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GENERAL NOTES

- ALL STRUCTURAL WORK SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND SHALL CONFORM TO THE PROJECT SPECIFICATIONS, INCLUDING THE XXX BUILDING CODE. ALL GOVERNING STANDARDS LISTED IN THESE NOTES SHALL BE THE EDITION REFERENCED IN THIS GOVERNING CODE.
- CONTRACTOR SHALL PROVIDE TEMPORARY SHORING, BRACING, AND SHEETING AND MAKE SAFE ALL FLOORS, ROOFS, WALLS, AND ADJACENT PROPERTY AS PROJECT CONDITIONS REQUIRE. SHORING AND SHEETING SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE PROJECT JURISDICTION, HIRED BY THE CONTRACTOR, WHO SHALL SUBMIT LICENSED IN THE PROJECT JURISDICTION, HIRED BY THE CONTRACTOR, WHO SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR THE OWNER'S REVIEW.
- DIMENSIONS AND ELEVATIONS OF EXISTING CONSTRUCTION GIVEN IN STRUCTURAL DRAWINGS ARE BASED ON INFORMATION CONTAINED IN VARIOUS ORIGINAL DESIGN AND CONSTRUCTION DOCUMENTS PROVIDED BY THE OWNER, AND LIMITED FIELD OBSERVATIONS AND MEASUREMENTS. THE CONTRACTOR SHALL VERIFY ALL INFORMATION PERTAINING TO EXISTING CONDITIONS BY ACTUAL MEASUREMENT AND OBSERVATION AT THE SITE. ALL DISCREPANCIES BETWEEN ACTUAL CONDITIONS AND THOSE SHOWN IN THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ENGINEER OF RECORD FOR EVALUATION BEFORE THE AFFECTED CONSTRUCTION IS PUT IN PLACE.
- THE CONTRACT DRAWINGS AND SPECIFICATIONS ARE COMPLEMENTARY. THESE NOTES HIGHLIGHT RATHER THAN REPLACE THE SPECIFICATIONS CONTAINED IN THE PROJECT MANUAL.

FOUNDATIONS

- BUILDING FOUNDATIONS SHALL BEAR ON UNDISTURBED SOIL HAVING MINIMUM BEARING AND SHALL CAPACITY OF 2000 PSF AS SPECIFIED BY THE GEOTECHNICAL CONSULTANT. ADEQUACY OF BEARING STRATUM SHALL BE VERIFIED IN FIELD PRIOR TO PLACING CONCRETE. ALL NECESSARY ADJUSTMENTS TO THE BOTTOM OF FOOTINGS TO BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
- DO NOT PLACE BACKFILL AGAINST BASEMENT WALLS UNTIL ALL FLOORS BRACING THESE ARE IN PLACE AND HAVE ATTAINED THEIR 28 DAY STRENGTH.
- ALL EXTERIOR WALLS SHALL BE PLACED A MINIMUM OF 4'-0" BELOW FINAL GRADE.
- CONCRETE SHALL BE POURED IN DRY EXCAVATIONS. CONTRACTOR SHALL NOTE SOIL AND WATER CONDITIONS AS SHOWN BY BORINGS INCLUDED IN THE REFERENCED GEOTECHNICAL SUBSURFACE INVESTIGATION REPORT(S) AND DEPTHS OF FOOTING AS SHOWN ON FOUNDATION PLANS.

CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO THE ACI FOLLOWING GOVERNING STANDARDS.
 - AMERICAN CONCRETE INSTITUTE (ACI) "BUILDING CODE REQUIREMENTS FOR CONCRETE" (ACI 318).
 - ACI "MANUAL OF CONCRETE PRACTICE", LATEST EDITION.
 - CONCRETE REINFORCING STEEL INSTITUTE (CRSI) "MANUAL OF STANDARD PRACTICE", LATEST EDITION.
- ALL CONCRETE COMPOSITE ON METAL DECK SHALL BE LIGHT WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS, UNLESS OTHERWISE NOTED.
- ALL OTHER CONCRETE SHALL BE NORMAL WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS, UNLESS OTHERWISE NOTED.
- REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60 OR A775 EPOXY COATED WHEN CALLED OUT ON PLAN. REINFORCING STEEL SHALL BE DETAILED ACCORDING TO THE ACI "DETAILS AND DETAILING OF REINFORCEMENT" (ACI 318).
- REINFORCING STEEL TO BE WELDED TO CONFORM TO ASTM A706 GRADE 60.
- WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A1064, WITH A MINIMUM YIELD STRENGTH OF 65,000 PSI.
- COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND PIPE SLEEVES WITH ALL OTHER DISCIPLINES DRAWINGS. MINIMUM CONCRETE BETWEEN SLEEVES SHALL BE 4".
- ALL GROUT SHALL BE NO SHRINK WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI.
- OVIDE CLEARANCE FROM FACE OF CONCRETE TO REINFORCEMENT AS FOLLOWS:
 - BEAMS & COLUMNS: 1-1/2"
 - FOOTINGS: 3"
 - EXTERIOR WALLS: 2" FOR #6 OR LARGER, 1-1/2" FOR #5 OR SMALLER
 - INTERIOR WALLS: 3/4"
- SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. NO CONCRETE WORK SHALL COMMENCE WITHOUT APPROVED SHOP DRAWINGS.
- CLEAN AND ROUGHEN TO 1/4" AMPLITUDE ALL EXISTING CONCRETE SURFACES TO RECEIVE NEW CONCRETE PRIOR TO PLACEMENT.
- SEE OTHER DRAWINGS IN THIS PROJECT FOR SIZE AND LOCATIONS OF EQUIPMENT PADS, INSERT AND EMBED ITEMS.
- REINFORCING DOWELS, WATER STOPS, AND OTHER EMBED ITEMS SHALL BE INSTALLED AND SECURED PRIOR TO CONCRETE PLACEMENT. "WET-SETTING" OF EMBEDDED ITEMS IS NOT PERMITTED.

CONCRETE BLOCK

- ALL CONCRETE BLOCK WORK SHALL CONFORM TO THE "NATIONAL CONCRETE MASONRY ASSOCIATION TEK MANUAL FOR THE DESIGN AND CONSTRUCTION OF CONCRETE MASONRY", LATEST EDITION AND ACI 530-BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES".
- CONCRETE BLOCK SHALL BE OF LIGHTWEIGHT AGGREGATE AND CONFORM TO THE FOLLOWING STANDARDS: SOLID/HOLLOW BLOCK: ASTM C90.

NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNIT (PSI)	NET AREA COMPRESSIVE STRENGTH OF MASONRY ASSEMBLY, F _m (PSI) USING TYPE S MORTAR
1900	1500
2800	2000
3750	1500
4800	1500

- UNLESS OTHERWISE NOTED ON PLANS AND/OR ELEVATIONS, CONCRETE BLOCK UNIT STRENGTH SHALL BE 1900 PSI MIN. NOTE: CONCRETE BLOCK WITH UNIT STRENGTH HIGHER THAN 1900 PSI REQUIRE LONGER DELIVERY LEAD TIMES.
- ALL MORTAR SHALL BE ASTM C270, TYPE S.
 - ALL GROUT FOR FILLING CELLS SHALL BE ASTM C 476 WITH MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI BUT NOT LESS THAN THE COMPRESSIVE STRENGTH OF THE MASONRY ASSEMBLY. F_m IN WHERE GROUT CELLS DO NOT EXCEED 4" IN DIAMETER FINE GROUT SHALL BE USED. ALL BLOCK.
 - DIMENSIONS INDICATED ON STRUCTURAL PLANS ARE NOMINAL DIMENSIONS.
 - ALL CONCRETE BLOCK BELOW GRADE SHALL BE FILLED SOLID WITH GROUT.
 - CONCRETE BLOCK BELOW BEAM OR TRUSS BEARING POINTS SHALL BE FILLED SOLID FOR A MINIMUM OF TWO COURSES IN DEPTH AND A MINIMUM OF 32" IN WIDTH, UNLESS NOTED OTHERWISE.
 - INSTALL STANDARD WEIGHT LADDER JOINT REINFORCEMENT AT 18" ON CENTER (SPACED VERTICALLY).
 - UNLESS NOTED OTHERWISE ALL MASONRY WALLS SHALL BE REINFORCED WITH #4@48" ON CENTER VERTICAL. GROUT ALL REINFORCED CELLS SOLID. PROVIDE DOWELS TO MATCH VERTICAL REINFORCING AT FOUNDATION.
 - WHERE CMU IS SUPPORTED ON STRUCTURAL STEEL MEMBERS, PROVIDE WELDED REBAR DOWELS BETWEEN STEEL AND CMU. DOWELS TO MATCH SIZE AND SPACING OF CMU REINFORCEMENT AND MUST PROVIDE FULL TENSION LAP SPLICE WITH CMU REINFORCEMENT. DOWELS MUST BE WELD ABLE AND CONFORM TO ASTM A706, GRADE 60.

STEEL DECK

- STEEL DECKING WORK SHALL CONFORM TO THE AISI NORTH AMERICAN "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
- STEEL DECKING UNITS AND ACCESSORY ITEMS SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM A611 OR A653 WITH A MINIMUM YIELD STRENGTH OF 33,000 PSI. BEFORE FORMING, THE STEEL SHEET SHALL RECEIVE A HOT DIP GALVANIZED COATING CONFORMING TO ASTM A653, GRADE 90.
- STEEL DECKING SHALL BE SHORED AS REQUIRED BY PLANS OR BY SPAN AND LOAD CONDITIONS TO SUPPORT WET WEIGHT OF CONCRETE AND ALL CONSTRUCTION LOADS.
- THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY APPROVED METHOD (TO BE SHOWN ON SHOP DRAWINGS) AT INTERVALS TO PROVIDE SUFFICIENT DIAPHRAGM STRENGTH TO MAINTAIN BUILDING ALIGNMENT AND TO SUSTAIN LOCAL CONSTRUCTION LOADS WITHOUT DISTORTION OR SEPARATION. MAXIMUM SPACING SHALL BE 3'-0" BETWEEN SUPPORT BEAMS. END LAPS OF SHEETS SHALL BE A MINIMUM OF 2 RIBS.
- EXCEPT AS OTHERWISE NOTED, DECK SHALL BE ATTACHED TO STRUCTURAL STEEL BY 3/4"x9 FUSION WELDS @12" ON CENTER AT END AND INTERIOR SUPPORTS PERPENDICULAR TO THE DECK SPAN AND AT EDGE AND INTERIOR SUPPORTS PARALLEL TO THE DECK SPAN. WELDS MAY BE OMITTED IN RIBS IN WHICH SHEAR CONNECTORS ARE TO BE APPLIED, EXCEPT THAT EACH DECK SECTION SHALL HAVE SUFFICIENT WELDS TO ADEQUATELY SECURE THE DECK, BRING THE DECK INTO DIRECT CONTACT WITH THE SUPPORTING STEEL AND TO PROVIDE SUFFICIENT DIAPHRAGM STRENGTH TO MAINTAIN BUILDING ALIGNMENT.
- PRIOR TO FABRICATION, THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR THE STEEL DECKING SHOWING DECK GAUGE, SIZE, AND LAYOUT AS WELL AS CLOSURE CONDITIONS, WELDS TO SUPPORTS AND SIDE LAP DETAILS.
- ALL REINFORCED OPENINGS IN STEEL DECK SHALL BE INSTALLED BY STEEL DECK SUBCONTRACTOR. STEEL DECK SUBCONTRACTOR TO PROVIDE REINFORCING AS PER TYPICAL DETAILS.
- AT STEEL DECK WITHOUT CONCRETE FILL THE FOLLOWING MAY BE ATTACHED WITHOUT SPECIFIC APPROVAL OF THE STRUCTURAL ENGINEER: ACOUSTICAL TILE AND GYPSUM BOARD CEILING ONLY; NO PIPING, DUCTING OR CONDUIT. MAXIMUM CEILING WEIGHT = 3.5 PSF. MAXIMUM WIRE HANGER LOAD = 60 LBS.
- WHERE SUSPENSION OF HANGER WIRES ARE REQUIRED BY OTHERS, VERIFY AND COORDINATE LOCATIONS, PATTERNS, SPACING, ETC. WITH THE APPROPRIATE TRADE. DRILL OR PUNCH HOLES AT BOTTOM OF DECK FLUTES OF SUFFICIENT SIZE TO PASS SUPPORT WIRES. WIRE SUPPORTS SHALL BE LOOPED AND SECURED WITH A MINIMUM OF THREE (3) TIGHT TURNS AROUND A MINIMUM 1-1/2" x 12" LONG FURRING CHANNEL OR NO. 3x12" LONG REINFORCING BAR CENTERED ABOVE THE HOLE AND LAID IN THE DECK FLUTES.

POST INSTALLED ADHESIVE AND MECHANICAL ANCHORS

- POST INSTALLED ANCHORAGE SHALL BE INSTALLED PER MANUFACTURER TECHNICAL DATA TO INTACT BASE MATERIAL. FOR INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS, THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ON-SITE INSTALLATION TRAINING FOR THE ANCHORING PRODUCTS SPECIFIED. PROVIDE STRUCTURAL ENGINEER OF RECORD WITH DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS PER ACI 318 APPENDIX D SECTION 9. NOTIFY ENGINEER OF RECORD PRIOR TO INSTALLATION IF BASE MATERIAL CONDITION DEVIATES FROM STRUCTURAL DRAWINGS OR MANUFACTURER TECHNICAL DATA. ALL HOLES SHALL BE DRY CONCRETE AND HAMMER DRILLED UNLESS OTHERWISE NOTED.
- MANUFACTURER DATA FOR ALTERNATE ANCHORAGE PROPOSED BY CONTRACTOR SHALL BE SUBMITTED TO ENGINEER OF RECORD FOR REVIEW AND APPROVAL. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC-ES SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE AND MUST PROVIDE INFORMATION ON THESE ITEMS.
- UNLESS OTHERWISE INDICATED, POST INSTALLED ANCHORAGE SHALL BE ADHESIVE TYPE HILTI HIT -HY 200 INTO CONCRETE, GROUT FILLED CMU OR STONE BASE MATERIAL OR HILTI HIT -HY 70 INTO BRICK MASONRY OR UNGROUTED CMU BASE MATERIAL.
- EXISTING REINFORCING BARS IN THE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE EXISTING REBARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS BY A MEANS APPROVED BY THE ENGINEER OF RECORD.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING GOVERNING STANDARDS:
 - AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" AND AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
 - ANSI 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
 - AMERICAN WELDING SOCIETY (AWS D1.1) "STRUCTURAL WELDING CODE - STEEL"
 - RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC) "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS"
 - ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS:
 - WIDE FLANGE BEAMS, COLUMNS, AND STRUCTURAL TEES: ASTM A992
 - HOLLOW STRUCTURAL SECTIONS: ASTM A4210, GRADE C
 - STRUCTURAL PIPE SECTIONS: ASTM A53, GRADE B
 - CHANNELS, ANGLES, AND PLATES: ASTM A36 UNLESS OTHERWISE NOTED.
 - BOLTED CONNECTIONS OF BEAMS/GIRDERS ARE TO BE SELECTED AS FOLLOWS:
 - STANDARD BEAM TO BEAM/GIRDER: ASTM F3125, GRADES A325, F1882, A490 OR F2280 BOLTS IN BEARING TYPE CONNECTIONS (3/4" DIAMETER MINIMUM WITH HARDENED WASHERS).
 - BEAM/GIRDER TO COLUMN CONNECTIONS, COLUMN SPLICES AND BOLTS EXPERIENCING TENSION LOADS (UNLESS OVERSIZED OR SLOTTED HOLES ARE USED, IN WHICH CASE SLIP-CRITICAL JOINTS SHALL BE USED): ASTM F3125, GRADES A325, F1882, A490 OR F2280 BOLTS IN PRETENSIONED JOINTS (3/4" DIAMETER MINIMUM WITH HARDENED WASHERS).
 - MOMENT CONNECTIONS AND BRACED FRAME CONNECTIONS: ASTM F3125, GRADES A325, F1882, A490 OR F2280 BOLTS IN SLIP CRITICAL JOINTS (3/4" DIAMETER MINIMUM WITH HARDENED WASHERS). FAYING SURFACES SHALL BE CLASS A UNLESS OTHERWISE NOTED.
 - PER AISI 341, ALL BOLTS SHALL BE INSTALLED AS PRETENSIONED HIGH STRENGTH BOLTS AND MEET THE REQUIREMENTS FOR SURFACE PREPARATION FOR SLIP CRITICAL CONNECTIONS WITH CLASS A SLIP COEFFICIENT OR HIGHER. THE AVAILABLE SHEAR STRENGTH OF BOLTED JOINTS USING STANDARD HOLES SHALL BE CALCULATED AS THAT FOR BEARING TYPE JOINTS.
 - ANCHOR BOLTS: ASTM F1554, GRADE 36.
 - STEEL CONNECTION SHALL BE STANDARD AISI-CRAM BEAM CONNECTIONS, AND SHALL BE DESIGNED BY A LICENSED ENGINEER WORKING FOR THE FABRICATOR, WHO SHALL PROVIDE CALCULATIONS, UTILIZING LRFD LOADS AND PROCEDURES.
 - UNLESS OTHERWISE NOTED ON PLAN, PROVIDE CONNECTIONS BASED ON MINIMUM SHEAR CAPACITY REQUIREMENTS IN THE FOLLOWING TABLE.
 - REINFORCING IS TO BE PROVIDED AT CONNECTIONS WHERE CUTS REDUCE THE SHEAR OR MOMENT CAPACITY BELOW THAT REQUIRED TO SUSTAIN THE REACTION. FLANGES AND WEB ARE TO BE REINFORCED WHERE THE LOCAL CAPACITY TO SUSTAIN CONNECTION LOAD IS INADEQUATE.
 - CONNECTIONS SHALL BE DESIGNED FOR SHEAR AND ECCENTRICITY, CONSIDERING THAT THE CONNECTION IS AN EXTENSION OF THE BEAMS AND GIRDERS.
- | MINIMUM SHEAR/MOMENT CAPACITY REQUIREMENTS | | | |
|--|--------------------------------|---------------------------------|------------------------------------|
| BEAM DEPTH (NOMINAL) | MIN. SHEAR CAPACITY ASD (KIPS) | MIN. SHEAR CAPACITY LRFD (KIPS) | MIN. MOMENT CAPACITY LRFD (KIP-FT) |
| 8", 10" | 16 | 24 | 140 |
| 12", 14" | 28 | 42 | 160 |
| 16" | 40 | 62 | 180 |
| 18" | 52 | 78 | 250 |
| 21" | 58 | 88 | N/A |
| 24", 27" | 74 | 108 | N/A |
| 30" | 94 | 126 | N/A |
| 33" | 94 | 142 | N/A |
| 36" | 102 | 155 | N/A |
| 40+" | 110 | 165 | N/A |
- MINIMUM WELD SIZE IS 1/4" FILLET UNLESS NOTED OTHERWISE.
 - ALL BEAMS EXCEPT CANTILEVER BEAMS SHALL BE FABRICATED AND INSTALLED WITH NATURAL CAMBER UP. CANTILEVER BEAMS SHALL BE FABRICATED AND INSTALLED SO THAT NATURAL CAMBER RAISES CANTILEVER END.
 - FIELD CUTTING OR BURNING OF STEEL IS PROHIBITED EXCEPT WITH THE EXPRESS WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD. (IN WHICH CASE, ALL BURNING OF STEEL MUST CONFORM TO THE THERMAL CUTTING REQUIREMENTS OF AISC AND AWS).
 - WELDING SHALL BE PERFORMED BY CERTIFIED LICENSED, AWS-QUALIFIED WELDERS. WELDING ELECTRODES FOR CARBON STEEL SHALL BE AWS E1.1, C OR E7018 GRADE 50 KSI PLATE USE ELECTRODE E7018 OR APPROVED EQUAL (OR ELECTRODES THAT MEET THE REQUIREMENT OF A.). WELDING ELECTRODES FOR ASTM A276 STAINLESS STEEL, TYPE 304, SHALL CONFORM TO AWS A5.4 FOR SHIELDED METAL ARC WELDING. ELECTRODE CLASS E308, OR AWS A5.9 FOR GAS METAL ARC WELDING, ELECTRODE CLASS ER308, WELDING ELECTRODES FOR JOINING STAINLESS STEEL TO CARBON STEEL SHALL CONFORM TO ELECTRODE CLASS E309/ER309.
 - SHOP PAINT EXTERIOR EXPOSED STEEL MEMBERS, STEEL MEMBERS NOT ENCASED IN CONCRETE OR PROTECTED AGAINST CORROSION, AND STEEL MEMBERS AT THE EXTERIOR WALL WITH NEMEC #10-99 "OR APPROVED EQUAL EXCEPT FOR MEMBERS TO BE HOT DIPPED GALVANIZED.
 - HOT DIP GALVANIZING SHALL CONFORM TO ASTM A123. REPAIR SCRATCHES OR ABRADED GALVANIZED SURFACE WITH ZINC RICH PAINT. ALL EXTERIOR EXPOSED STEEL AND STEEL SUPPORTING EXTERIOR SHALL BE HOT DIPPED GALVANIZED.
 - UNTELS SHALL BE INSTALLED OVER ALL OPENINGS IN MASONRY WALLS AS FOLLOWS:

MASONRY LINTELS	LINTEL
4'-0" OR LESS	L4x3-12x5/16 LLV
4'-1" TO 7'-0"	L6x3-12x5/16 LLV

- 3-1/2" LEGS ARE HORIZONTAL
- PROVIDE ONE ANGLE FOR EACH 4' OF WALL THICKNESS
- PROVIDE L5x5x5/16 ANGLES FOR 6" THICK WALLS AND PARTITIONS WITH OPENINGS UP TO 6'-0".
- PROVIDE MINIMUM 6" BEARING AT EACH END.
- UNTELS OVER 6'-0" SHALL BE REPROPOSED.

- SHOP AND ERECTION DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. NO FABRICATION OF STEEL SHALL COMMENCE WITHOUT APPROVED SHOP DRAWINGS.
- PROVIDE MECHANICALLY GALVANIZED BOLTS FOR EXTERIOR APPLICATIONS.
- ALL STEEL CONNECTIONS MUST MEET THE REQUIREMENTS OF SECTION 2213 OF THE NYC BUILDING CODE.
- ALL EXPOSED WELDS TO BE GROUND SMOOTH.

COLD FORMED METAL FRAMING

- ALL COLD FORMED METAL FRAMING WORK SHALL COMPLY WITH THE AISI NORTH AMERICAN "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", AS WELL AS ANSI A42.4 "SPECIFICATIONS FOR INTERIOR LATHING AND FURRING."
- ALL PLYWOOD APPLIED TO METAL JOISTS SHALL BE SCREWED AND GLUED TO THE JOISTS. THE ADHESIVE SHALL BE AN APA APPROVED ELASTOMERIC ADHESIVE.
- INSTALL METAL FRAMING IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS AND RECOMMENDATIONS, UNLESS OTHERWISE INDICATED. ALL MATERIALS SHALL BE GALVANIZED.
- ALL LOAD BEARING STUDS, JOISTS, AND ACCESSORIES SHALL BE MADE OF THE MINIMUM TYPE, SIZE, GAUGE, AND SPACING SHOWN ON DRAWINGS AND PROVEN IN THE CALCULATIONS.
- SUBMIT SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR ALL LOAD BEARING COLD FORMED METAL FRAMING (JOISTS, STUDS, ETC.) PRIOR TO FABRICATION SHOP DRAWINGS SHALL INDICATE PLACING OF ALL FRAMING MEMBERS SHOWING TYPE, SIZE, GAGE, NUMBER, LOCATION AND SPACING. THEY SHALL ALSO INDICATE SUPPLEMENTAL STRAPPING, BRACING, SPLICES, BRIDGING, ACCESSORIES AND DETAILS REQUIRED FOR PROPER INSTALLATION. SEE SPECIFICATIONS, LOADING DIAGRAMS AND SCHEDULE FOR STRUCTURAL PERFORMANCE CRITERIA.
- SHOP DRAWINGS SHALL SHOW SIZE AND LENGTH OF WELDS FOR ALL WELDED CONNECTIONS AND TYPE, SIZE AND NUMBER OF SCREWS FOR ALL SCREWED CONNECTIONS. SUBMIT MANUFACTURER DATA GIVING STRENGTH VALUES FOR ALL FASTENERS USED. WELDED CONNECTIONS SHALL BE WIRE BRUSHED AND COATED WITH A ZINC-RICH PAINT.
- ALL GALVANIZED STUDS AND/OR JOISTS, NO. 12, 14 AND 16 GAGE, SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF ASTM A446, GRADE D, WITH A MINIMUM YIELD OF 50,000 PSI. ALL GALVANIZED 18 AND 20 GAGE STUDS AND/OR JOISTS, AND ALL GALVANIZED TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF ASTM A446, GRADE A, WITH A MINIMUM YIELD OF 33,000 PSI.
- ALL STUDS, JOIST AND ACCESSORIES SHALL BE PAINTED WITH RUST - INHIBITIVE PAINT MEETING THE PERFORMANCE REQUIREMENTS OF TT-P-3636, OR SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING.
- FRAMING COMPONENTS MAY BE PRE-ASSEMBLED INTO PANELS PRIOR TO ERECTING. PREFABRICATED PANELS SHALL BE SQUARE WITH COMPONENTS ATTACHED IN A MANNER AS TO PREVENT RACKING.
- AXIALLY LOADED STUDS SHALL BE INSTALLED IN A MANNER WHICH WILL ASSURE THE ENDS OF THE STUDS ARE POSITIONED AGAINST THE INSIDE TRACK WEB, PRIOR TO STUD AND TRACK ATTACHMENT.
- STUDS SHALL BE PLUMBED, ALIGNED AND SECURELY ATTACHED TO THE FLANGES OR WEBS OF BOTH UPPER AND LOWER TRACKS.
- WALL STUD BRIDGING SHALL BE ATTACHED IN A MANNER TO PREVENT STUD ROTATION. BRIDGING ROWS SHALL BE SPACED ACCORDING TO THE FOLLOWING SCHEDULE. WALLS UP TO 10'-0" HEIGHT: ONE ROW AT MID-HEIGHT. WALLS EXCEEDING 10'-0" HEIGHT: BRIDGING ROWS SPACED NOT TO EXCEED 5'-0" ON CENTER.
- CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL TEMPORARY BRACING AND SHORING AS REQUIRED UNTIL ERECTION IS COMPLETED AND ALL ATTACHED ADJACENT FRAMING IS COMPLETE.
- BRIDGES IN AXIALLY LOADED STUDS ARE NOT PERMITTED.
- JOISTS SHALL BE LOCATED DIRECTLY OVER BEARING STUDS OR LOAD DISTRIBUTION MEMBER TO BE PROVIDED AT THE TOP TRACK.

SPECIAL INSPECTIONS (BC)

- INSPECTIONS REQUIRED BY THE LOCAL JURISDICTION SHALL BE PERFORMED BY A TESTING AGENCY PROVIDED BY THE OWNER FOR THE FOLLOWING ITEMS:
 - INSPECTION OF FABRICATORS (IBC 1704.2.5)
 - STEEL CONSTRUCTION (IBC 1705.2)
 - STRUCTURAL STEEL (IBC 1705.2.1)
 - STRUCTURAL STEEL WELDING (AISC 360, AWS D1.1)
 - HIGH STRENGTH BOLTS (AISC 303)
 - STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL: (IBC 1705.2.2, TABLE 1705.2.2)
 - COLD-FORMED STEEL DECK (IBC 1705.2.2)
 - OPEN-WEB STEEL JOISTS AND JOIST GIRDERS (IBC 1705.2.3, TABLE 1705.2.3)
 - COLD-FORMED STEEL TRUSSES SPANNING 60 FEET OR GREATER (IBC 1705.2.4)
 - CONCRETE CONSTRUCTION (IBC 1705.3, TABLE 1705.3)
 - WELDING OF REINFORCING BARS (IBC 1705.3.1, TABLE 1705.3.1)
 - MATERIALS TESTS (IBC 1705.3.2, TABLE 1705.3)
 - POST-INSTALLED MECHANICAL ANCHORS (IBC TABLE 1705.3, ACI 318 CHAPTER 17)
 - MASONRY CONSTRUCTION (IBC 1705.4, ACI 530 AND ACI 530.1 LEVEL B QUALITY ASSURANCE)
 - WOOD CONSTRUCTION (IBC 1705.5)
 - HIGH-LOAD DIAPHRAGMS (IBC 1705.5.1)
 - METAL PLATE-CONNECTED WOOD TRUSSES SPANNING 60 FEET OR GREATER (IBC 1705.5.2)
 - SOILS (IBC 1705.6, TABLE 1705.6)
 - DRIVEN DEEP FOUNDATIONS (IBC 1705.7, TABLE 1705.7)
 - CAST-IN-PLACE DEEP FOUNDATIONS (IBC 1705.8, TABLE 1705.8)
 - HELICAL PILE FOUNDATIONS (IBC 1705.9)
 - FABRICATED ITEMS (IBC 1705.10)
 - SPECIAL INSPECTIONS FOR WIND RESISTANCE (IBC 1705.11)
 - STRUCTURAL WOOD (IBC 1705.11.1)
 - COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION (IBC 1705.11.2)
 - WIND-RESISTING COMPONENTS (IBC 1705.11.3)
 - SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE (IBC 1705.12)
 - STRUCTURAL STEEL (IBC 1705.12.1)
 - STRUCTURAL WOOD (IBC 1705.12.2)
 - COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION (IBC 1705.12.3)
 - COLD-FORMED STEEL SPECIAL BOLTED MOMENT FRAMES (IBC 1705.12.9)
 - TESTING AND QUALIFICATION FOR SEISMIC RESISTANCE (IBC 1705.13)
 - STRUCTURAL STEEL (IBC 1705.13.1)
- STRUCTURAL OBSERVATIONS REQUIRED BY THE LOCAL JURISDICTION AND IBC 1704.5 SHALL BE PERFORMED BY A REGISTERED DESIGN PROFESSIONAL PROVIDED BY THE OWNER. STRUCTURAL OBSERVATIONS SHALL BE THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS.
- TESTING AGENCY FOR THE INSPECTIONS SHALL FILE ALL APPROPRIATE FORMS WITH THE BUILDING DEPARTMENT.

STANDARD ABBREVIATIONS

ADPL	ADDITIONAL	ENGR	ENGINEER
ADJ.	ADJACENT	E.O.R.	ENGINEER OF RECORD
A/E	DESIGN TEAM OF RECORD	EQ.	EQUAL
ALT.	ALTERNATE	E.S.	EACH SIDE
ANCH.	ANCHOR	E.W.	EACH WAY
APPROX.	APPROXIMATE/APPROXIMATELY	EXP.	EXPANSION
ARCH.	ARCHITECT/ARCHITECTURAL	EXT.	EXTERIOR
BLDG.	BUILDING	FDN.	FOUNDATION
B.F.	BRACE FRAME	FIN.	FINISH
BM	BEAM	FLR.	FLOOR
B.O.	BOTTOM OF	FRMG.	FRAMING
BOT.	BOTTOM	F.S.	FAR SIDE
BRG.	BEARING	FT.	FEET
BSMT.	BASEMENT	FOOT.	FOOTING
CANT.	CANTILEVER	G.A.	GAGE
CFS	COLD FORMED STEEL	GALV.	GALVANIZED
C.F.	CAST IN PLACE	G.B.	GRADE BEAM
C.J.P.	CONTRACTION JOINT	HDR.	HEADER
CLG.	CEILING	HGR.	HANGER
CLR.	CLEAR	HORIZ.	HORIZONTAL
CMU	CONCRETE MASONRY UNIT	H.P.	HIGH POINT
COL.	COLUMN	HT.	HEIGHT
COMP.	COMPOSITE	HVAC	HEATING, VENTILATION, & AIR CONDITIONING
CONC.	CONCRETE	I.D.	INSIDE DIAMETER
CONSTR.	CONSTRUCTION	I.F.	INSIDE FACE
CONT.	CONTINUOUS	I.J.	ISOLATION JOINT
COORD.	COORDINATE/COORDINATION	INFO	INFORMATION
CONTR.	CONTRACTOR	INT.	INTERIOR
COTR.	CONTRACT OFFICER'S TECHNICAL REPRESENTATIVE	J.	JOINT
CTR.	CENTER	K.	KIP
DBL.	DOUBLE	LB.	POUND
DEMO	DEMOLITION/DEMOLISH	L.L.	LIVE LOAD
DIA.	DIAMETER	L.L.B.B.	LONG LEGS BACK-TO-BACK
DIAG.	DIAGONAL	L.W.	LONG LEG HORIZONTAL
DIM.	DIMENSION	LLV.	LONG LEG VERTICAL
D.L.	DEAD LOAD	L.P.	LOW POINT
DN.	DOWN	L.W.	LIGHTWEIGHT
DTL.	DETAIL	L.W.	LONG WAY
DWG(S)	DRAWING(S)	MAS.	MASONRY
DWL.	DOWEL	MAX.	MAXIMUM
EAL.	EACH	MECH.	MECHANICAL
E.F.	EACH FACE	MEP	MECH., ELECT., PLUMBING, & FIRE PROTECTION
ELEV.	EXPANSION JOINT	MANU.	MANUFACTURER
ELEC.	ELEVATION	MIN.	MINIMUM
ELEC.	ELECTRICAL	MISC.	MISCELLANEOUS
ELEV.	ELEVATOR	M.O.	MASONRY OPENING
EMBED.	EMBEDMENT	N.F.	NEAR FACE
E.O.	EDGE OF	N.I.C.	NOT IN CONTRACT

DESIGN PARAMETER TABLE		
GOVERNING CODE:	2015 INTERNATIONAL BUILDING CODE, AS ADOPTED BY NEW YORK STATE	
BUILDING CATEGORY:	II	
SNOW LOAD:	35 PSI	P _g GROUND SNOW LOAD
	1.0	C _e SNOW EXPOSURE FACTOR
	1.0	I _s SNOW LOAD IMPORTANCE FACTOR
	1.0	C _t THERMAL FACTOR
WIND LOAD:	120 MPH	BASIC WIND SPEED
	1.0	I WIND IMPORTANCE FACTOR
	0	C WIND EXPOSURE CATEGORY
	0.18 +/-	G _{CFI} INTERNAL PRESSURE COEFFICIENT
	34 PSF	CAC VELOCITY PRESSURE AT MEAN ROOF HEIGHT
	99 KIP	V DESIGN BASE SHEAR
SEISMIC DESIGN:	1.0	I SEISMIC IMPORTANCE FACTOR
	0.195	S _s SHORT PERIOD SPECTRAL RESPONSE ACCELERATION
	0.065	S ₁ 1-SECOND PERIOD SPECTRAL RESPONSE ACCELERATION
	0	S ₀ SITE CLASS
	0.300	S ₀ (b) 5%-DAMPED SPECTRAL RESPONSE COEFFICIENT AT SHORT PERIODS
	0.156	S ₀ (d) 5%-DAMPED SPECTRAL RESPONSE COEFFICIENT AT 1-SECOND PERIODS
	B	SEISMIC DESIGN CATEGORY
STEEL MOMENT FRAME:	95 KIP	BASIC SEISMIC FORCE RESISTING SYSTEM
	0.067	C _s SEISMIC RESPONSE COEFFICIENT
	3	R RESPONSE MODIFICATION FACTOR
EQUIVALENT STATIC FORCE:		ANALYSIS PROCEDURE

NOTE:

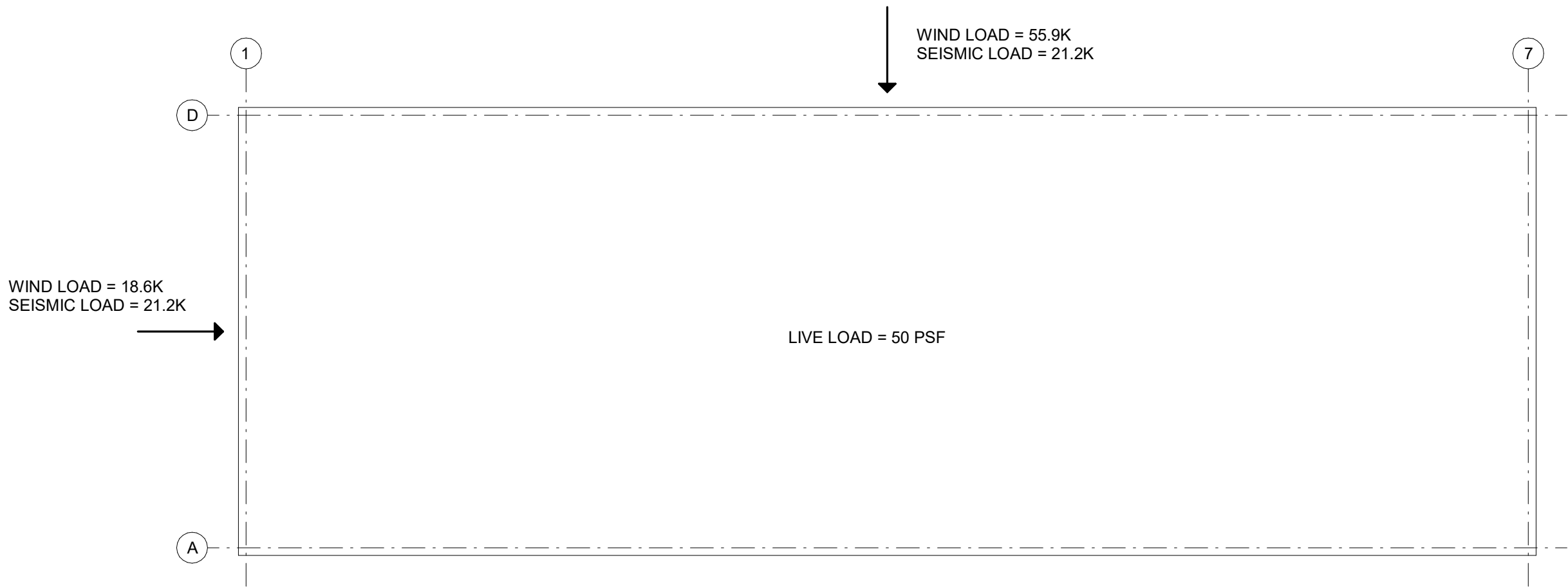
3" = 1'-0"

02 - Structural Steel Framing Key CONNECTION REQUIREMENTS.

