks.
- Splices in stude and other framing components: Not permitted.
- 21. Shape designations and section properties are based on the universal system for light gage steel framing members. For example, "600S162-54" denotes 6" deep, 1-5/8" wide flange, stud, 54 mils(=16 gage) thickness.
- 22. Submittals to the engineer are required for certificates of compliance for framing members (studs. joists, tracks, etc.), screws, and accessories (connection clips, stiffeners, etc.) prior to delivery to the
- 23. Shop drawings: Show layout, spacing, sizes, thickness, material specification, and types of coldformed metal framing. Show fastening and anchorage details, including mechanical fasteners. Show reinforcing channels, opening framing, supplemental framing, strapping, bracing, bridging, splices, accessories, connection details, and attachment to adjoining work. Regarding delegated design of cold formed framing, the cold-formed metal engineer and fabricator shall design only members and connections that are not shown on the drawings and shall submit calculations of all designs, signed and sealed by a qualified professional engineer registered in the

#### the cold-formed metal framing, including sizes, gage, spacing and connections, that are indicated on the drawings.

state of jurisdiction who is responsible for their preparation. They shall not design any changes to

- Wood design is based on the AF&PA NDS, "National Design Specification for Wood Construction with Supplement", as specified in the code reference section of these general notes
- Roof and floor decks shall be framed with the member sizes and/or types at the spacings shown on plan. The contractor shall coordinate locations of all plumbing piping, HVAC ducting and recessed lighting fixtures, etc prior to layout to minimize interference that may require the altering or strengthening of the installed framing.
- All wood framing is to be stored on site above the ground on "stickers" indoors or under tarps with adequate clearances to allow air circulation.
- Floors shall be installed level at the proper elevation. Roofs shall be installed at the pitches indicated on the architectural drawings
- Joists and rafters shall be installed directly over bearing studs unless otherwise detailed. At bearing walls above floor joists where the joists span perpendicular to the bearing wall above, provide blocking between the joists. Blocking width shall match bearing wall width above. Blocking shall be LVL material where joists are LVL joists and shall be dimensioned lumber where joists are dimensioned wood framing, unless otherwise noted.
- Joists and rafters shall be supported laterally at each support by full depth solid blocking two inches in thickness, except where joists are supported by a flush header or nailed to a rim joist. Flush framed connections shall be made with prefabricated galvanized steel hangers made by Simpson Strong-Tie, Co., Inc. or Kant-Sag connectors by United Steel Products Co. of width and depth appropriate for the supported member. Install with the type and quantity of fasteners recommended by the manufacturer. Prefabricated steel hangers used in contact with preservative pressure treated wood shall be hot dipped galvanized in accordance with ASTM A 123 or stainless steel, Type 316, or have a "Triple Zinc" (ASTM G 185) coating. Fasteners in contact with
- preservative pressure treated wood shall be hot dipped galvanized in accordance with ASTM A 153 or stainless steel, Type 316. Do not mix stainless steel and galvanized fasteners and connectors. Contractor shall choose metal connector (Simpson, USP, or approved equal) based on member reactions shown on drawings and/or connection configuration and provide product data to the engineer for approval. Metal connections are required at flush framed conditions, roof rafter holddowns, post caps and bases, and where indicated on plan.
- 10 Structural wood framing used in exterior applications or in contact with concrete or masonry shall be Southern Yellow Pine No. 2 or better, ACQ (Alkaline Copper Quaternary) or CA (Copper Azole) preservative pressure treated wood with a retention appropriate for end use.
- 11 Built-up members of three plies or less shall have adjacent plies nailed together with two rows of nails at 12" o.c. (10d common nails for 1-1/2" plies, 12d common nails for 1-3/4" plies). Built-up members of more than 3 plies shall be assembled with 1/2" diameter thru bolts at 16" o.c. staggered up and down with 2 inch clearance at top and bottom edges.

#### <u>Dimensioned Wood Framing Notes:</u>

General Wood Notes:

- 1 The structural wood stress grade stamped lumber shall be graded as follows:
  - Douglas Fir-Larch or Douglas Fir-Larch (North), No. 2.  $F_b$  (base) = 850 psi F<sub>c</sub> (parallel to grain) = 1400 psi
    - F<sub>c</sub> (perpendicular to grain) = 625 psi  $F_v$  (base) = 180 psi E (base) = 1.600.000 psi
- The design of the dimensional lumber members and their connections is based on the lumber having a moisture content at the time of installation of 19% or less.
- Joists or rafters are to be installed with "crown" up (i.e. positive camber) and within 1/4 inch in 10 feet straight, end-to-end alignment.
- Severely distorted (twisted, bowed, cupped, checked, etc.) lumber shall not be used. Notches in the top or bottom of dimensioned lumber joists or rafters shall not exceed one-sixth the member depth and shall not be located in the middle third of the span. End notches shall not exceed one-fourth the member depth. Bored holes shall not be within two inches of the top and bottom of the member and their diameter shall not exceed one-third the member depth, unless

#### Wood Sheathing Panels:

otherwise noted on plans.

- Sheathing panels, plywood or oriented strand board ("OSB"), shall conform to U.S. Product Standard PS-1, and bear the APA grade-trademark of the American Plywood Association. Only plywood shall be used on floors.
- Sheathing panels for floors, roof, and walls shall be APA rated sheathing, Exposure 1, with a minimum span index rating of 32/16. See architectural plans for thickness.
- Sheathing panels on flat surfaces shall be installed with face grain perpendicular across supports and continuous over two or more spans. Provide 1/8" space between panel edges parallel to face
- grain, 1/16" space between panel edges over supporting members. Floor sheathing shall be glued to supporting members with construction adhesive such as PL 200, laid in a continuous 1/4 inch wide bead along the member length.

#### Structural Insulated Panels:

- Panels shall be capable of supporting the self-weight of the panel plus 25 psf dead load, snow loads (supplier must calculate roof snow load, drift load, unbalanced load, and sliding load as needed based upon the information shown in the design loads notes section and the building code requirements), and wind loads (supplier must calculate components and cladding wind loads based upon the information shown in the design load notes section and the building code requirements) for
- the spans shown on the drawings. 2 Submit signed and sealed calculations prepared by a professional engineer licensed in the jurisdiction of the project.
- Panels must be interconnected to create a diaphragm capable of transferring 275 plf shear force to the main lateral force resisting systems. Wood top and bottom panels must be APA rated sheathing and adequate for fastening roofing
- bottom surface of panel. Supplied product must have been evaluated by the International Conference of Building Officials (ICBO). ICBO report and product data showing compliance with the above stated loading shall be submitted to the architect for approval.

material (nailable). See architectural drawings for information regarding quality/species of exposed

#### <u>Laminated Veneer Lumber:</u>

- Laminated Veneer Lumber (LVL) shall be "Micro-Lam" as manufactured by Weyerhaeuser, "G-P Lam" as manufactured by the Georgia Pacific Corporation or "Gang-Lam" as manufactured by the Louisiana Pacific Corporation
- Minimum allowable stress and stiffness characteristics of LVL and PSL material shall be as follows:

	LVL
F <sub>b</sub> =	2600 psi
F <sub>c</sub> (parallel to grain) =	2510 psi
F <sub>c</sub> (perpendicular to grain) =	750 psi
F <sub>v</sub> =	285 psi
E =	1,900,000 p

Member sizes shown on plan (width x depth) specified as LVL may be constructed of multiple LVL plies, fastened together by nailing or bolting as required.

Members may not be bored or notched without written permission from the engineer.

Structural Glued Laminated (Glu-Lam) Timber:

- Glued laminated timber shall be manufactured in conformance with the AITC "Manufacturing, Standard Specifications for Structural Glue Laminated Timber of Softwood Species", as specified in the code reference section of these general notes.
- 2 Appearance of timbers shall conform to the AITC "Standard Appearance Grade for Structural Glued Laminated Timber", as specified in the code reference section of these general notes. Typically, concealed timbers shall be industrial appearance grade, exposed or partially exposed timbers shall be architectural appearance grade unless noted otherwise on the architectural drawings or in the
- specifications. [All structural glued laminated timber shall be premium appearance grade.] Minimum allowable stresses and stiffness characteristics of timbers shall be as follows:
- F<sub>b</sub> = 2400 psi (tension face),= 1200 psi (compression face)\*
- $F_{c1} = 650 \text{ psi}$  $F_{v} = 165 \text{ psi}$
- E = 1,800,00 psi
- These stresses are met or exceeded by Douglas Fir 24F V4 and Southern Pine 24F-V3 combinations.
- \*Members subjected to stress reversal due to negative movements created by cantilevers or continuous span conditions shall be balanced combinations, Douglas Fir 24F-V8 and Southern Pine 24F-V5 (E = 1,700,000 psi minimum for Southern Pine).
- Unless specifically detailed otherwise on the drawings, install timbers in substantial conformance with AITC 104 "Typical Construction Details".
- Structural glued laminated timbers shall be designed in accordance with AITC 117 referred to in Note No. 1 above for the structural loads shown on this drawing without exceeding the allowable design working stresses listed in this section.
- The fabricator shall be an AITC licensed firm and shall provide factory-glued structural units that comply with AITC 190.1, "Structural Glued Laminated Timbers". Each piece of structural glued laminated timber shall be factory marked with AITC quality mark. Place mark on surface that will not be exposed in completed work.
- 7 The fabricator shall submit structural analysis and design data signed and sealed by the qualified professional engineer registered in the jurisdiction of the project and responsible for their preparation to the architect for review prior to fabrication.
- The fabricator shall engage an experienced installer who has completed structural glued laminated timber construction similar in material, design and extent to that indicated for this project and with a record of successful in-service performance.
- 9 Delivery, storage, and handling of structural glued laminated timbers shall comply with AITC 111 "Recommended Practice for Protection of Structural Glued Laminated Timber During Transit, Storage, and Erection." Individually wrap members with plastic coated paper covering, with waterresistant seams, before shipping or exposed to outdoor conditions.
- 10 Structural glue laminated timber members that extend beyond the building wall and that are subject to exterior conditions or that are in contact with exterior wall masonry, shall be ACQ or CA preservative-treated before gluing in accordance with AWPA C 28-95 for waterborne preservatives. After dressing and end cutting to final size and shape, apply a field treatment preservative to comply
- 11 Adhesive shall be wet-use type complying with ASTM D 2559. Do not use melamine-ureaformaldehyde adhesive for preservative treated glued laminated timber.

with AWPA M 4-95 to any portions of the member that fall outside the building wall.

- Connectors, anchors and accessories shall be fabricated from structural steel shapes, plates, and bars complying with ASTM A 36. All fabricated assemblies shall be hot-dipped galvanized after fabrication in accordance with ASTM A 123.
- 13 Bolts shall conform to ASTM A 307. Nuts shall conform to ASTM A 563. All fasteners shall be hotdipped galvanized in accordance with ASTM A 123.
- 14 Factory finish members with the manufacturer's standard, dry appearance, penetrating acrylic stain and sealer, oven dried and resistant to mildew and fungus. Provide color selected by architect from manufacturer's full range of colors.
- 15 Members may not be notched or bored in the field without written permission of the engineer, except as required for connections indicated on the approved shop drawings.
- 16 Repair damaged surfaces and finishes after installation is complete. Replace damaged structural glued laminated timbers if repairs are not approved by the architect or engineer. 17 At the completion of fabrication, the fabricator shall submit a certificate of compliance stating that the

work was performed in accordance with the approved contract documents, as required by Section

#### Heavy Timber Framing Notes:

fv = 170 psi

- All heavy timber framing shall comply with AITC 108-93 "standard for heavy timber construction". Shop drawings showing plan layout of the heavy timber system, full dimensions of each member, and details of each connection shall be submitted to the architect for review and approval prior to
- The heavy timber shall be of the following species and grade Timber species: Douglas Fir-Larch or
- Douglas Fir-Larch (North) Timber grade:

1704.2 of the building code indicated at the beginning of these general notes.

