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d. e. DESIGN DAT/ Building Code Floor Load: Dead Live Lobb 1st F Stair Mecl Roof Load: Dead Live Snov Lateral Soil Lo Soil At-R Activ Wind Load: Earthquake De Build Seis Map Spec Seis Basi Desi Seis Basi Desi Seis Basi	Architectural of when in confli- structural men Specifically tit indications of TA de = 2018 Internationa ead Load 6" Thick Conc Precast plank ve Loads (reduced as bbies = 100 PSF t Floor Corridors = 100 echanical Rooms = 110 echanical Rooms = 110 cond Snow Flat Roof Snow Snow Exposu Snow Load Im	Irawings, if applicable, shall take prece ct with Mechanical and Structural draw nbers. led drawings and sections of the speci items in a collateral way. I Building Code rete composite deck = 95 PSF (include = 150 psf (includes 100 psf plank weig permitted by Code): I0 PSF 0 PSF 50 PSF 15 PSF; Bottom Chord 10 PSF (include Load May Control)	edence in regard to dimensions, vings, except for the size of ifications take precedence over es structural weight) ght)	15. 16. 17. 18. 19.	Specifications, water may be added only if ne the maximum slump is exceeded. The water least half of the total mixing required. Dischar hours, or before the truck drum has revolved a introduction of the mixing water to the cement aggregates. Truck batch slips must include tir site, and quantity of water (in gallons) per cub design water-cement ratio. Reinforcing steel shall be deformed, billet-stee reinforcing steel must be approved by the CO requirements of the AWS Code, D1.4, Structu be performed only by qualified welders. When specified, welded wire fabric shall confe chairs are required at the recommended spac Reinforcing steel splices shall be ACI Class E Provide adequate bolsters, hi-chairs, support length of all reinforcing bars. Provide accesso up ends for reinforcement at all faces of expo Shop drawings for the fabrication and placem	wither the maximum shall be incorpora ge of the concrete 300 revolutions, wit and aggregates of me of batching, tot bic yard available to el conforming to <i>A</i> OR and shall be per ural Welding Code form to ASTM A188 sing for structural s a splices unless ind bars, etc., to main pries which are pla sed concrete, inter	n permissil ted by add shall be o hichever o or the intro cal drum re o be adde ASTM A61 rformed in - Reinforc 5 and supp slabs. dicated oth tain special stic tipped rior or exte
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Roof Load: Dead Live Snow Live Snow Lateral Soil Lo Soil At-R Activ Wind Load: Basi Build Expo Inter Wind Net U Earthquake De Build Seis Map Spec Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi	ead Load = Top Chord ve Load = 20 PSF now Load: (Roof Live I Ground Snow Flat Roof Sno Snow Exposu Snow Load Im	15 PSF; Bottom Chord 10 PSF (inclue	des structure weight)			ionic of Formoroning c	
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Soil At-R Activ Wind Load: Basi Build Expo Inter Wind Net I Earthquake De Build Seis Map Spec Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi	Flat Roof Sno Snow Exposu Snow Load Im	Load Po = 30 PSF			special inspection agency or the COR. Contra concrete placement.		
Soil At-R Activ Wind Load: Basi Build Expo Inter Wind Net I Earthquake De Build Seis Map Spec Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi	Snow Load Im	w Load, Pf = 25.4 PSF		21. 22.	Schedule the pouring of foundations on the sa Cure all concrete in accordance with accepted		
Soil At-R Activ Wind Load: Basi Build Expo Inter Wind Net I Earthquake De Build Seis Map Spec Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi	Thormal East	re Factor, Ce = 1.0 portance Factor, Is = 1.10			Standard Specification for Curing Concrete, la anticipated during the curing period. All concr	atest edition, for th	e worst ca
Soil At-R Activ Wind Load: Basi Build Expo Inter Wind Net I Earthquake De Build Seis Map Spec Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi		or, Ct = 1.10		23.	requirements of ACI 306.1, Standard Specific No aluminum of any type shall be allowed in t		
Activ Wind Load: Basi Build Expo Inter Wind Net I Earthquake De Build Seis Map Spec Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi Desi Seis Basi	oil Unit Weight = 120 F	ĊF		24.	concrete reaction. Mechanically vibrate concrete. Slabs-on-grad	e need only be vib	orated arou
Basi Builo Expo Inter Wino Net I Earthquake De Builo Seis Map Spec Seis Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	-Rest Pressure = 40 F stive Pressure = 60 PS			25.	embedded items. Do not place pipes, ducts, reglets or chases in	n structural concre	ete or com
Build Expo Inter Wind Net I Earthquake De Build Seis Map Sped Seis Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	asic Wind Speed (3-se	cond gust) = 122 MPH		26. 27.	approval of the COR. Maximum free drop of any concrete = 5'-0". Chamfer all exposed concrete edges a minim	$y_{1} = \frac{1}{2} \frac{1}$	