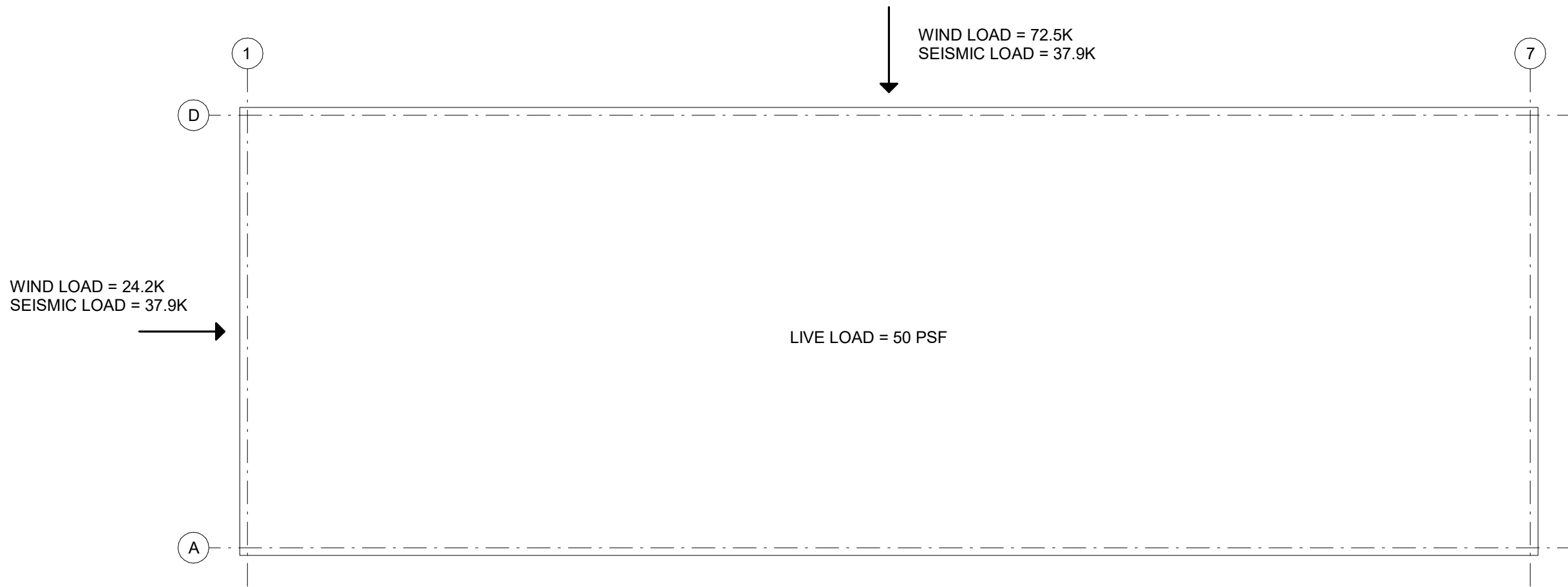
LEGEND

- CONCRETE FOOTING, GRADE BEAM OR PILE CAP
- WALL OR CONCRETE BEAM BELOW
- CONCRETE WALL
- COLD FORMED METAL FRAMING WALL, CFW6 U.O.N. ON PLAN
- COLD FORMED METAL SHEAR WALL WITH FLAT PLATE CROSS BRACING: CFW6 U.O.N. ON PLAN
- STEEL COLUMN
- L4x4x3/8 PARTITION HANGER SUPPORT
- STEEL BEAM
- STEEL LINTEL (SEE SCHEDULE)
- STEEL BRACING (L4x4x5/16 UNO)
- MECHANICAL WEB PENETRATION
- LEDGER ANGLE (SEE DETAIL)
- SLOPE STEEL BEAM DIRECTION
- BENT STEEL FRAMING
- SHEAR CONNECTION
- MOMENT CONNECTION
- COLUMN BASE PLATE
- COLUMN TRANSFER
- CONCRETE ON METAL DECK (SEE SCHEDULE), OPEN ARROW INDICATES SPAN DIRECTION
- METAL DECK (SEE SCHEDULE), OPEN ARROW INDICATES SPAN DIRECTION
- METAL GRATING FLOOR (SEE SCHEDULE), OPEN ARROW INDICATES SPAN DIRECTION
- SLAB ON GRADE (SEE SCHEDULE)
- SLOPE RAMP
- COLUMN ABOVE / BELOW
- HANGER COLUMN ABOVE / BELOW
- TOP OF SLAB ELEVATION
- SPOT ELEVATION
- INDICATES STEP DOWN
-

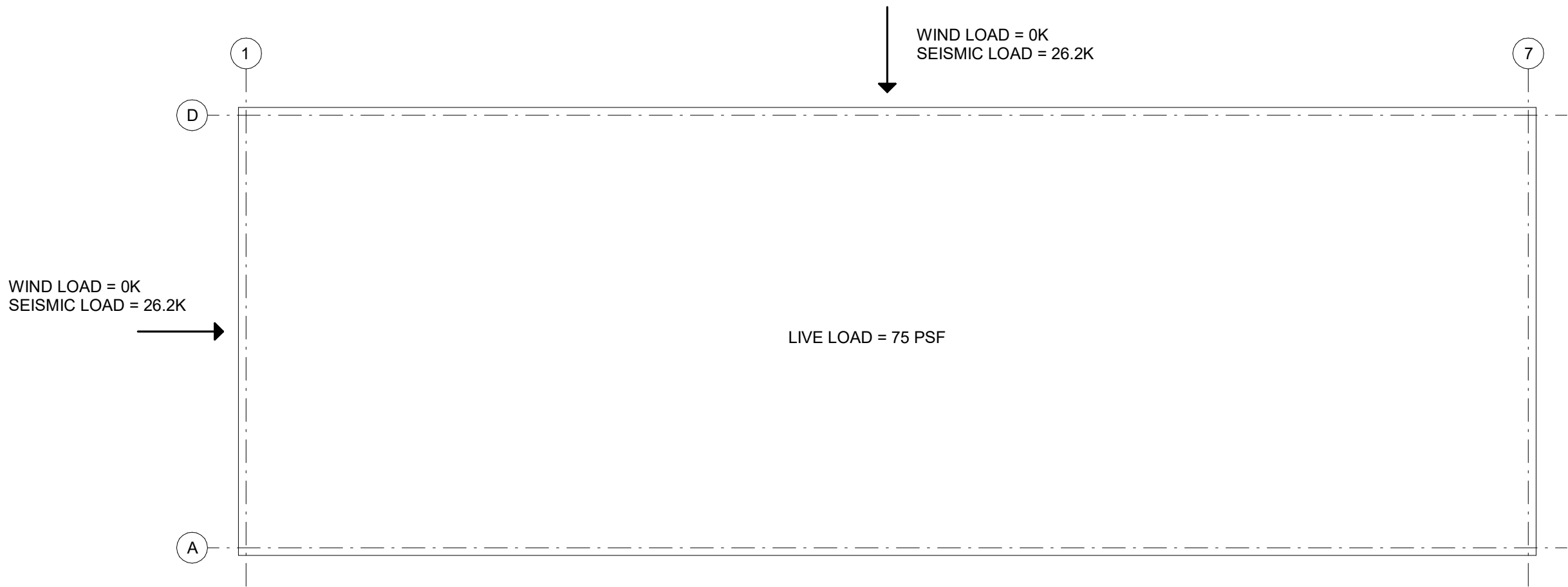
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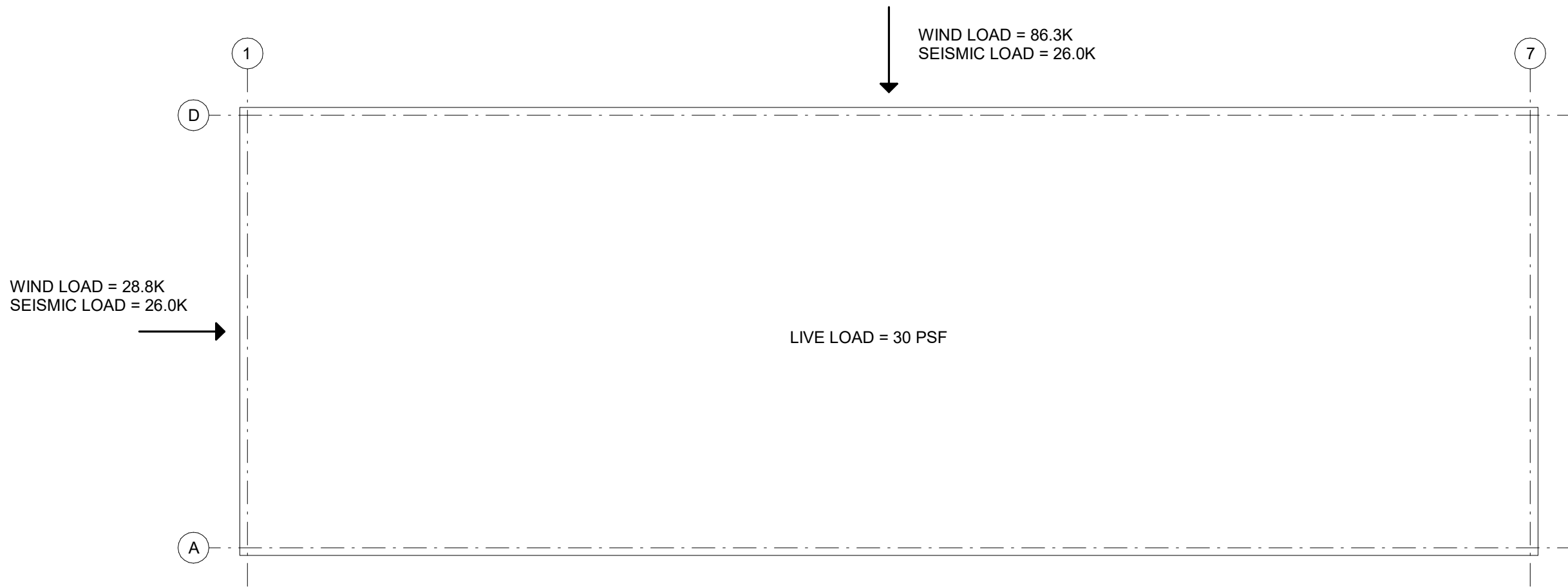
INN SECOND FLOOR LOADING



INN THIRD FLOOR LOADING



INN MECHANICAL MEZZANINE FLOOR LOADING

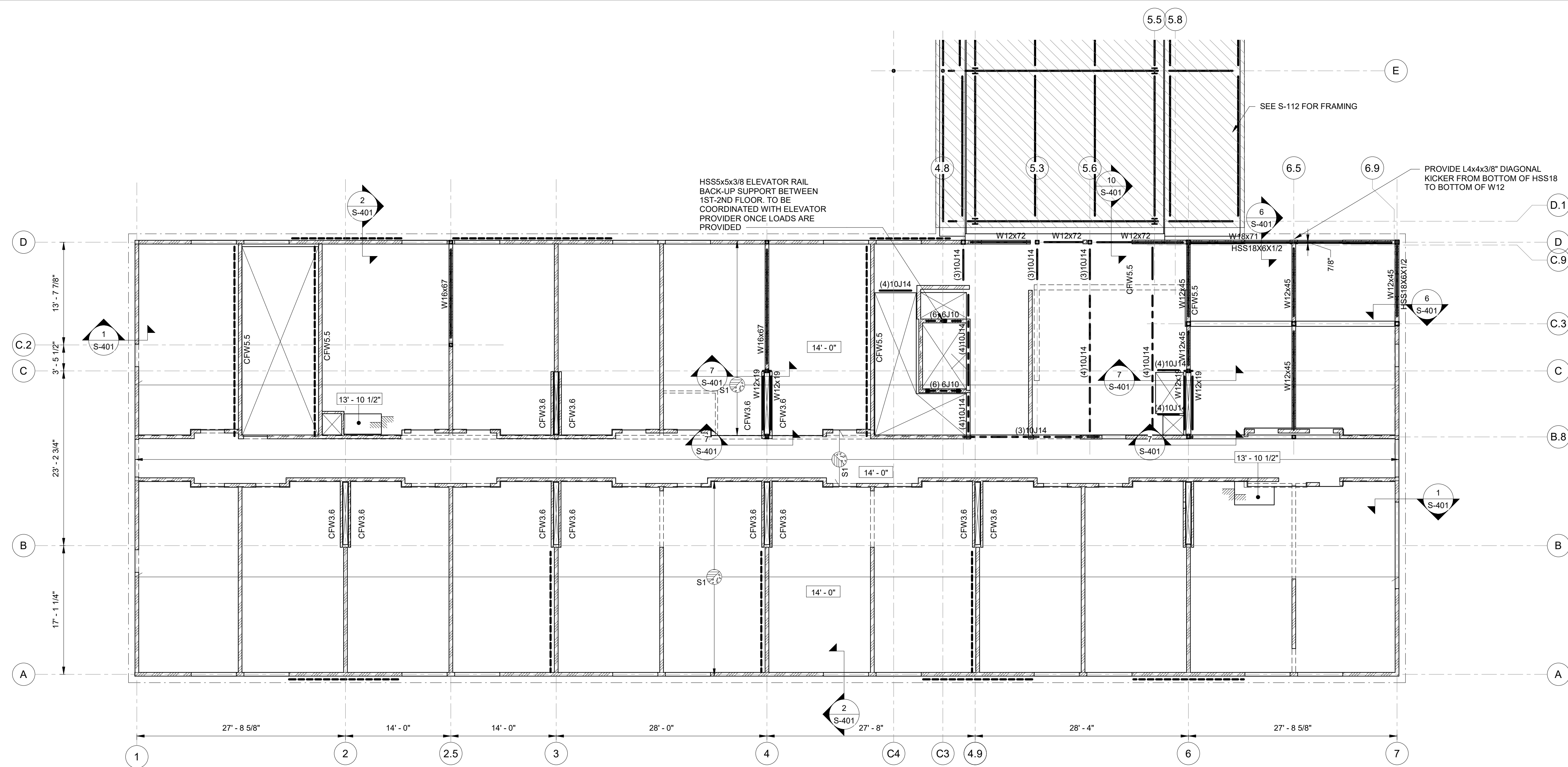


INN ROOF FLOOR LOADING

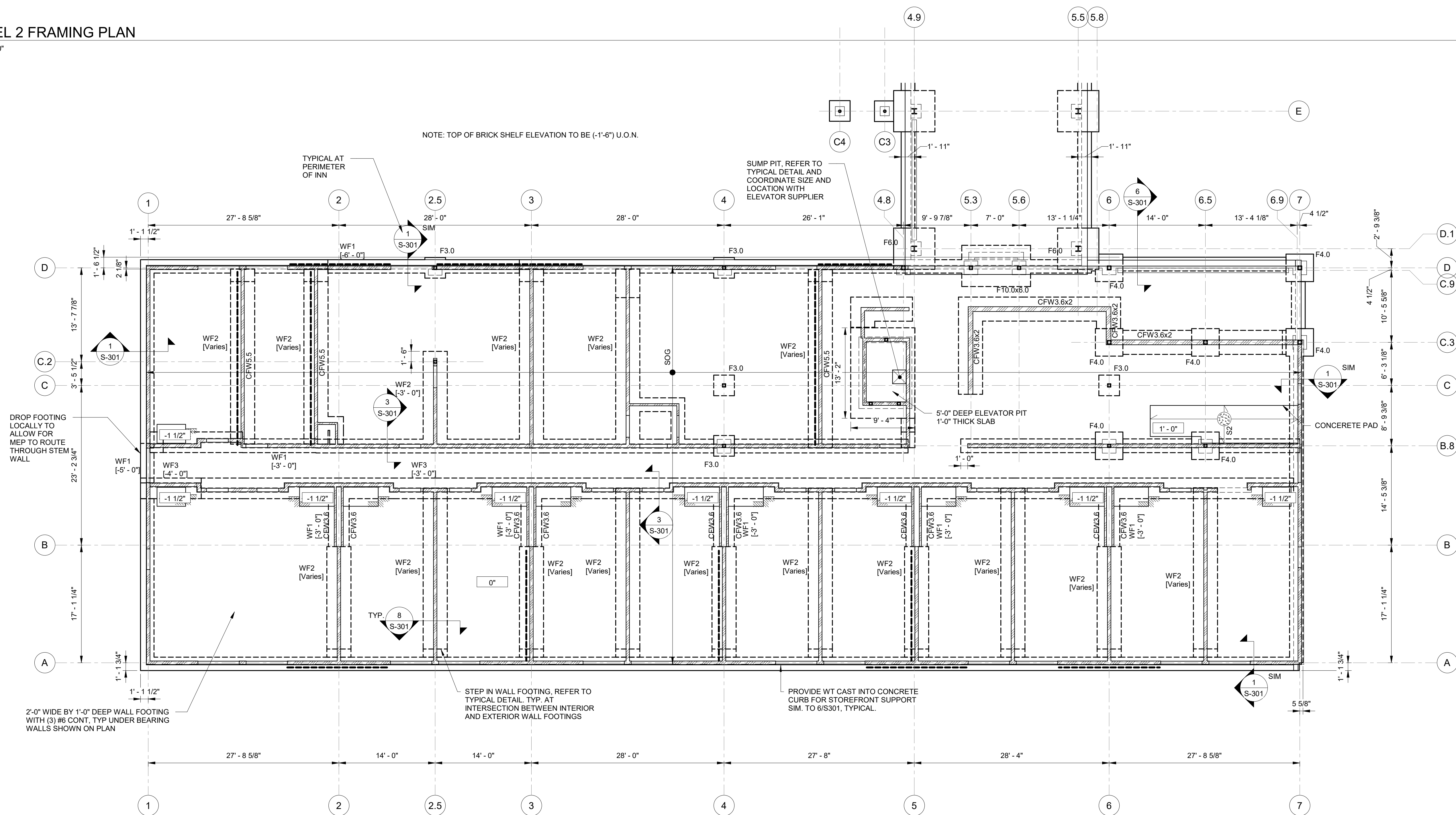
INN LOADING
N.T.S.

NOTES:
1. WIND LOADING IS SHOWN FOR MWFRS
2. REFER TO S-001 FOR ALL OTHER GENERAL NOTES AND DESIGN LOAD PARAMETERS

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2 LEVEL 2 FRAMING PLAN
S-101 1/8" = 1'-0"



1 FOUNDATION FRAMING PLAN
S-101 1/8" = 1'-0"

- # NOTES
1. TOP OF SLAB ELEVATION IS NOTED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF FINISH FLOOR ELEVATION AND DATUM INFORMATION.
 2. BOTTOM OF FOOTING ELEVATIONS ARE [8'-0"] BELOW GRADE AT EXTERIOR AND [2'-0"] BELOW TOP OF SLAB AT INTERIOR, UNLESS NOTED THIS [X]-[X].
 3. TOP OF BRICK SHELF ELEVATION IS [1'-0"] BELOW TOP OF SLAB.
 4. SEE S-501 FOR GENERAL NOTES.
 5. SEE S-500 SERIES FOR TYPICAL DETAILS.
 6. SEE S-500 FOR TYPICAL SLOPE SCHEDULES.
 7. COORDINATE ALL DIMENSIONS WITH ARCHITECT, INTERFERE WITH OTHER CONTRACTORS.
 8. COORDINATE ALL SLAB OPENINGS, SLOPES, SLEEVES, DEPRESSIONS, DIME DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, MEP AND OTHER PRIME CONTRACTORS.
 9. FXX INDICATES FOOTING SETBACK.
 10. SEE S-601 FOR TYPICAL DETAILS.
 11. WALL FOOTINGS TO BE W1, U O N PLAN.
 12. MINIMUM DIRT AT GROUND AND 2ND FLOOR LEVEL IS A MINIMUM OF 100 PSF. ROOF LEVEL LOAD IS A MINIMUM OF 20 PSF.

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CONTRACTOR

CONSULTANT

ISSUE DATES

07/23/2019	75% DESIGN DEVELOPMENT
09/03/2019	100% DESIGN DEVELOPMENT
12/13/2019	50% CONSTRUCTION DOCUMENTS
04/15/2020	ISSUED FOR PERMIT
05/01/2020	ISSUED FOR CONSTRUCTION
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12/18/2020	FINAL GMP SET
08/21/2021	BID SET

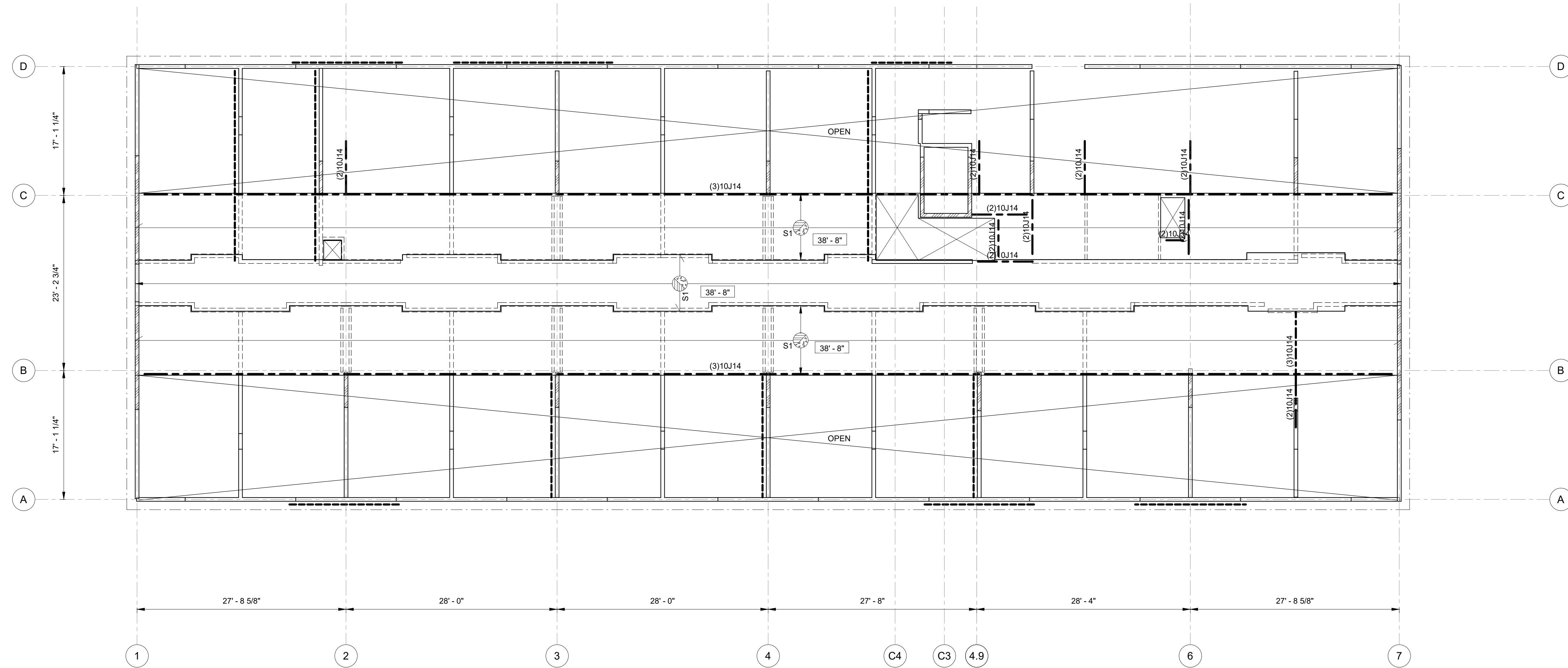
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INN - FIRST AND SECOND FLOOR FRAMING PLANS

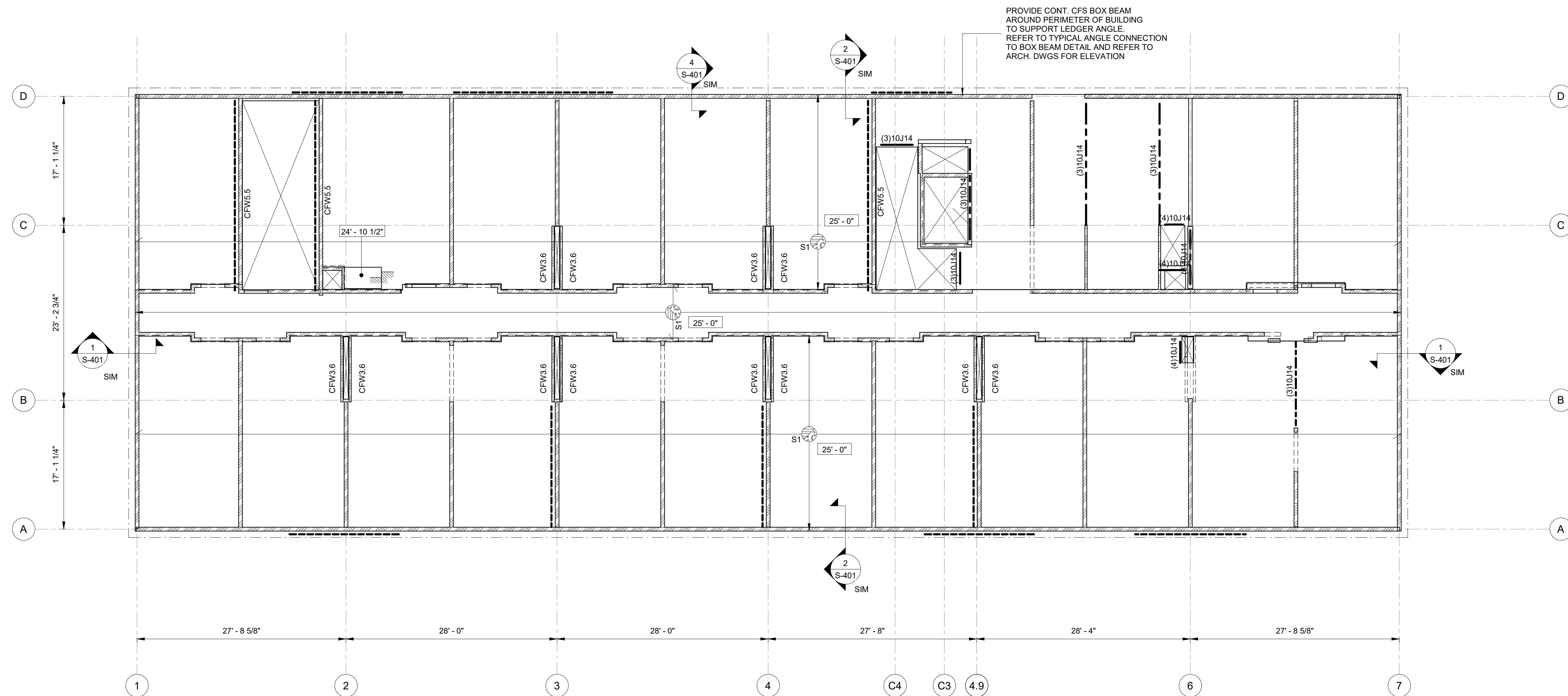
S-101



2 MECHANICAL LEVEL FRAMING PLAN
S-102 1/8" = 1'-0"



1 LEVEL 3 FRAMING PLAN
S-102 1/8" = 1'-0"



PROVIDE CONT. CFS BOX BEAM
AROUND PERIMETER OF BUILDING
TO SUPPORT LEDGER ANGLE.
REFER TO TYPICAL ANGLE CONNECTION
TO BOX BEAM DETAIL AND REFER TO
ARCH. DWGS FOR ELEVATION

NOTES

1. TOP OF SLAB ELEVATIONS NOTED ON PLAN, REFER TO ARCHITECTURAL DRAWINGS FOR FINISH FLOOR ELEVATION AND DATUM INFORMATION.
2. TOP OF STEEL ELEVATION IS $<0.6 - 1/4"$ FROM TOP OF SLAB NOTED THIS $<X-X>$ ON PLAN.
3. REFER TO S-500 SERIES DRAWINGS FOR TYPICAL DETAILS.
4. COORDINATE ALL SLAB OPENINGS, SLEEVES, CURBS, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
5. COORDINATE ALL ALUMINUM LOCATIONS AND WALL OPENING WITH ARCHITECT, CIVIL, MEP, AND ALL OTHER PRIME CONTRACTORS.
6. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF MASONRY, SHELF ELEVATIONS AND CONFIGURATION.
7. LIVE LOAD AT GROUND AND SECOND FLOOR LEVEL IS 100 PSF. ROOF LIVE LOAD IS A MINIMUM OF 20 PSF.

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DUPLICATE DATES

3/2019	75% DESIGN DEVELOPMENT
3/2019	100% DESIGN DEVELOPMENT
3/2019	50% CONSTRUCTION DOCUMENTS
5/2020	ISSUED FOR PERMIT
1/2020	ISSUED FOR CONSTRUCTION
1/2020	GMP SET
8/2020	FINAL GMP SET
1/2021	BID SET

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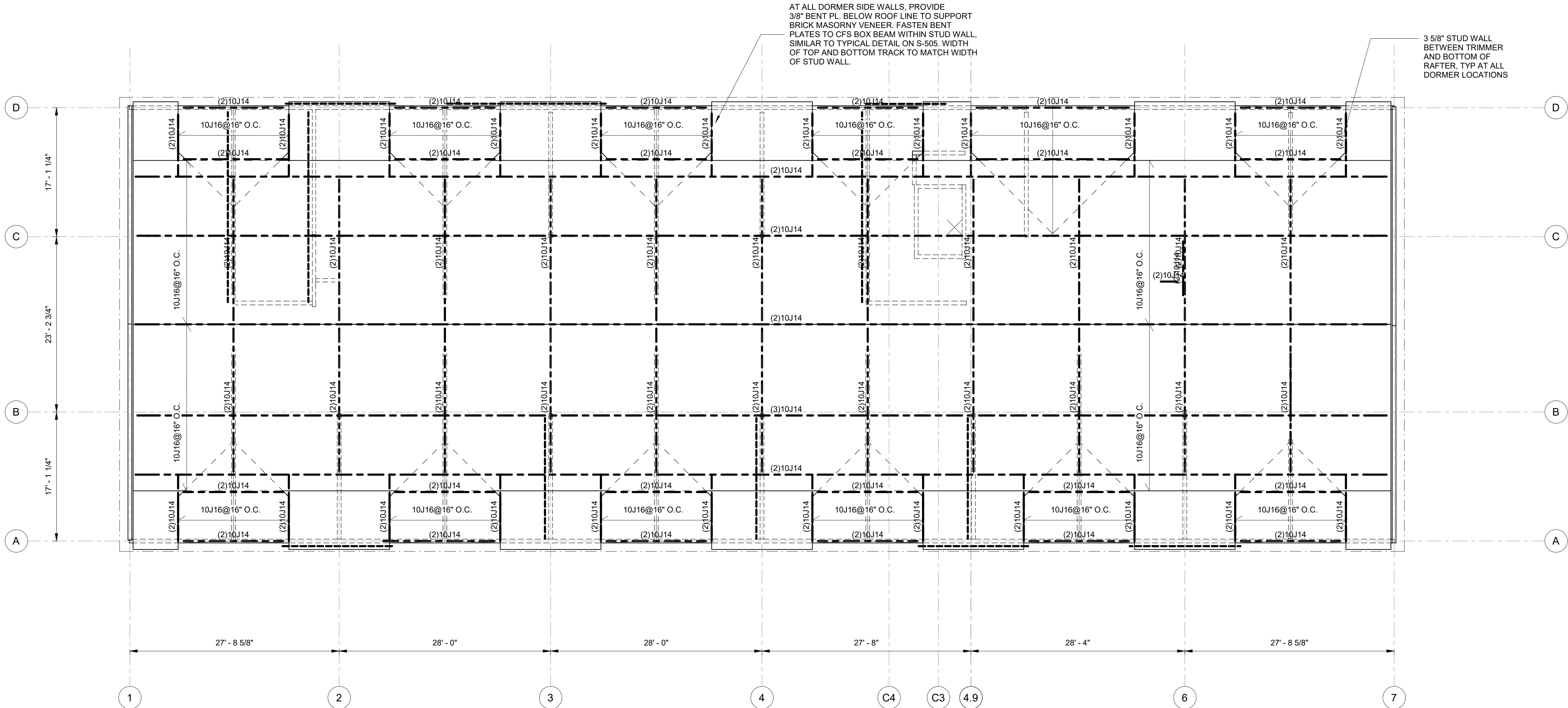
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150 West 28th St, Suite 1802, New York, NY 10001
(310) 820-6680 | fisherpartners.net

ANN - THIRD
FLOOR AND
MECH
FRAMING
PLANS

SCALE: $1/8" = 1'-0"$

S-102

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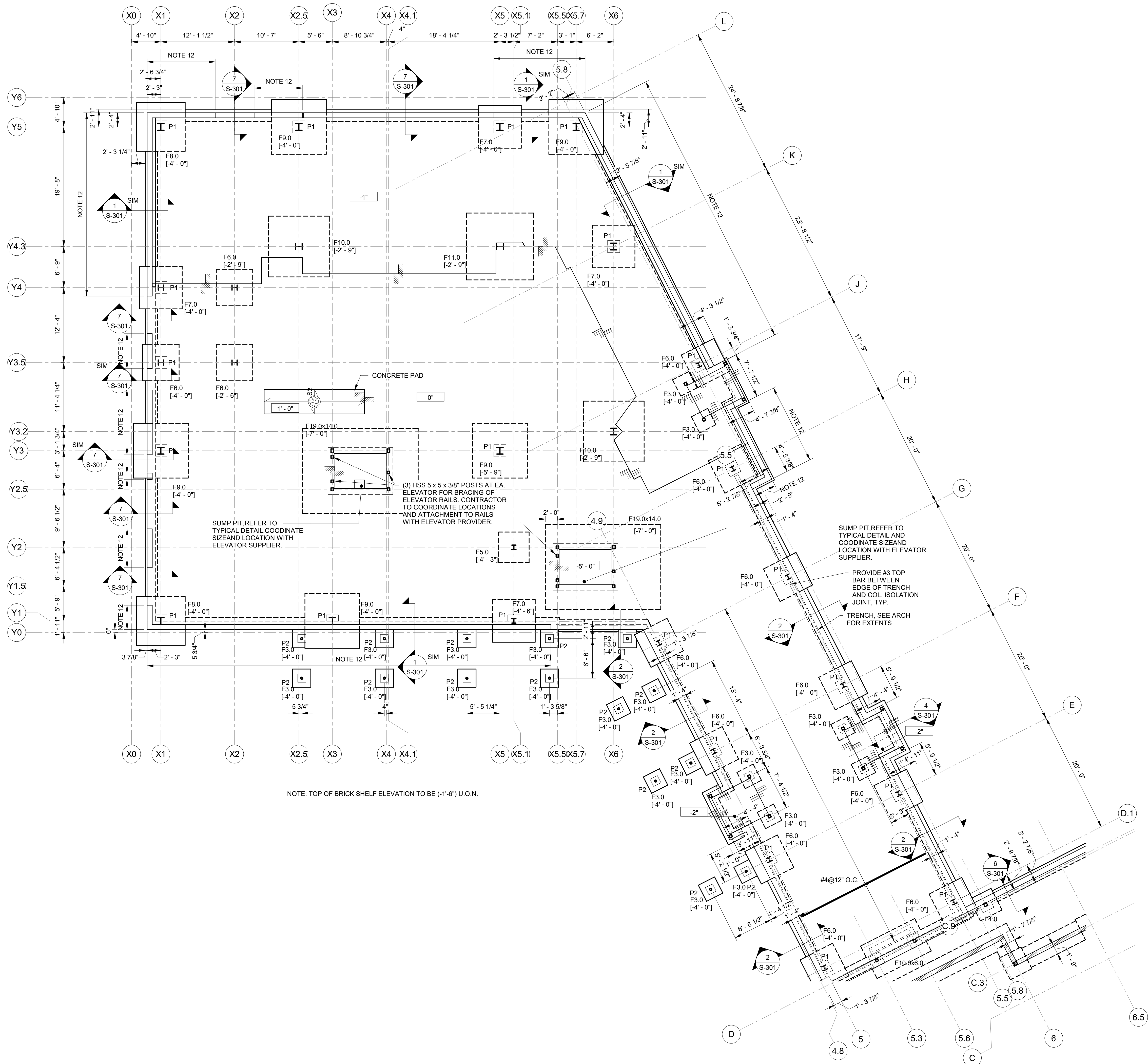
1
S-103 / 1/8" = 1'-0"

ROOF FRAMING PLAN

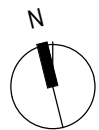
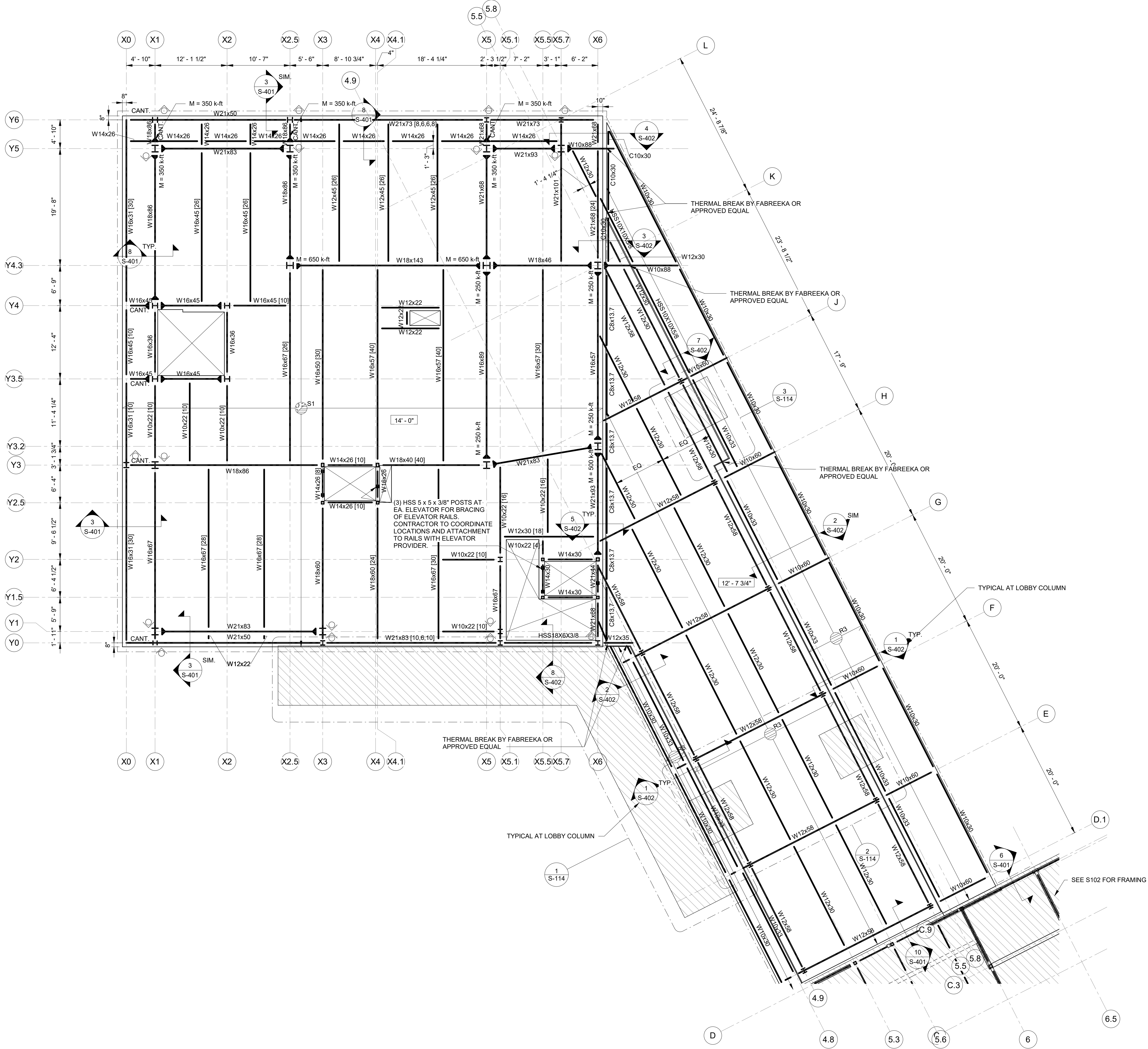


- NOTES
1. TOP OF SLAB ELEVATIONS NOTED ON PLAN, REFER TO ARCHITECTURAL DRAWINGS FOR FINISH FLOOR ELEVATION AND DATUM INFORMATION.
 2. TOP OF STEEL ELEVATION IS <7'-6 1/4"> FROM TOP OF SLAB NOTED THUS <X'X"> ON PLAN.
 3. REFER TO S-001 FOR GENERAL NOTES.
 4. REFER TO S-500 SERIES DRAWINGS FOR TYPICAL DETAILS.
 5. COORDINATE ALL SLAB OPENINGS, SLEEVES, SLOPES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
 6. COORDINATE ALL DIMENSIONS, LOCATIONS AND WALL OPENINGS WITH ARCHITECT, CIVIL, MEP, AND ALL OTHER PRIME CONTRACTORS.
 7. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF MASONRY SHELF ELEVATIONS AND CONFIGURATION.
 8. LIVE LOAD AT GROUND AND 2ND FLOOR LEVEL IS A MINIMUM OF 100 PSF. ROOF LIVE LOAD IS A MINIMUM OF 20 PSF.