- 4 Minimum allowable stresses and stiffness characteristics of timbers shall be as follows:
- fb = 1.350 psifc (perpendicular to grain) = 625 psi
- e = 1,600,000 psi
- Provide heavy timber with each piece factory marked with a grade stamp. Apply grade stamps to
- surfaces that will not be exposed to view.
- Heavy timber shall have a maximum moisture content of 19% at the time of installation. Heavy timber shall be dressed (s4s) surfaced four sides. Connectors, anchors, and accessories shall be fabricated from structural steel shapes, plates, and
- bars complying with ASTM A36. All fabricated assemblies shall be hot dip galvanized after fabrication in accordance with ASTM A123. Submit for approval structural calculations signed and sealed by a registered professional engineer licensed in the jurisdiction of the project. 9 Bolts shall conform to ASTM A307. Nuts shall conform to ASTM A563. All fasteners shall be hot
- dip galvanized in accordance with ASTM A153. 10 Shop fabricate members with crown up to provide camber. Predrill members for fasteners and
- assembly of units. Machine sand exposed surfaces to remove planing or surfacing marks. 11 Heavy timber shall be saturated with one coat of a clear penetrating sealer after fabrication and
- 12 Unless specifically detailed otherwise on the drawings, install timbers in substantial conformance with AITC 104-03 "typical construction details". 13 Erect timber framing true and plumb. Roof framing shall be installed at the pitch indicated on the
- architectural drawings. Connect members using all the connectors, anchors, and accessories indicated on the shop drawings. 14 Members may not be notched or bored in the field without written permission of the engineer, except
- as required for connections indicated on the approved shop drawings. 15 All damaged surface and finishes shall be repaired after erection is complete. Replace damaged

heavy timber if repairs are not approved by the architect.

#### Wood Fasteners Notes:

1 Wood components are to be fastened together as indicated in the following schedule unless specifically indicated otherwise on the plans. Nailing schedule

Connection location	Nail size/type	Quantity or spacing and
Floor construction Built up girder/beam	10d common for 1.50" plies 2	hariz rows (tan & hat)
(3 plies or less):	16d common for 1.75" plies 12	
Floor joists to	10d common	4 toe-nail
Sill or girder:	Tou common	i too man
Rim/box joist to	16d common	3 end direct
Joist end:		
Bridging to joists:	8d common	3 toe-nail or
	10d common	2 direct
Floor truss chord	16d common	3 direct
To sill or girder:		
Band joist to	10d common	3 direct
Truss end:	40.1	011
Edge floor truss Bottom chord to sill:	16d common	8" o.c. direct
	10d common	8" o.c. toe-
Edge floor joist/ nail	Tod Common	o o.c. toe-
Truss to sill:		
Floor sheathing	8d common	6" o.c. edges
. roor encouning	10" o.c. other	5 5.6. 5 tig 5 5
Roof & ceiling constructio	1	
Ceiling joist	16d common	4 toe-nail
to plate:		
Ceiling joists laps	10d common	4 direct
(over partition):		
Ceiling joists	10d common	5 direct

to rafter:		
Collar tie	10d common	5 direct
to rafter:		
Roof rafter	10d common	4 toe-nail
to plate:		
Roof rafter	16d common	4 toe-nail or
to ridge:	16d common	3 end direct
Valley/hip rafter	16d common	2" o.c. toe-
nail or		
to ridge, header	16d common	3" o.c. end
direct		
or trimmer:		
Jack rafter	10d common	3 toe-nail or
to hip:	16d common	2 direct
Blocking between	10d common	2" o.c. toe-
nail		
rafters to top plate:		
Roof sheathing	8d common	6" o.c. edges
-	10" o.c. other	_
Naila and anikaa ahall aanfarm t	a the naminal cizes enegified in Enderal C	nocifications EE N 10ED

Nails and spikes shall conform to the nominal sizes specified in Federal Specifications FF-N-105B and as noted below:

nu as note	u below.					
			Peni	nyweight		
Type		8d	10d	12d	16d	20d
ommon	Shank diameter	0.131"	0.148"	0.148"	0.162"	0.192"
	Length	2.50"	3.00"	3.25"	3.50"	4.00"
	Head diameter	0.281"	0.312"	0.12"	0.344"	0.406"

Pneumatic or electric powered hammers typically utilize lighter gage fasteners and normally require additional fasteners to be installed. Fastener specifications must be submitted to the engineer for approval prior to installation if powered hammers are to be used. Bolts shall conform to ASTM A 307 or ASTM A 36.

All fasteners used in contact with preservative pressure treated wood shall be hot dipped galvanized

- in accordance with ASTM A 153 or stainless steel, Type 316. Do not mix stainless steel and galvanized fasteners and connectors.
- Bored lead holes for fasteners shall be as follows: A Nail and spike lead holes are not required unless to prevent splitting of wood. If required, lead hole diameter shall not exceed 75% of nail/spike diameter.
- B Wood screws lead hole diameter equals 7/8 of unthreaded shank diameter in connected wood part and 7/8 of diameter at root of thread in wood receiving thread.

Insert threaded screw type fasteners by turning with screwdriver or wrench. Do not drive by

Lag and wood screws shall conform to ANSI/ASME standard B18.6.1 - 19.81.

- Lag screws lead hole diameter equals shank diameter for extent of unthreaded shank, and 60% of shank diameter for threaded portion of shank. Thru bolts - lead hole diameter 1/32" to 1/16" larger than nominal bolt diameter.
- hammering. Facilitate installation by placing soap or other lubricant on threads. Provide standard round washers under the heads of all thru bolts and lag screws and under all nuts unless otherwise indicated on the plans. Tighten fasteners without crushing wood fibers under washers.

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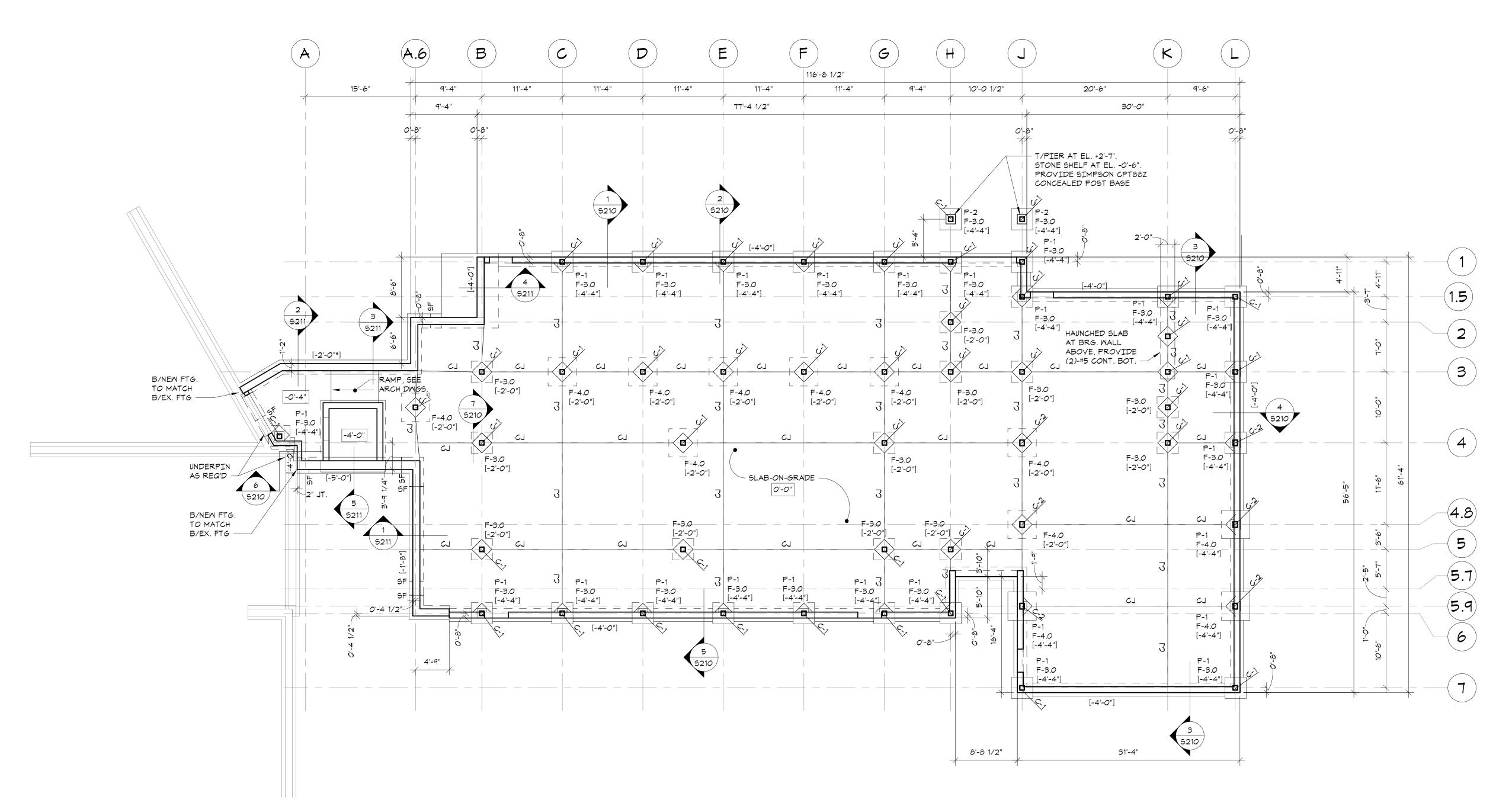
4 08/12/2021 ISSUED FOR CONSTRUCTION 3 06/25/2021 ISSUED FOR PERMIT 2 | 05/08/2020 | 90% CONSTRUCTION DOCUMENTS 1 02/21/2020 DESIGN DEVELOPMENT No. Date

GENERAL NOTES AND MATERIAL **SPECIFICATIONS** 

11/26/2019

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#### FOUNDATION AND FIRST FLOOR SLAB-ON-GRADE PLAN

1/8"=1'-0"

NORTH:

- TOP OF FIRST FLOOR CONCRETE SLAB ON GRADE ELEVATION = DATUM ELEVATION O'-O" = TOPOGRAPHICAL ELEVATION = 221.0' +/- (MATCH EXISTING).
- FLOOR CONSTRUCTION: 5" CONCRETE SLAB ON GRADE, REINFORCED WITH 6x6 - W2.0xW2.0 WELDED WIRE FABRIC, UNLESS OTHERWISE NOTED. FOR DETAILS, SEE "TYPICAL SLAB ON GRADE DETAILS".
- FOR FOUNDATION CONDITION AT EXTERIOR DOORS, SEE "TYPICAL DETAIL AT EXTERIOR DOORS".
- COORDINATE ALL PITS, TRENCHES, POCKETS, BRICK SHELVES, SLEEVES, PENETRATIONS AND INSERTS IN CONCRETE WALLS AND SLABS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- FOR MASONRY SHEAR WALL FOUNDATION INFORMATION, INCLUDING DOWELS BETWEEN FOUNDATIONS AND MASONRY WALLS, SEE "TYPICAL CONCRETE MASONRY WALL REINFORCEMENT DETAIL". PROVIDE DOWELS BETWEEN FOUNDATIONS AND MASONRY WALLS EQUAL TO SIZE AND SPACING OF MASONRY WALL VERTICAL REINFORCEMENT.
- CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS BEFORE PROCEEDING WITH ANY WORK.
- SEE GENERAL NOTES, FOR ADDITIONAL INFORMATION.
- \* INDICATES BOTTOM OF FOOTING ELEVATION IS
   APPROXIMATE. BOTTOM OF NEW FOOTING SHALL MATCH BOTTOM
   OF EXISTING FOOTING, UNLESS EXISTING FOOTING ELEVATION
   IS ABOVE FROST DEPTH IN WHICH CASE CONTRACTOR SHALL
   NOTIFY ARCHITECT AND ENGINEER PRIOR TO PROCEEDING
   WITH ANY WORK AND SHALL UNDERPIN EXISTING FOUNDATIONS
   AS REQUIRED TO COMPLY WITH THE BUILDING CODE. SEE
   "TYPICAL UNDERPINNING DETAIL".