Inter Wind Net I Earthquake De Build Seis Map Spec Seis Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	ilding Category = III posure Category = B	60114 gusty - 122 Wi 11		27. 28.	Provide a minimum of concrete cover for reinf drawings:1.) Footings = 3 inches earth face, 2	forcing bars as foll	
Net I Earthquake De Build Seis Map Spec Seis Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	ernal Pressure Coeffi	cient, GCpi = ±0.18 MWFRS) = 25 PSF (1.0W)		29.	and piers = 2 inches earth face, 1-1/2 inches Six (6) test cylinders shall be prepared for ever	all others, U.N.O.	
Build Seis Map Sped Seis Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas		See Load Diagram. Min. Net Uplift load	i = 10 PSF	20.	to represent all concrete placed on that day. and three (3) cylinders shall be tested at 28 d	Three (3) cylinders	s shall be
Map Spec Seis Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	ilding Category = III			30.	Concrete Finishes, as defined in Chapter 5 of surfaces not exposed to public view: Rough-fu	ACI 301, shall be	as follows
Seis Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	eismic Importance Fac apped Spectral Respo				Smooth-form finish. 3.) Walls as indicated: Ru floor: Troweled finish 5.) Floors to receive cer	, <b>.</b>	•
Seis Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	Ss = 0.286 S1 = 0.061			31.	<ol> <li>Exterior slabs on grade, walkways, etc.: N Provide vertical control joints in concrete wall:</li> </ol>	s at a maximum 30	0'-0" O.C.
Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	ectral Response Coe	: (PER GEOTECH REPORT) fficients:			joint location plan to the Engineer for approva in concrete shear walls.		
Basi Desi Seis Resp Anal Blast Loading: All b (PSF Octo Blas	Sds = 0.300 Sd1 = 0.098			32.	Concrete submittals required to be reviewed a construction include, but are not limited to: Mi	ix designs, Reinfor	
Seis Resp Anal Blast Loading: All b (PSF Octo Blas	eismic Design Categor asic Seismic Force-Re	sisting System = Structural Steel not s	specifically designed for seismic Gauge X-braced shear walls.	33.	Plans for Walls, and Control Joint Plans for Sl Slabs on Grade:		a marile
Resp Anal Blast Loading: All b (PSF Octo Blas	esign Base Shear = 12	6 kips (0.7E For ASD Load Combinati Combinations)			<ul> <li>a. Slabs on grade shall be placed in a</li> <li>b. Slab finishing operations shall meet</li> <li>Chapter 8 of ACI 302. Floors received</li> </ul>	t ACI Class 5 floor	requireme
Anal Blast Loading: All b (PSF Octo Blas	eismic Response Coef esponse Modification I	ficient, Cs = 0.114			floor requirements. c. Refer to the floor flatness requirement	-	
All b (PSF Octo Blas	•	quivalent Lateral Force Procedure			<ul> <li>c. Refer to the hoor namess requirements section 3.13.11.</li> <li>d. Curing of slabs shall begin as soon</li> </ul>		
(PSF Octo Blas		all be designed per the Physical Secur	rity and Resiliency Design Manual		curing, wet covering, or liquid meml Chapter 9 of ACI 302.1R for a perio	brane-forming curi	ng compo
	SRDM) for blast criter ctober 1,2020.	a and requirements for Life-Safety Pro	otected (LSP) Facilities Dated		e. Sawcut control joints must be comp depth of 1/3 the slab thickness (1/4	pleted within 1 to 4	hours of f
Basi	All building str	pject Design Narrative section 3.5 for a ucture shall resist min. vehicle level th	nreat of (W1) at 47 ft standoff.		maximum 15'-0" O.C. spacing Subr approval.	nit a sawcut joint le	ocation pla
	asis of design Informat	ocks, Mailrooms, and Lobbies shall res ion is included in the structural drawir	ngs for convenience to the		f. Slab joint sealant shall be a two-cor of 80 in accordance with ASTM D 2	240.	
	ements:	ect Narravtive for basis of design loads	s incluaing but limited to the following		g. Do not allow construction equipmer days.		
	Exterior Metal				<ul> <li>h. When strip pouring, allow seven (7)</li> <li>i. A slab on grade pre-construction co</li> </ul>	onference involving	g the COR
	Masonry Wall				recommended to review installation	procedures and re	equiremer
	Concrete Floo above noted asterisks	r and Roots s the contractor is responsible for the and shall submit signed and sealed c		Г			MAX.
proje		S. Davidson has ensured these elemeters to confidential signed and sealed co	ents meet or exceed typical building		TYPE OF CONSTRUCTION	MIN. 28-DAY COMP.	W/C RATIO
	<sup>r</sup> the noted blast loads oject specifications. C	ce the contract is awarded for additiona		┝		STRENGTH	
Roof	the noted blast loads oject specifications. C de requirements. Refe	gn:		•	<ol> <li>Footings, interior slabs-on-grade &amp; slabs-on- netal deck</li> </ol>	3,000 PSI	0.53
	the noted blast loads oject specifications. C de requirements. Refe	RAMP-UP/RAMP-DOWN	RAMP-UP/RAMP-DOWN DYNAMIC BLAST LOADING		<ol> <li>Exterior slabs, walls, beams, columns, piers</li> <li>grade beams</li> </ol>	4,500 PSI	0.45