1. TOP OF SLAB ELEVATION IS NOTED ON PLAN.
REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF
SLAB ELEVATION, FINISH FLOOR ELEVATION AND DATUM
INFORMATION.
2. BOTTOM OF FOOTING ELEVATIONS IS 4'-0" BELOW
FINISH FLOOR EXTERIOR AND 1'-0" BELOW
FINISH FLOOR INTERIOR UNLESS NOTED THIS
WAY.
3. TOP OF BRICK SLAB ELEVATION IS 1'-0" BELOW
TOP OF SLAB.
4. SEE S-801 FOR GENERAL NOTES.
5. SEE S-801 FOR DETAILS FOR TYPICAL DETAILS.
6. SEE S-801 FOR COLUMN SCHEDULES.
7. SEE S-801 FOR ALL DETAILS WITH ARCHITECT.
8. MECHANICAL, ELECTRICAL, PLUMBING AND
CIVIL MEP AND OTHER PRIME CONTRACTORS
COORDINATE ALL SLAB OPENINGS, SLOPES,
PENETRATIONS, RISERS, DRAINAGE, VENTILATION
AND CURBS WITH ARCHITECT, CIVIL MEP AND OTHER
PRIME CONTRACTORS.
9. SEE ARCHITECTURAL DRAWINGS FOR FLOOR FINISH
S-801 FOR SCHEDULE.
10. WALL FOOTINGS TO BE: W/1, U/O N PLAN
AND W/2, U/O PLAN.
11. WALL FOOTINGS TO BE: W/1, U/O N PLAN PLUS
A MINIMUM OF 100 PSF ROOF LOAD PLUS A
MINIMUM OF 20 PSF
12. APPROXIMATE 12" MINIMUM OF BRICK SLAB
COORDINATE WITH ARCHITECTURE



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NOTES

1. TOP OF SLAB ELEVATIONS NOTED ON PLAN, REFER TO ARCHITECTURAL DRAWINGS FOR FINISH FLOOR ELEVATION AND DATUM INFORMATION.
2. TOP OF STEEL ELEVATION IS $\pm 0.46 \frac{1}{4}"$ FROM TOP OF SLAB NOTED THUS $\times\text{X}\times$ ON PLAN.
3. REFER TO S-001 FOR GENERAL NOTES.
4. REFER TO S-500 SERIES DRAWINGS FOR TYPICAL DETAILS.
5. COORDINATE ALL SLAB OPENINGS, SLEEVES, SLOPES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
6. COORDINATE ALL DIMENSIONS, LOCATIONS AND WALL OPENING WITH ARCHITECT, CIVIL, MEP, AND ALL OTHER PRIME CONTRACTORS.
7. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF MASONRY SHELF ELEVATIONS AND CONFIGURATION.
8. LIVE LOAD AT GROUND AND 2ND FLOOR LEVEL IS A MINIMUM OF 100 PSF. ROOF LIVE LOAD IS A MINIMUM OF 20 PSF.

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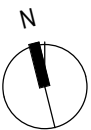
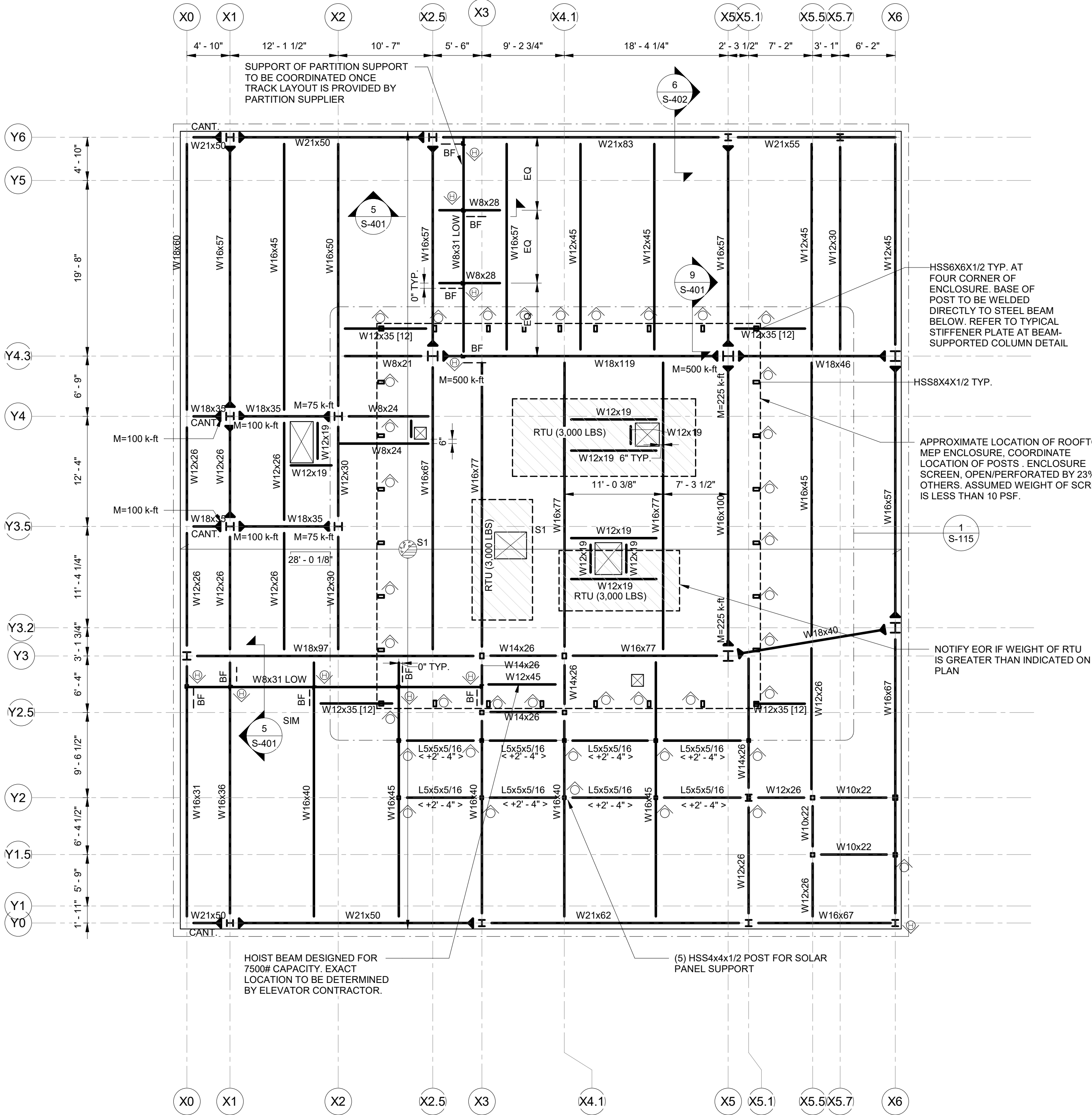
CONSULTANT

ISSUE DATES

01/23/2019	75% DESIGN DEVELOPMENT
04/03/2019	100% DESIGN DEVELOPMENT
12/13/2019	90% CONSTRUCTION DOCUMENTS
04/15/2020	ISSUED FOR PERMIT
05/01/2020	ISSUED FOR CONSTRUCTION
07/01/2020	GMP SET
12/18/2020	FINAL GMP SET
05/21/2021	BID SET

SCALE: 1/8" = 1'-0"

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- NOTES
1.

TOP OF SLAB ELEVATIONS NOTED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR FINISH FLOOR ELEVATION AND DATUM INFORMATION.
2.

TOP OF STEEL ELEVATION IS <-0'-6 1/4"> FROM TOP OF SLAB NOTED THUS <X-X"> ON PLAN.
3.

REFER TO S-001 FOR GENERAL NOTES.
4.

REFER TO S-500 SERIES DRAWINGS FOR TYPICAL DETAILS.
5.

COORDINATE ALL SLAB OPENINGS, SLEEVES, SLOPES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
6.

COORDINATE ALL DIMENSIONS, LOCATIONS AND WALL OPENING WITH ARCHITECT, CIVIL, MEP, AND ALL OTHER PRIME CONTRACTORS.
7.

REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF MASONRY SHELF ELEVATIONS AND CONFIGURATION.
8.

LIVE LOAD AT GROUND AND 2ND FLOOR LEVEL IS A MINIMUM OF 100 PSF. ROOF LIVE LOAD IS A MINIMUM OF 20 PSF.

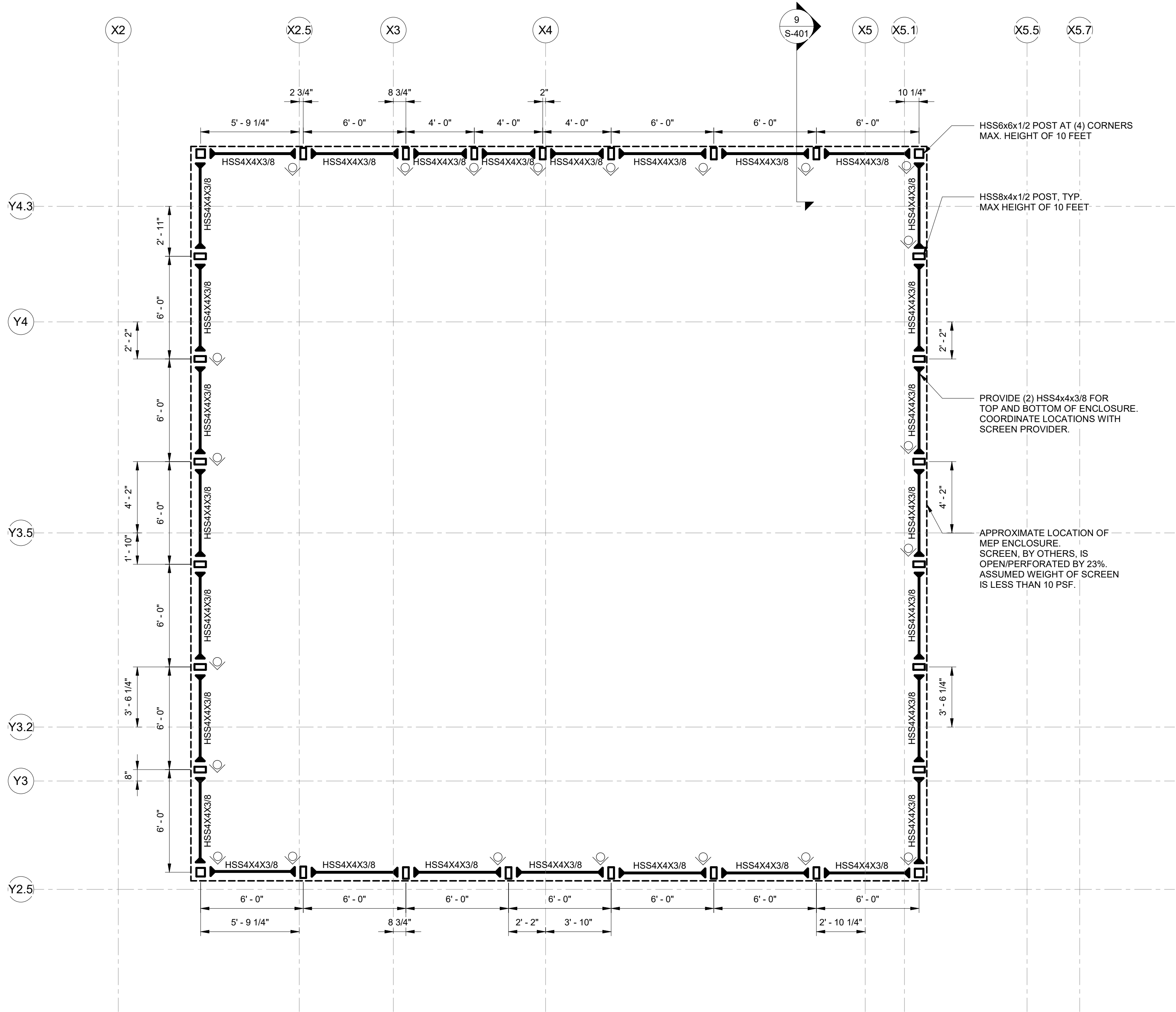
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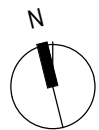
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09/03/2019	100% DESIGN DEVELOPMENT
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09/21/2021	BID SET

SCALE: 1/8" = 1'-0"

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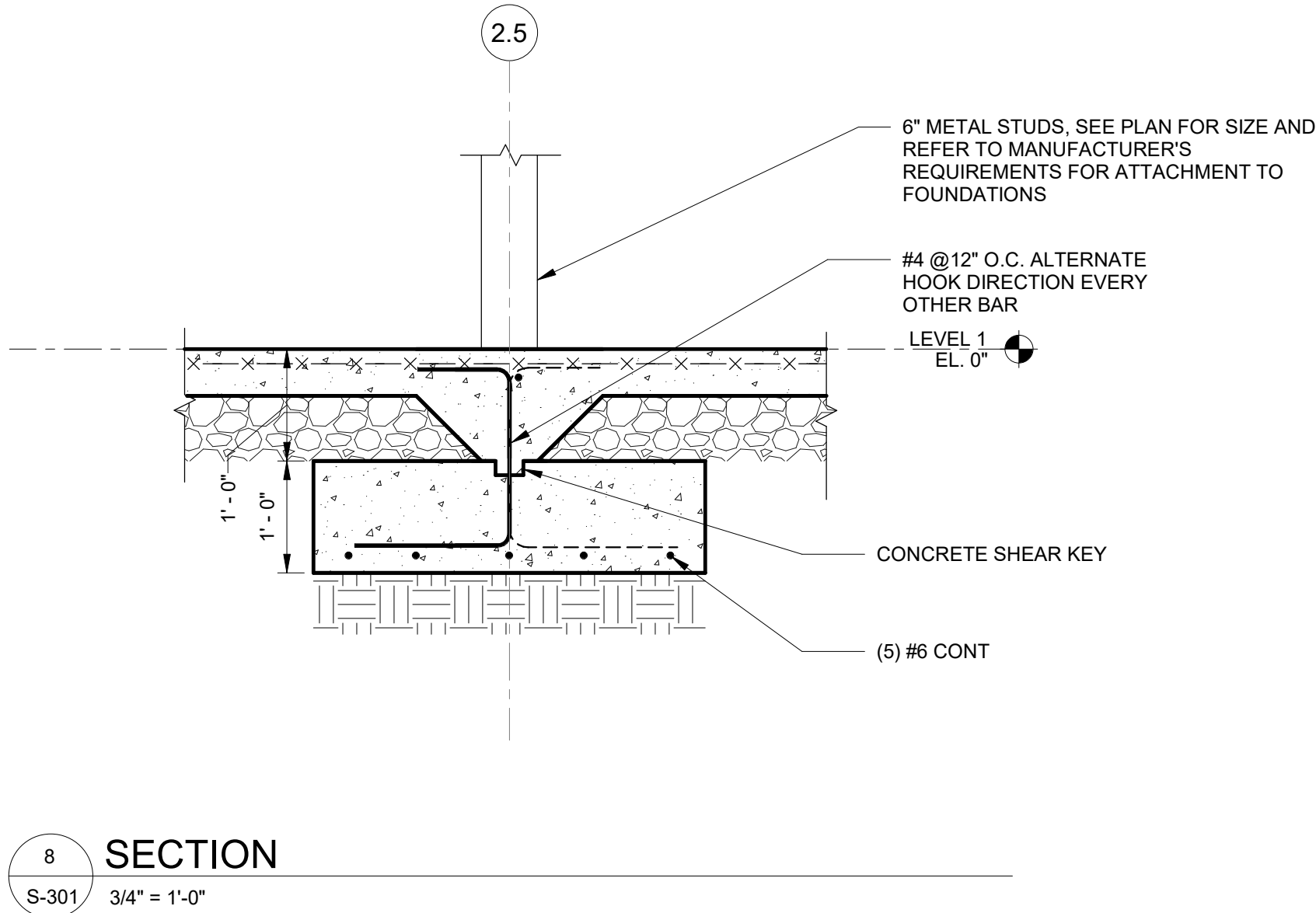
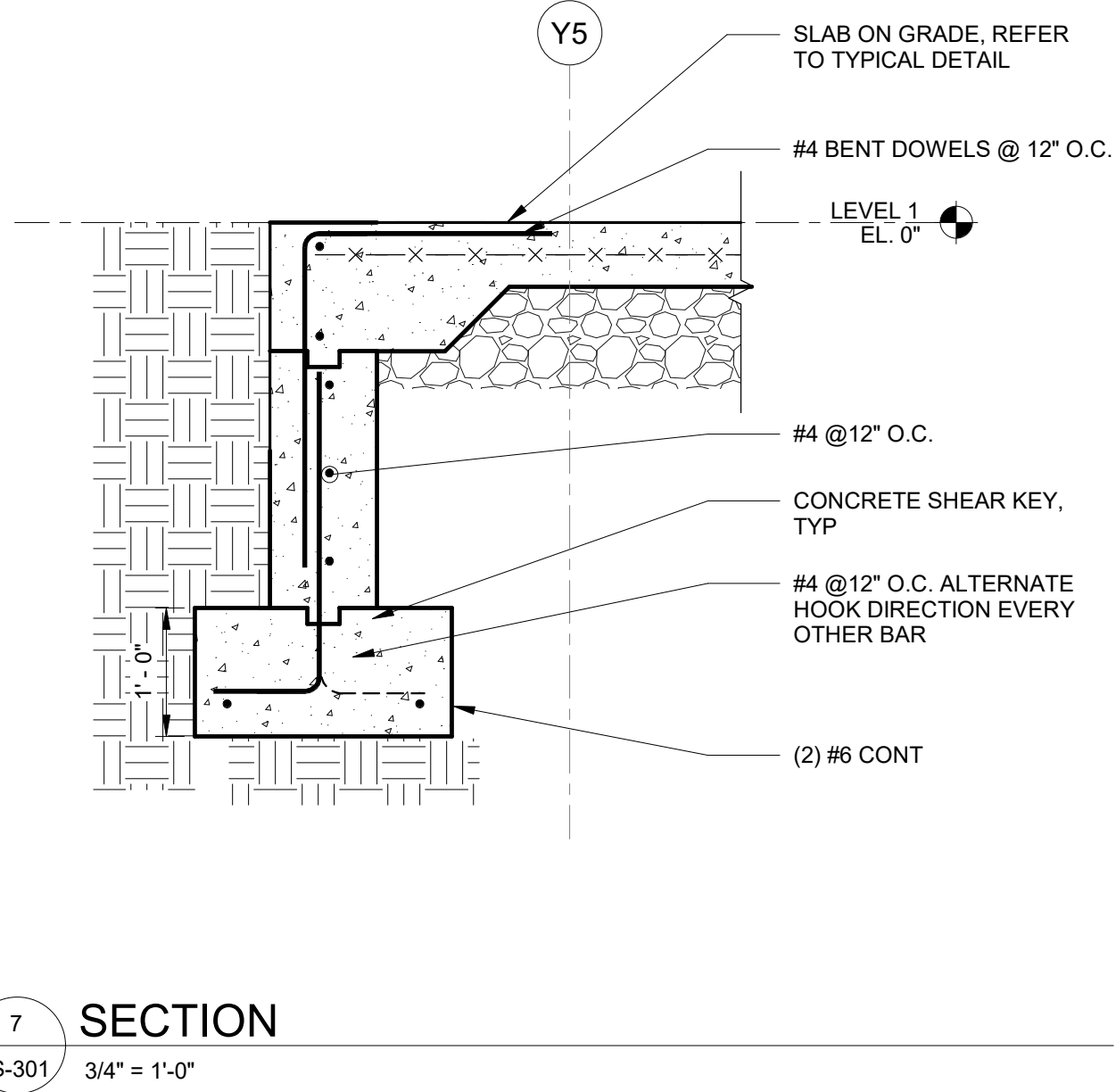
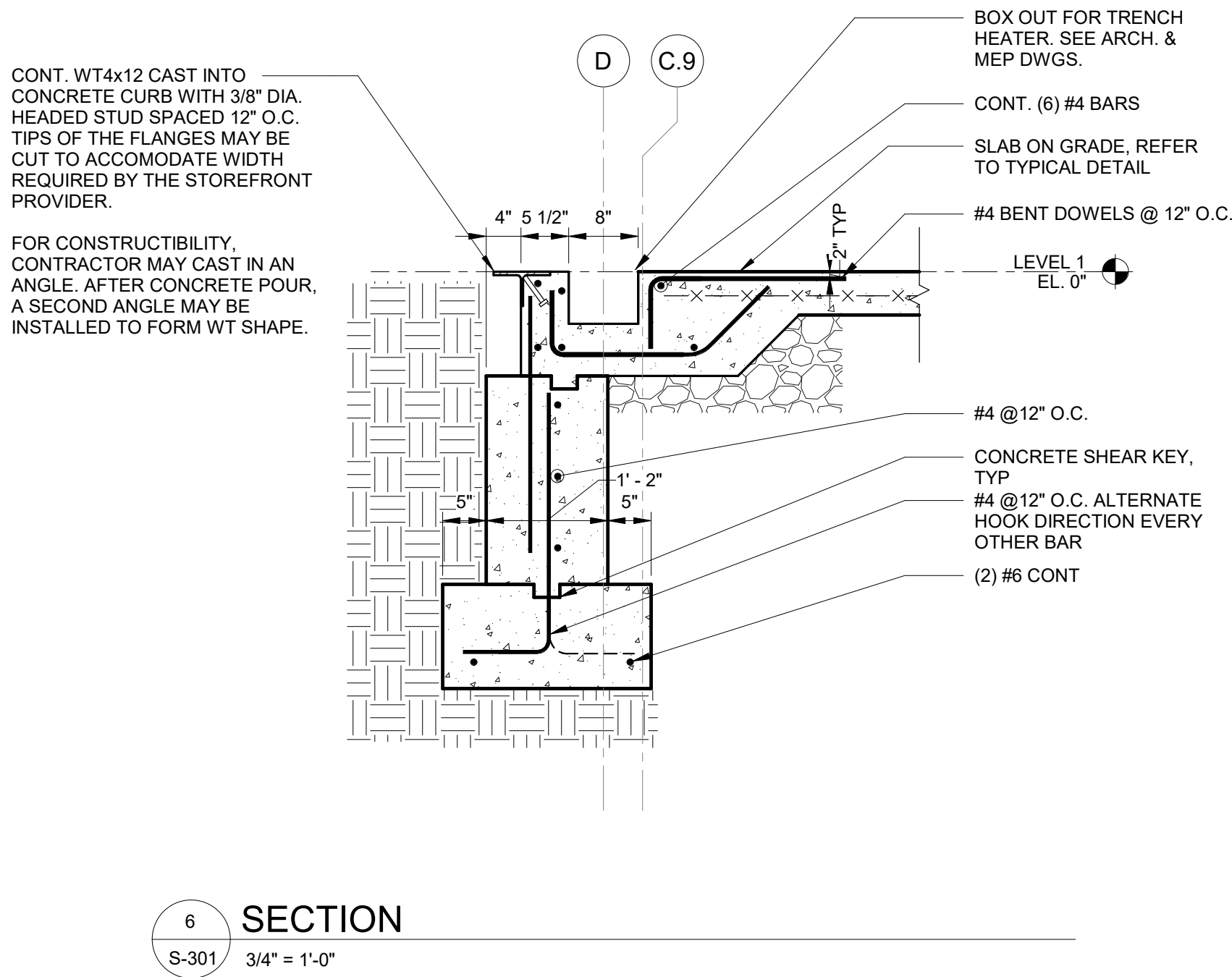
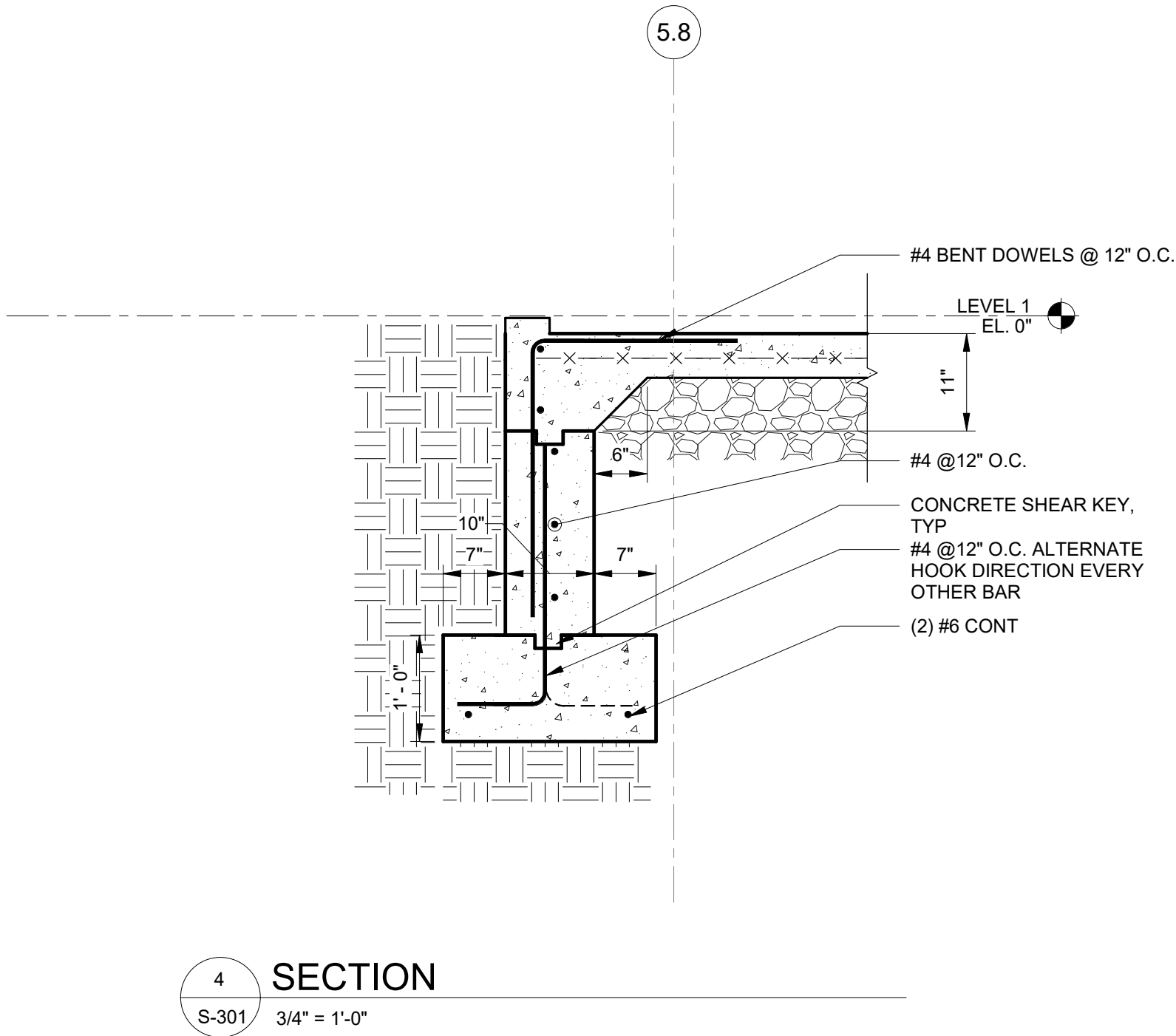
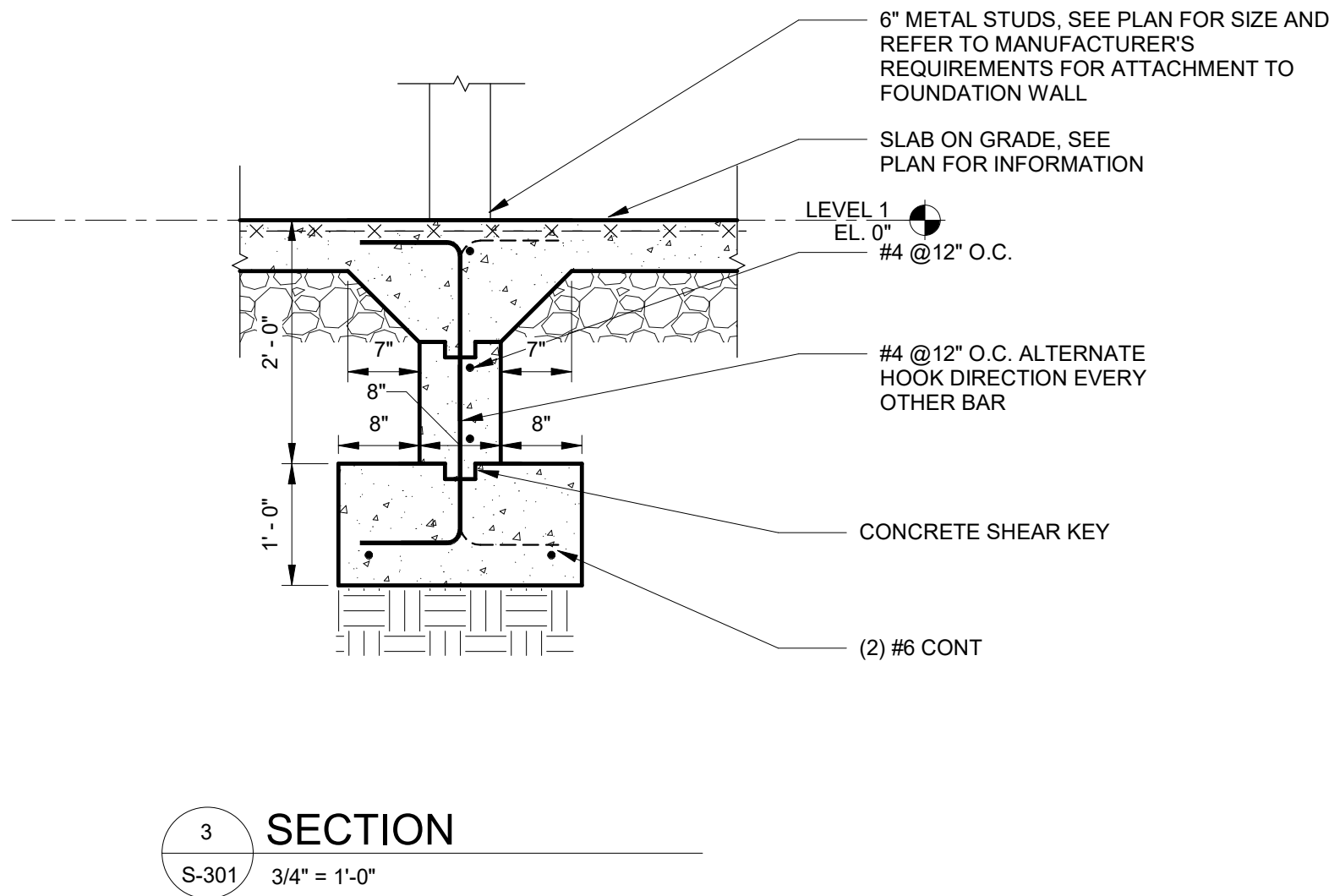
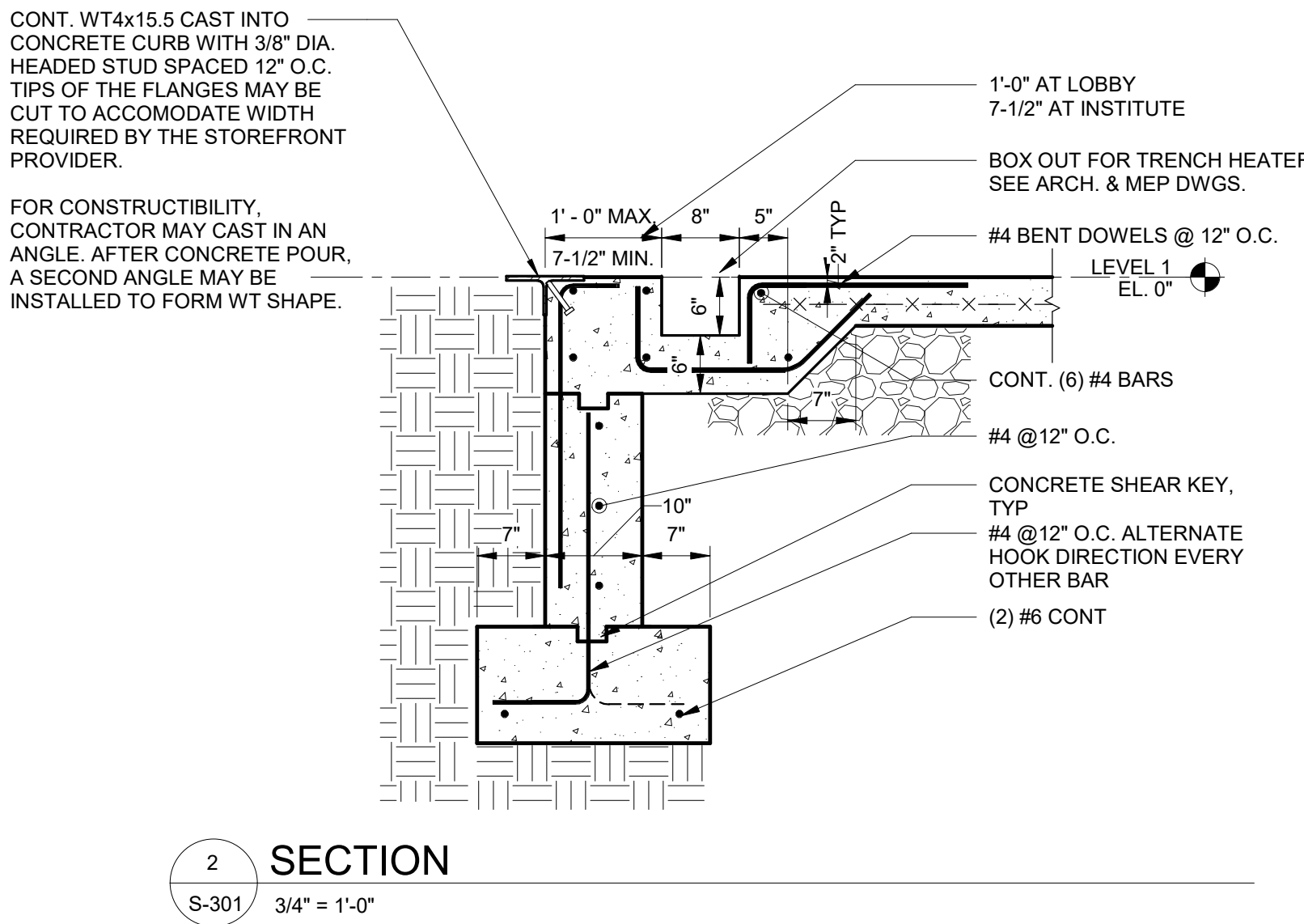
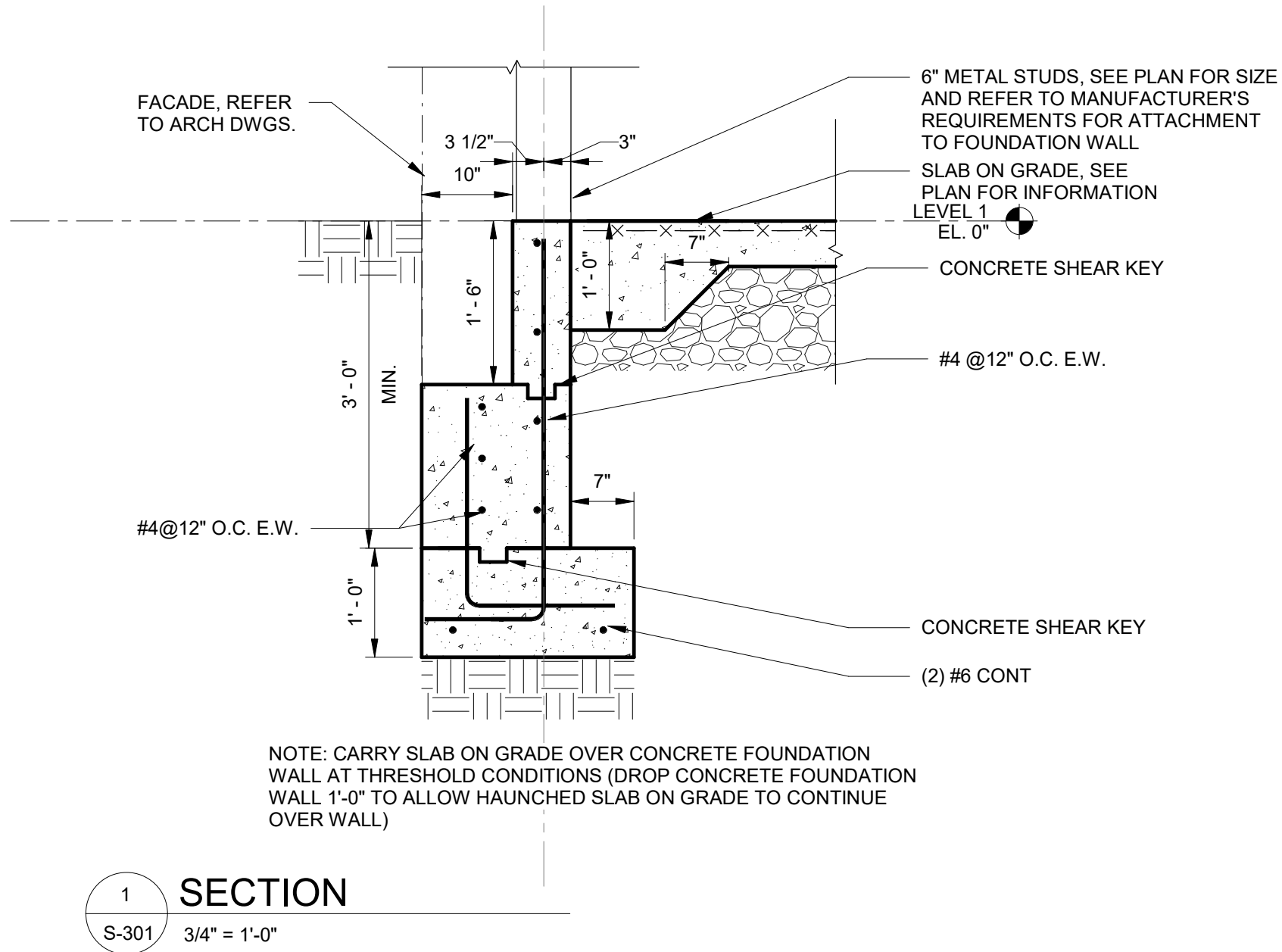
1 INSTITUTE - MECHANICAL ENCLOSURE SCREEN FRAMING PLAN
S-115 1/4" = 1'-0"