#### LEGEND:

- [+/- ....] INDICATES BOTTOM OF FOOTING ELEVATION FROM TOP OF FIRST FLOOR CONCRETE SLAB DATUM ELEVATION.
- (+/-...) INDICATES TOP OF WALL ELEVATION FROM TOP OF FIRST FLOOR CONCRETE SLAB DATUM ELEVATION.
- INDICATES TOP OF CONCRETE SLAB ELEVATION
  FROM TOP OF FIRST FLOOR CONCRETE SLAB DATUM ELEVATION.
  MAINTAIN FULL SLAB THICKNESS AT SLOPED AND DEPRESSED
  AREAS. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND
  EXTENT.
- "SF" INDICATES APPROXIMATE LOCATION OF STEPS IN FOOTINGS ON PLAN. COORDINATE LOCATION AND ELEVATION WITH SITE GRADING AND MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS. FOR DETAILS, SEE "TYPICAL STEPPED FOOTING DETAIL".
- "CJ" INDICATES APPROXIMATE LOCATION OF CONTROL/CONSTRUCTION JOINTS IN SLABS ON GRADE. FOR DETAILS, SEE "TYPICAL SLAB ON GRADE DETAILS".
- F-## INDICATES FOOTING TYPE. SEE "FOOTING SCHEDULE AND FOOTING DETAIL".
- P-# INDICATES PIER TYPE. SEE "PIER SCHEDULE AND PIER DETAILS". TOP OF PIER ELEVATION TO BE -0'-8", UNLESS OTHERWISE NOTED.
- "C-#" INDICATES TIMBER COLUMN, SEE COLUMN SCHEDULE ON DRAWING 5301.

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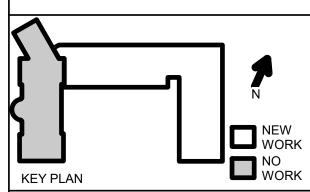


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1	12/19/2019	SCHEMATIC DESIGN

FIRST FLOOR SLAB-ON-GRADE AND FDN PLAN

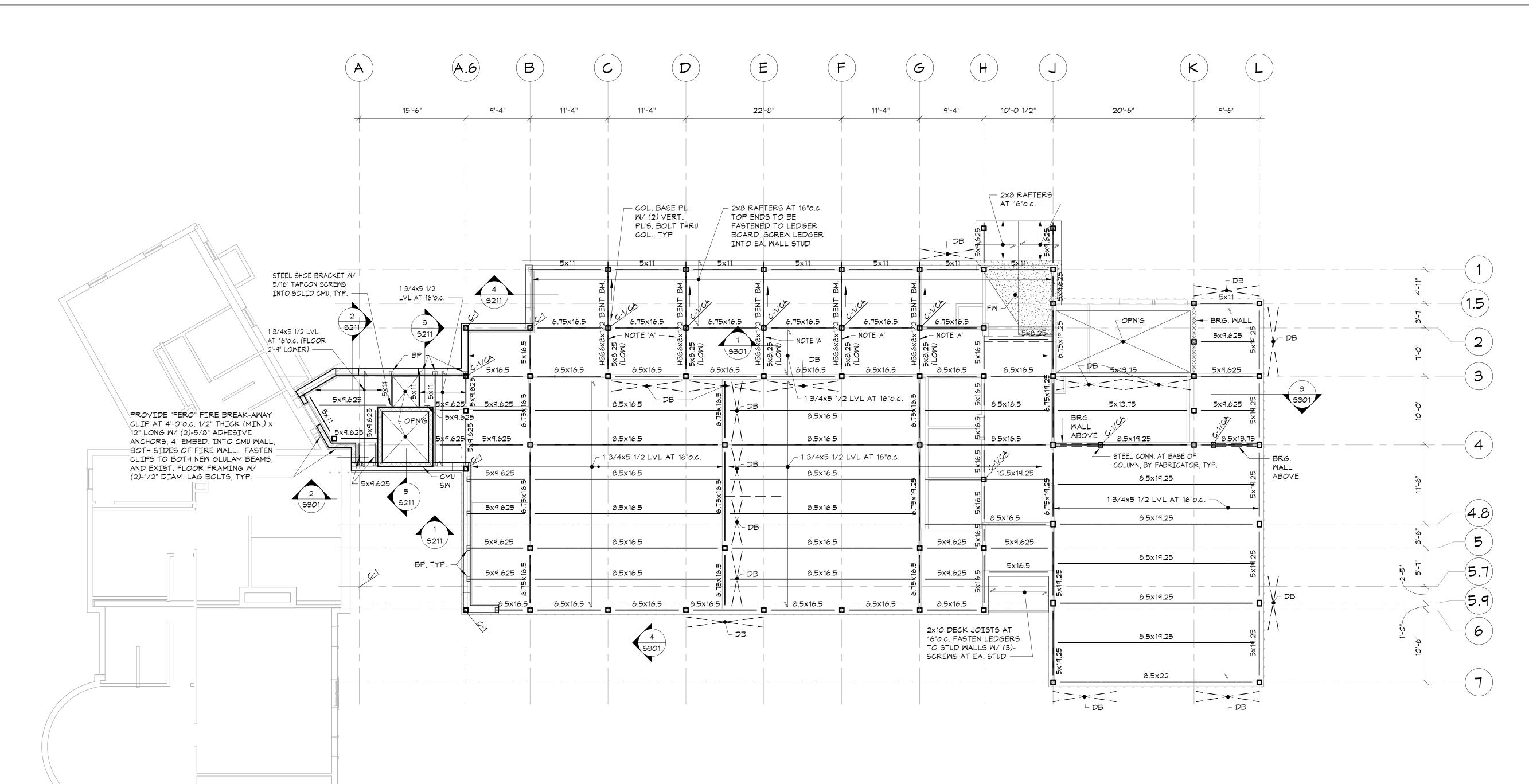
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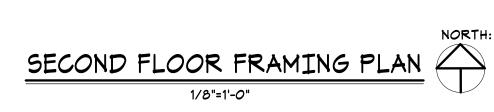
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- TOP OF SECOND FLOOR TOPOGRAPHICAL ELEVATION: 233'-O". TOP OF FLOOR FRAMING AT CONVENT GROUND FLOOR TO BE AT ELEVATION 230'-3 1/4"+/- (MATCH EXISTING).
- GLULAM BEAMS ARE NOTED BY SIZE, WIDTH (INCHES) X DEPTH (INCHES). ALL GLULAM BEAMS TO BE -0'-7 1/2" BELOW FLOOR ELEVATION, UNLESS OTHERWISE NOTED.
- BELOW FLOOR ELEVATION, UNLESS OTHERWISE NOTE
- "C-#" INDICATES TIMBER COLUMN, SEE COLUMN SCHEDULE ON DRAWING \$301.
- "CA" INDICATES COLUMN ABOVE.

"CMU SM" INDICATES 8" CMU SHEARWALL.

- "NOTE 'A" INDICATES TO 'RIP' GLULAM BEAM AS REQUIRED TO PROVIDE 1/2" CLEARANCE TO UNDERSIDE OF BENT BEAM. BOTTOM OF GLULAM BEAM TO ALIGN WITH ADJACENT GLULAM BEAMS. SEE SECTION 1/5301.
- "FM" INDICATES FALSEMORK OR OVERBUILD FRAMING AT 16"O.C. NAIL 2x12 IN VALLEY MITH (2)-16d COMMONS TO EACH RAFTER BELOW THAT IS CROSSED. NAIL BOTTOM END OF FALSEMORK RAFTER TO 2x12 VALLEY WITH (2)-10d COMMON TOE-NAILS EACH SIDE OF RAFTER FOR EACH 4 FEET OF RAFTER LENGTH, USE 2x6'S FOR SPANS UP TO 10 FEET, 2x8'S FOR SPANS UP TO 13 FEET, 2x10'S FOR SPANS UP TO 16 FEET, AND 2x12'S FOR SPANS UP TO 20 FEET. RAFTERS BELOW FALSEMORK MUST BE FULLY SHEATHER AND NAILED. ISOLATED OPENINGS IN SHEATHING MAY BE ALLOWED WITH THE ENGINEER'S APPROVAL.
- "DB" INDICATES DIAGONAL BRACING. PROVIDE 2"x16 GA. STRAPPING, EACH SIDE OF WALL, WITH #12 SCREWS INTO BUILT-UP COLD-FORMED STEEL POSTS.
- "BP" INDICATES BEAM BEARING POCKET, SEE TYPICAL DETAILS.
- ALL FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN LINES, UNLESS OTHERWISE INDICATED.
- COORDINATE SIZE AND LOCATION OF ALL FLOOR PENETRATIONS WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. ALL FLOOR PENETRATIONS SHALL BE REVIEWED BY THE ENGINEER PRIOR TO FABRICATION AND INSTALLATION OF PENETRATION FRAMING ELEMENTS. DO NOT SCALE OPENINGS.
- SEE GENERAL NOTES FOR ADDITIONAL INFORMATION.

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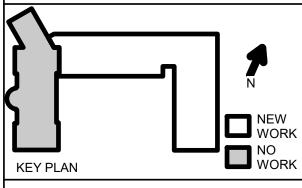


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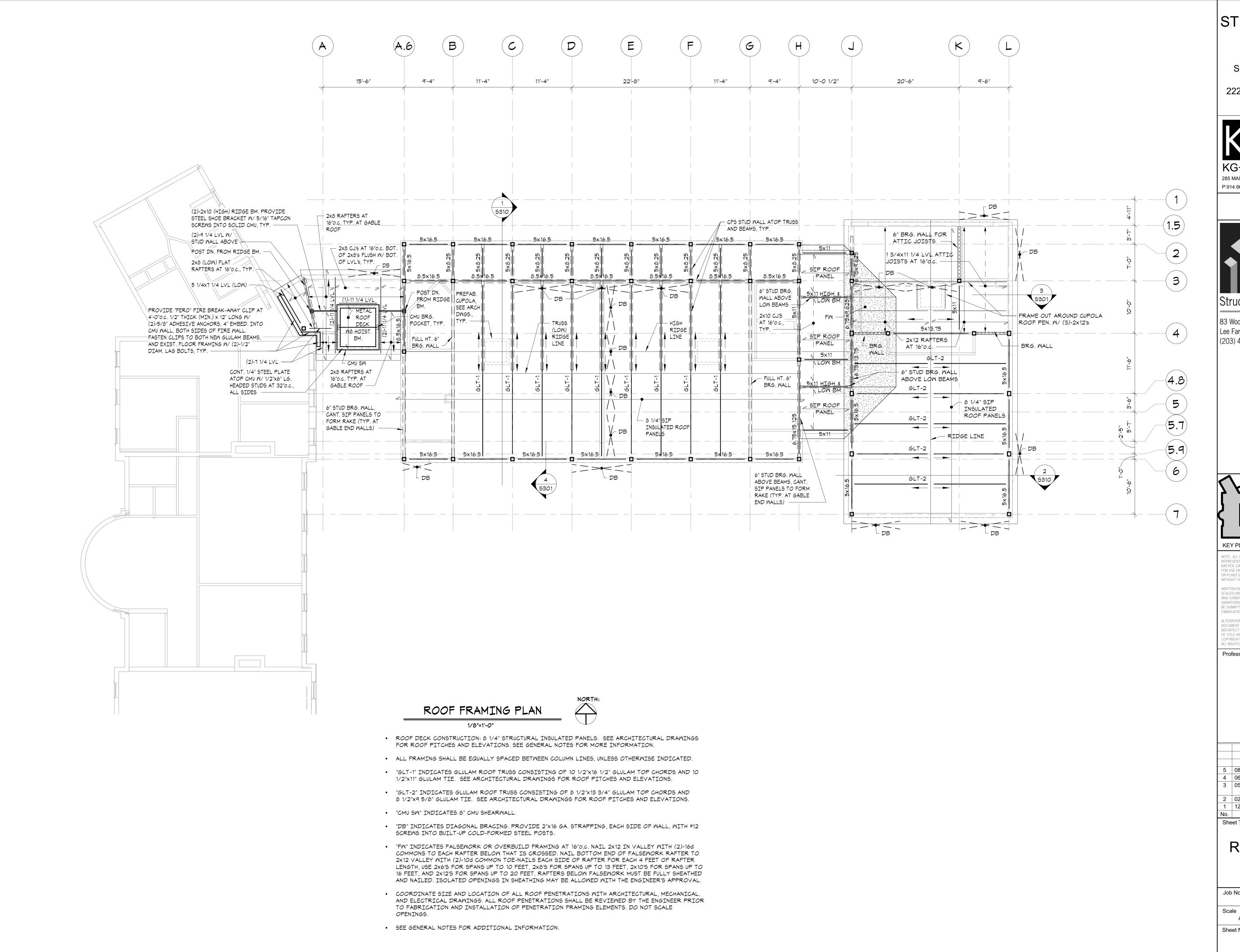
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## SECOND FLOOR FRAMING PLAN