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otherwise indicated, they do not indicate the means or methods of construction. Provide all measures necessary to protect the workmen and other persons during construction. Provide all necessary measures to avoid excessive stresses and to hold the structural elements in place during construction. Such measures shall include, but not be limited to, bracing, shoring for construction equipment, shoring for earth banks, forms, scaffolding, planking, safety nets,

support and bracing for cranes and hoists, guying, etc. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used. Observation visits to the site by structural engineer's field representative shall not include the items noted above.

Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. Retain the services of a professional structural engineer licensed in the state in which the project is located to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

Revisions:	Date:

1

PRESSURE = 7.81 PSI

TOTAL DUR. = 10.49 MSEC

IMPULSE = 27.94 PSI-MSEC

RISE TIME = 3.58 MSEC

TOTAL DUR. = 7.15 MSEC

PRESSURE = 2.07 PSI

RISE TIME = 16.30 MSEC

PRESSURE = 233.12 PSI

IMPULSE = 182.39 PSI-MSEC

TOTAL DUR. = 1.56 MSEC

TOTAL DUR. = 41.96 MSEC

IMPULSE = 33.75 PSI-MSEC

TOTAL DUR. = 32.61 MSEC

LOW ROOF

Lobby Blast Loading Design:

LOBBY

Concrete Design Method:

Design per LRFD

Structural Steel Design Method: Design per ASD

Loads indicated are ASD loads

Loads indicated are ASD loads Cold-formed Steel Framing Design Method: Design per ASD