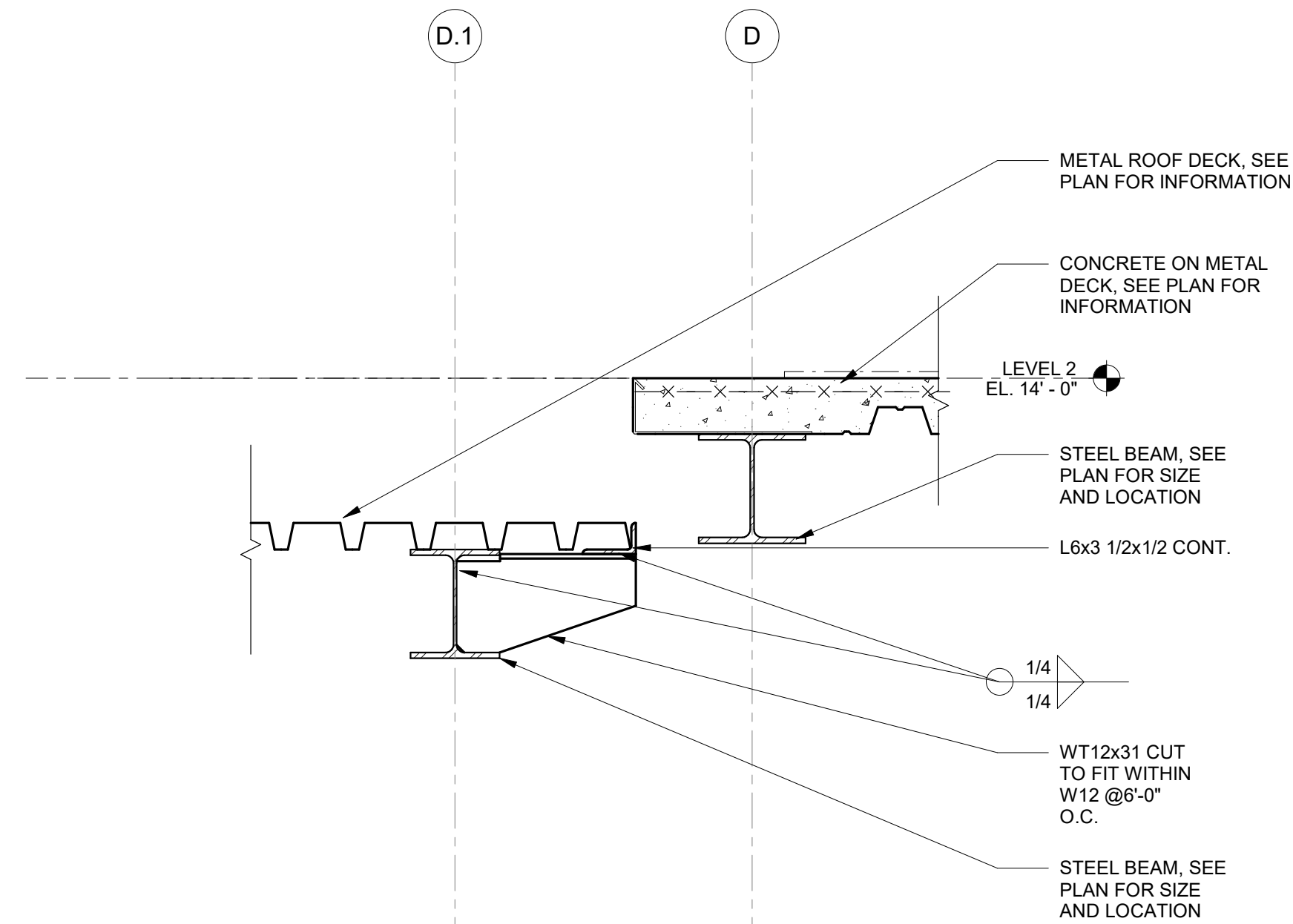
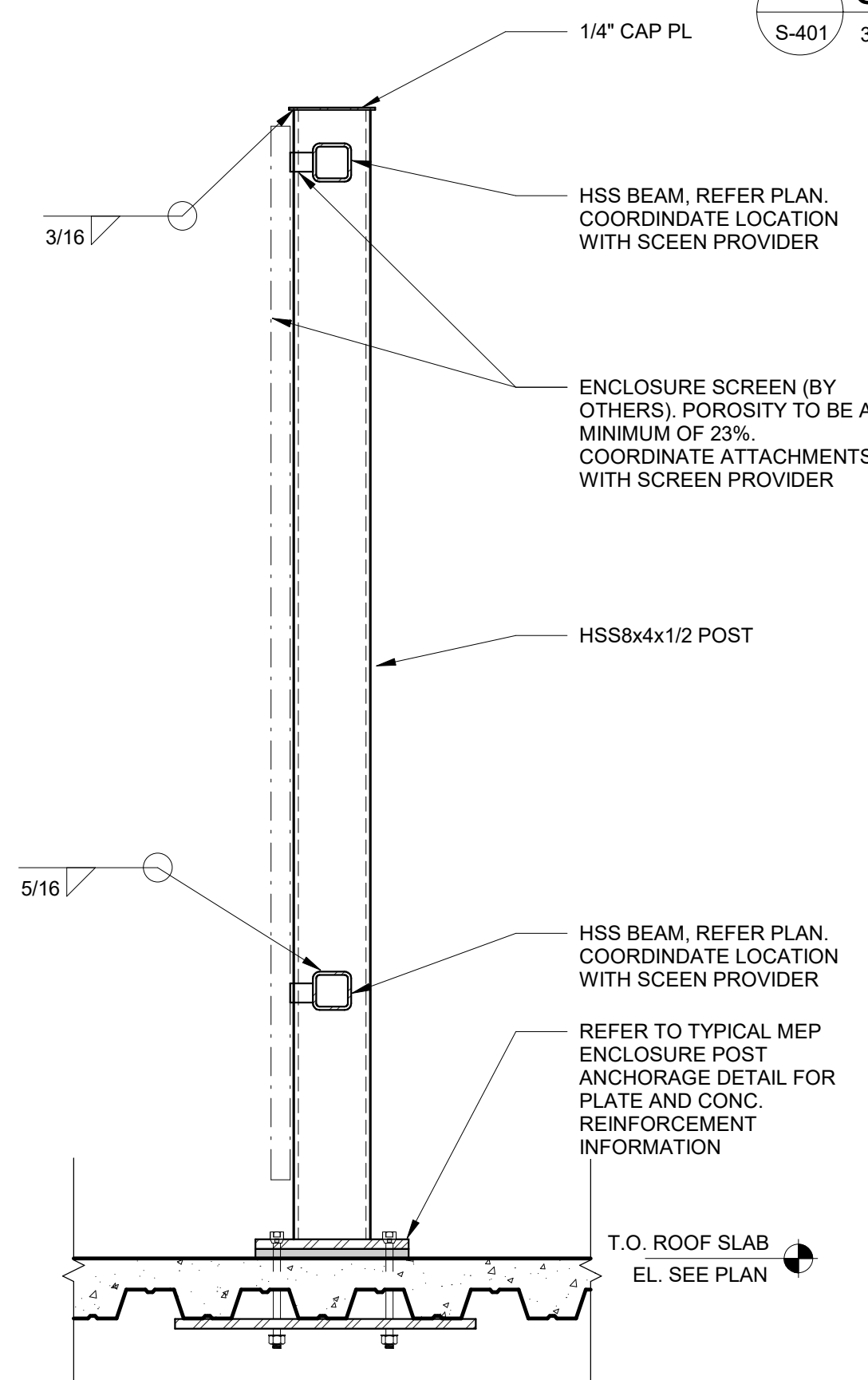
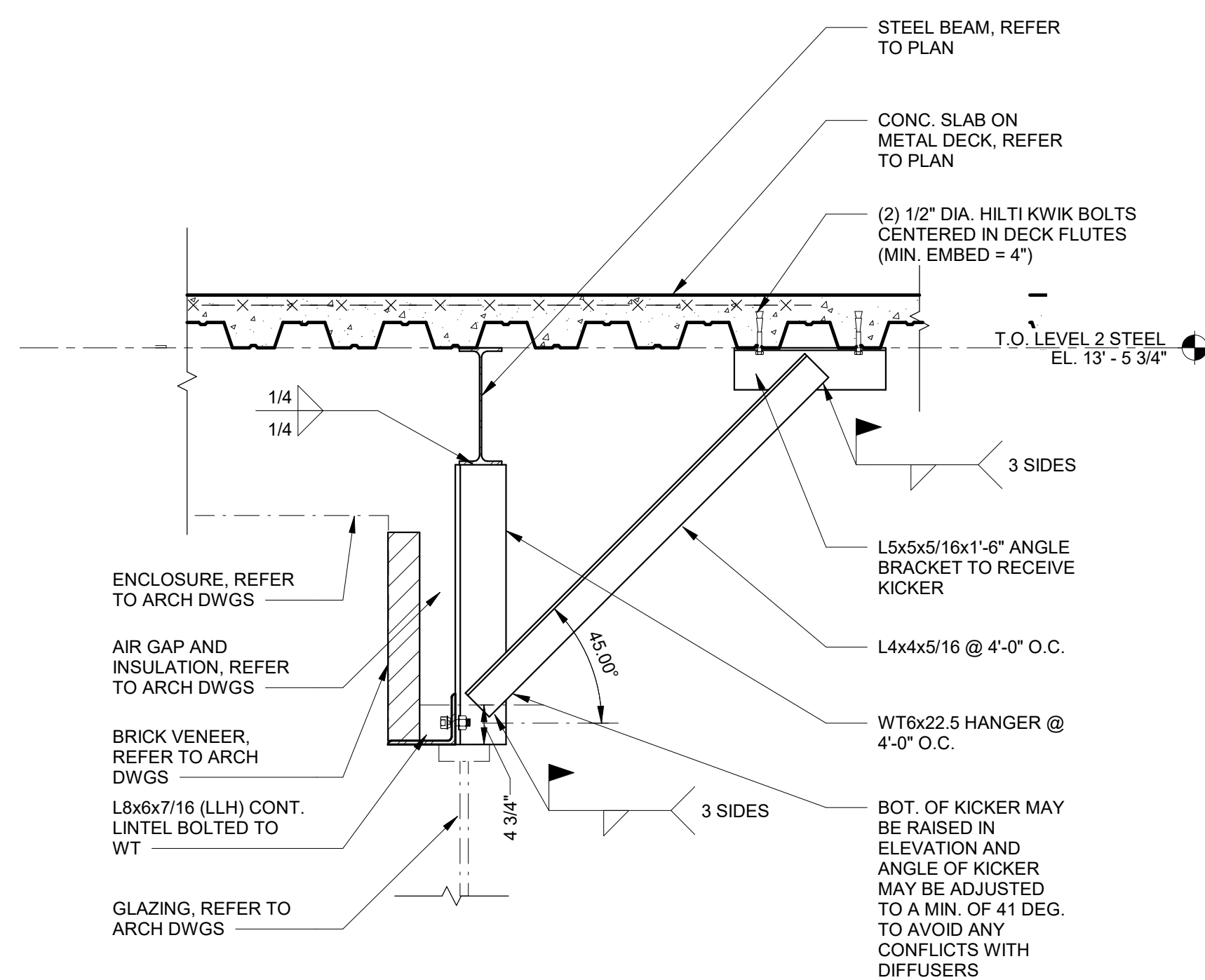
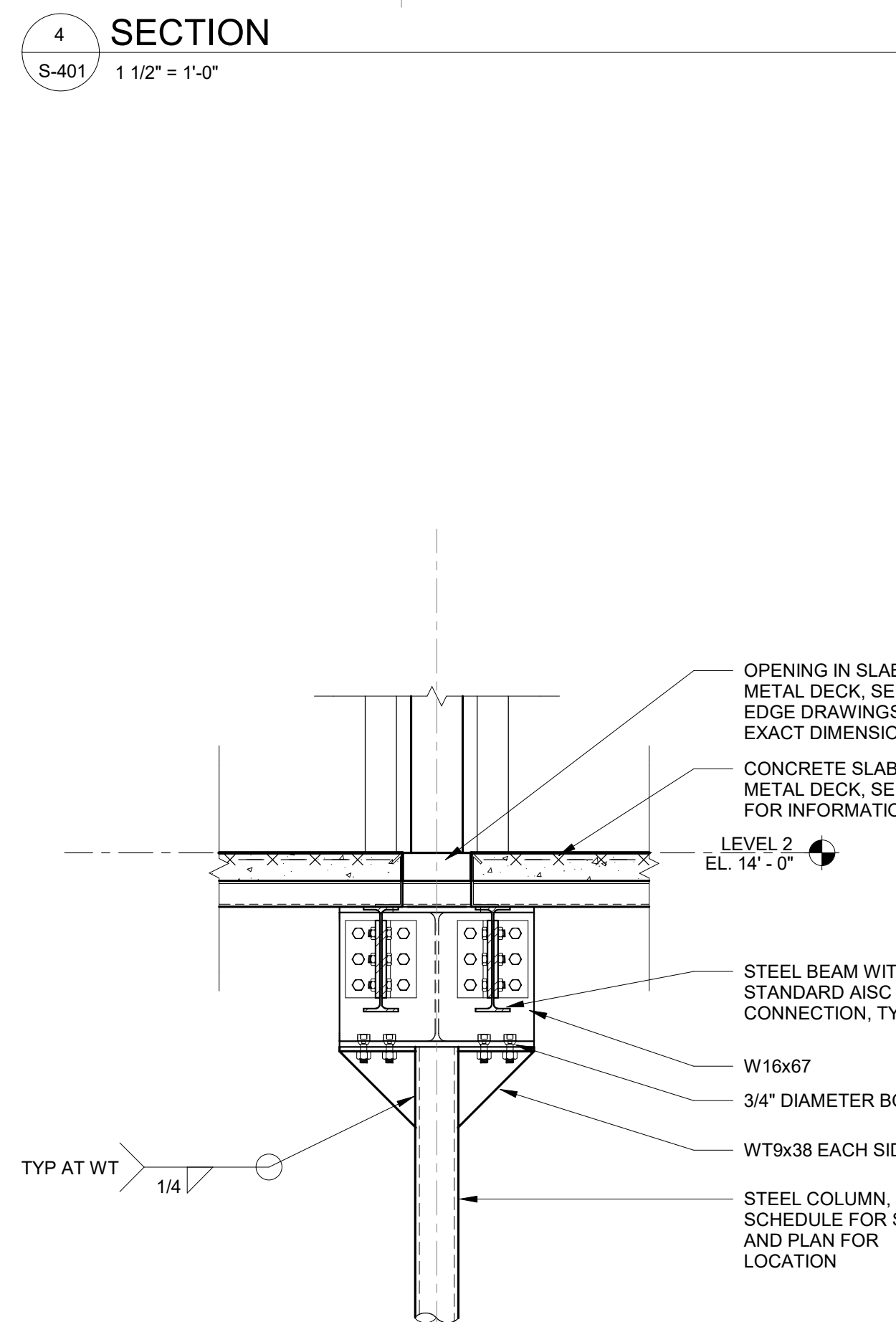
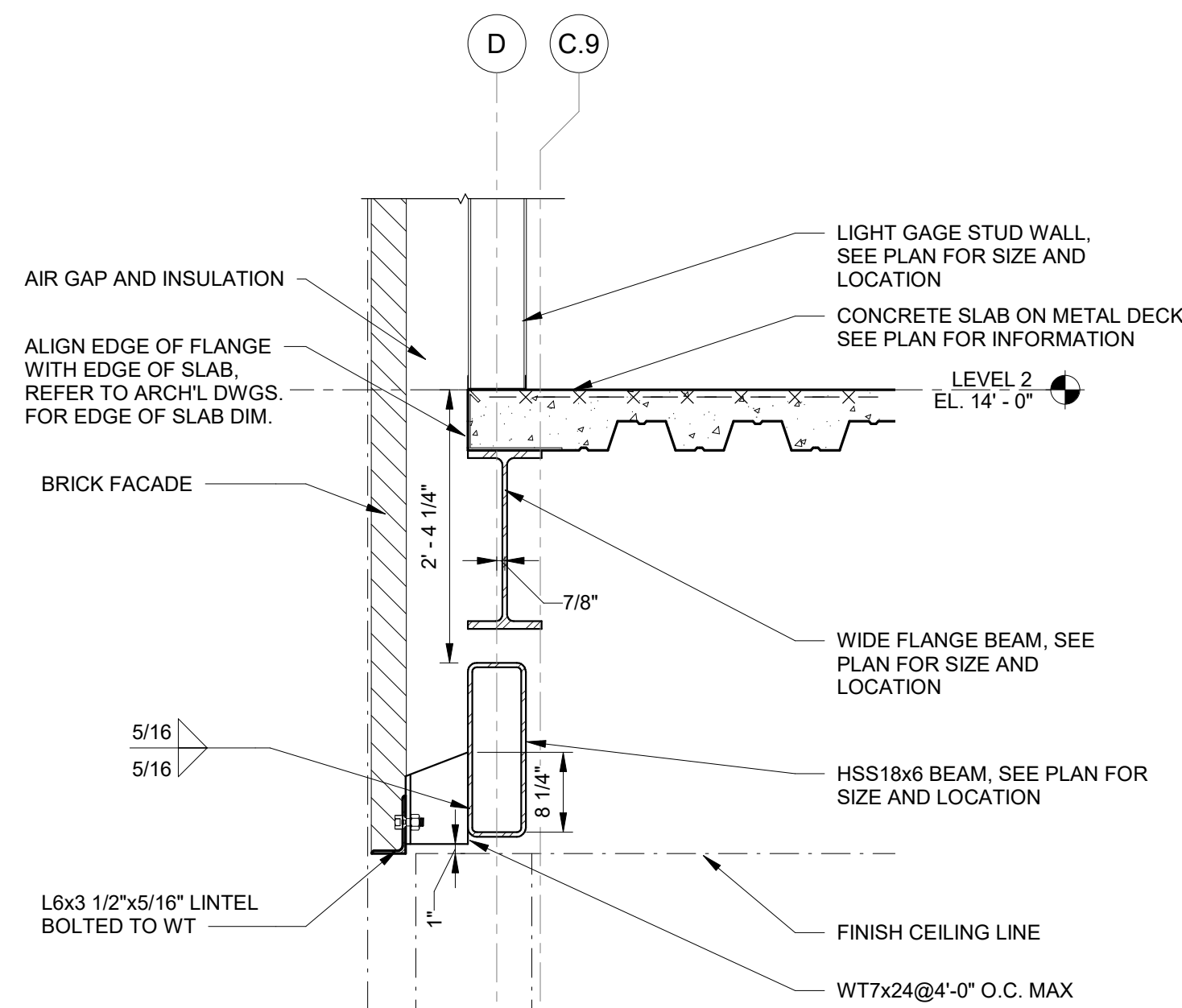
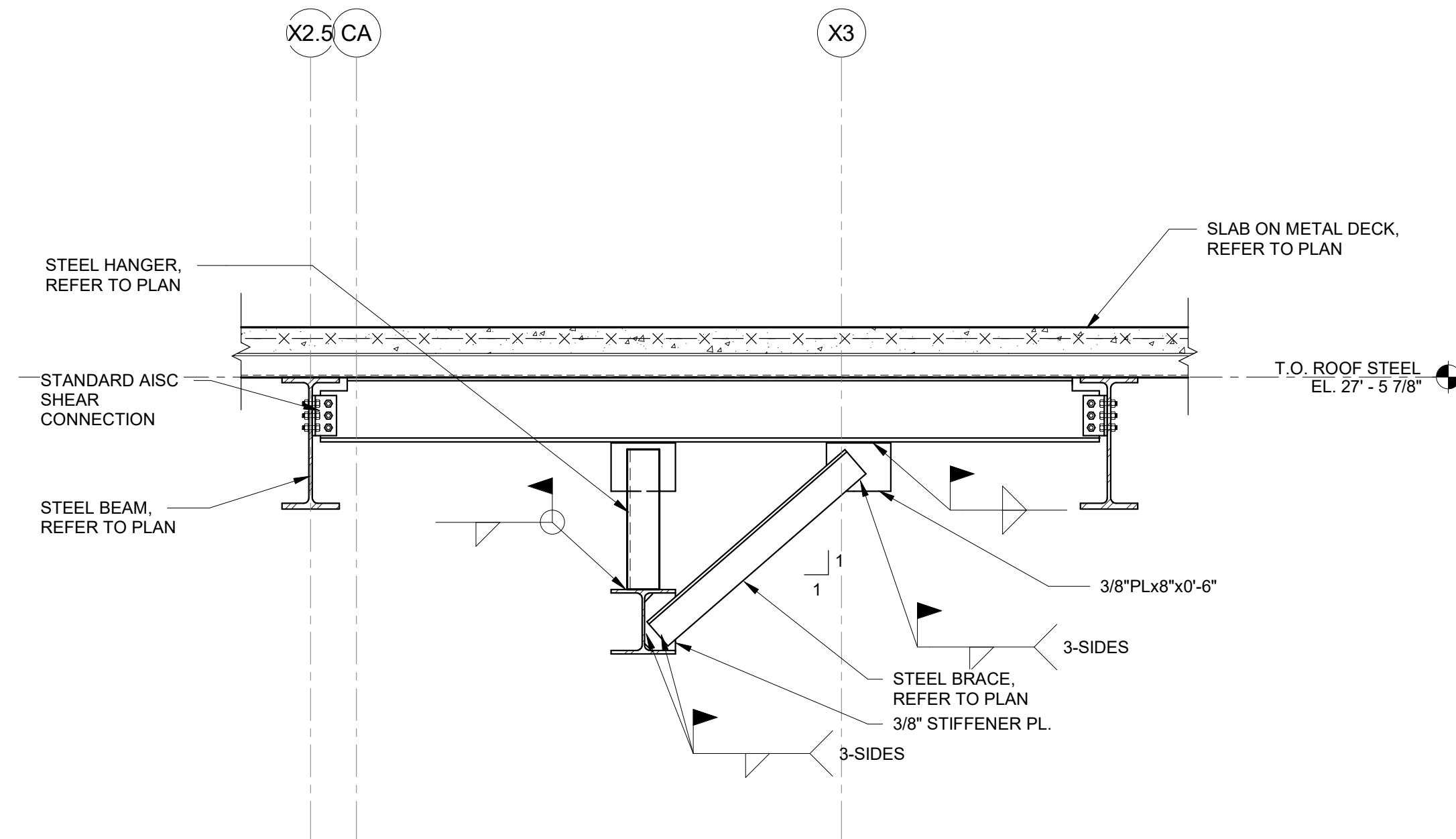
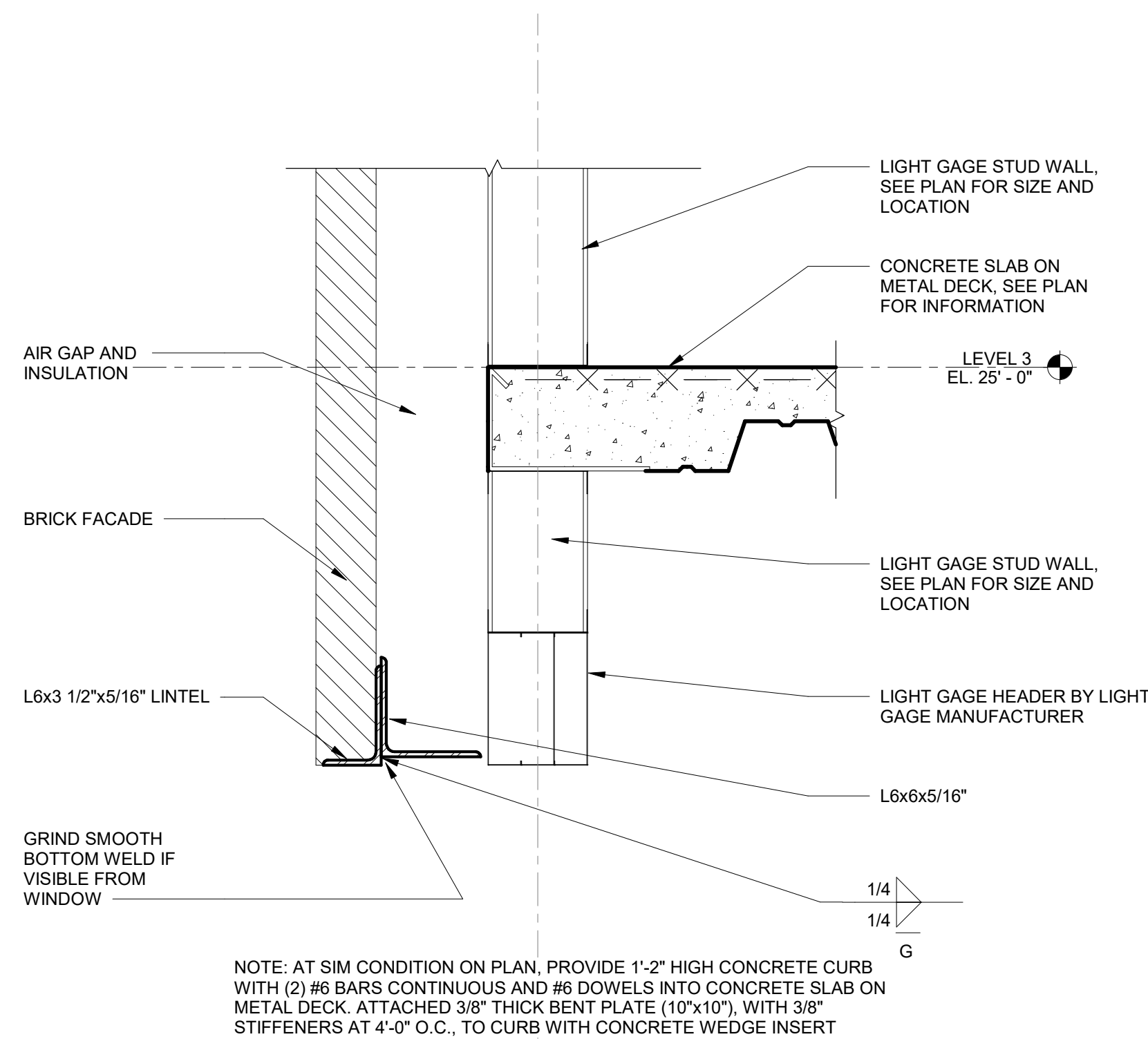
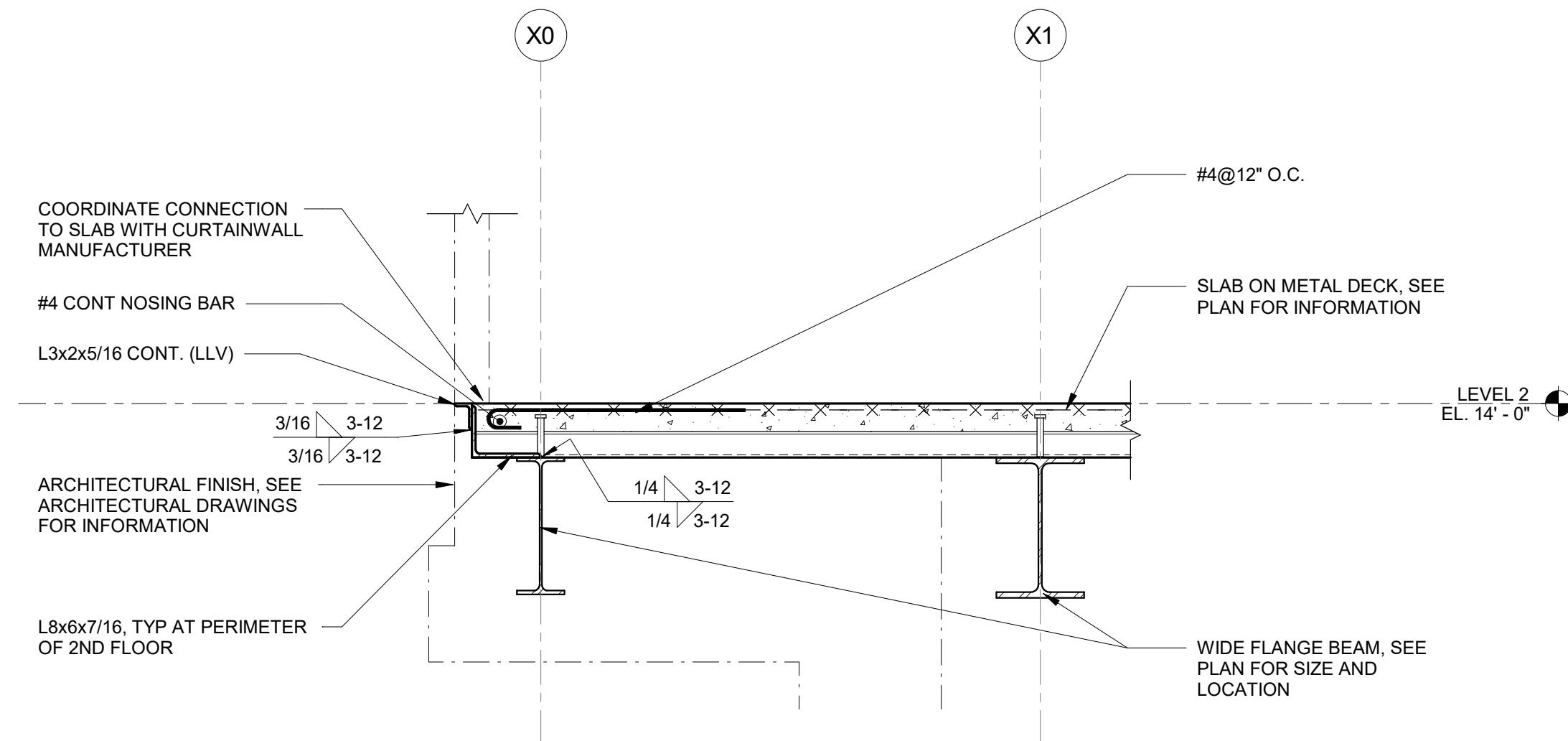
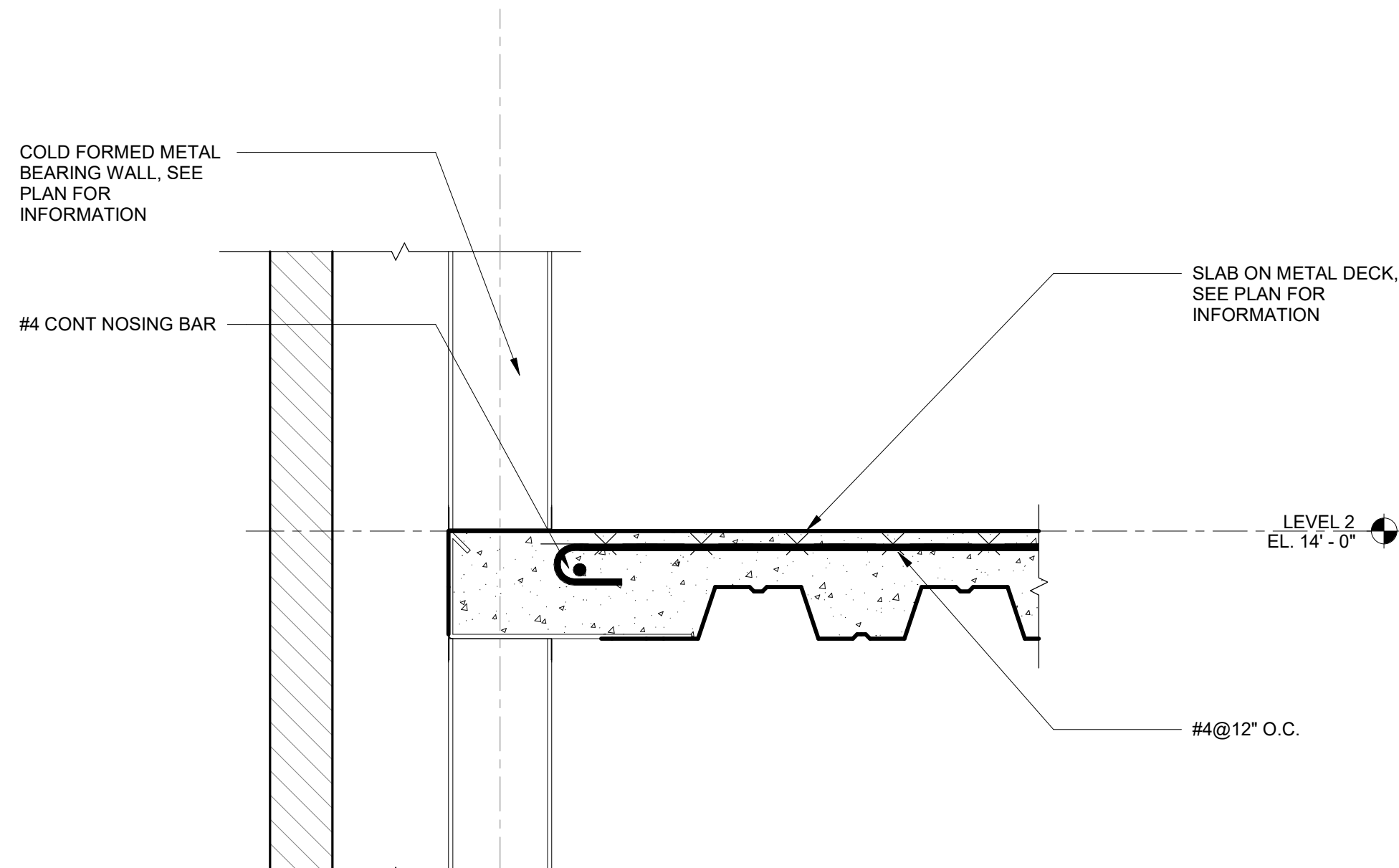
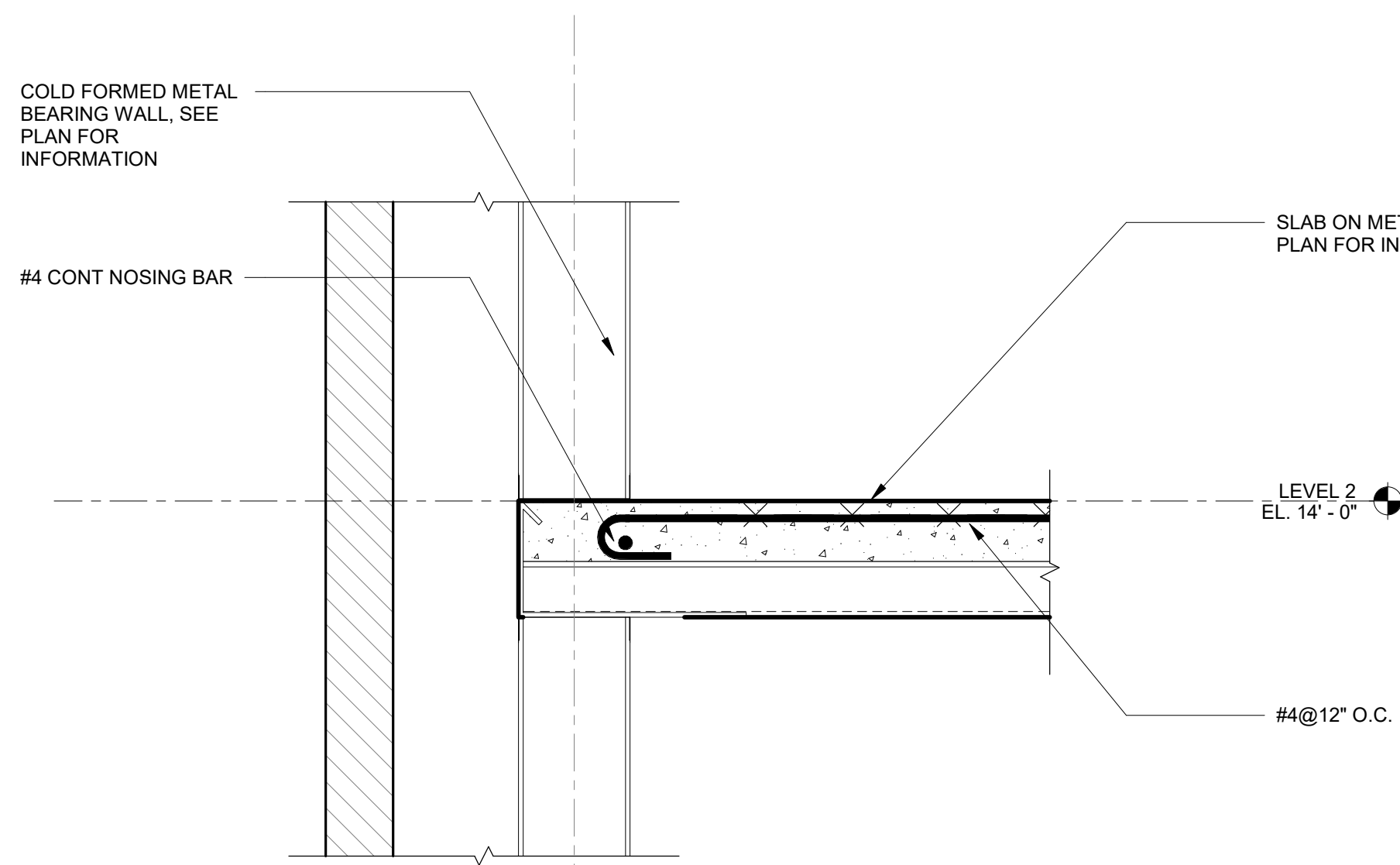