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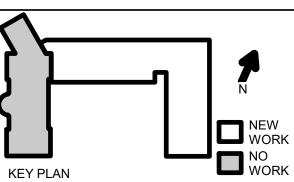
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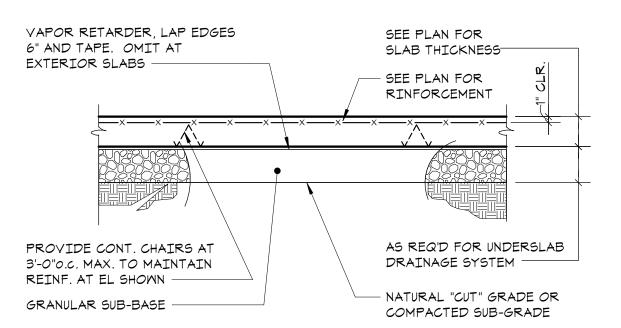
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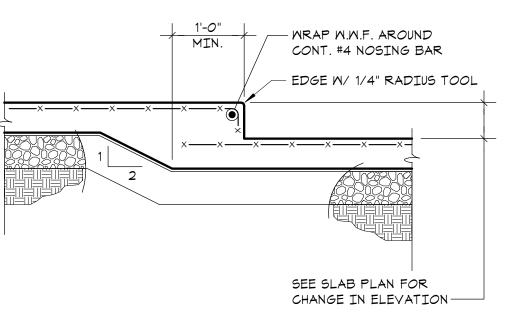
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#### ROOF FRAMING **PLAN**

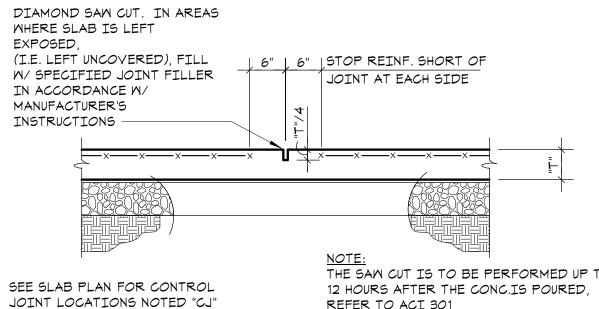
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SLAB ON GRADE DETAIL



STEPPED SLAB DETAIL



CONTROL JOINT DETAIL

THE SAW CUT IS TO BE PERFORMED UP TO REFER TO ACI 301

FIRST POUR | SECOND POUR 6" ISTOP REINF. SHORT OF TONGUE AND GROOVE JOINT AT EACH SIDE CONSTRUCTION JOINTS ARE TO BE USED WHEN CONCRETING OPERATIONS ARE TO BE INTERRUPTED.

SLAB ON GRADE CONSTRUCTION JOINT DETAIL

SLAB AT COLUMN DETAIL dol.\FOOTING\PIER PREFORMED JT. STRIP FULL DEPTH AND FLUSH W/ T/SLAB IN AREAS WHERE THE SLAB - COLUMN IS LEFT EXPOSED (I.E. LEFT UNCOVERED) HOLD JT. STRIP - SECOND POUR TO T/SLAB 1/2" DOWN FROM T/SLAB AND FILL W/ SPECIFIED JT. FILLER IN ACCORDANCE - STIRRUPS M/MANUFACTURER'S INSTRUCTIONS - FOOTING OR PIER STOP REINF. SHORT OF6" JOINT AT EACH SIDE

<u>PLAN</u>

COLUMN -

SEE SLAB PLAN FOR

LOCATIONS NOTES

(2)-#3 STIRRUPS

CONTROL JT.

COL.\FOOTING\PIER

PROVIDE

DIAMOND SHAPE

FILLED W/ CONC.

AFTER COLUMN IS

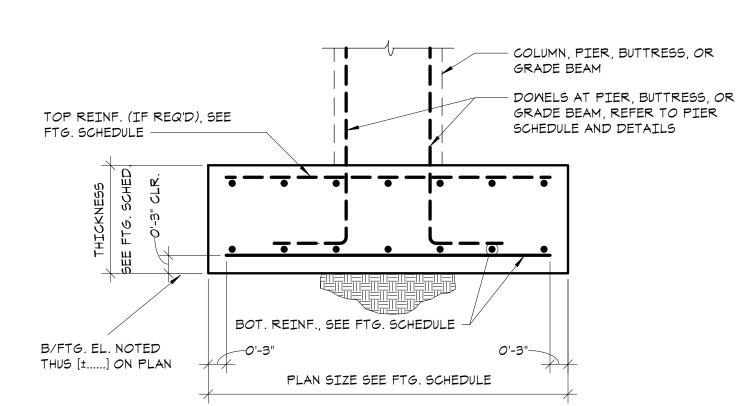
FORM, TO BE

INSTALLED

SLAB AT COLUMN DETAIL

#### TYPICAL SLAB ON GRADE DETAIL

NO SCALE



MARK -	PLAN	SIZE	THICKNESS	BOTTON	M REINF.	REMARKS
	N-S	E-M	111201411200	N-S	E-M	TALL!!! ALA
F-3.0	3'-0"	3'-0"	1'-4"	(4)-#5	(4)-#5	
F-4.0	4'-0"	4'-0"	1'-4"	(5)-#5	(5)-#5	
NOTES:						
NOTES:						
	:R TO F001	TING DETA	ILS AND FOUND,	ATION NOI	TES IN USI	NG THIS SCHEDULE.
	:R TO FOO <sup>-</sup>	TING DETA	ILS AND FOUND,	ATION NOI	TES IN USI	NG THIS SCHEDULE.

## ROUND 4 BARS <u>6 BARS</u> 8 BARS <u>10 BARS</u> <u> 12 BARS</u> <u>OR MORE</u>

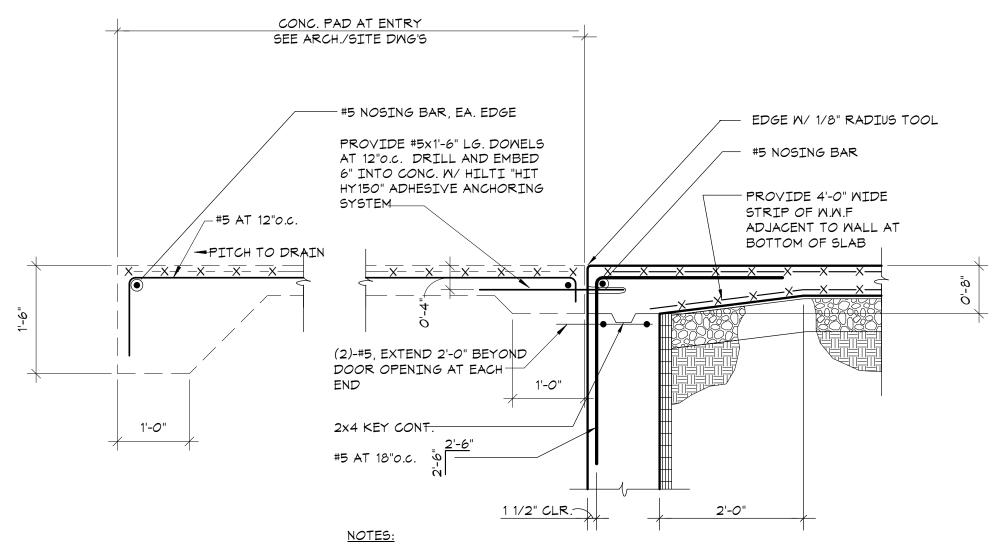
#### TYPICAL TIE ARRANGEMENT "\*" INDICATES WHEN 6" OR LESS OMIT INTERIOR TIE.

	MAXIMUM SPACING OF TIES			
VERTICAL	9	IZE AND SPACING OF TIES		
BAR SIZE	#3	#4	#5	
#5	10"	-	-	
#6	12"	-	-	
#7	14"	-	-	
#8	16"	16"	-	
#9	18"	18"	-	
#10	18"	20"	-	
#11	†	22"	22"	
#14	+	24"	27"	
#18	†	24"	30"	

"\*" INDICATES MAX. SPACING NOT TO EXCEED LEAST DIMENSION OF COMPRESSION MEMBER. TINDICATES #3 TIES NOT PERMITTED.

- 1. THE VERTICAL BARS SHALL BE SPACED AROUND THE PERIMETER SO AS TO ACHIEVE APPROXIMATELY EQUAL SPACING.
- 2. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF EXTERIOR DOORS. 3. TIES SHALL BE ARRANGED SUCH THAT EVERY CORNER AND ALTERNATE VERTICAL BAR SHALL HAVE LATERAL SUPPORT PROVIDED BY THE CORNER OF A TIE WITH AN INCLUDED ANGLE OF NOT MORE THAN 135 DEGREES AND NO BAR SHALL BE FURTHER THAN 6" CLEAR ON EACH SIDE ALONG THE TIE
- FROM SUCH A LATERALLY SUPPORTED BAR. 4. TIES SHALL BE LOCATED VERTICALLY NOT MORE THAN ONE-HALF A TIE SPACING ABOVE THE TOP OF FOOTING OR SLAB IN ANY STORY, AND SHALL BE SPACED AS PROVIDED HEREIN TO NOT MORE THAN ONE-HALF A TIE SPACING BELOW THE LOWEST HORIZONTAL REINFORCEMENT IN SLAB OR DROP PANEL ABOVE.
- 5. WHERE BEAMS OR BRACKETS FRAME FROM FOUR DIRECTIONS INTO A COLUMN, PIER OR BUTRESS, TERMINATION OF TIES NOT MORE THAN 3" BELOW REINFORCEMENT IN SHALLOWEST OF SUCH BEAMS OR BRACKETS SHALL NOT BE PERMITTED. 6. SEE THE GENERAL NOTES ON DRAWING S1 FOR THE MINIMUM CONCRETE COVER REQUIREMENTS FOR
- REINFORCEMENT. 7. WHERE PIERS ARE ADJACENT TO FOUNDATION WALLS, PIERS ARE TO BE POURED MONOLITHICALLY WITH WALLS, PIER REINFORGING SHALL EXTEND WITHIN 2" OF OPPOSITE FACE WALL.