Loads indicated are ASD loads

F

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3		4	5	6
uilding Code Requirements for	STRU( 1.	ICTURAL STEEL All steel construction shall be in accordance with the Specification for Structural Steel Buildings,	STEEL	L STAIR DESIGN
or Structural Concrete for Buildings,	1.	July 7, 2016 (AISC 360), the Specification for Structural Joints Using High-Strength Bolts, August 1, 2014, the Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016 (AISC 303), and	1.	The stair supplier shall be responsible for the design of all stairs, landings, and connections, unless noted otherwise. Drawings and calculations sealed by a structural engineer licensed in the
ant in accordance with the	2.	the AISC Steel Construction Manual, 15th edition. All welding shall be performed in accordance with the requirements of the AWS Code, D1.1,	2.	state in which the project is located shall be submitted to the COR for approval. Stairs & landings shall be designed for a minimum live load of 100 PSF in addition to any
exterior slabs, all concrete exposed	Ζ.	Structural Welding Code - Steel, latest edition, and AISC Specifications using the proper electrode	3.	applicable dead loads; including their self-weight. Contractor shall coordinate the steel stair design with masonry wall construction to ensure proper
gregate size = 1.5"		from AWS D1.1 Table 3.1 and performed only by qualified welders. Complete Joint Penetration (CJP) welds shall be tested per AISC's 15th Edition Steel Manual, Part 16, Chapter N, Section N5.5.		bearing conditions & attachments are provided.
hitted without Engineer's approval.	2	The Risk Category for this building is III. As indicated in AISC's 15th Edition, 100% of the CJP welds shall be tested. This work should be coordinated by the Contractor with the firm hired by the Owner.	COLD 1.	-FORMED STEEL FRAMING All structural members shall be in conformance with the Specification for the Design of Cold-Formed
gineer. )s.	3.	Structural steel shall conform to the requirements of ASTM A992, with a minimum yield stress of 50 ksi for all wide flange and WT shapes. All other shapes, miscellaneous plates, and connection		Steel Structural Members, AISI, latest edition. Manufacturer shall verify all design applications of framing members shown on the drawings.
opolymer polypropelene fibrillated	4.	material shall conform to ASTM A36. Square or rectangular structural steel sections (HSS) shall conform to the requirements of ASTM	2.	Cold-formed steel supplier shall prepare complete structural design calculations for all framing members and accessories. Framing shall be designed for the loads listed on the drawings and all
m rate of 1.5 lb/cu.yd. 2.) Fibrous approved equal.	5.	A500, Grade B, with a minimum yield stress of 46 ksi. Round hollow structural steel sections (HSS) shall conform to the requirements of ASTM A500,		applicable building code requirements. All exterior cold-formed framing shall be design for the blast requirements indicated by section 08 56 53 blast resistant facade systems. Framing shall also be
erature and shrinkage a shall be added at a minimum rate	6.	Grade B, with a minimum yield stress of 42 ksi. Structural steel pipe shall conform to the requirements of ASTM A53, Type S, Grade B, with a	3.	designed to meet the deflection criteria listed in items 3, 4, & 5. Interior and Exterior Walls: Horizontal deflection due to wind loas shall not exceed L/360 of the wall
lefin shall have an equivalent length / equivalent diameter) of 50,	7.	minimum yield stress of 35 ksi. All galvanized structural steel shall conform to the requirements of ASTM A123.All galvanizing holes	-	height unless supporting masonry veneer, in which case deflection shall not exceed L/600 of the wall height.
ement shall be manufactured by -screeded, U.N.O.	8.	shall be plug welded or sealed with zinc plugs. Shop drawings for the fabrication and erection of all structural steel shall be submitted to and	4.	Floor Joists or Trusses: Vertical deflection shall not exceed L/360 of the span for Live Load, and L/240 of the span for Total Load.
shrinkage reinforcement, cold- a minimum rate of 25 lb. / cu. yd. or	9.	approved by the COR prior to fabrication. The Contractor shall notify the COR of any fabrication or erection errors or deviations and receive	5.	Roof Joists or Trusses: Vertical deflection shall not exceed L/360 of the span for Live Load, and L/240 of the span for Total Load. (If members do not support a plaster or drywall ceiling, deflection
equivalent diameter between 0.016" ter) of 50, and added at a minimum	10.	written approval before field corrections are made. Fabricator and erector of structural steel shall be AISC certified. A current copy of the AISC	6.	shall not exceed L/240 of the span for Live Load, and L/180 of the span for Total Load). Cold-formed steel trusses shall be designed for the following loads indicated in items 7-11:
red by Propex, BASF, Grace, or		certificate shall be submitted to the COR prior to fabrication and erection. If not certified by AISC, fabricator shall be subjected to the special inspection requirements.	7.	All wind loads and snow loads, including balanced, unbalanced, drift, & sliding snow, shall be determined using the 2018 International Building Code (IBC).