NOTES

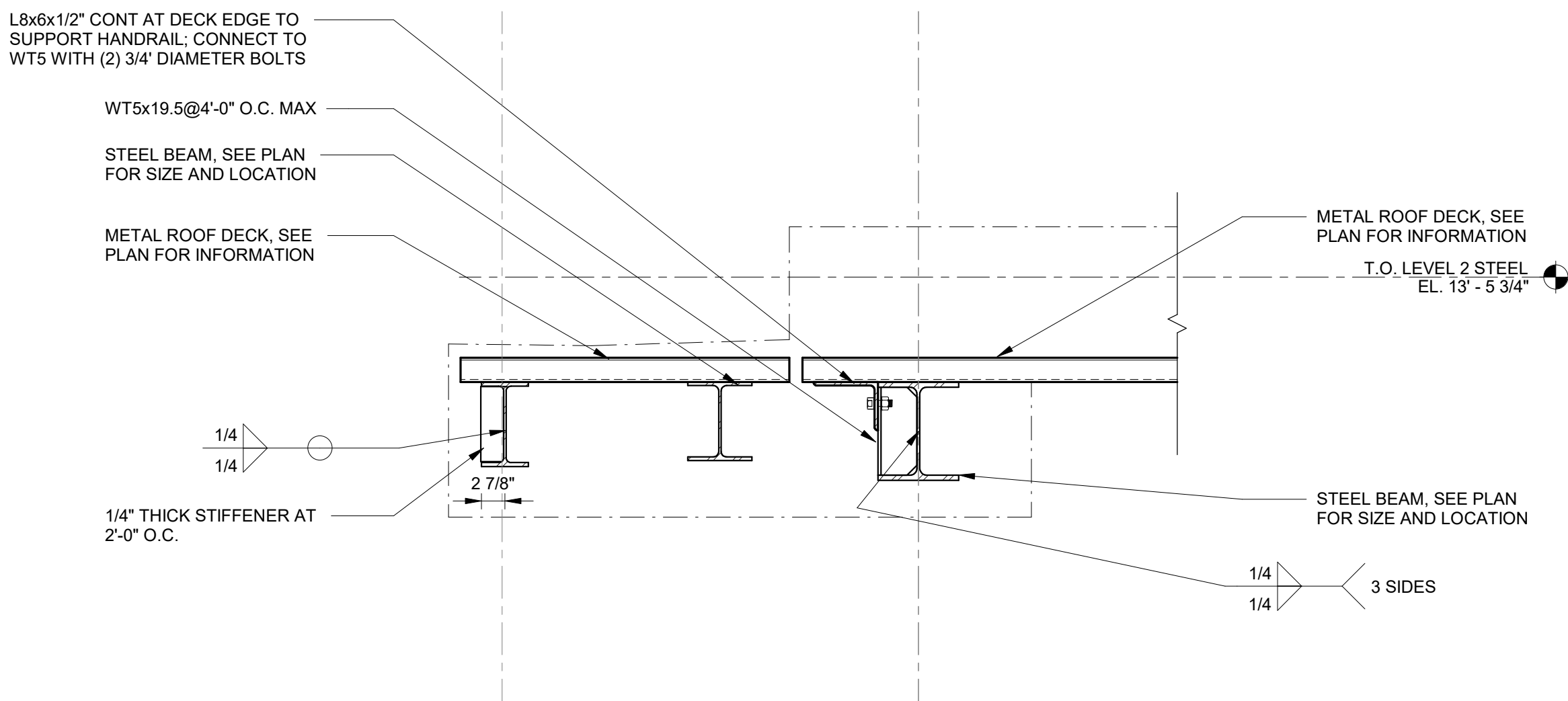
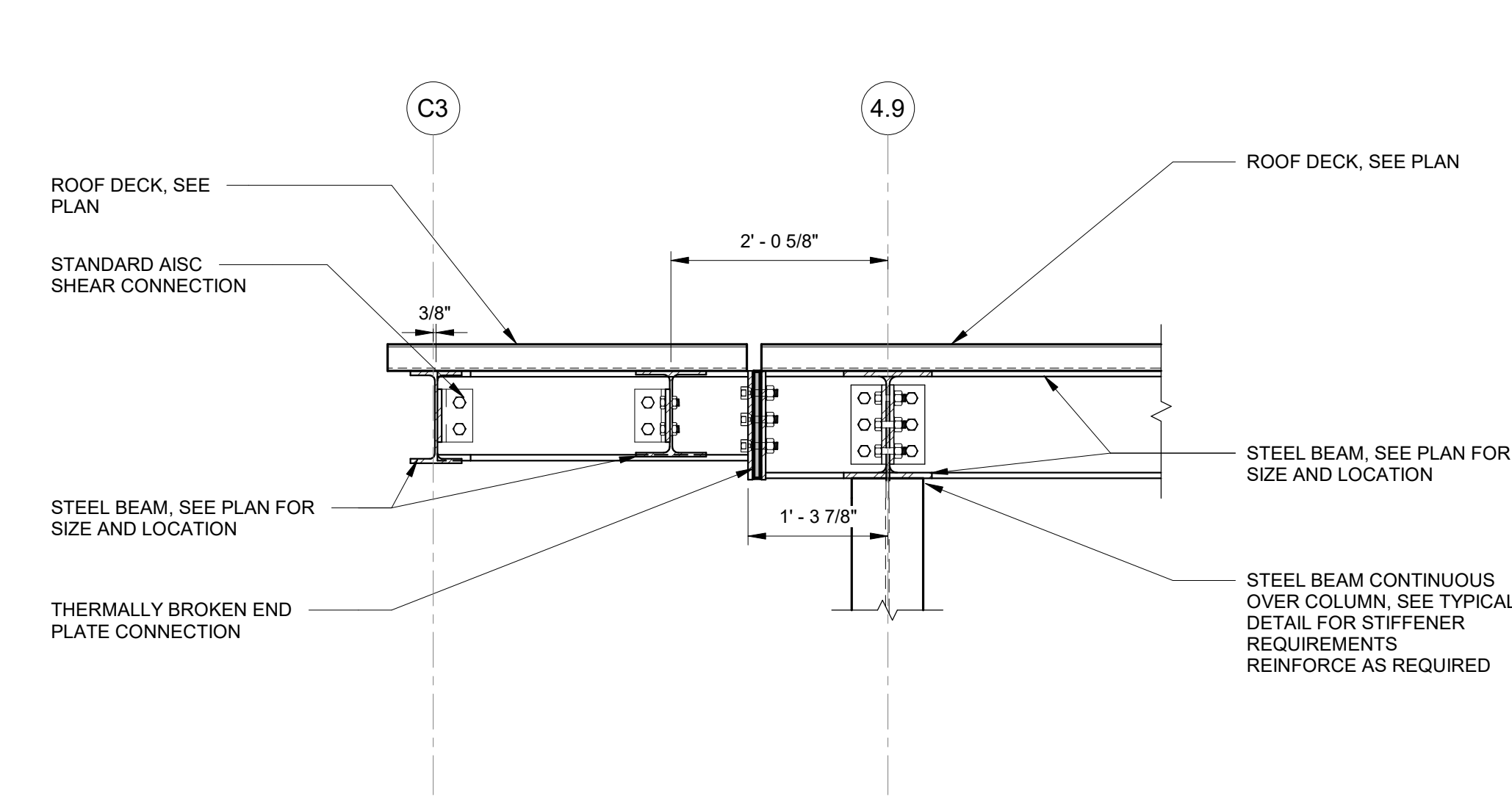
1. REFER TO ARCH FOR TOP OF STEEL ELEVATIONS.
2. REFER TO S-001 FOR GENERAL NOTES.
3. REFER TO S-000 SERIES DRAWINGS FOR TYPICAL DETAILS.
4. COORDINATE ALL SLAB OPENINGS, SLEEVES, SLOPES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
5. COORDINATE ALL DIMENSIONS, LOCATIONS AND WALL OPENING WITH ARCHITECT, CIVIL, MEP, AND ALL OTHER PRIME CONTRACTORS.

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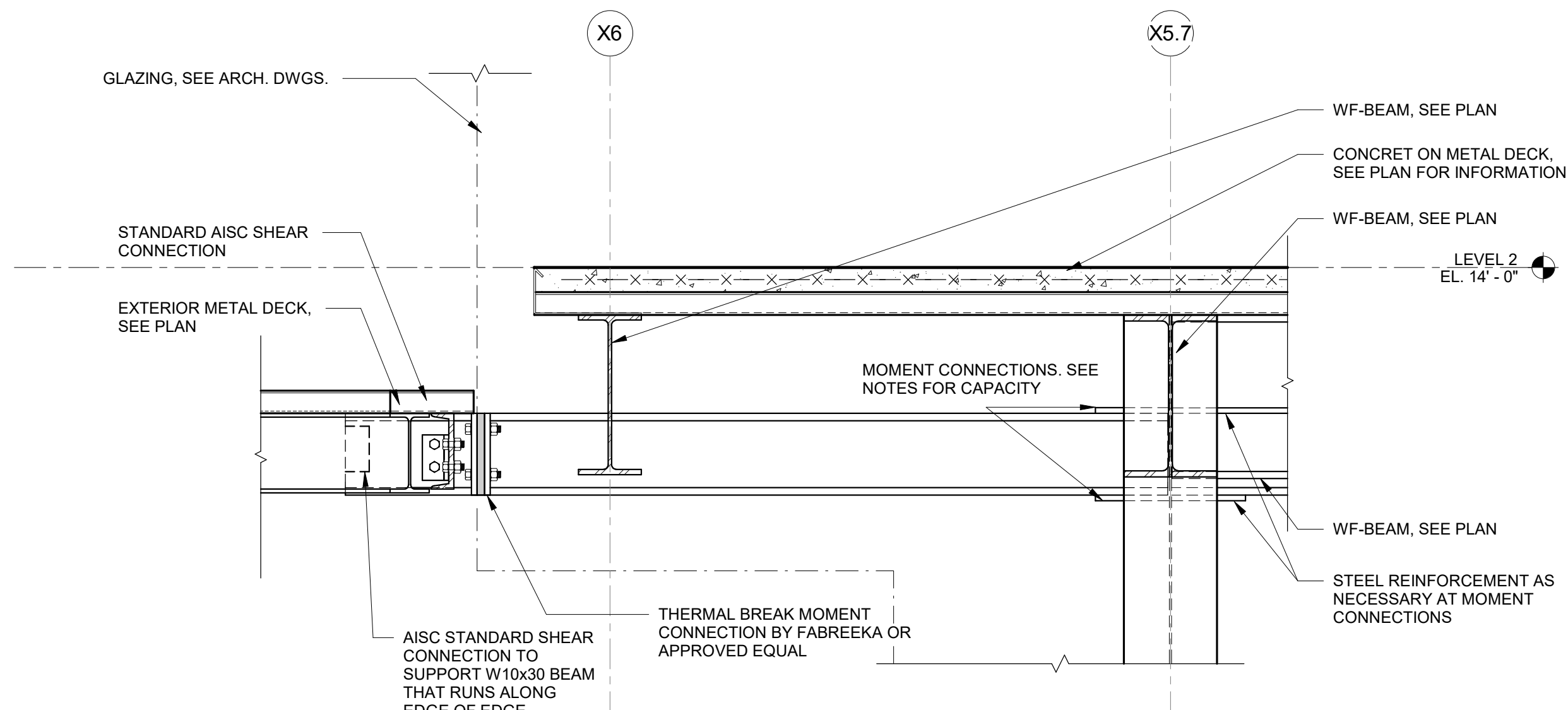
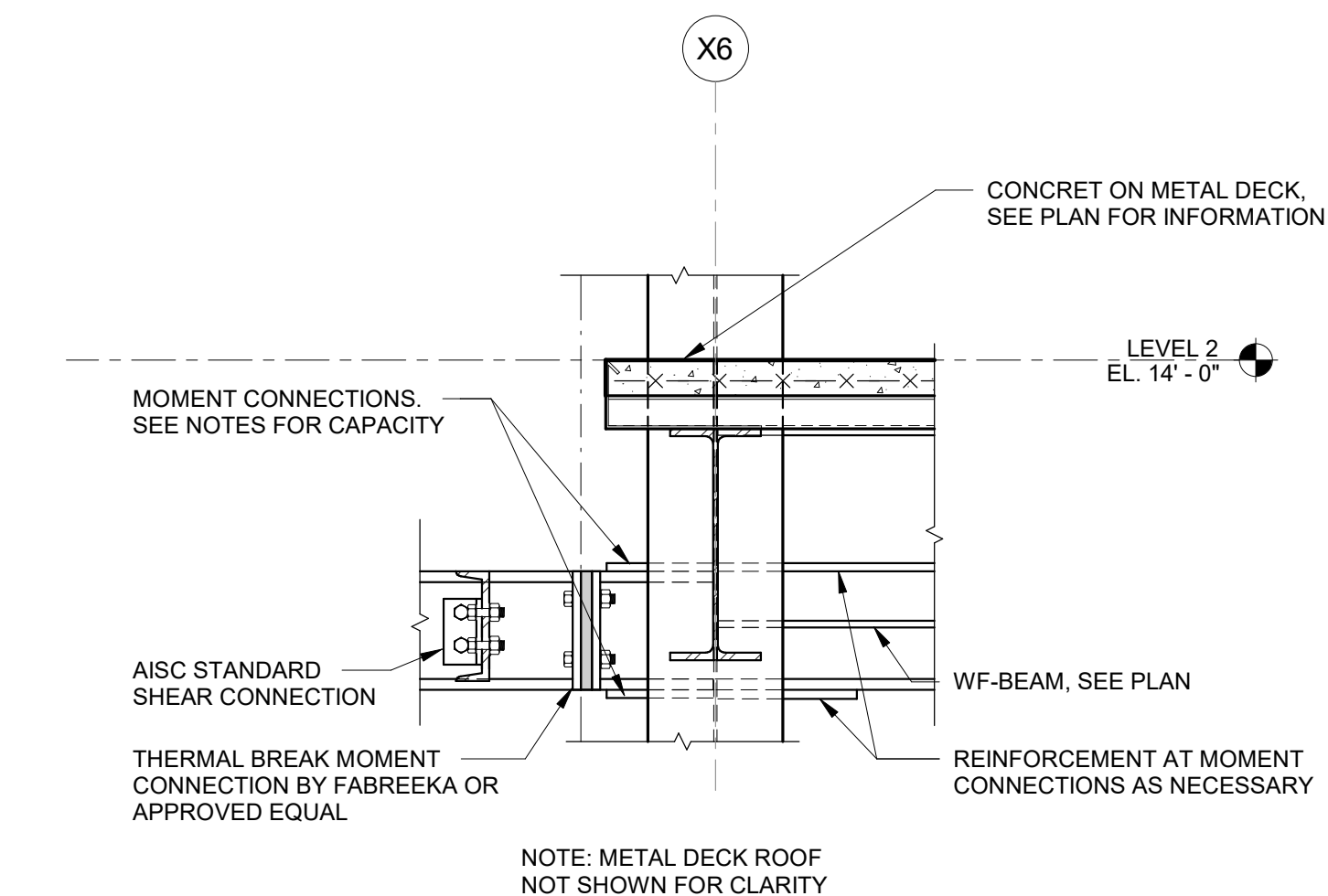


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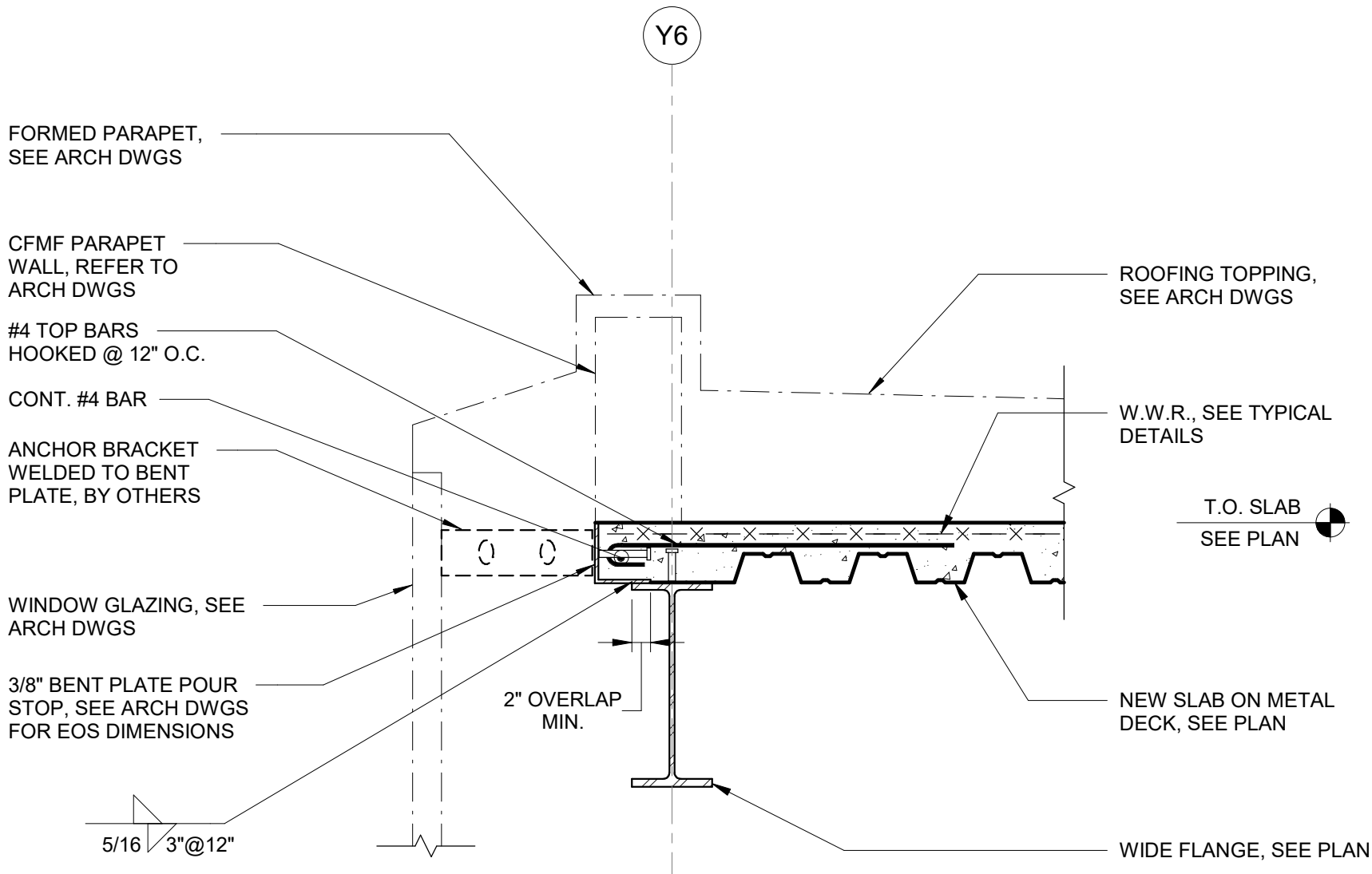
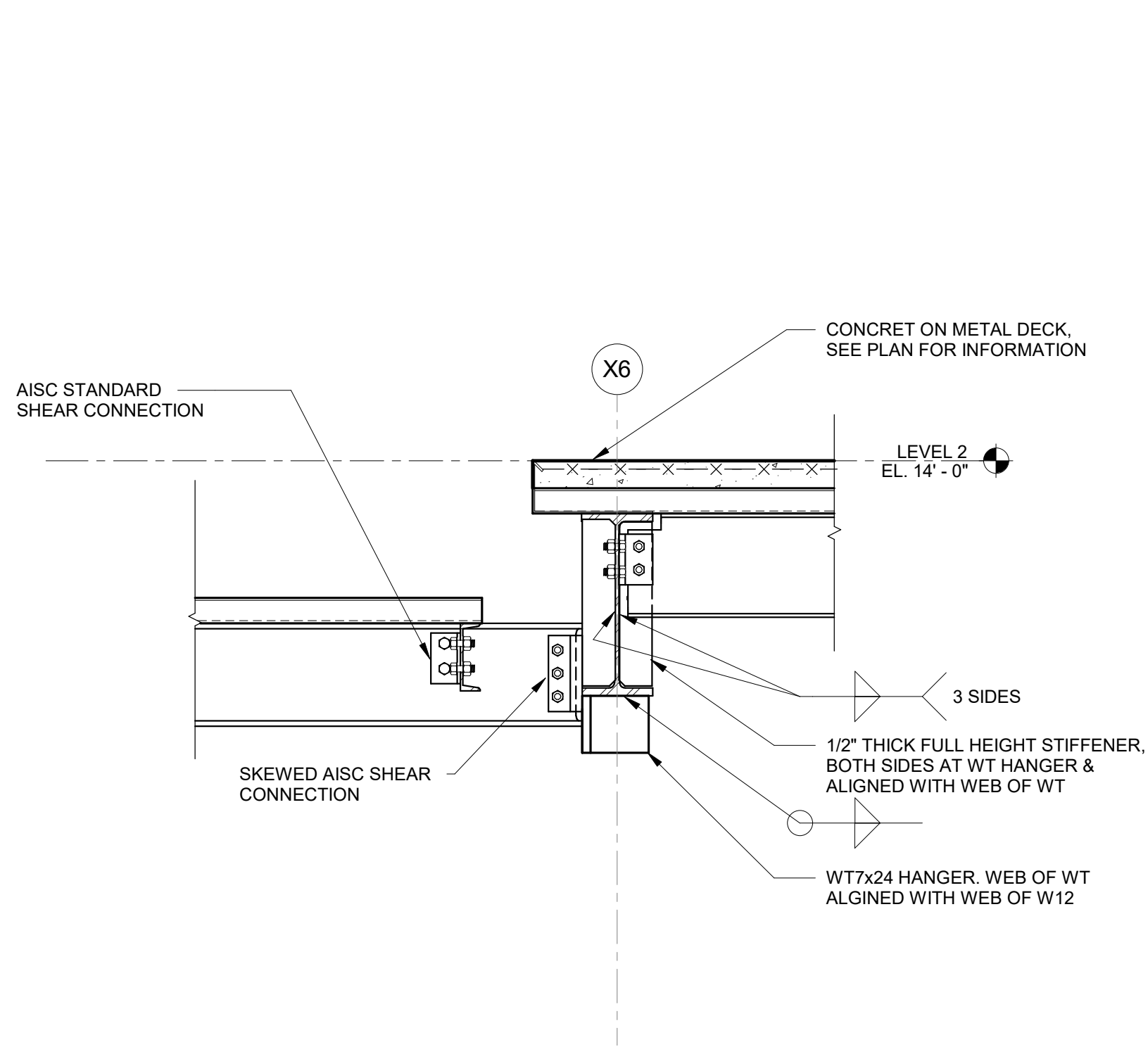
1 SECTION
S-402 3/4" = 1'-0"

2 SECTION
S-402 3/4" = 1'-0"



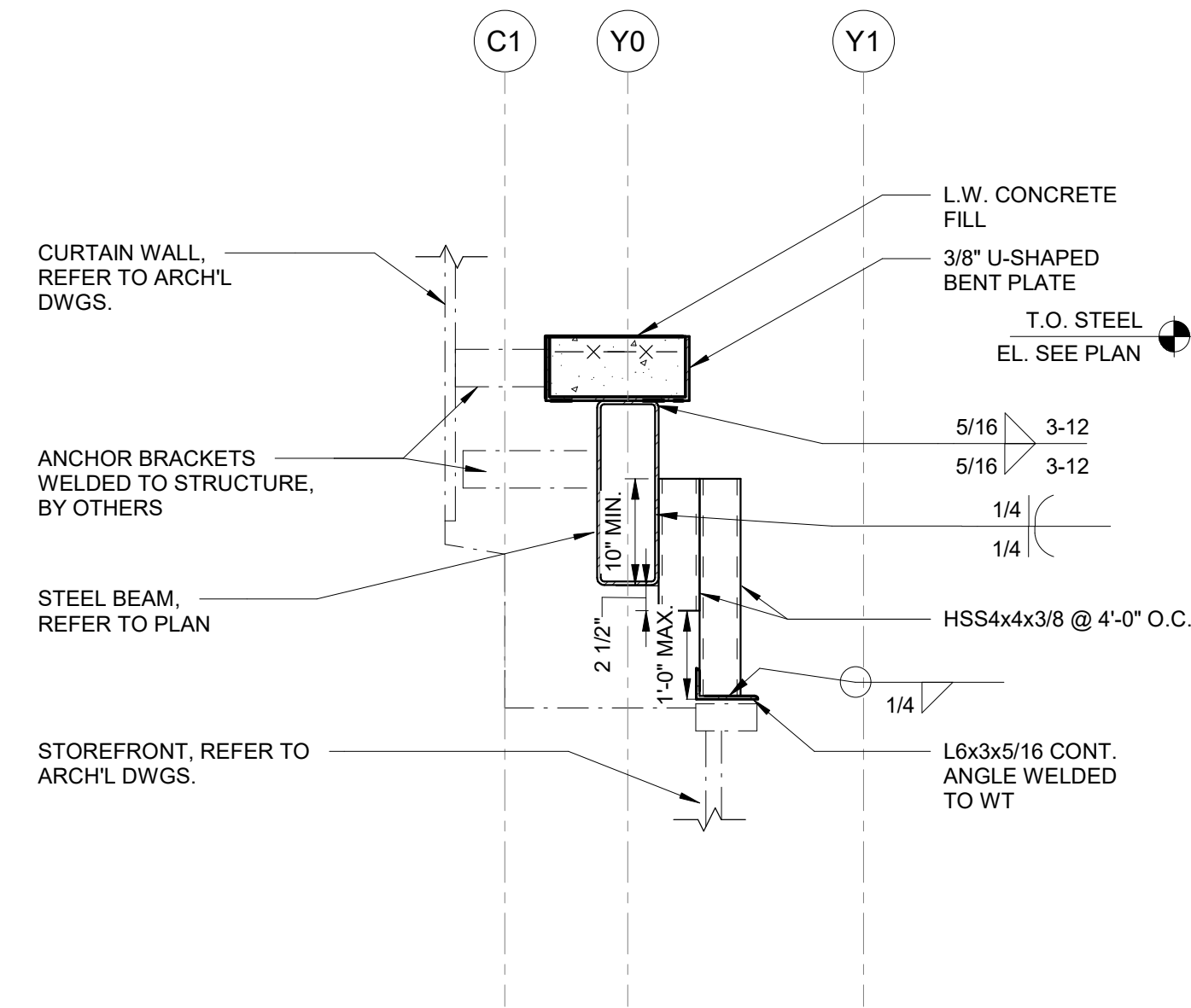
3 SECTION
S-402 3/4" = 1'-0"

4 SECTION
S-402 3/4" = 1'-0"

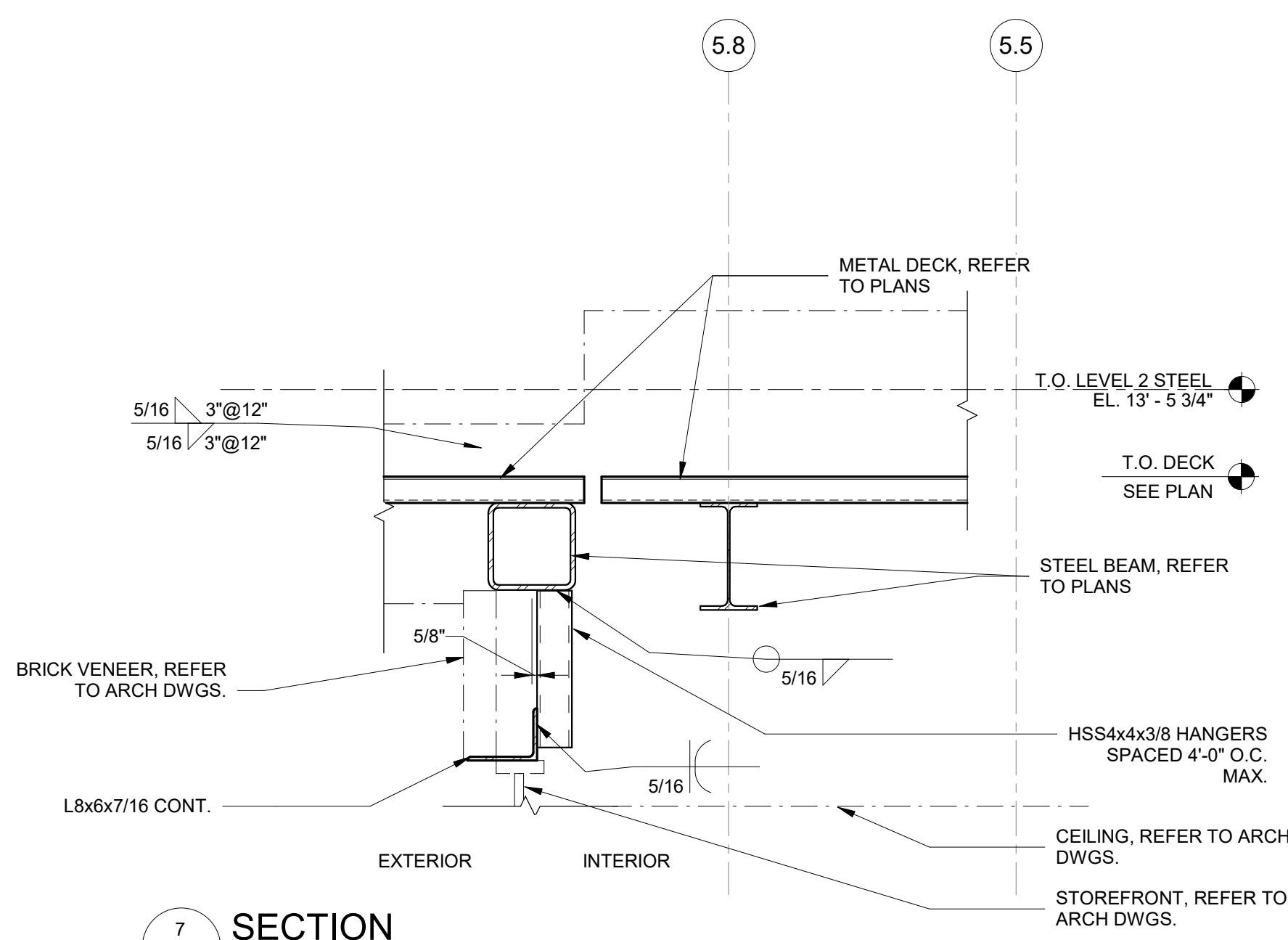


5 SECTION
S-402 3/4" = 1'-0"

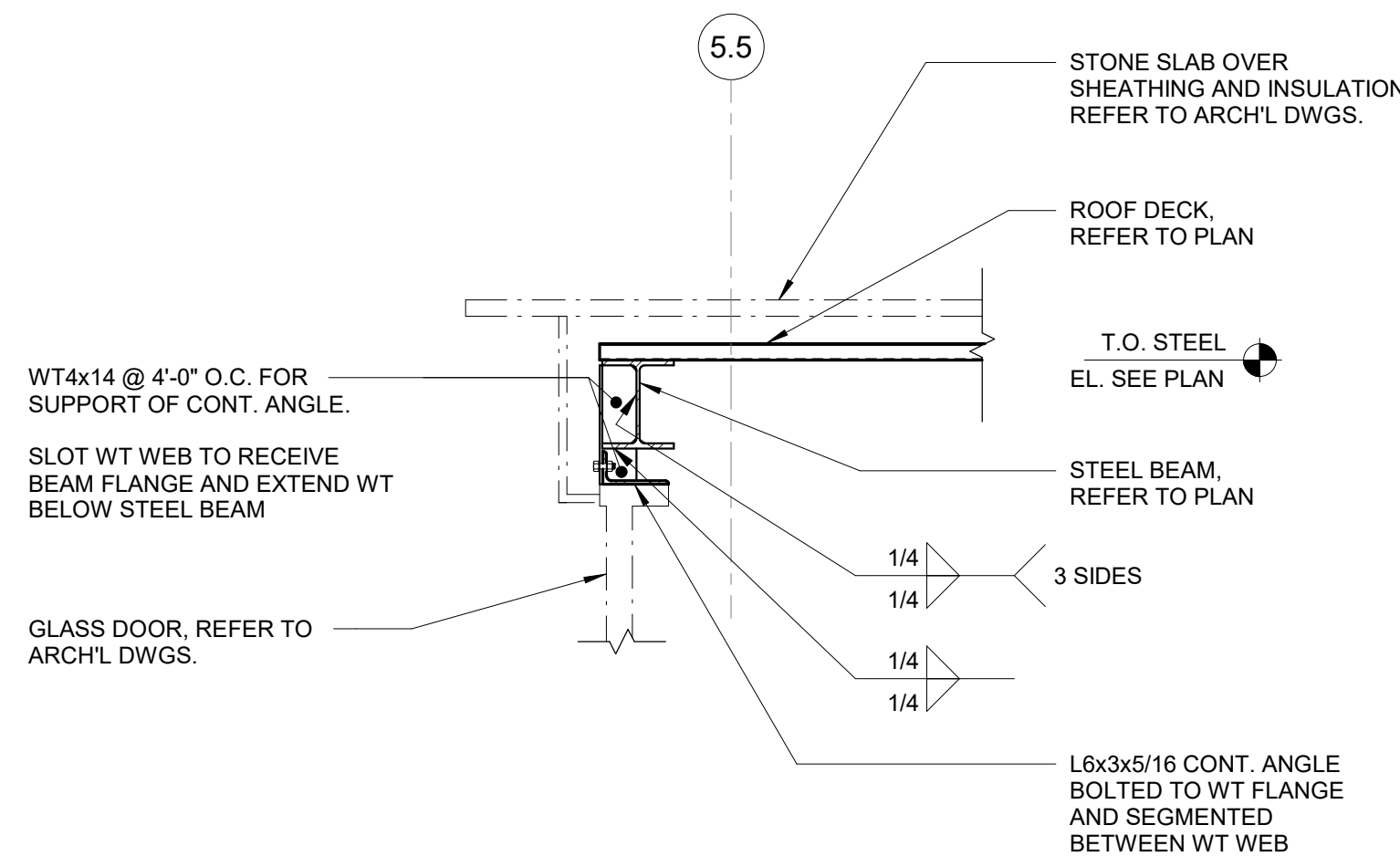
6 SECTION
S-402 3/4" = 1'-0"