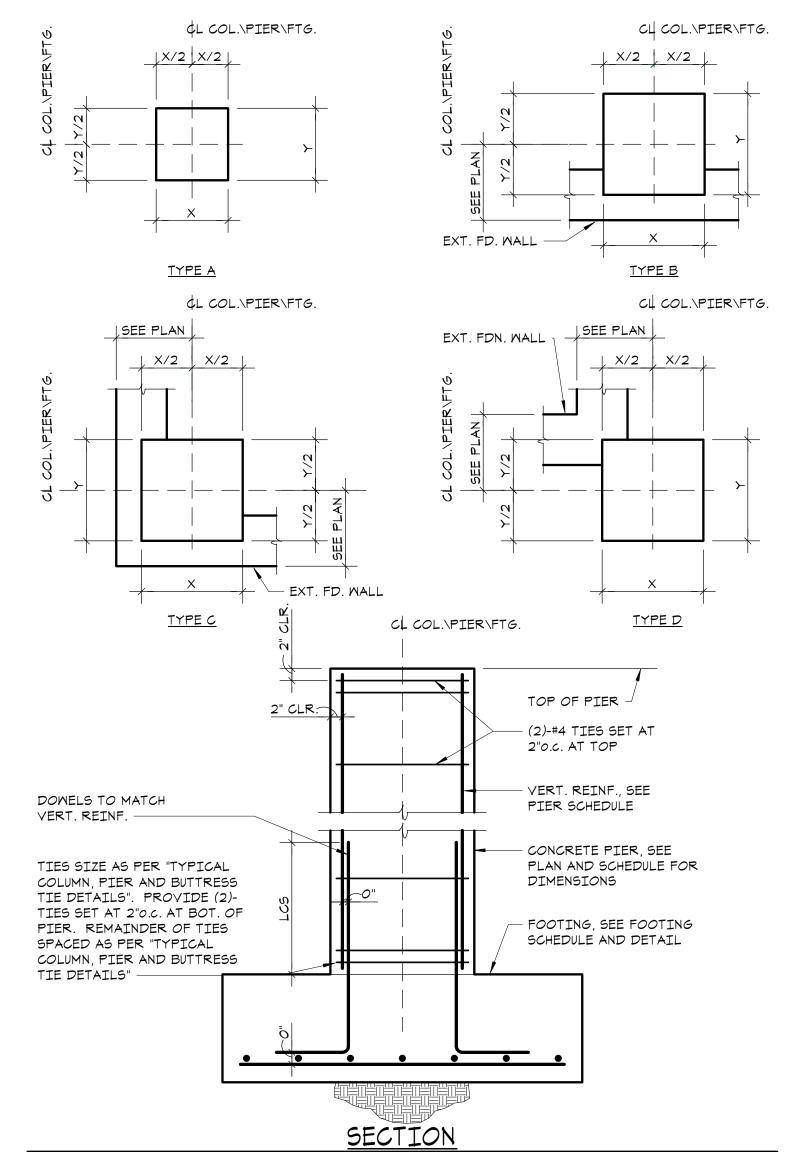
TYPICAL COLUMN, PIER AND BUTTRESS TIE DETAILS
NO SCALE



 SEE APPROPRIATE SECTION FOR REMAINDER OF FOUNDATION INFORMATION NOT REPEATED HERE.

 SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF EXTERIOR DOORS.

## TYPICAL FOUNDATION WALL DETAIL AT EXTERIOR DOORS NO SCALE



PIER SCHEDULE MARK TYPE REMARKS E-W DIR. N-S DIR. REINF. 1'-4" (4)-#6 16" PROVIDE 2" STONE SHELF, ALL 1'-4" (4)-#6 (4)-SIDES, W/ 12" x 12" STEM

TYPICAL PIER SCHEDULE AND PIER DETAILS
NO SCALE

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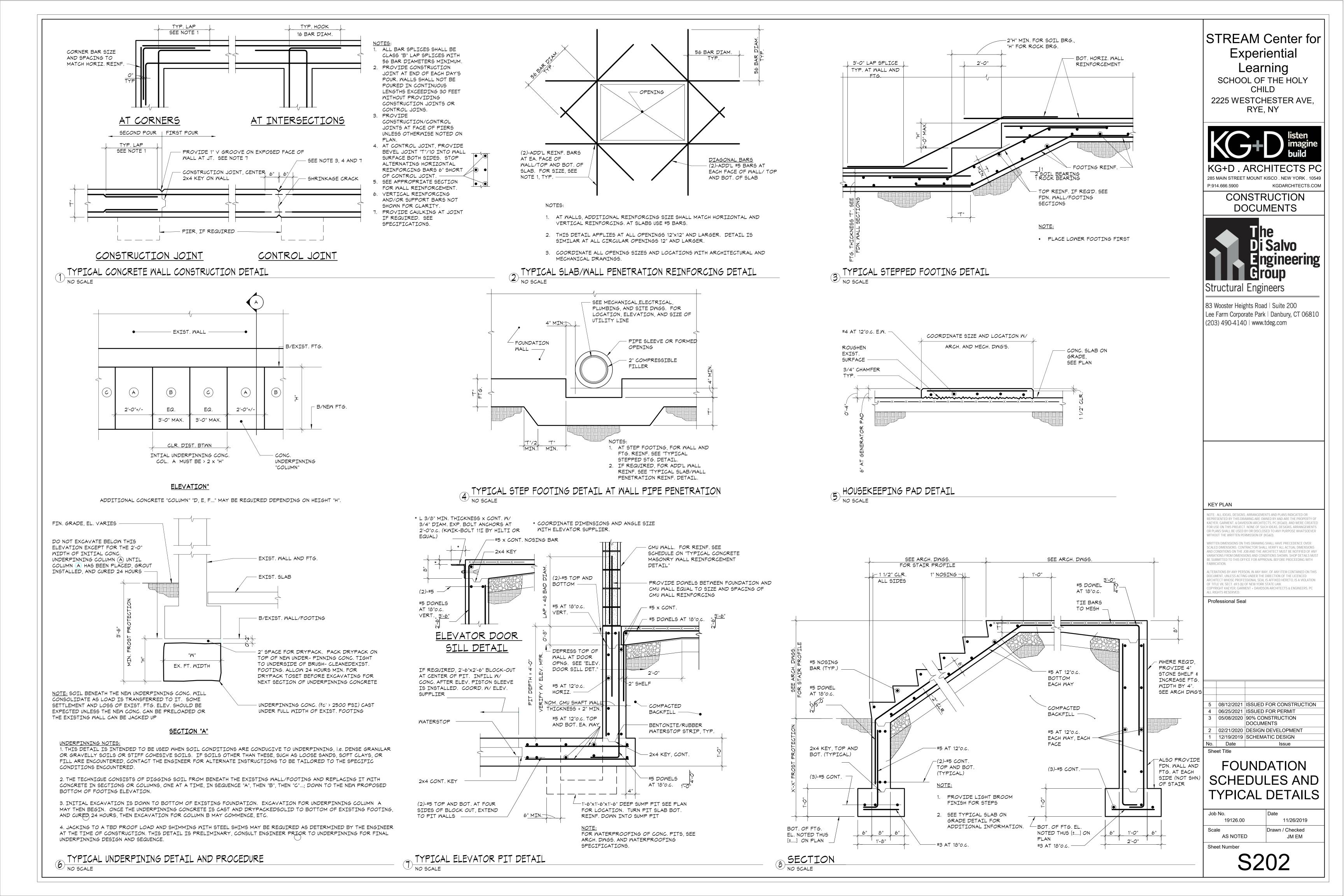
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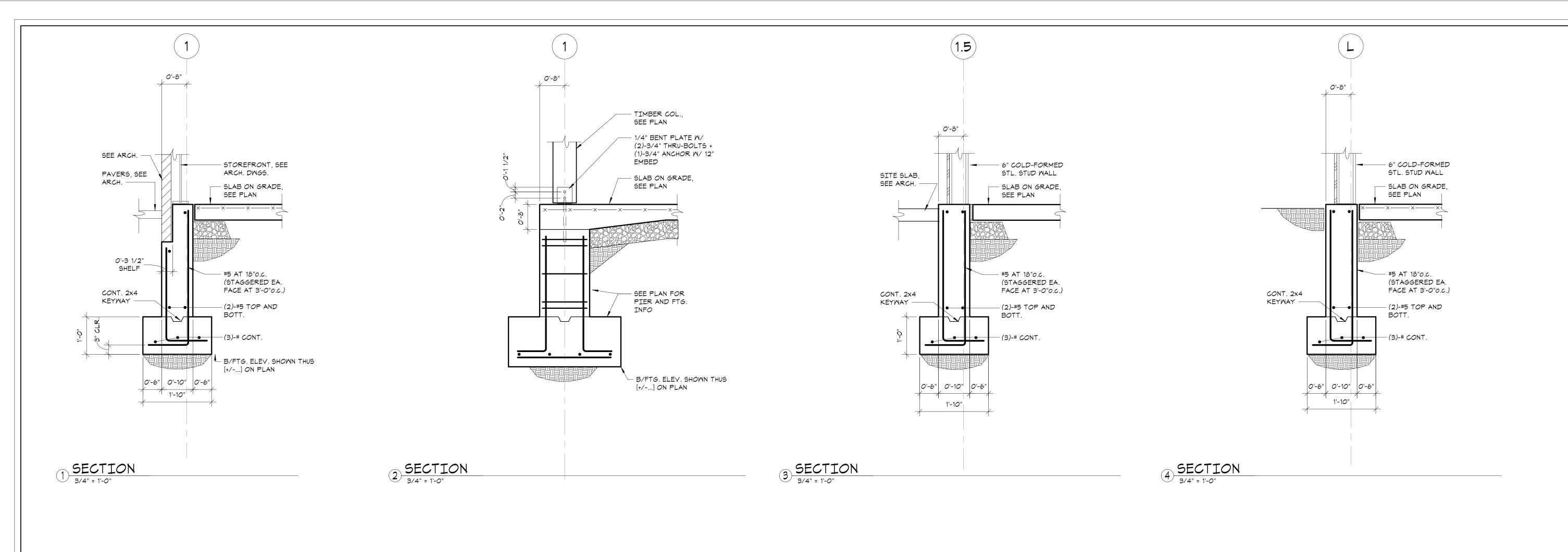
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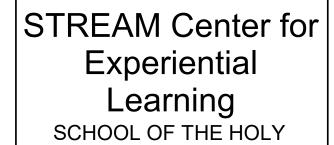
FOUNDATION SCHEDULES AND TYPICAL DETAILS