orcement, U.N.O. ag, or other pozzolans may be	11.	Bolted connections of structural steel shall be made using high strength steel bolts conforming to ASTM A325. Bolts shall be 3/4" minimum in diameter unless noted otherwise.	8. 9.	Minimum Top chord live load = 30 psf Top chord dead load = 15 psf
ntent. Concrete mixes using these	12.	The faying surfaces at slip-critical connections shall be Class A surfaces, and shall not be painted or primed. Galvanized surfaces shall be roughened by means of hand wire brushing as indicated in	10. 11.	Bottom chord dead load = 10 psf All trusses shall be designed for the blast requirements indicated by section 05 40 00 cold-formed
le for placement, as indicated by the permissible water-cement ratio nor	13.	the 2004 RCSC specification. All steel connections shall be designed by a professional engineer licensed in the state in which the	12.	metal framing. Supplier shall submit the following items, at a minimum, for review: 1.) Product data. 2.) Shop
d by additional mixing equal to at hall be completed within 1-1/2		project is located. The design shall be in accordance with the AISC steel manual and AISC 360 specification referenced by the adopted Building Code for the project. The fabricator shall submit	12.	drawings showing framing, accessories, anchorages, and connection details. 3.) Structural design calculations sealed by a structural engineer licensed in the state in which the project is located. 4.)
chever comes first, after the the introduction of the cement to the		sealed connection design calculations for all shear, moment, and bracing connections before shop drawings are submitted for review. Connection designs varying from typical details shown on the	10	Manufacturer's installation instructions for framing members and accessories.
drum revolutions upon arrival at be added to attain the maximum		construction documents shall be submitted with design calculations for approval prior to submitting show drawings. The licensed engineer responsible for the connection design shall review the steel	13.	All framing members shall be formed from corrosion-resistant steel corresponding to the following requirements: 1.) 16 Gage (54 Mils), 14 Gage (68 Mils), and 12 Gage (97 Mils) framing
TM A615 - Grade 60. Welding of		shop drawings and confirm in writing that the steel shop drawings have properly incorporated the connections as designed. The document shall be submitted along with the steel shop		components shall be formed from steel conforming to the minimum requirements of ASTM A653, Grade D, with a minimum yield stress of 50,000 psi. 2.) 18 gage (43 Mils) and 20 gage (33 Mils)
ormed in accordance with the Reinforcing Steel latest edition and	14.	drawings for the COR to review. Simple shear connections shall be standard double angle or shear plates, unless noted otherwise.		framing components shall be formed from steel conforming to the minimum requirements of ASTM A653, Grade A with a minimum yield stress of 33,000 psi. 3.) Galvanized framing products shall be
and supplied in flat sheets only. Bar	15.	All bolted connections shall have a minimum of 2 bolts. All high strength bolts shall be snug tight unless noted otherwise. A snug tight condition is as	14.	coated in accordance with the requirements of ASTM A525. When masonry veneer is anchored to cold-form steel backing, framing shall be corrosion resistant
bs. cated otherwise.		defined in Section 1.2.2. in the AISC Design Guide 17. Bolts used in slip-critical connections shall be pretensioned in accordance with the 2004 RCSC specification.	15.	and have a minimum base metal thickness of 0.043 in. (43 mils 18 gage). Connections of all framing components shall be with self-drilling screws or welding. When
in specified clearance for the entire ic tipped or galvanized with turned-	16.	Anchor bolts shall conform to the requirements of ASTM F1554, with a minimum yield strength of 36 ksi, unless noted otherwise. Bolts shall be 3/4" minimum diameter unless noted otherwise. All		connecting materials greater than 1/4" in thickness, holes shall be pre-drilled prior to installation of fastener. Screws shall be of sufficient size to insure the strength of the connection. Wire tying of
or or exterior.	17.	columns shall have a minimum of 4 anchor bolts unless noted otherwise. The Contractor shall be responsible for the temporary bracing of all steel during erection and until		framing components shall not be permitted. All welds shall be touched up with paint conforming to the manufacturer's requirements.
and mix designs by the acting	18.	construction is complete. The Contractor shall furnish all plates, clip and seat angles, and connections for completion of the	16.	All welding shall be performed in accordance with the requirements of the AWS Code, D1.3, Structural Welding Code - Sheet Steel, latest edition, and shall be performed only by qualified