8 SECTION
S-402 3/4" = 1'-0"



7 SECTION
S-402 3/4" = 1'-0"



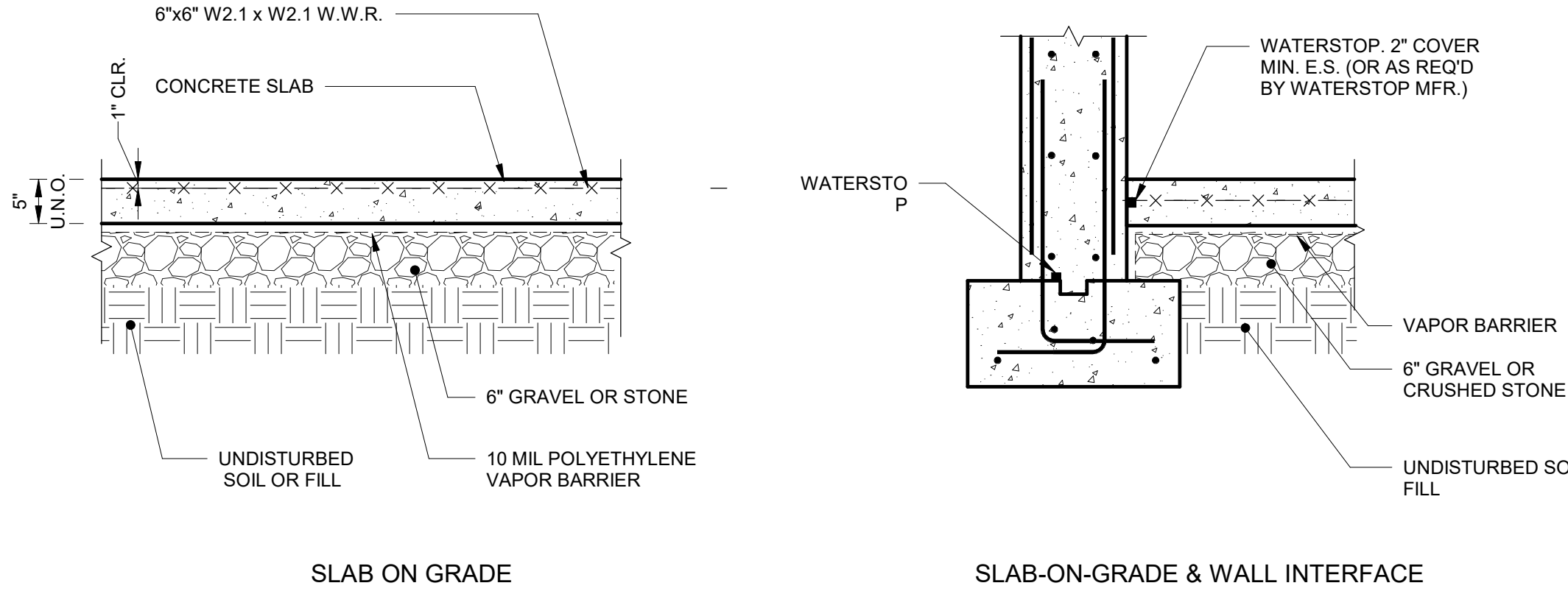
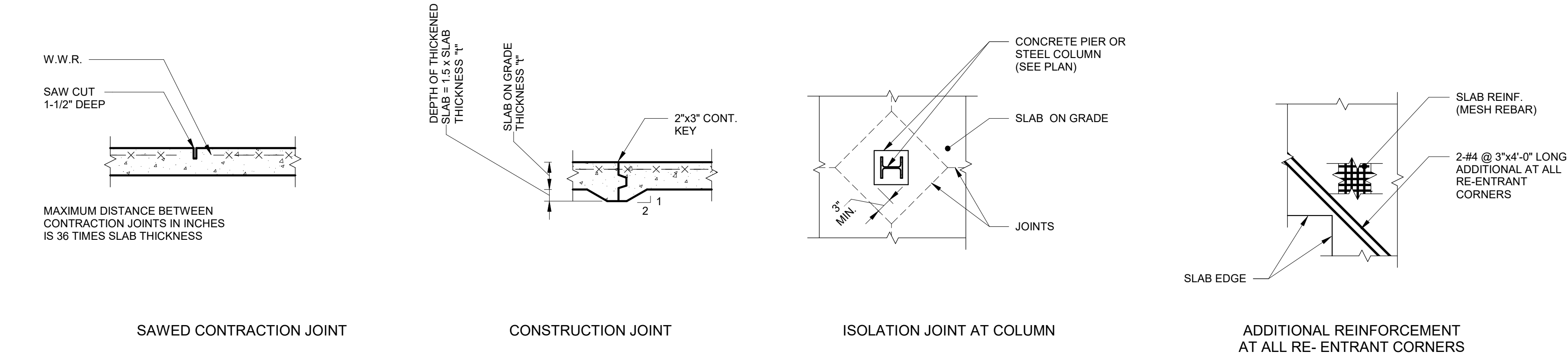
9 SECTION
S-402 3/4" = 1'-0"

CONTRACTOR

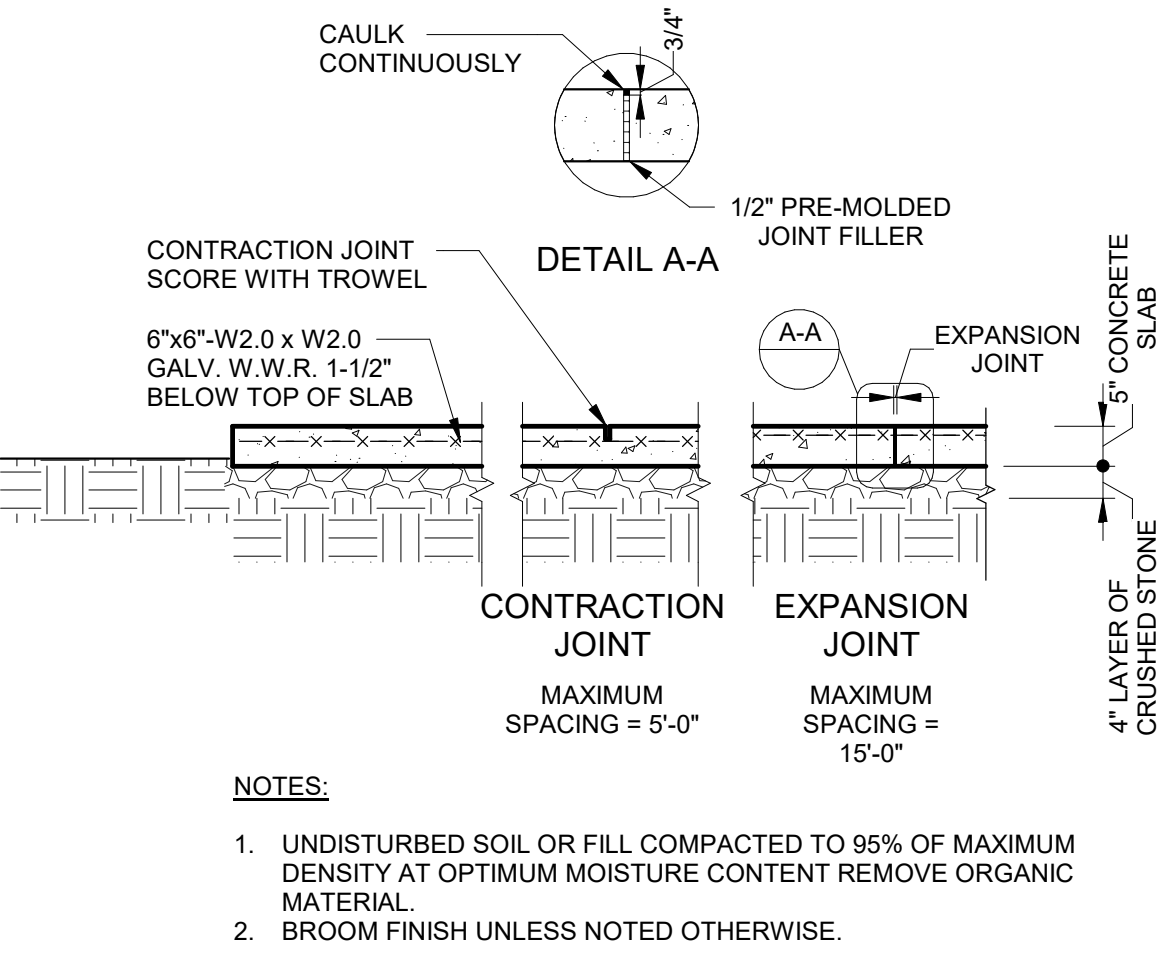
CONSULTANT

ISSUE DATES
07/20/2019 75% DESIGN DEVELOPMENT
08/03/2019 100% DESIGN DEVELOPMENT
12/13/2019 100% CONSTRUCTION DOCUMENTS
04/15/2020 ISSUED FOR PERMIT
05/01/2020 ISSUED FOR CONSTRUCTION
07/01/2020 GMP SET
12/18/2020 FINAL GMP SET
06/21/2021 BID SET

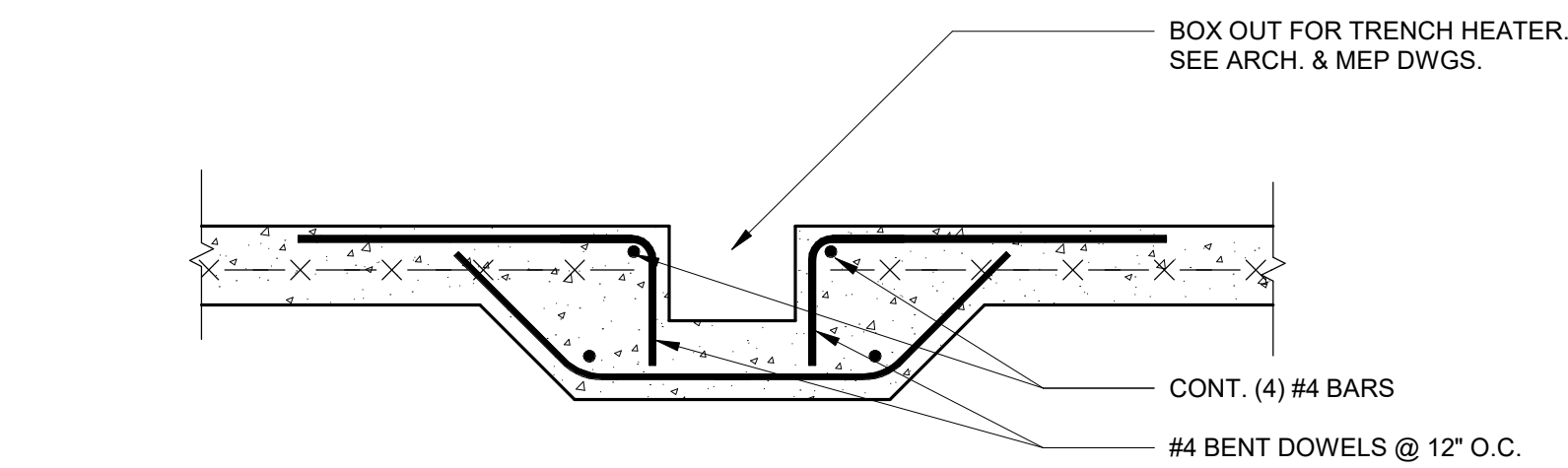
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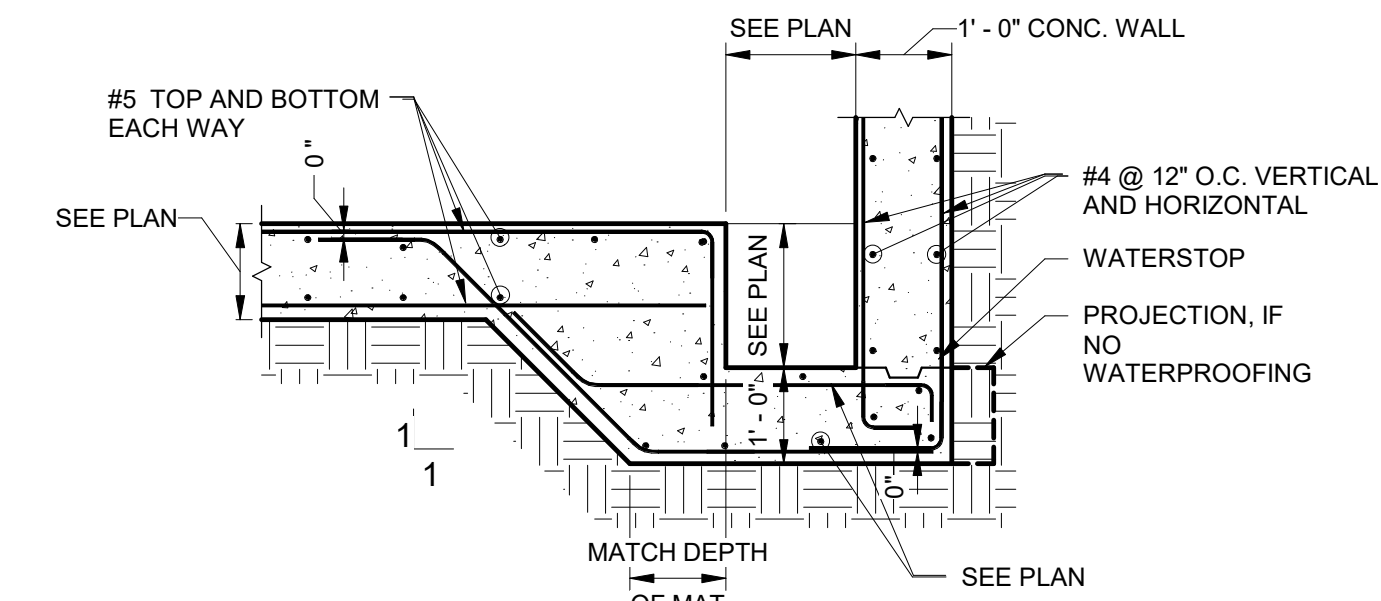
TYPICAL SLAB ON GRADE
N.T.S.



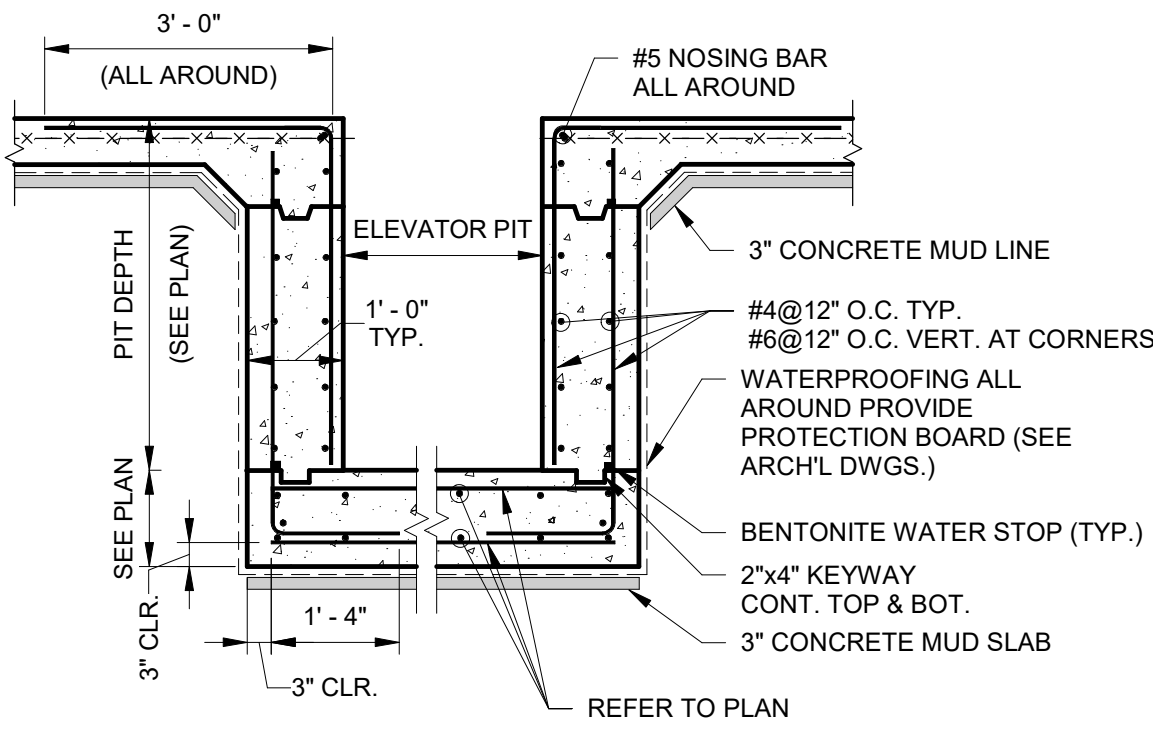
TYPICAL EXTERIOR PAVING
N.T.S.



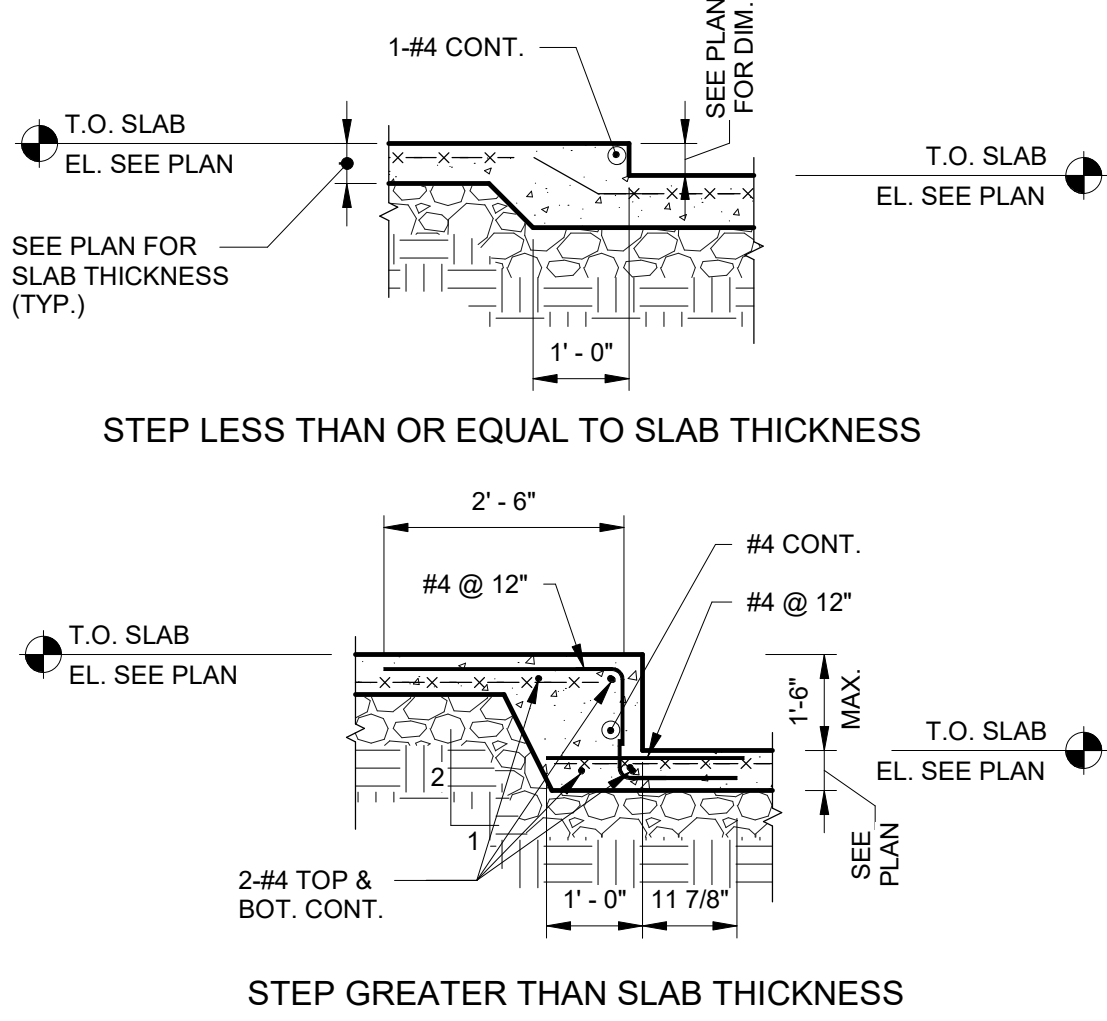
TYPICAL TRENCH DRAIN DETAIL
N.T.S.



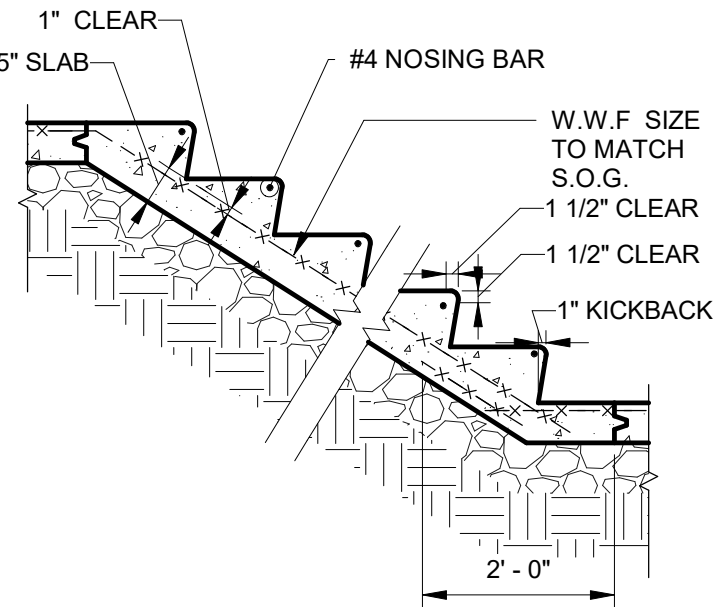
TYPICAL ELEVATOR SUMP PIT
N.T.S.



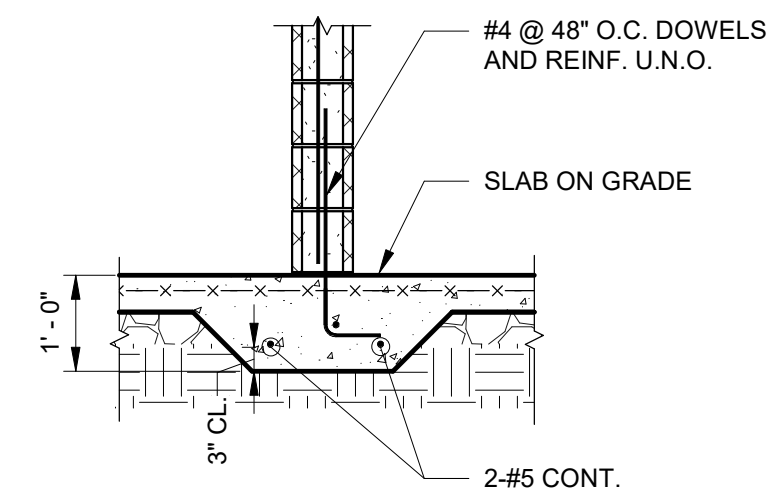
TYPICAL ELEVATOR PIT (EXTERIOR WATERPROOFING)
N.T.S.



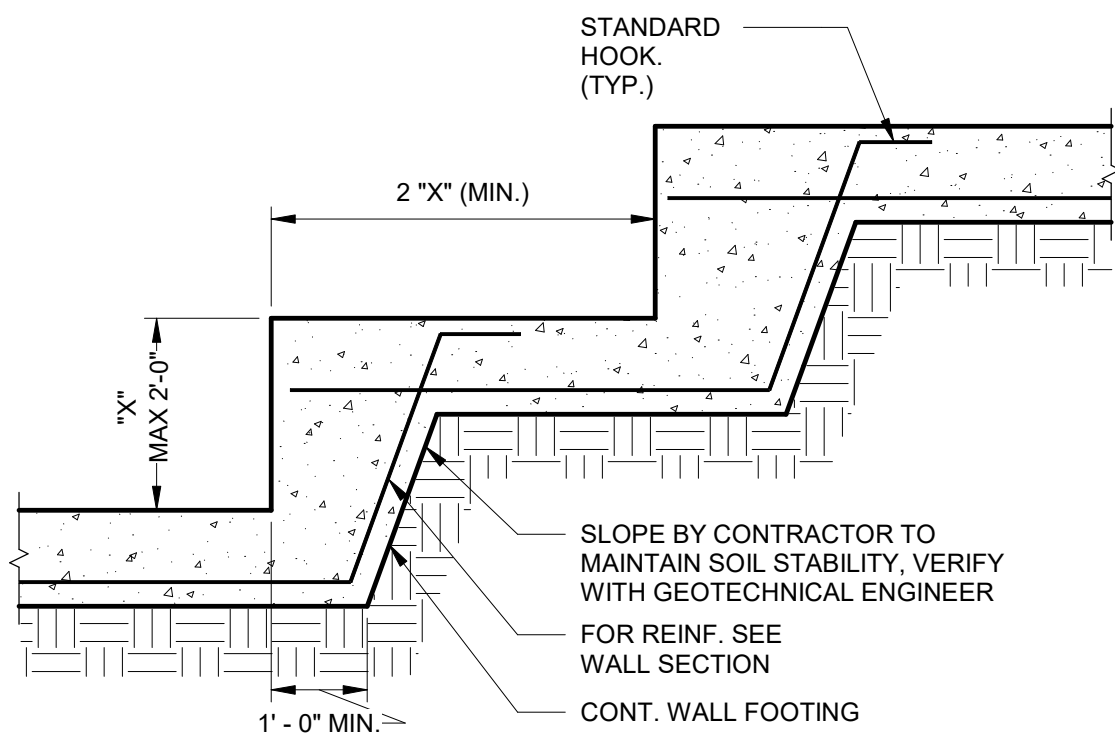
TYPICAL STEP IN SLAB ON GRADE
N.T.S.



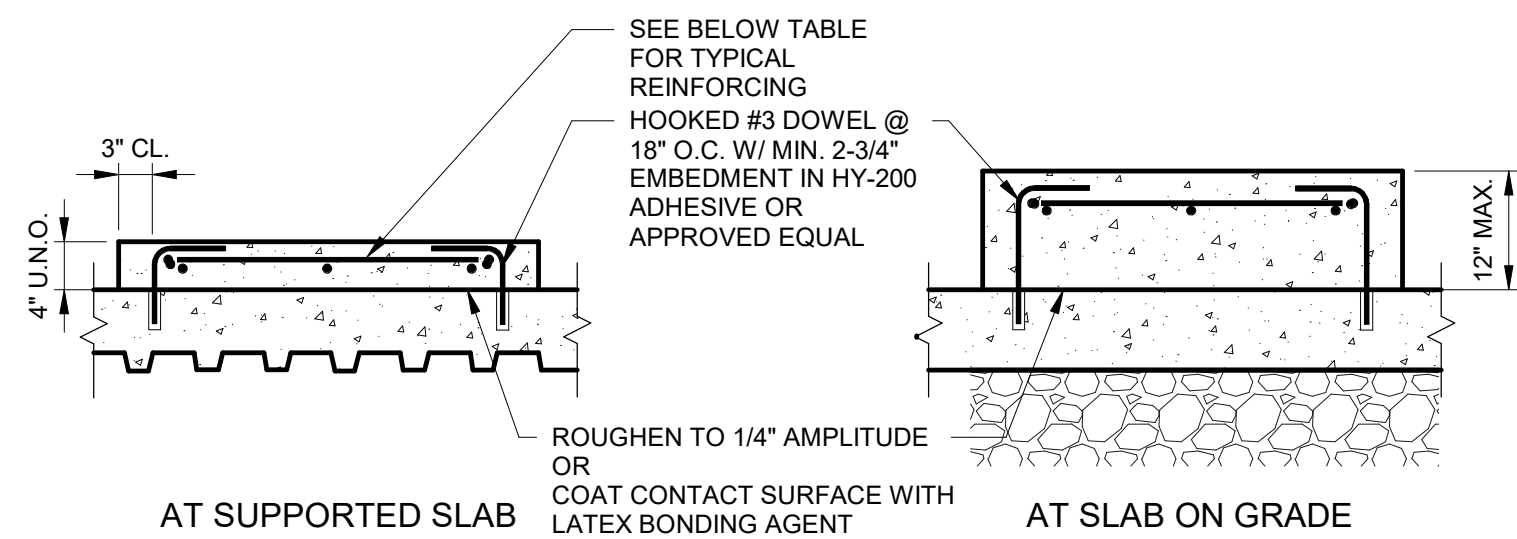
TYPICAL STAIR ON GRADE
N.T.S.



NOTE: SEE TYPICAL SLAB ON GRADE DETAIL FOR ADDITIONAL INFORMATION
TYPICAL SUPPORT FOR MASONRY PARTITIONS AT SLAB ON GRADE
N.T.S.



TYPICAL STEPPED WALL FOOTING
N.T.S.



EQUIPMENT PAD TEMPERATURE AND SHRINKAGE REINFORCING	
PAD THICKNESS	REINFORCING
4"-5"	#3 @ 12" O.C. EACH WAY
6"-9"	#4 @ 12" O.C. EACH WAY
10"-12"	#5 @ 12" O.C. EACH WAY

- NOTES:**
1. FOR SIZE AND LOCATION SEE ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. CONCRETE FOR PADS SHALL BE NORMAL WEIGHT WITH $f_c = 4,000$ PSI.

TYPICAL EQUIPMENT PAD
N.T.S.



DEFORMED TENSION BAR NOTES:

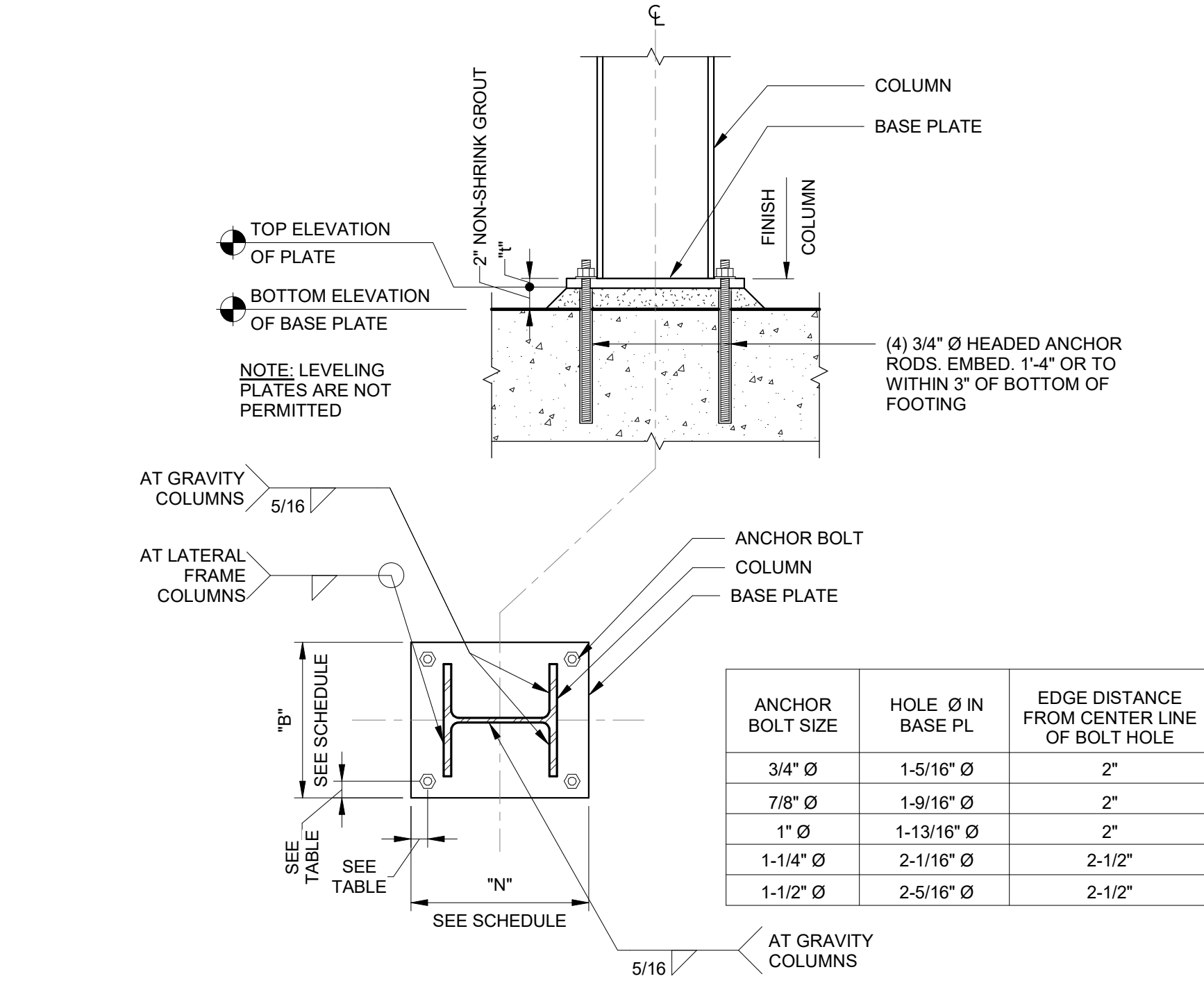
1. FOR HORIZONTAL REINFORCEMENT WITH 12 INCH OR MORE FRESH CONCRETE CAST BELOW IT, TENSION DEVELOPMENT LENGTH/ TENSION LAP SPlice LENGTH SHALL BE 1.3X THE VALUES GIVEN.
2. FOR EPOXY-COATED BARS:
 - A. WHERE CONCRETE COVER IS LESS THAN 3X BAR DIAMETER, OR CLEAR SPACING IS LESS THAN 6X BAR DIAMETER, TENSION DEVELOPMENT LENGTH/ TENSION LAP SPlice LENGTH SHALL BE 1.5X THE VALUES GIVEN
 - B. WHERE CONCRETE COVER IS EQUAL TO OR GREATER THAN 3X BAR DIAMETER AND CLEAR SPACING IS GREATER THAN 6X BAR DIAMETER, TENSION DEVELOPMENT LENGTH/ TENSION LAP SPlice LENGTH SHALL BE 1.2X THE VALUES GIVEN.

CASE 1: CLEAR SPACING OF BARS BEING DEVELOPED OR SPICED NOT LESS THAN DB, CLEAR COVER NOT LESS THAN DB, AND STIRRUPS OR TIES THROUGHOUT LD NOT LESS THAN THE CODE MINIMUM OR CLEAR SPACING OF BARS BEING DEVELOPED OR SPICED NOT LESS THAN 2DB AND CLEAR COVER NOT LESS THAN DB.