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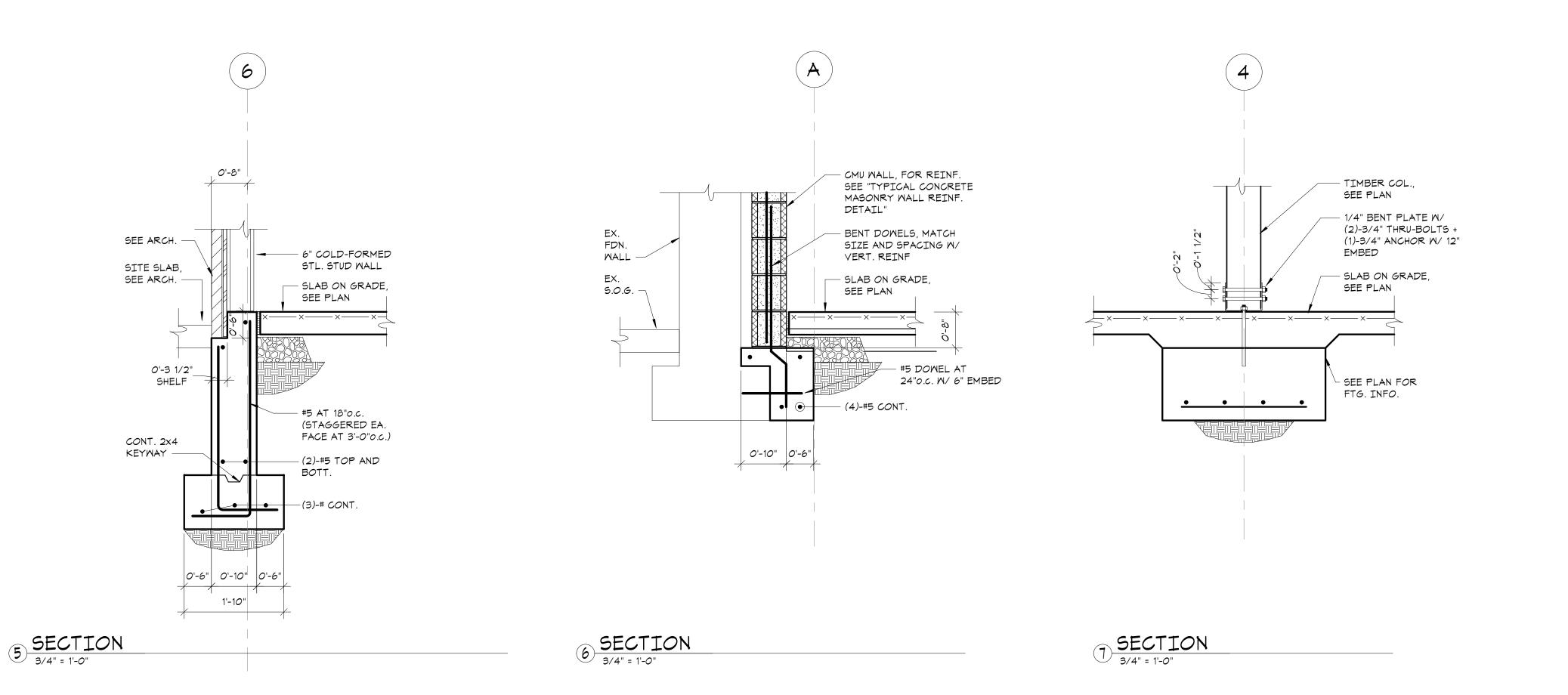
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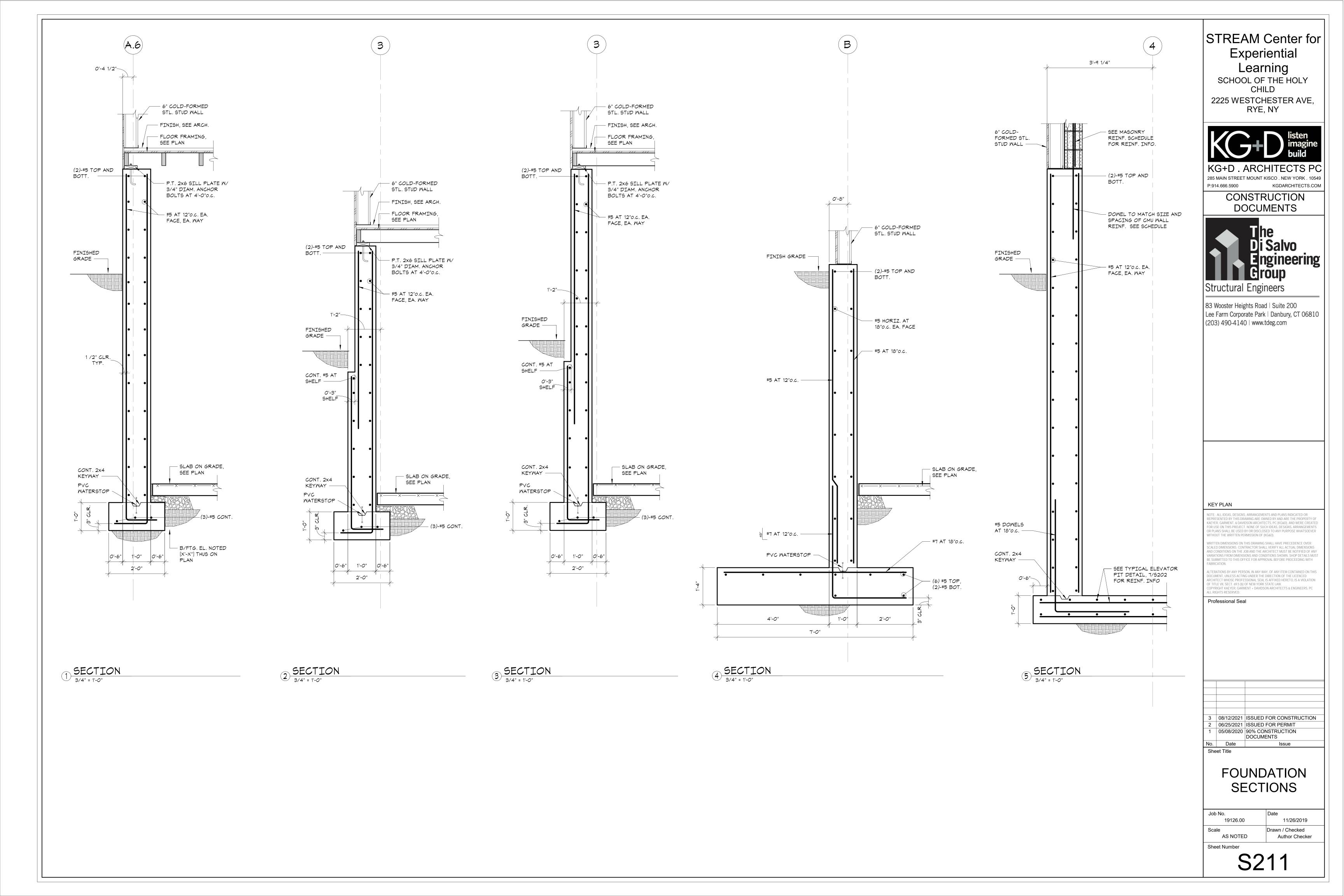
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#### FOUNDATION SECTIONS

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Sheet Number	
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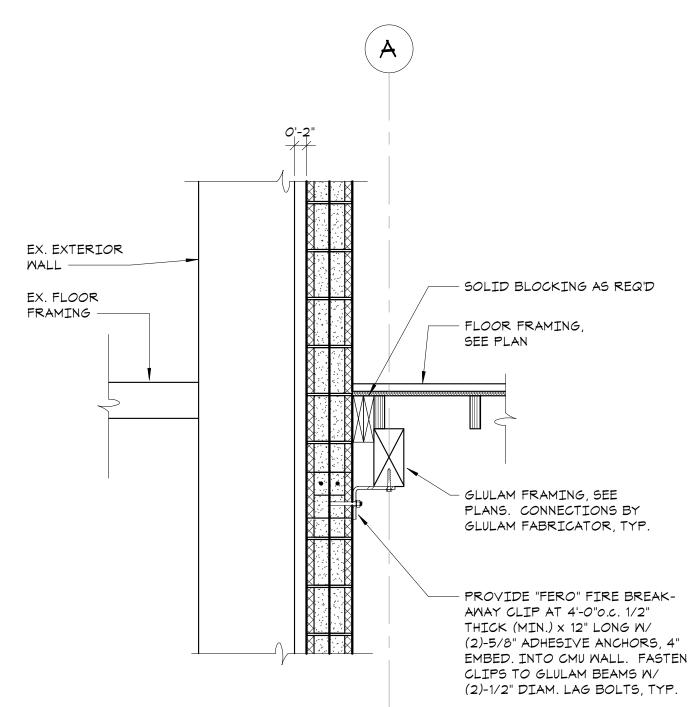


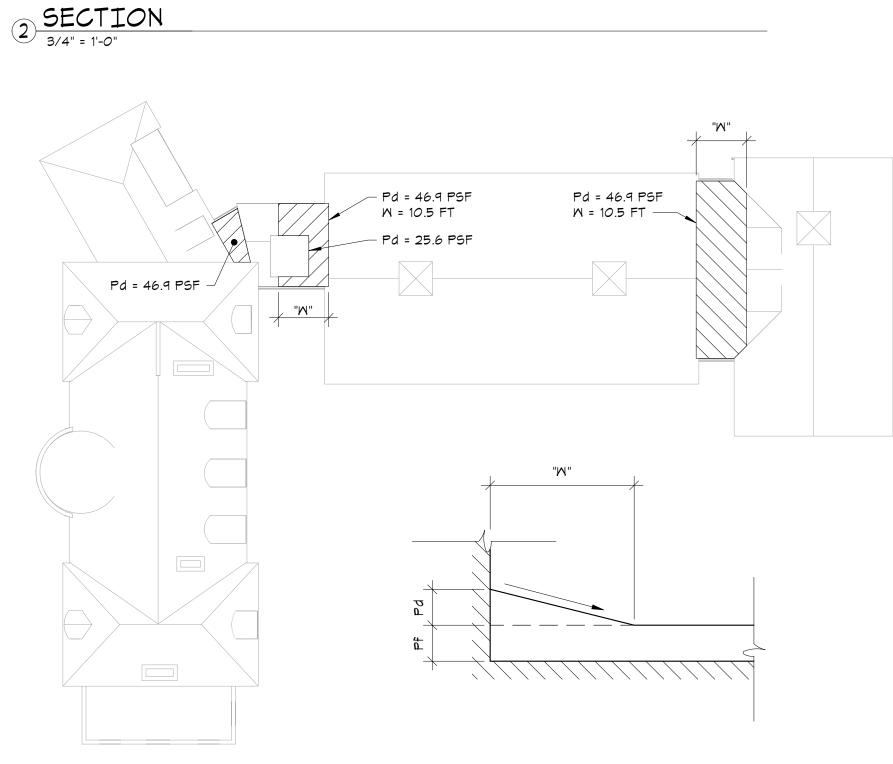


TIMBER COLUMN SCHEDULE			
DESIGNATION	SIZE	BASE CONNECTION	
C-1	8x8	6 1/2"x1/4"x1'-8" BENT STEEL PLATE	
C-2	8×10	6 1/2"x1/4"x1'-10" BENT STEEL PLATE	
C-3	6x6	5"x1/4"x1'-6" BENT STEEL PLATE	
		THRU-BOLTS, AND (1)-3/4" ANCHOR ROD O CONCRETE. SEE SECTIONS 2/5210 AND	

7/S210 FOR MORE INFORMATION.