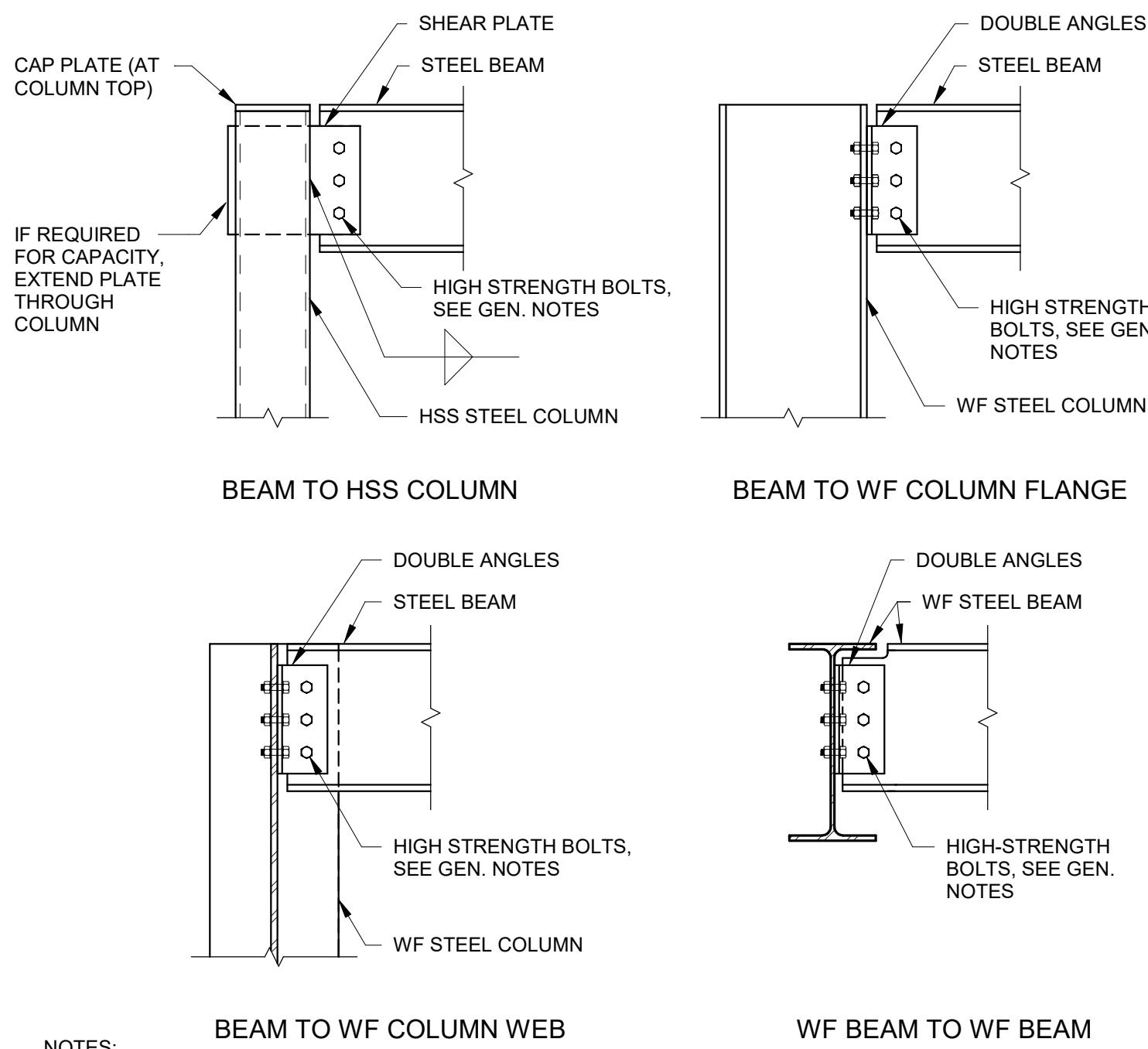
CASE 2: OTHER CASES

DEFORMED BAR COMPRESSION DEVELOPMENT LENGTH (L _{dc})					DEFORMED BAR COMPRESSION LAP SPLICE				
FOR NORMAL WEIGHT STONE CONCRETE & UNCOATED BARS					FOR NORMAL WEIGHT STONE CONCRETE & UNCOATED BARS				
BAR SIZE	3000 PSI CONCRETE	4000 PSI CONCRETE	5000 PSI CONCRETE	6000 PSI CONCRETE	BAR SIZE	3000 PSI CONCRETE	4000 PSI CONCRETE	5000 PSI CONCRETE	6000 PSI CONCRETE
#3	9	8	8	8	#3	12	12	12	12
#4	11	10	9	9	#4	15	15	15	15
#5	14	12	12	12	#5	19	19	19	19
#6	17	15	14	14	#6	23	23	23	23
#7	20	17	16	16	#7	27	27	27	27
#8	22	19	18	18	#8	30	30	30	30
#9	25	22	21	21	#9	34	34	34	34
#10	28	25	23	23	#10	39	39	39	39
#11	31	27	26	26	#11	43	43	43	43

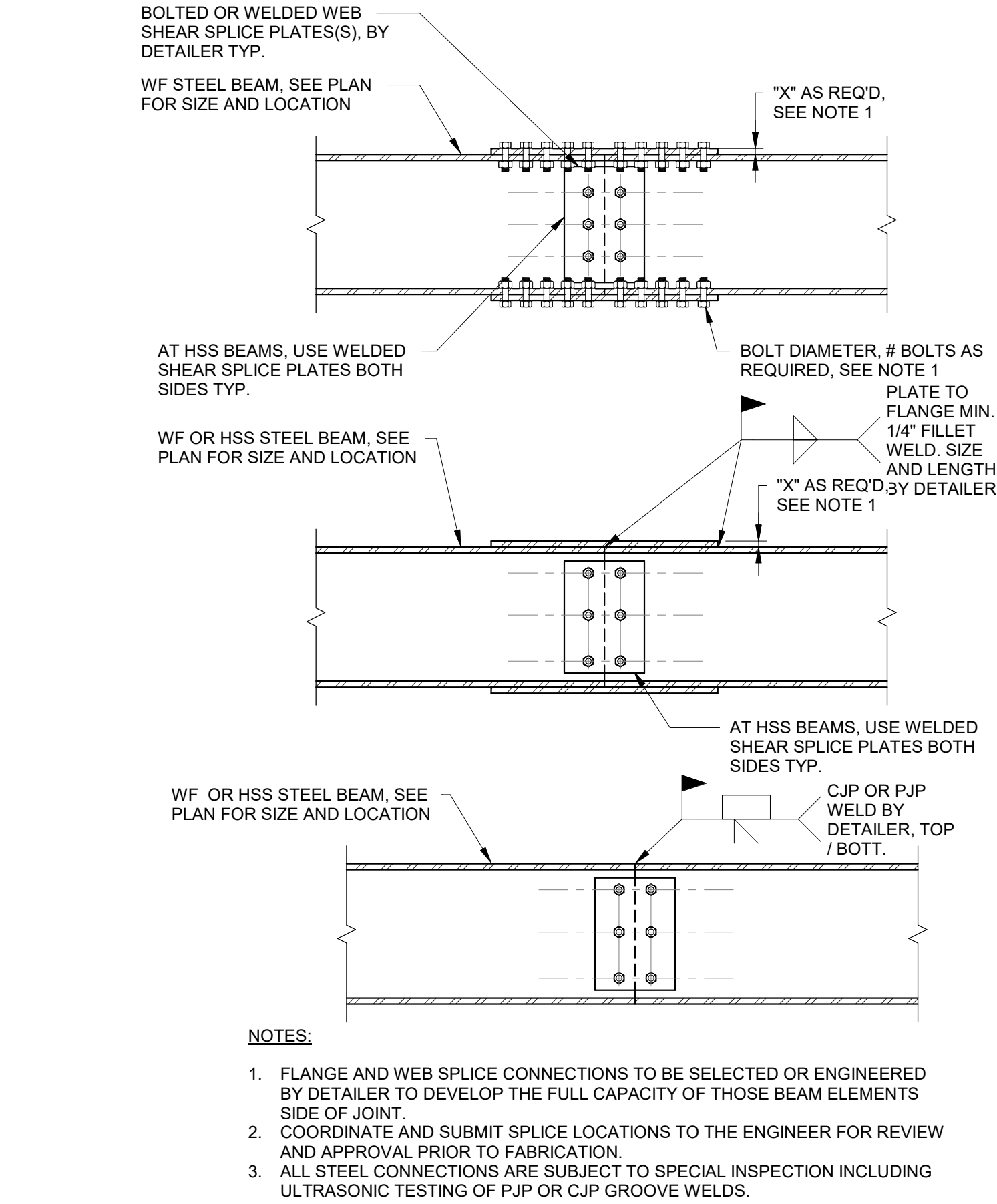
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TYPICAL COLUMN BASE PLATE AT WIDE FLANGE COLUMN
N.T.S.



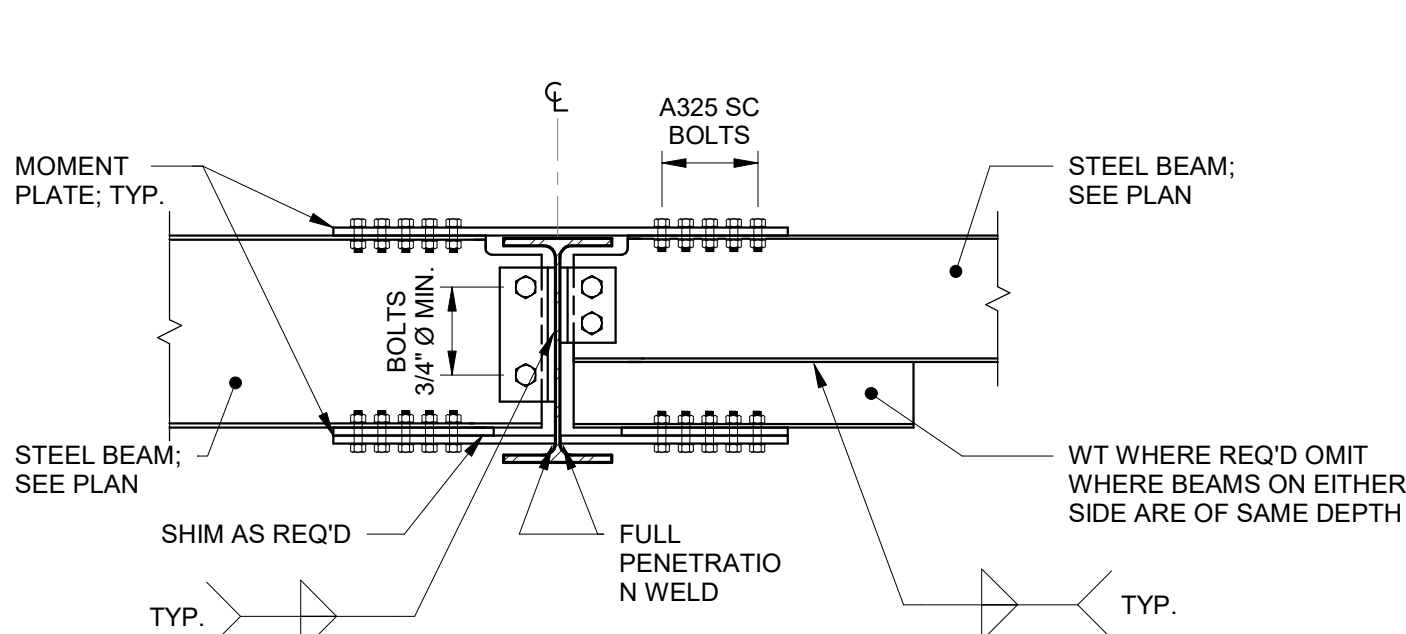
TYPICAL COLUMN BASE PLATE AT PIPE OR TUBE COLUMN
N.T.S.



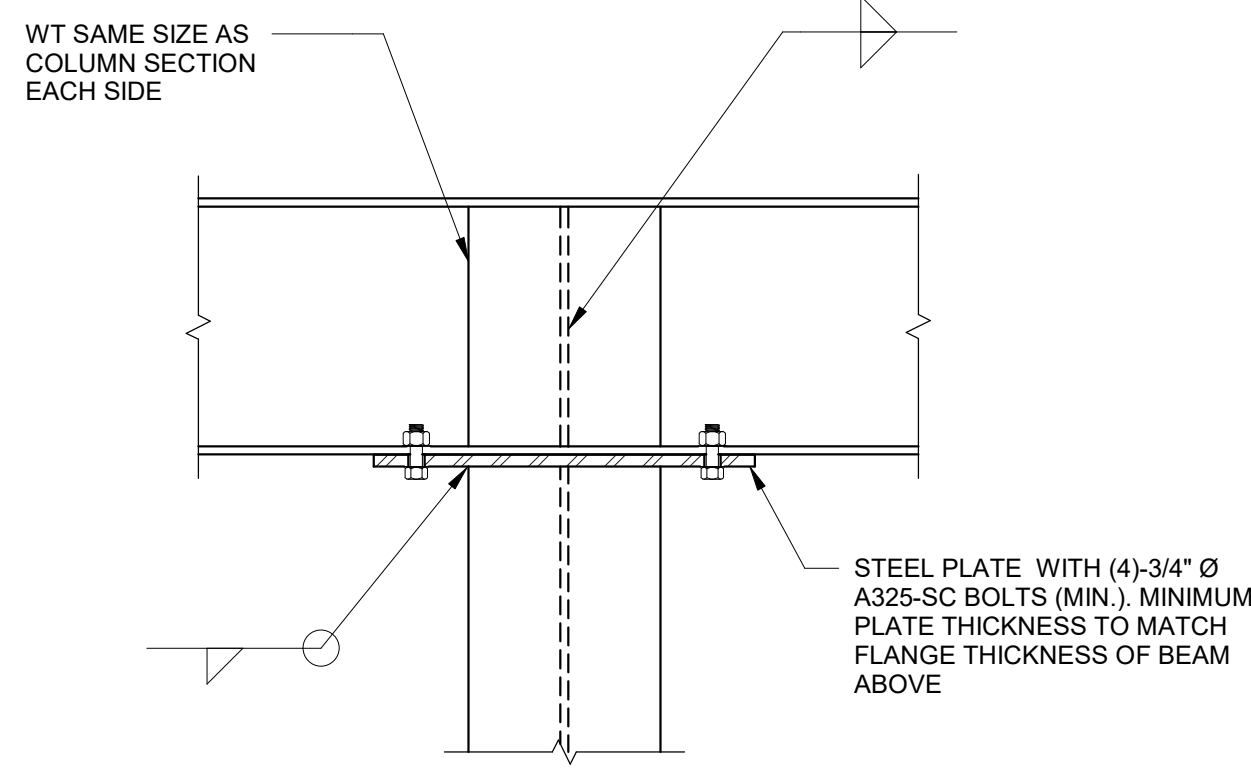
TYPICAL STEEL BEAM SPLICE
N.T.S.

- NOTES:
1. CONNECTION DETAILS SHOWN ABOVE ARE SCHEMATIC ONLY. THE CONTRACTOR MAY SUBMIT ALTERNATE DETAILS FROM THOSE SHOWN ABOVE, BUT IN ANY CASE THE CONTRACTOR IS RESPONSIBLE FOR PRODUCING STEEL SHOP DRAWINGS IN ACCORDANCE WITH THE PROJECT GENERAL NOTES AND AISC GUIDELINES. CALCULATIONS SHALL BE SUBMITTED AS REQUIRED IN THE GENERAL NOTES AND ELSEWHERE IN THE CONTRACT DOCUMENTS.
 2. THE CONTRACTOR SHALL DETAIL THE PROJECT CONNECTIONS FOR THE LOADS AS INDICATED IN THE CONTRACT DOCUMENTS, PER AISC GUIDELINES. THE SCHEMATIC DETAILS ABOVE ARE NOT SUGGESTIVE OF SPECIFIC CAPACITIES. THE NUMBER AND SIZE OF BOLTS, SIZE AND LENGTH OF WELDS, AND SIZE OF STEEL PIECES MUST BE DETERMINED PER AISC GUIDELINES AND THE CONTRACT DOCUMENTS.
 3. PROVIDE FULL DEPTH CONNECTIONS AT ALL PERIMETER BEAM CONNECTIONS AND AT BEAM CONNECTIONS TO PERIMETER BEAMS, UNLESS OTHERWISE NOTED.

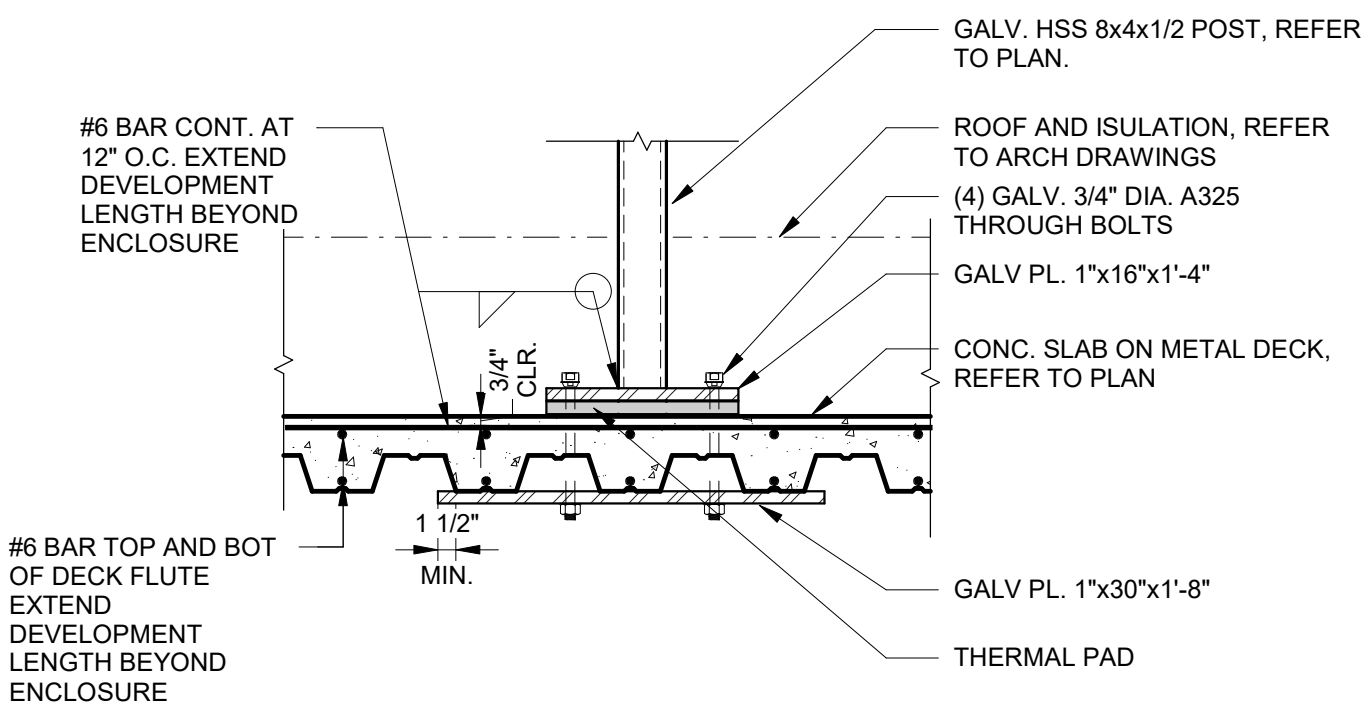
TYPICAL SUGGESTED SHEAR CONNECTIONS AT GRAVITY LOADS ONLY
N.T.S.



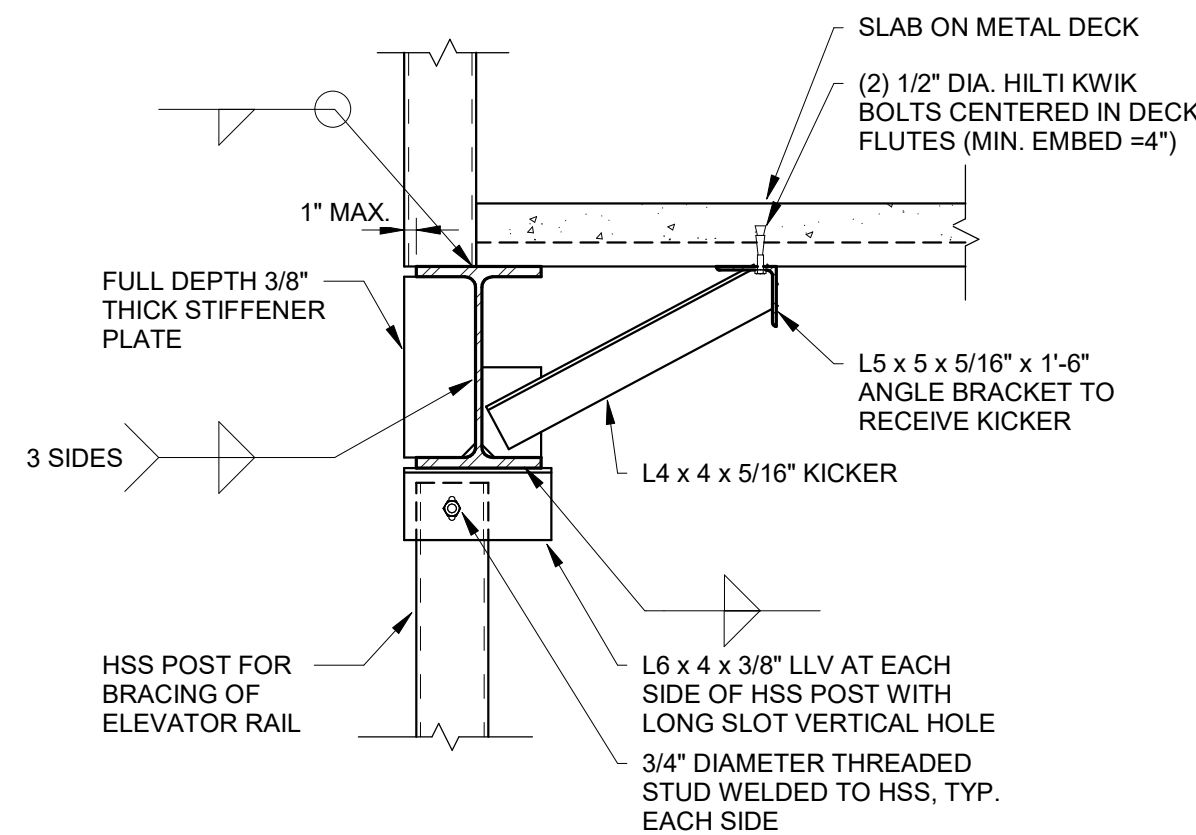
TYPICAL BEAM CONTINUOUS OVER COLUMN (PARALLEL WEBS)
N.T.S.



TYPICAL BEAM CONTINUOUS OVER COLUMN (PERPENDICULAR WEBS)
N.T.S.



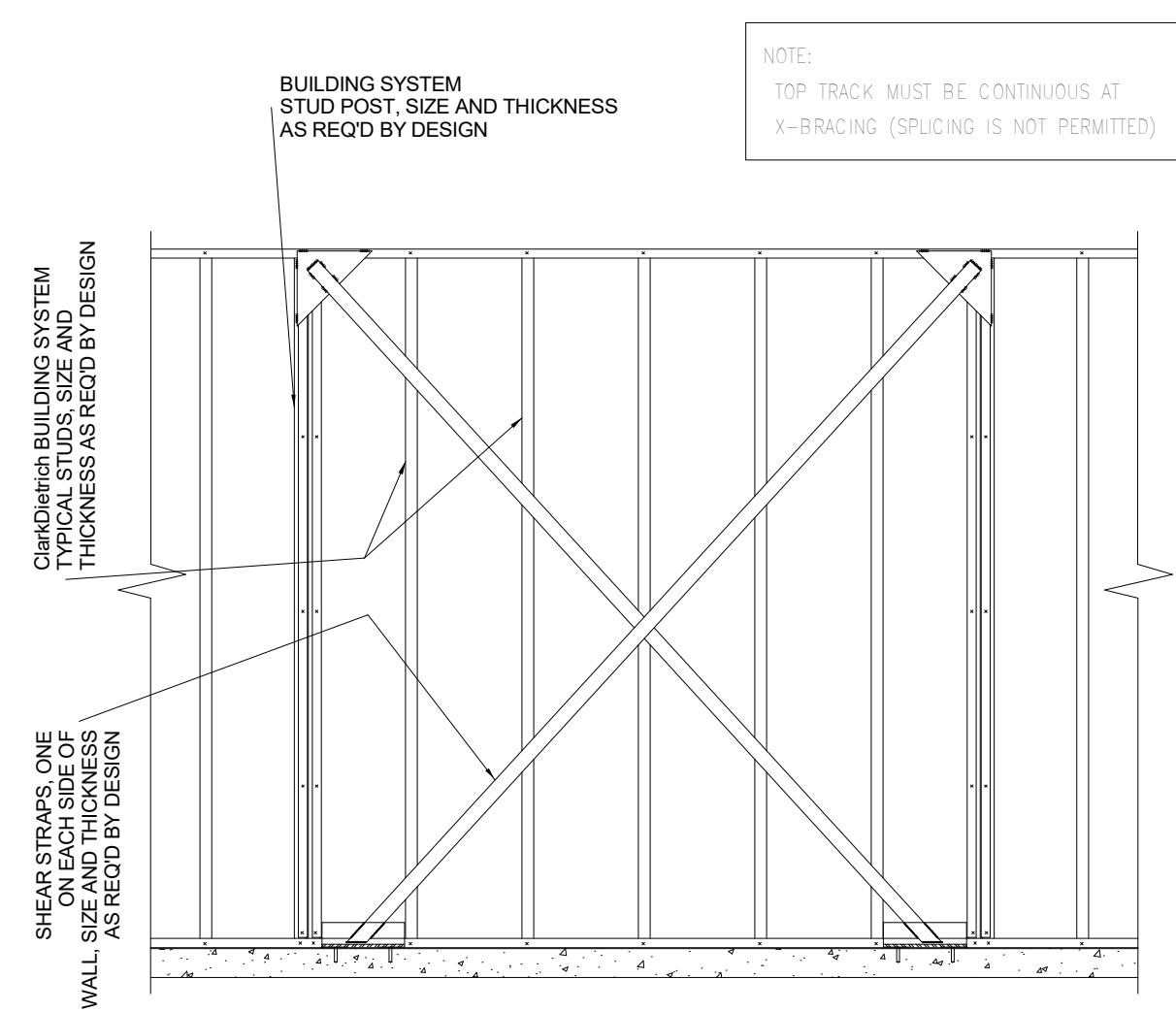
TYPICAL MEP ENCLOSURE POST ANCHORAGE
N.T.S.



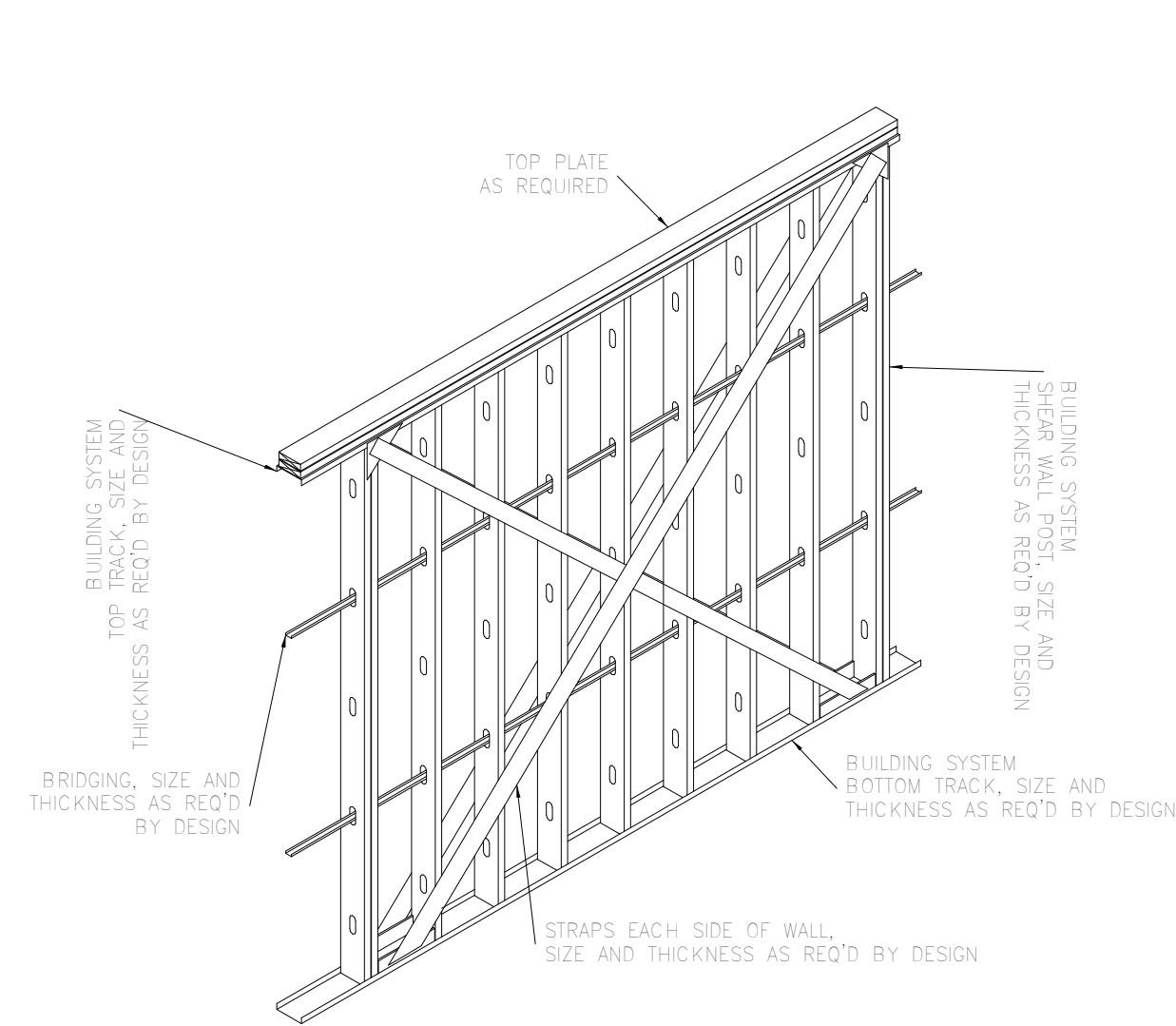
TYPICAL DETAIL OF HANGER AT PARTITION WALL
N.T.S.

TYPICAL DETAIL OF ATTACHMENT OF HSS POST REQUIRED FOR ELEVATOR RAIL SUPPORT
N.T.S.

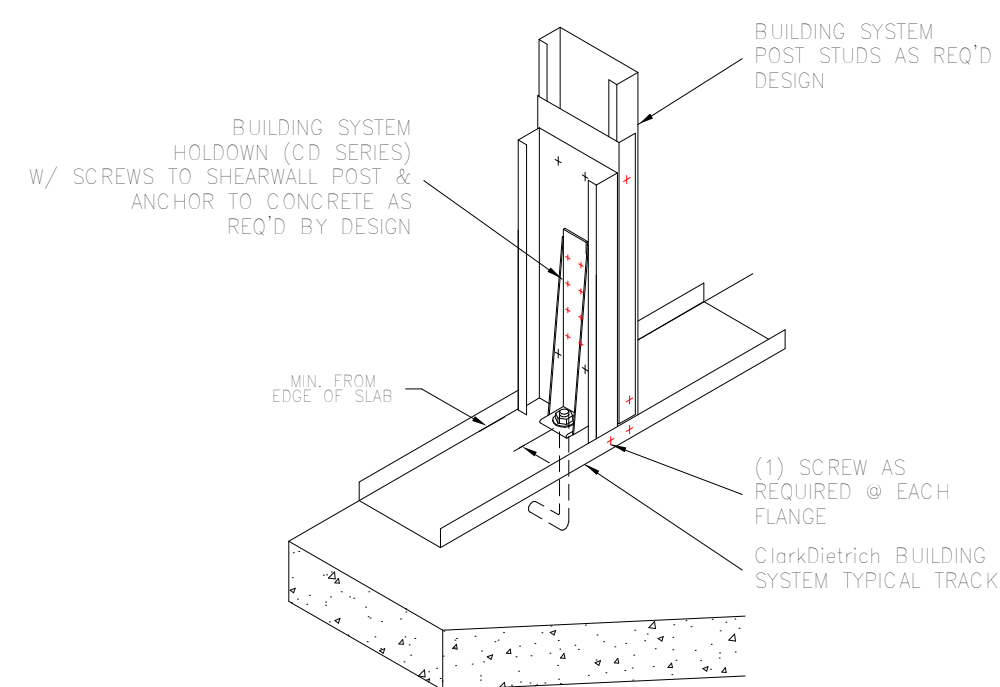
TYPICAL BEAM-TO-BEAM MOMENT CONNECTION
N.T.S.



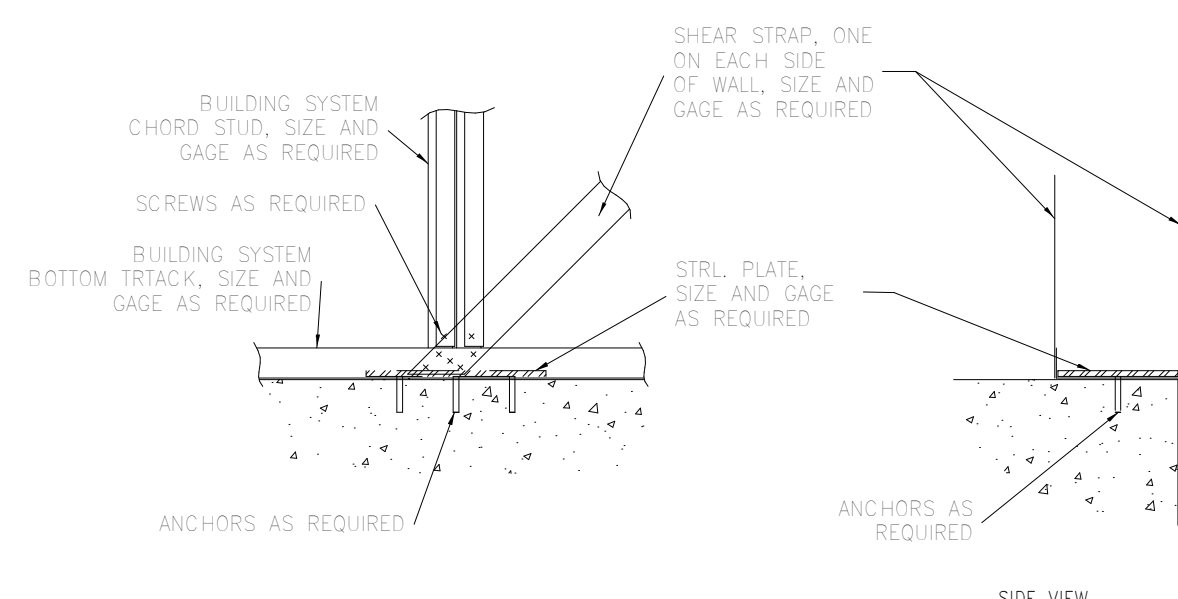
SHEAR WALL ELEVATION



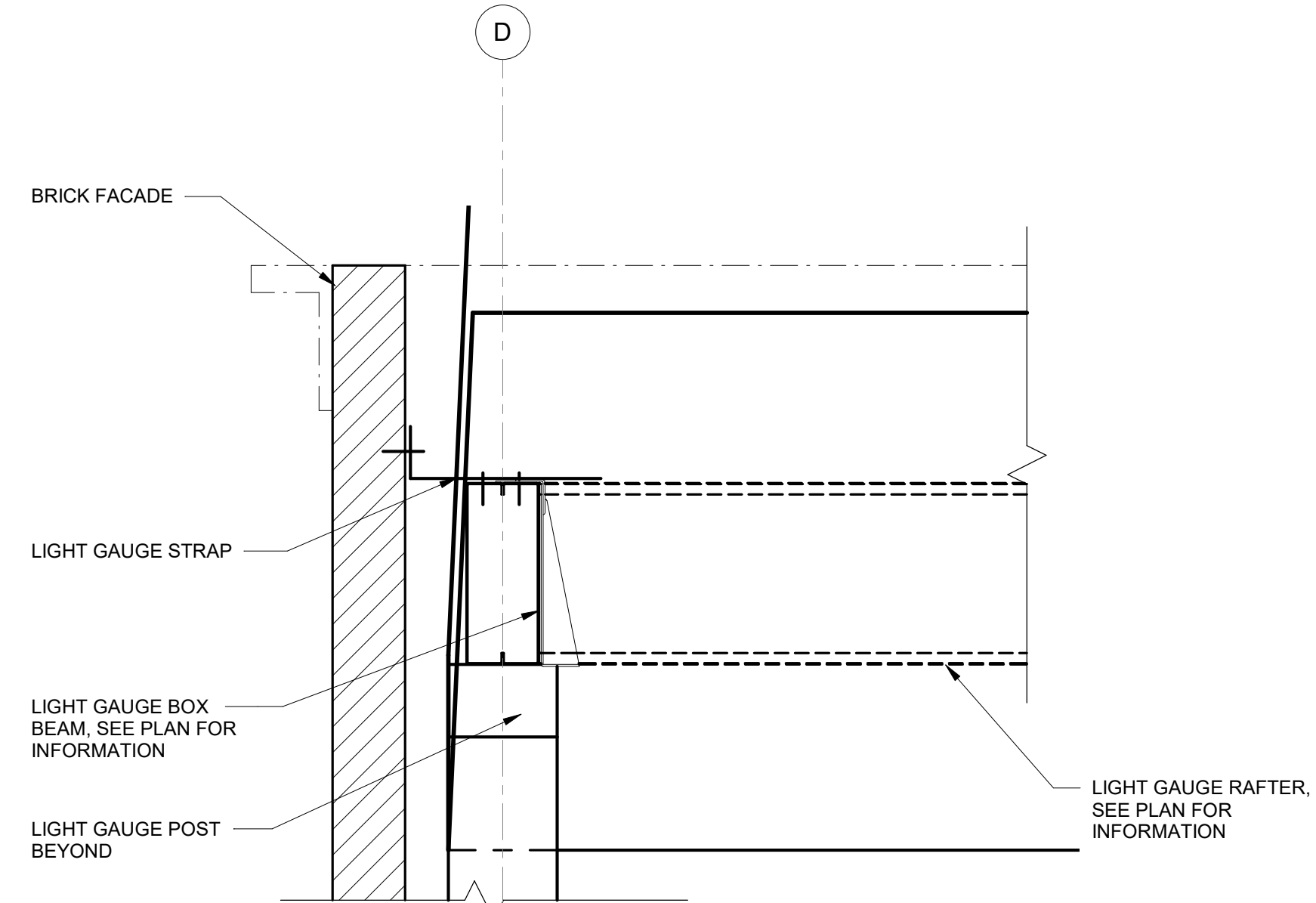
SHEAR WALL ELEVATION



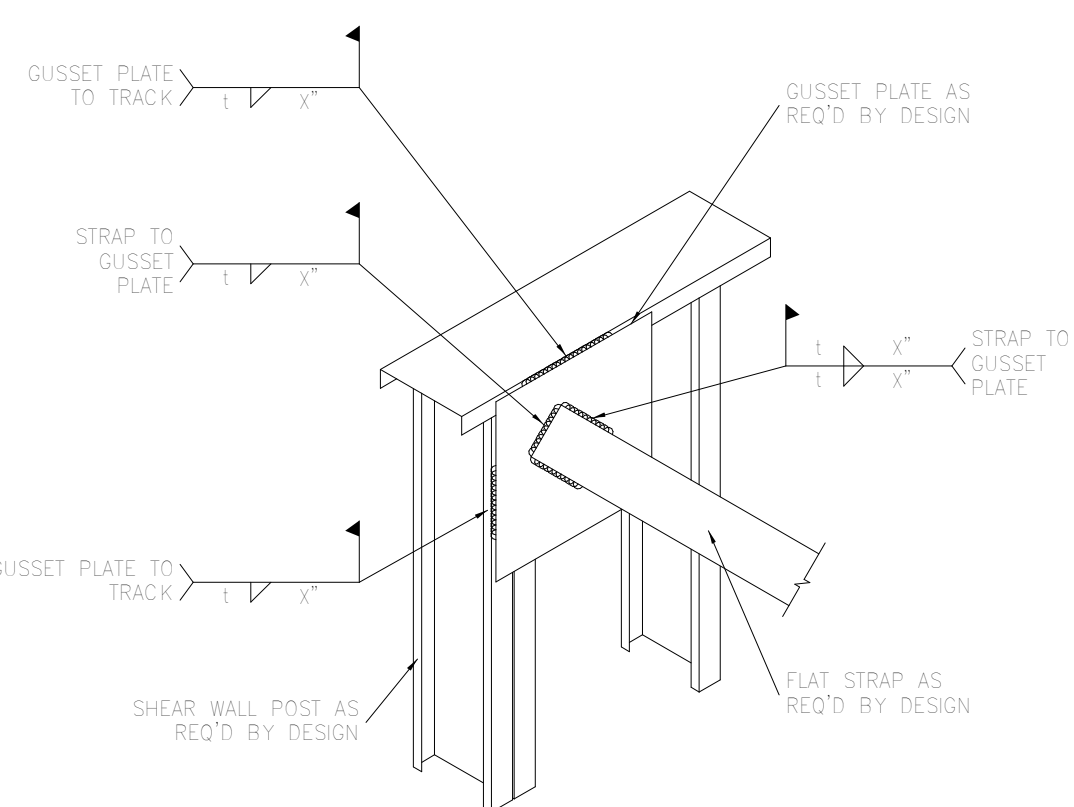
HOLD DOWN DETAIL



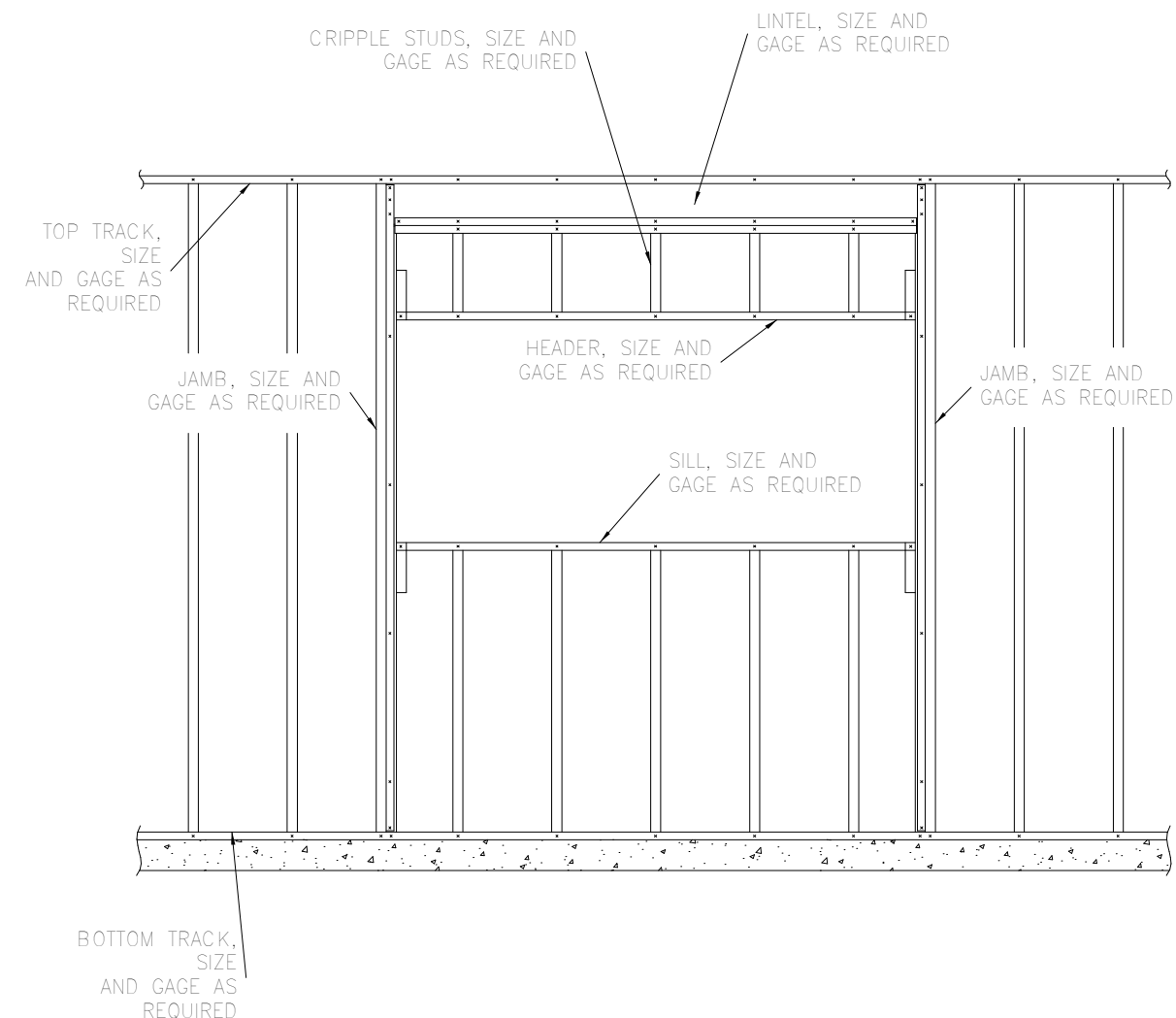
SHEAR ANCHOR DETAIL AT BASE



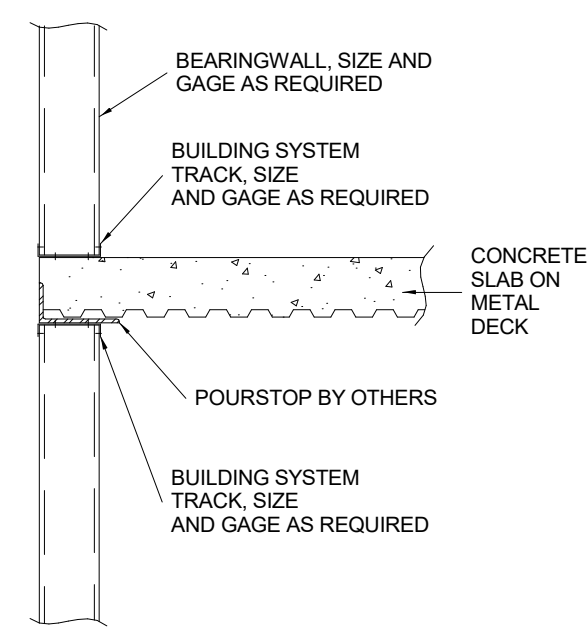
TYPICAL ROOF JOIST AT DORMER
N.T.S.



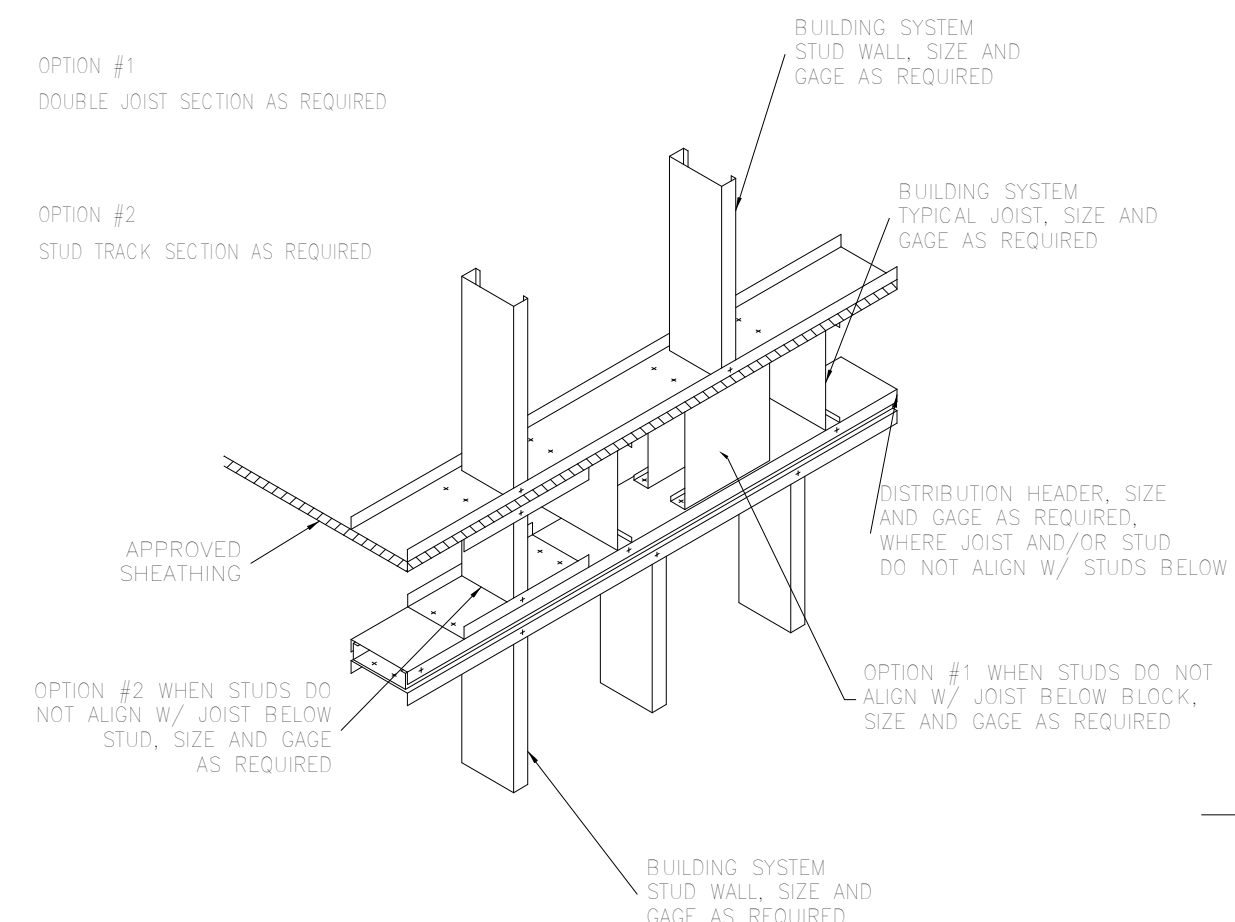
GUSSET PLATES



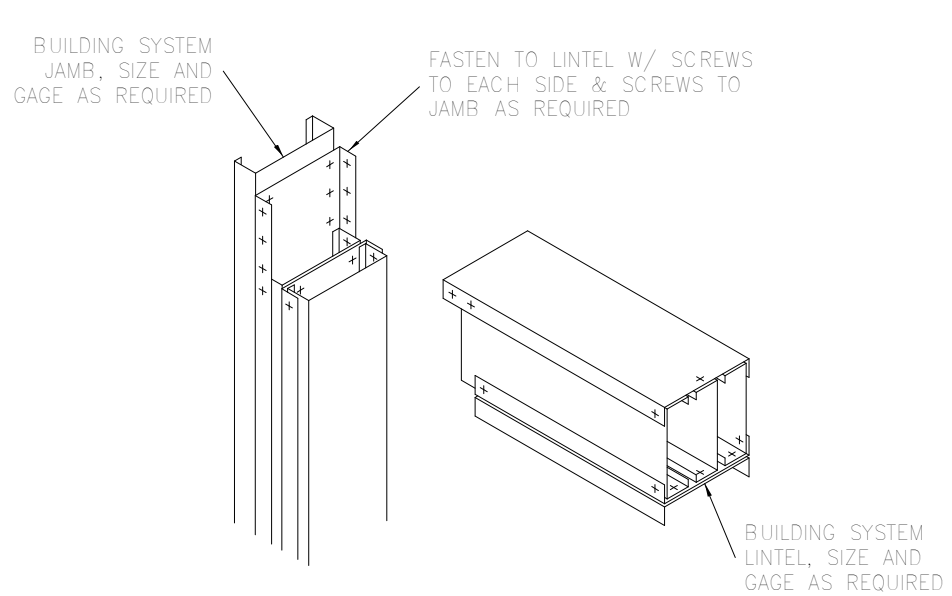
LOAD BEARING WINDOW OPENING ELEVATION



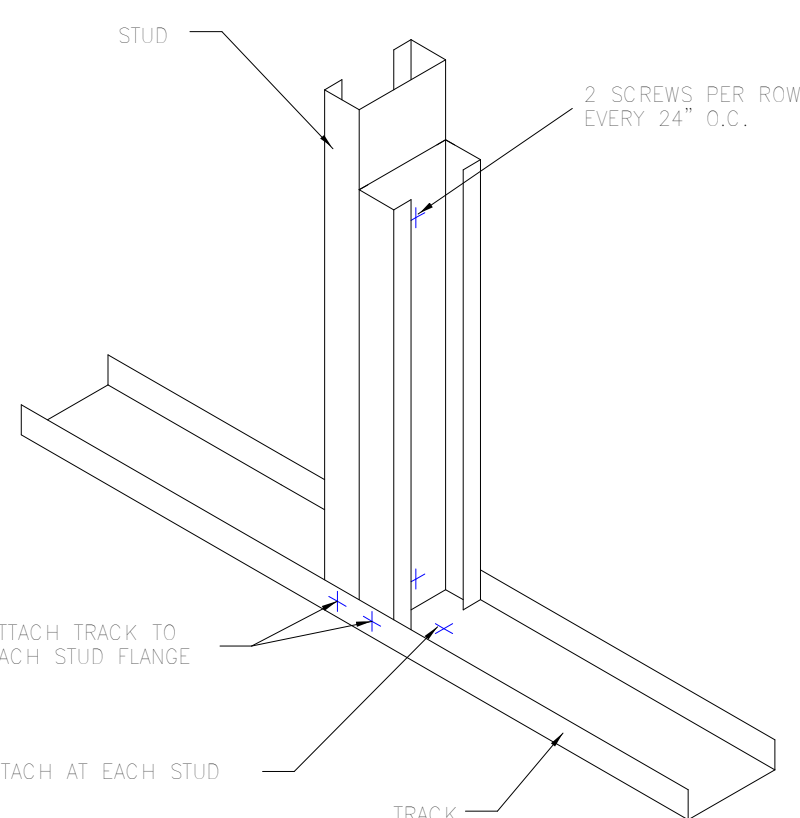
LOAD BEARING EXTERIOR



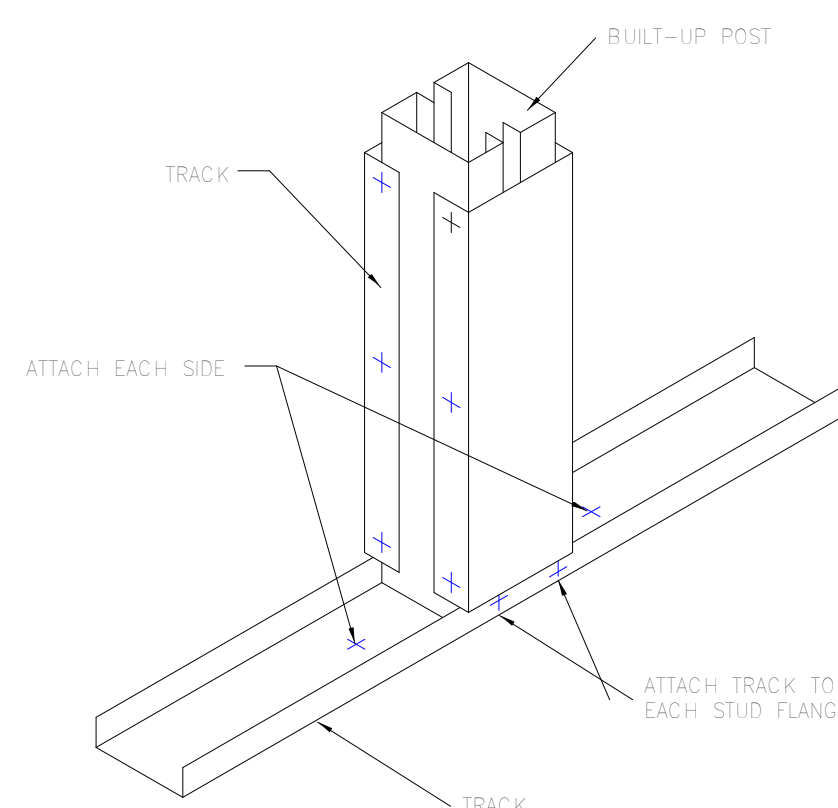
LOAD DISTRIBUTION HEADER



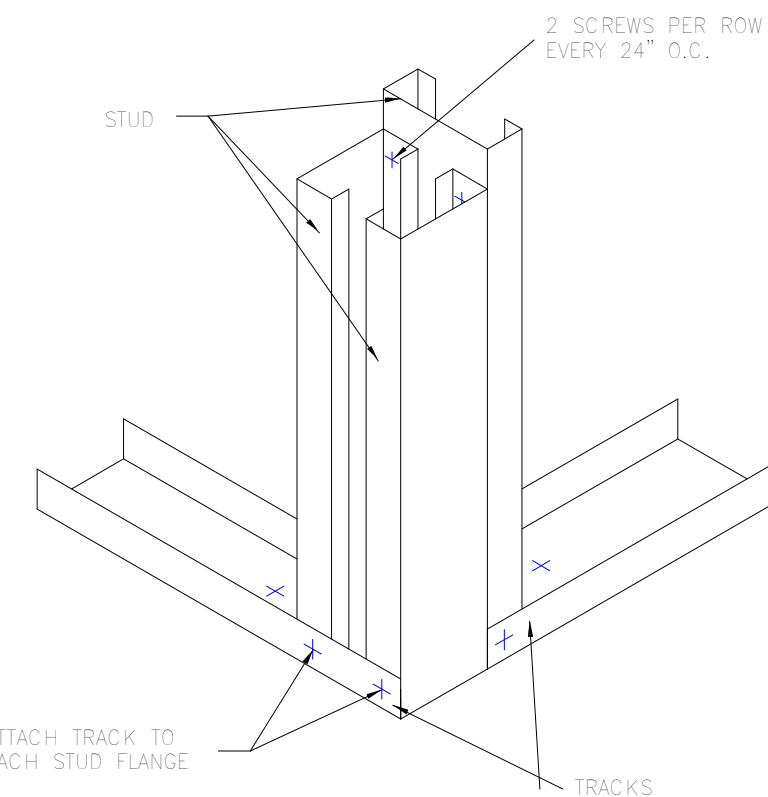
LOAD BEARING BOX HEADER LINTEL



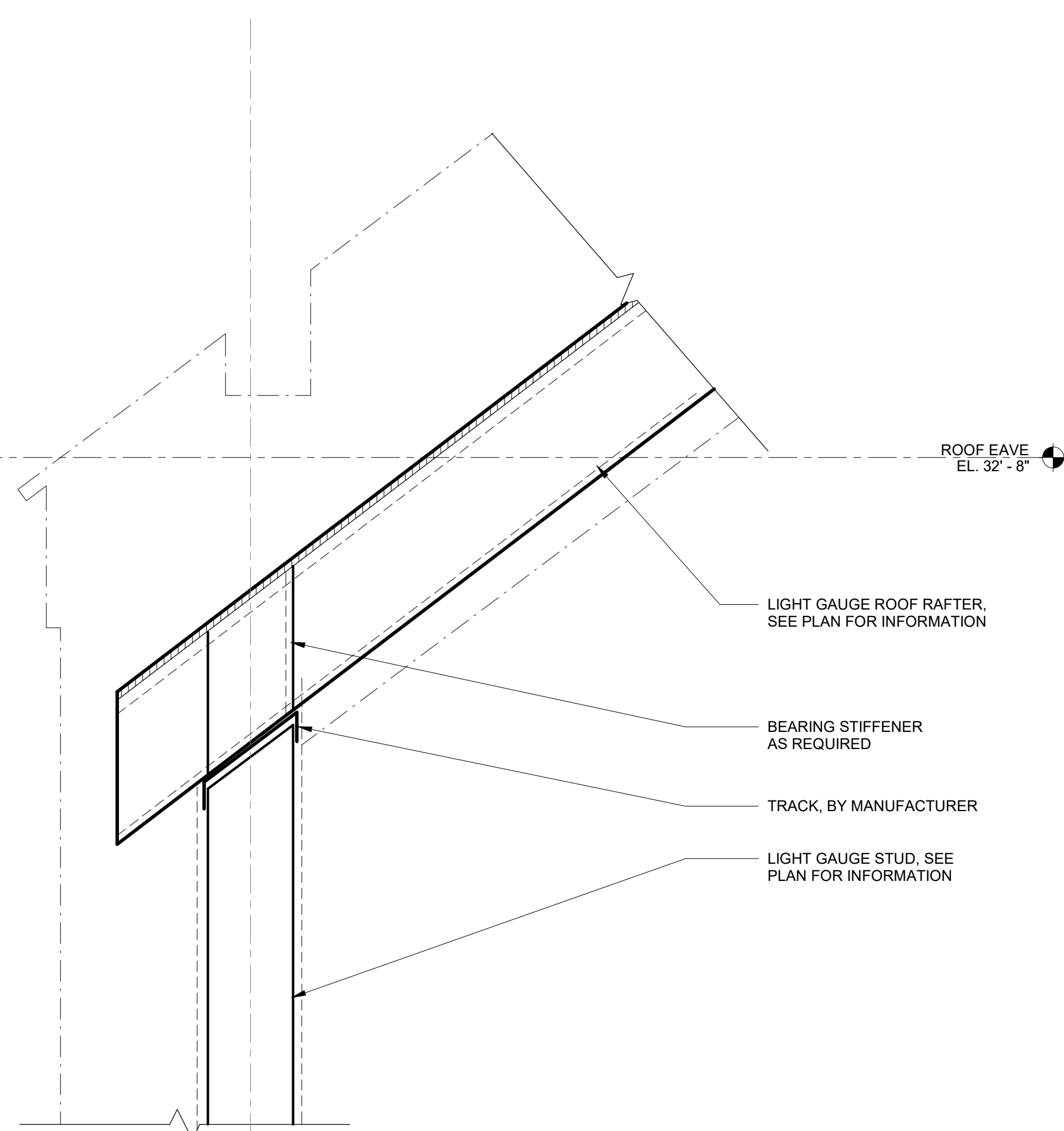
BACK-TO-BACK STUDS



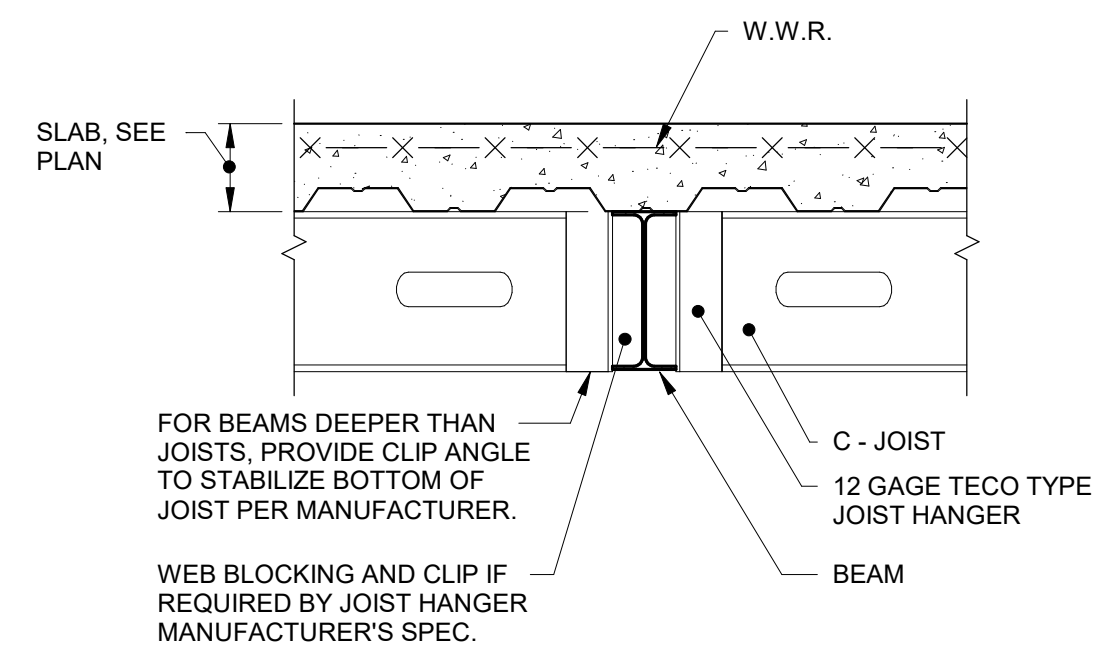
BUILT-UP POST



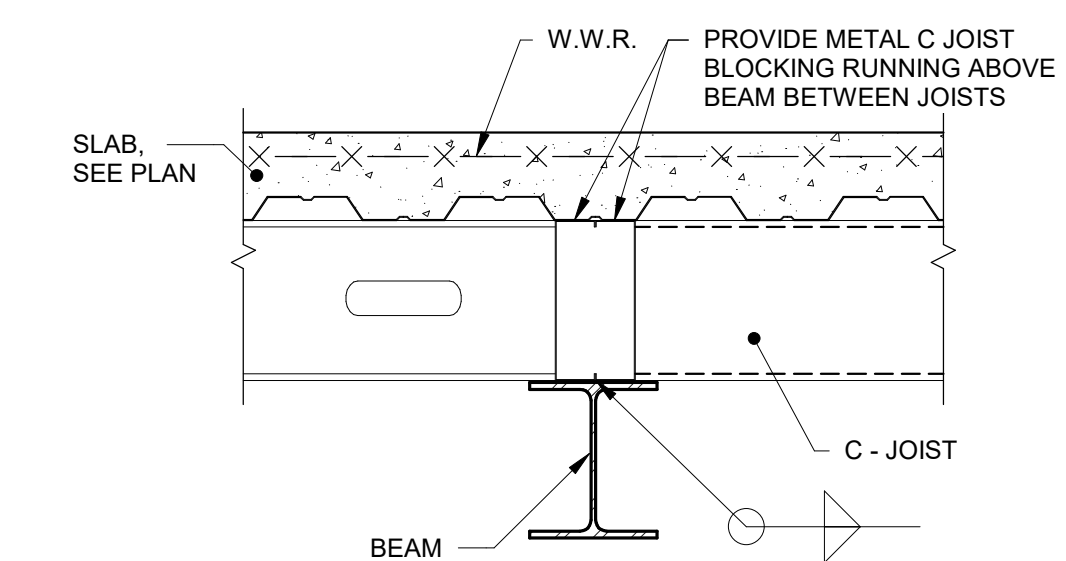
THREE-STUD CORNER



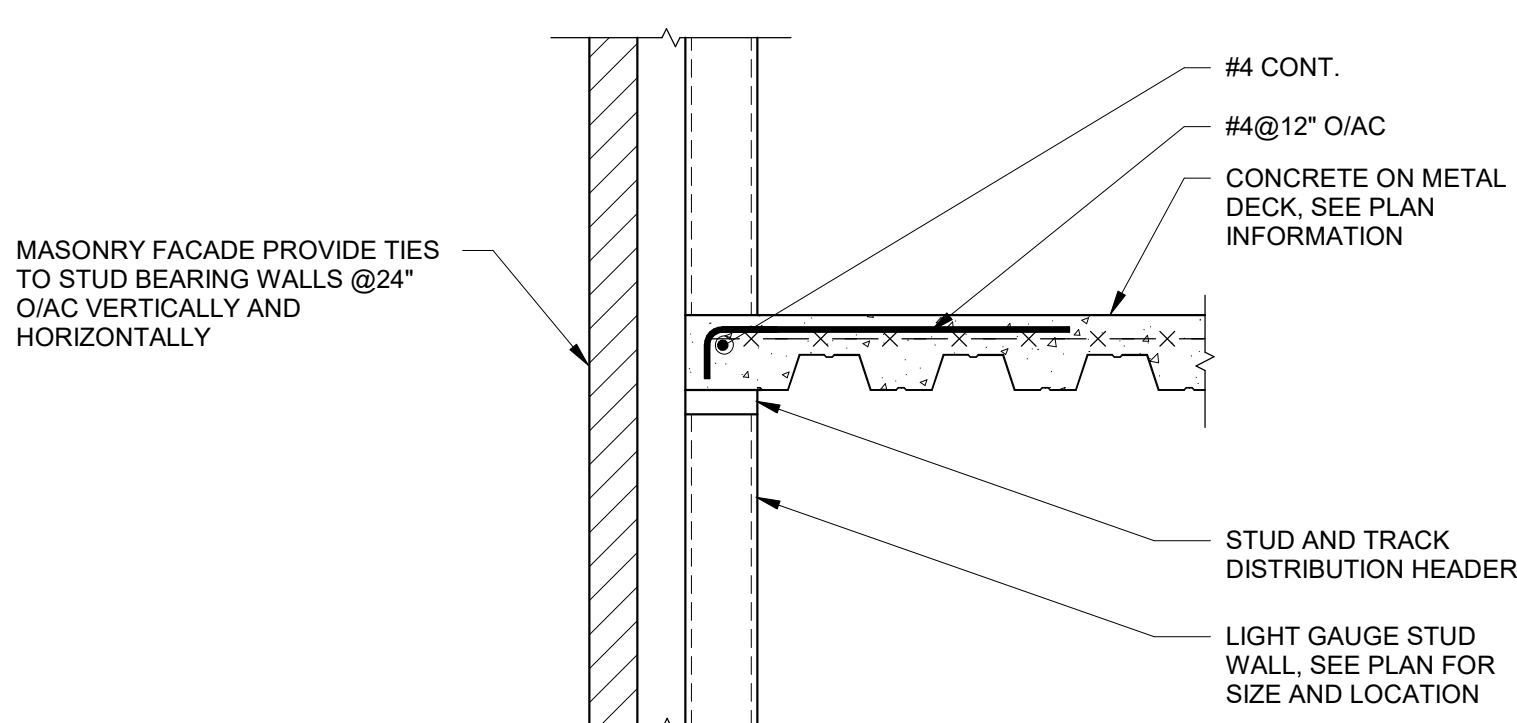
TYPICAL ROOF JOIST SUPPORT ON BEARING WALL
N.T.S.



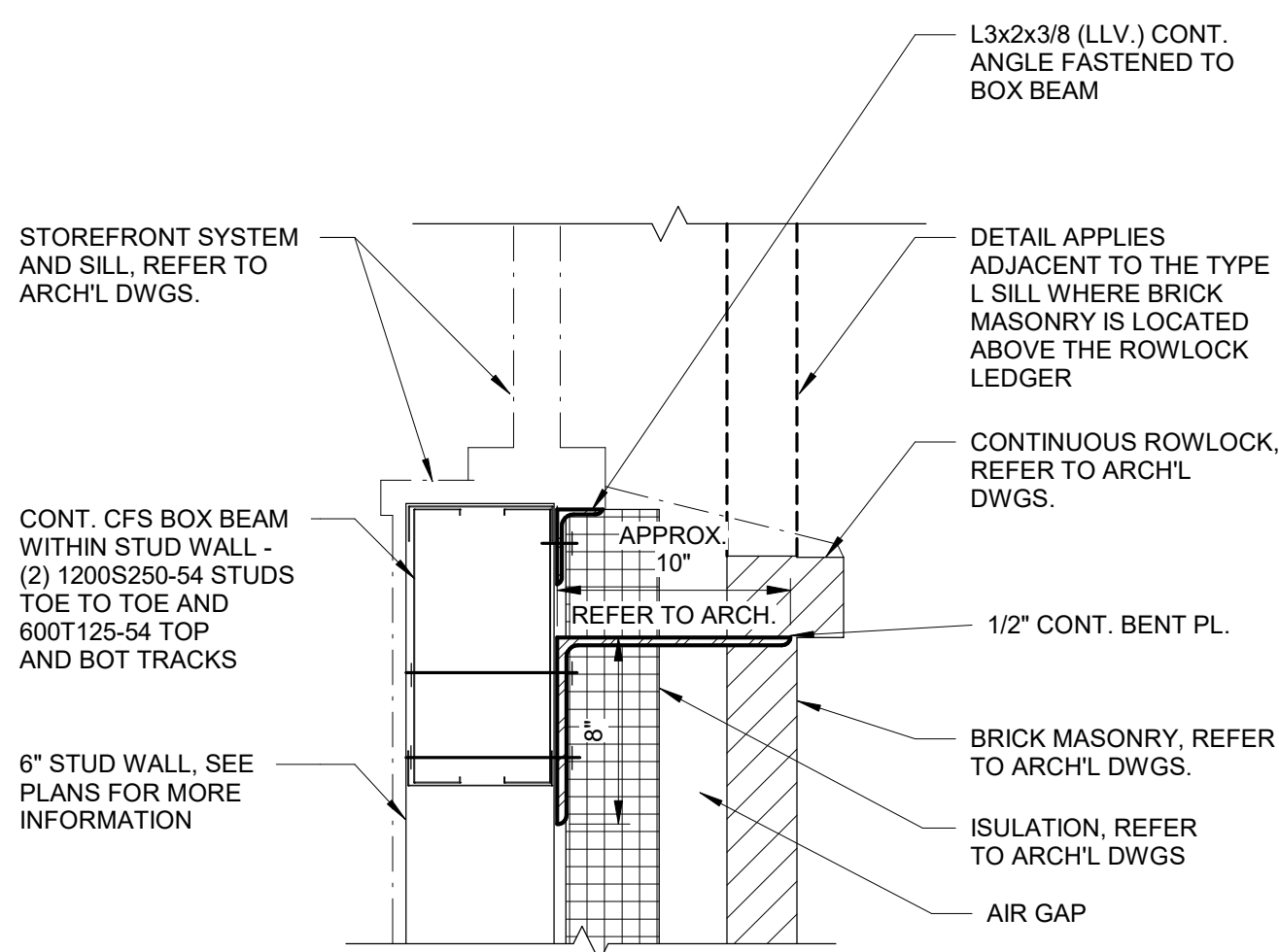
WHERE BEAM IS ANY DEPTH



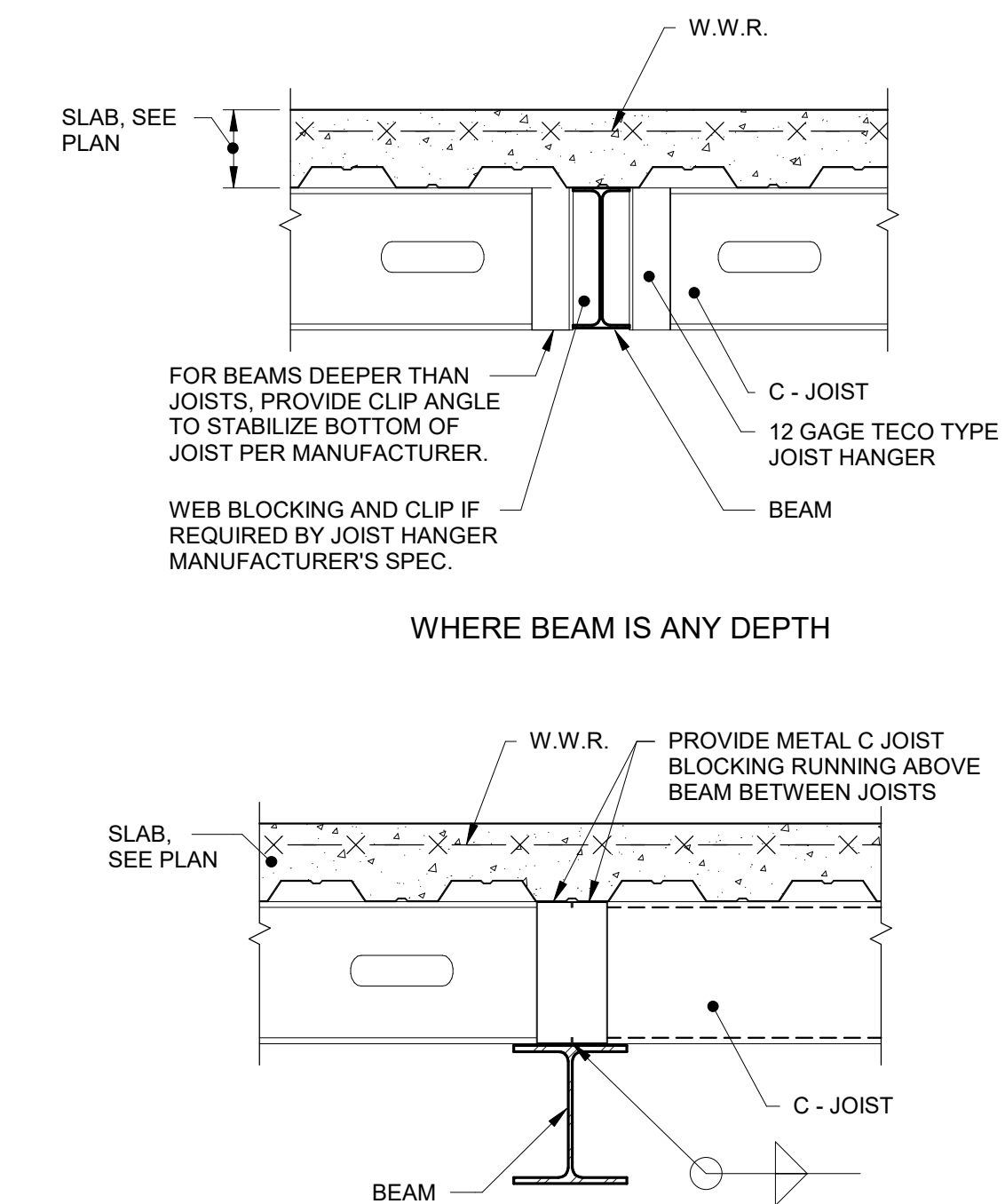
WHERE BEAM IS ANY DEPTH



TYPICAL DETAIL EXTERIOR BEARING WALL AT INN
N.T.S.



TYPICAL BENT PL. CONNECTION TO BOX BEAM (TYPE L)
N.T.S.



TYPICAL C JOISTS SUPPORTED ON STEEL BEAM
N.T.S.