## 1 COLUMN SCHEDULE-WOOD/TIMBER

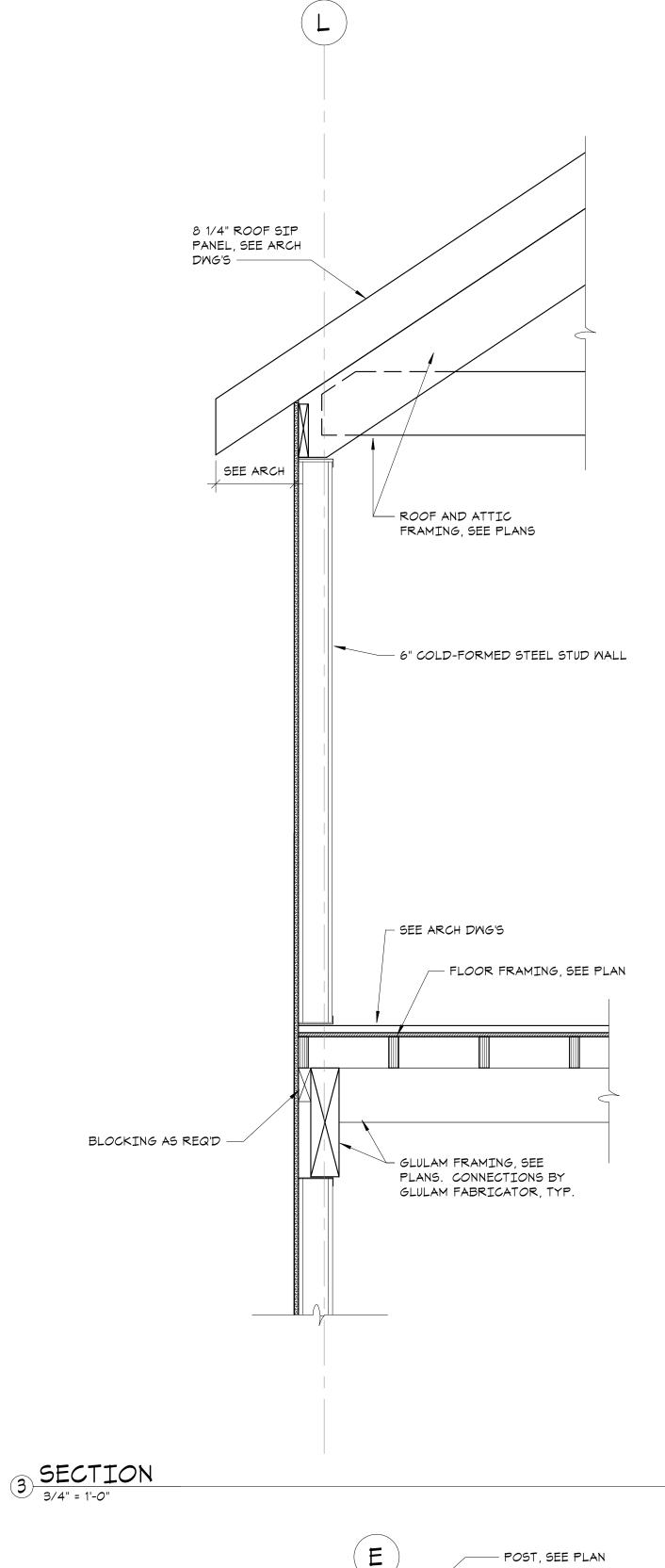


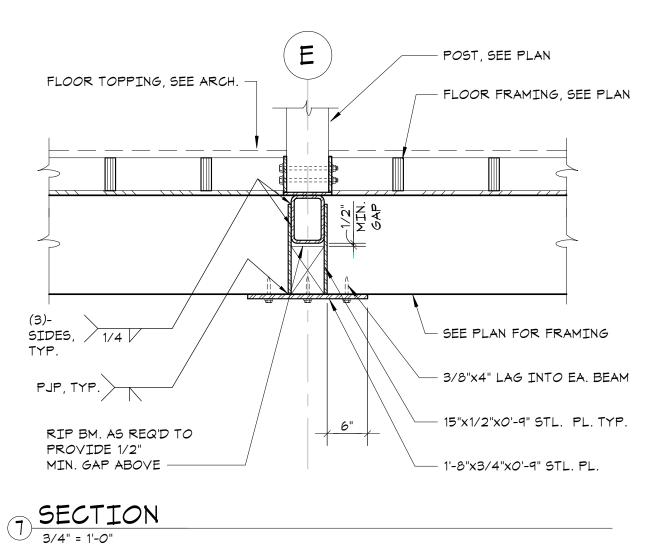


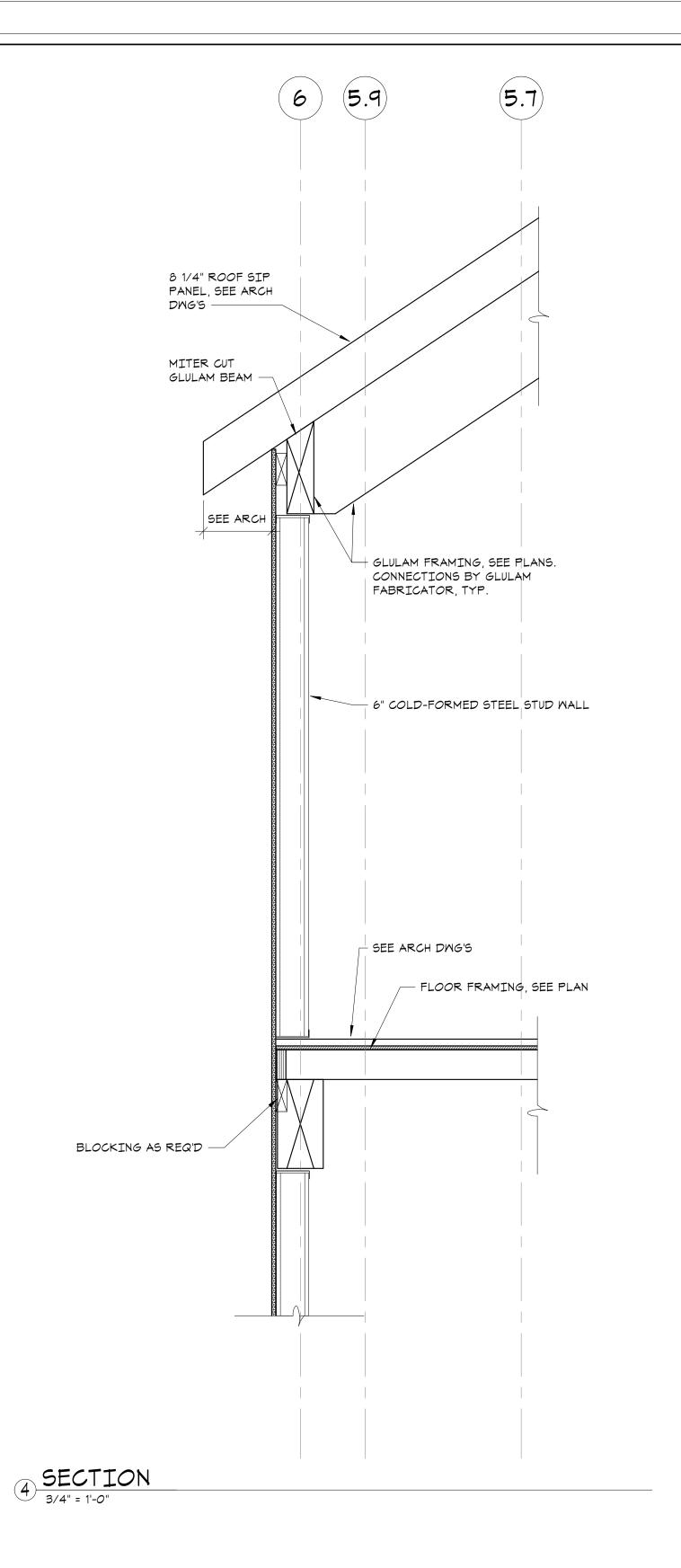
5 ROOF SNOW DRIFT PLAN

1" = 20'-0"

• THE VALUES SHOWN ON LOADING PLAN INDICATE Pd, THE SNOW DRIFT SURCHARGE LOAD AND "W", THE DRIFT WIDTH. THE TOTAL SNOW LOAD IS THE COMBINATION OF PF (FLAT ROOF SNOW LOAD) + Pd. REFER TO THE BUILDING CODE AND DRIFT DIAGRAM BELOW FOR FURTHER INFORMATION.







#### STREAM Center for Experiential Learning

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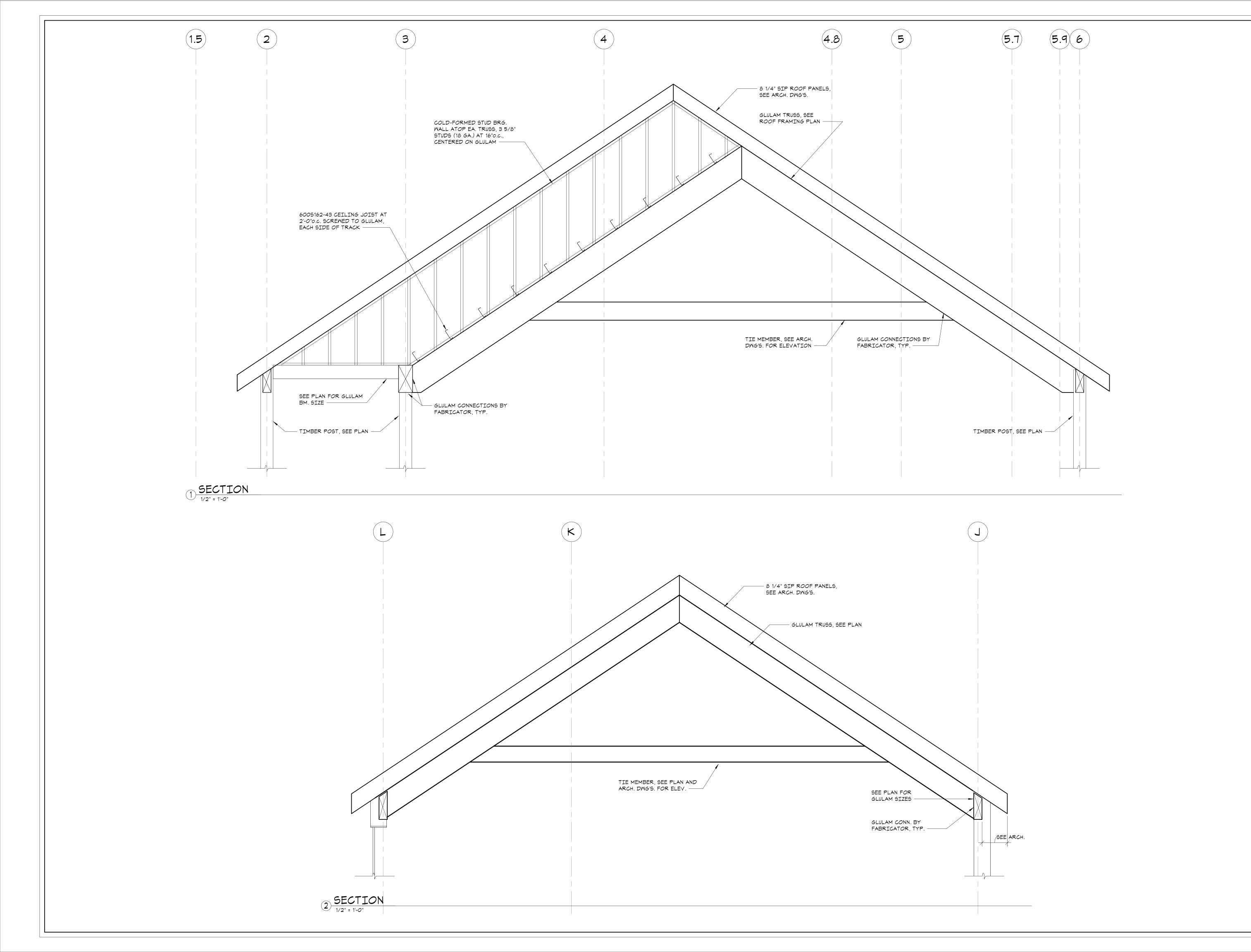
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#### FRAMING **DETAILS AND** SECTIONS

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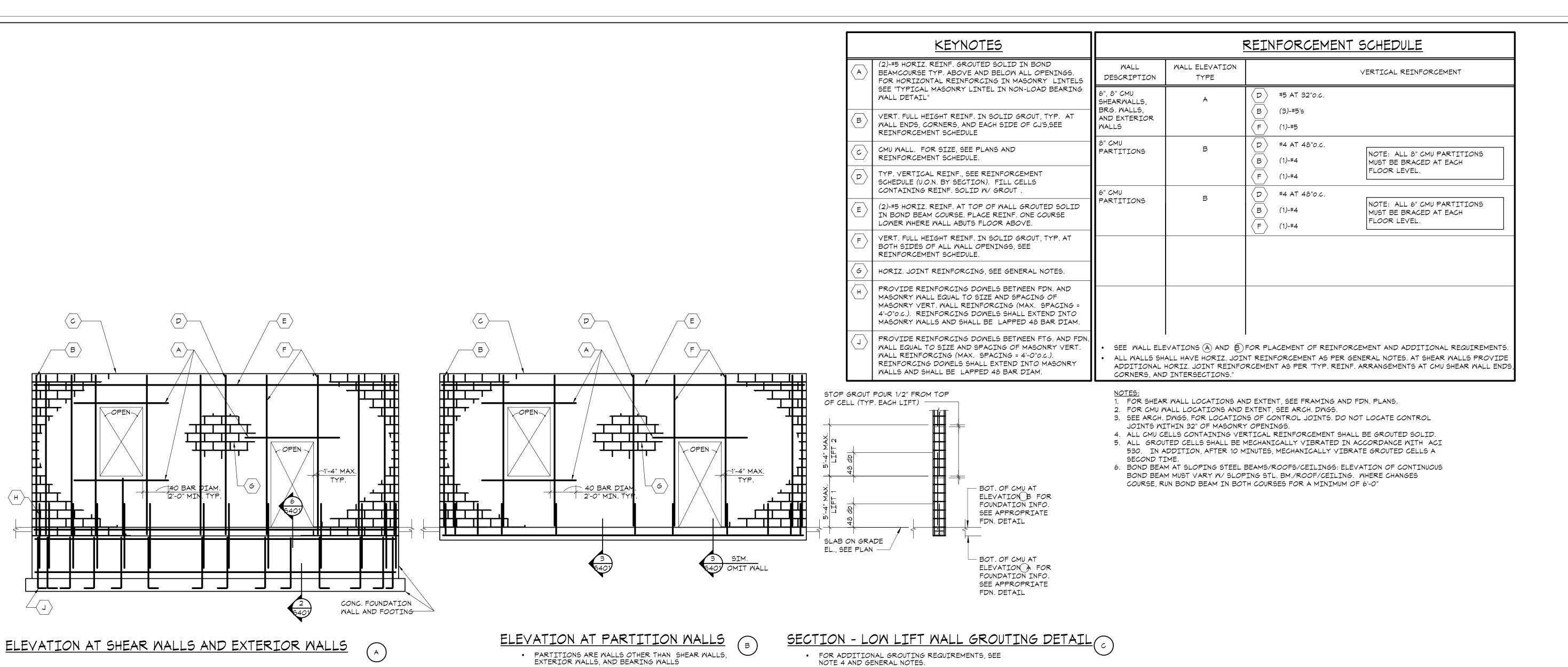
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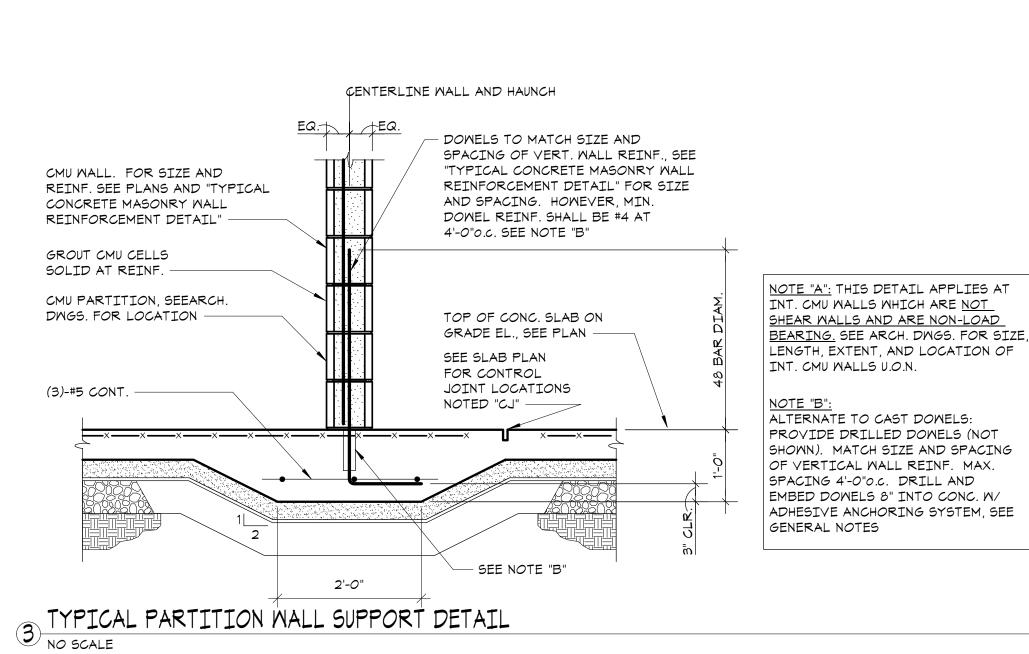


TYPICAL CONCRETE MASONRY WALL REINFORCEMENT DETAIL

NOTE "A": PROVIDE REINF. DOWELS BETWEEN FDN. AND NOTES: MASONRY WALL EQUAL TO SIZE AND SPACING OF PROVIDE 90° BARS AT ALL MASONRY VERT. WALL REINF. FOR SIZE AND CORNERS TO LAP MAIN REINF. SPACING OF REINF., SEE "TYP. CONCRETE 2'-6" MIN. MASONRY WALL REINFORCEMENT DETAIL". DOWELS SHALL EXTEND INTO MASONRY AND FOR CONDITION AT WALL SHALL BE LAPPED 48 BAR DIAM. OPENINGS, SEE "TYP. FOUNDATION DETAIL FOR CL WALL AND FTG. MASONRY WALLS AT WALL NOTE "B": MATCH NOMINAL CMU WALL THICKNESS OPENINGS." NOTE "C": PROVIDE REINF. DOWELS BETWEEN FTG. AND FDN. WALL EQUAL TO SIZE AND SPACING OF MASONRY VERT. WALL REINF. FOR SIZE AND SPACING OF REINF., SEE "TYP. CONC. MASONRY WALL REINFORCEMENT DETAIL". CMU WALL. FOR SIZE AND REINF. SEE PLANS AND "TYPICAL CONCRETE MASONRY WALL REINFORCEMENT SEE NOTE "A" -DETAIL" 1/2" PREMOLDED JOINT FILLER, TYP. (2)-#5 CONT., TOP AND BOT. T/WALL OR FTG. FOR SLAB ON GRADE CONSTRUCTION SEE "TYPICAL SLAB ON GRADE T/CONG. SLAB ON DETAILS" GRADE EL., SEE PLAN 2X4 KEY CONT. #5 AT 12"o.c. SEE NOTE "C" (3)-#5 CONT - B/FTG. EL. NOTED THUS [±.....] ON PLAN U.O.N. NOTE U.O.N.

TYPICAL FOUNDATION DETAIL FOR SHEAR WALLS/MASONRY WALLS

NO SCALE



CL OF MASONRY CONTROL JOINT VERT. REINF., SEE REINF. SCHEDULE ON SEE NOTE 3 ON "TYP. CONC. "TYPICAL CONCRETE MASONRY WALL MASONRY WALL REINFORCEMENT REINFORCEMENT DETAIL" — CMU WALL BACKER ROD AND JOINT SEALER AT CONTROL JOINT HORIZ. JT. REINF. SHALL BE TERMINATED 2" FROM CONTROL JOINTS, TYP. PLAN DETAIL AT WALL ENDS AND CONTROL JOINTS -AT CORNERS AND INTERSECTIONS, PROVIDE PREFABRICATED CORNER AND TEE W1.7 (9 GAGE) HORIZ. JOINT LADUR REINF. AT 8"O.C., TYP. EXCEPT AT CONTROL JOINTS (SEE ARCH. -YERT. REINF., SEE REINF. DWG'S. FOR LOCATIONS) WHERE HORIZ. JT. SCHEDULE ON "TYPICAL LADDER REINF. SHALL NOT CROSS CONTROL CONCRETE MASONRY WALL REINFORCEMENT DETAIL" VERT. REINF., SEE REINF. SCHEDULE ON "TYPICAL CONCRETE MASONRY WALL - CMU WALL REINFORCEMENT DETAIL"

PLAN DETAIL AT CORNERS AND INTERSECTIONS

TYPICAL HORIZONTAL JOINT REINFORCING (NOT SHOWN FOR CLARITY).

TYPICAL MASONRY WALL PLAN DETAILS
NO SCALE

2'-6"

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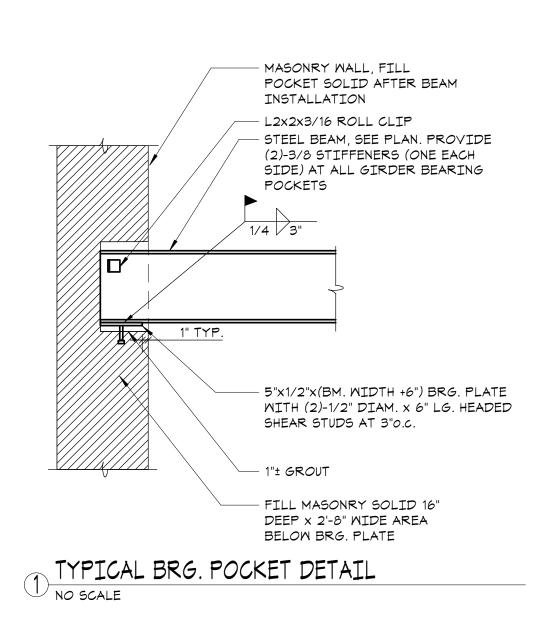
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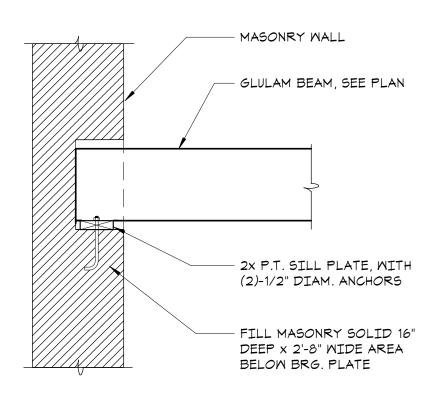
MASONRY SCHEDULE AND TYPICAL DETAILS

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2 TYPICAL GLULAM BRG. POCKET DETAIL

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MASONRY DETAILS

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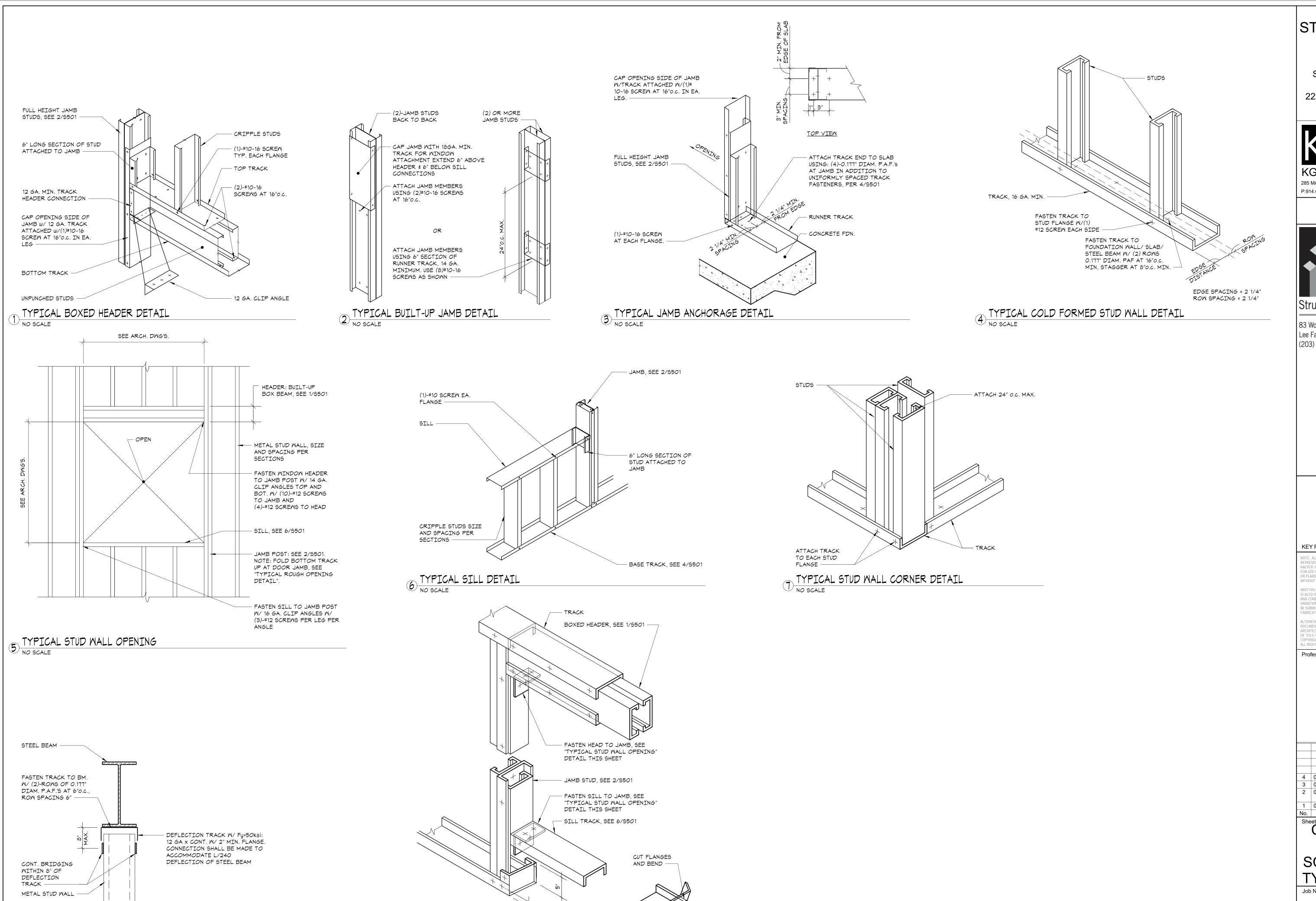
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TYPICAL ROUGH OPENING DETAIL
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8 TYPICAL DEFLECTION TRACK DETAIL
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COLD FORMED FRAMING **SCHEDULES AND** TYPICAL DETAILS

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