for coordinating inspections prior to	19.	structure, even if such items are not specifically called for on the structural drawings. Structural and miscellaneous steel fabricators shall be responsible for obtaining all field dimensions	17.	welders. End bearing and bridging details shall be as stipulated in the AISI Specifications and as shown on
tion is completed. d conformance to ACI 308.1,	20.	necessary for the completion of their work. Minimum size of fillet welds, unless otherwise noted, is to be 3/16-inch fillet. Chip, wirebrush clean,	18.	the drawings. Field cutting of steel framing members shall be by saw or shear. Torch cutting will not be permitted.
worst case weather conditions I procedures shall conform to the	20.	and prime paint all field welds. Summary of Paint System (Reference Steel Structures Painting Council - A Guide to the Shop	19. 20.	Temporary bracing shall be provided and remain in place until work is permanently stabilized. Installation - Stud Wall Systems:
her Concreting, latest edition.	۷۱.	Painting of Structural Steel): a. For all work temporarily exposed to weather for six (6)		<ul><li>a. Provide web stiffeners at support locations where indicated.</li><li>b. Mechanical bridging of the type and spacing described shall be installed before loading.</li></ul>
nted around floor ducts and other		months or less : No paint required.		<ul><li>c. End blocking shall be provided where joist ends are not restrained against rotation.</li><li>d. Provide additional framing around openings when the width of the opening exceeds the</li></ul>
		<ul> <li>For all work temporarily exposed to weather for greater than six (6) months:</li> </ul>		typical joist spacing. e. During construction, the joist system shall not be loaded beyond the limits for which it was
or composite floor systems without		aa. Surface Preparation = SSPC-SP 2 bb. Pretreat = None required		designed. f. Minimum end bearing shall be 1 1/2".
		cc. Paint Application = SSPC-PA 11 dd. Number of Coats = One	21.	Installation - Horizontal Joists: a. Where splicing of track is necessary between stud spacings, a piece of stud shall be
vs unless noted otherwise on J.N.O. 2.) Concrete walls, beams,		ee. Primer = SSPC-Paint 15 ff. Touch-up = As per manufacturer specifications		placed between adjacent tracks and fastened by weld or screw to each side of the track, each end.
f concrete placed on any given day		c. For all work permanently exposed to weather: aa. Surface Preparation = SSPC-SP 3		<li>b. Splicing of framing components, other than track or multi-story stud walls, is not permitted.</li>
shall be tested at seven (7) days f concrete design strength.		bb. Pretreat = None required cc. Paint Application = SSPC-PA 1		<ul> <li>Studs shall be plumbed, aligned and secured to the continuous runner tracks at each end and each side.</li> </ul>
s follows, U.N.O.: 1.) Formed d surfaces exposed to public view:		dd. Number of Coats = Three minimum ee. Primer = SSPC-Paint 25		d. Mechanical bridging of the type and spacing described shall be installed prior to the installation of facing materials. Minimum spacing of bridging shall be 4'-0", unless
cal interior commercial/industrial ement, terrazzo, etc.: Scratch finish		ff.       Touch-up = As per manufacturer specifications         gg.       2nd Coat = SSPC Paint 21         Finish Operate SOPO Desire 21		indicated otherwise by light-gauge framing engineer. e. Installation of sheathing, wallboard or any other collateral material shall be performed in
0" O.C. spacing. Submit a control		hh. Finish Coat = SSPC Paint 21 ii. Dry Film Thickness =		accordance with the product manufacturer's specification
n. Control joints shall not be placed		First Coat: 1.5 to 2.0 mils. Second Coat: 20. mils	PREC	AST CONCRETE FLOOR AND WALL PANELS
COR prior to cast-in-place concrete ement Shop Drawings, Control Joint		Finish Coat: 2.0 mils Total = 5.5 to 6.0 mils	1.	Planks shall be pretensioned, solid or hollow core, flat slabs machine cast by a member of the Precast Concrete Institute and designed for the loads listed on the drawings and in accordance with
		d. Surfaced within 2 inches of welds shall be free of material that would prevent proper welding or prouce objectionable fumes while welding is being done.	2.	Precast Concrete Institute standards. Shop drawings & sealed calculations prepared by a structural engineer licensed in the state in which
equirements of ACI 302.1R. equirements in accordance with		e. For Structural steel that will receive sprayed fireproofing: no primer or finish paint. If steel is painted, a proper bonding agent must be applied to		the project is located shall be submitted to the COR for approval prior to fabrication and shall include all material strengths and specifications.
ner must be finished to ACI Class 6		ensure proper adhesion of the sprayed fireproofing.	3. 4.	See drawings for thickness and spanning directions. All grout keys shall be properly filled for full length and properly tied into bearing points as detailed
st in Place project specification	FOUN	IDATION CONSTRUCTION Allowable soil bearing pressure (net) assumed in design is 3,000 PSF (pounds per square foot)	5.	on the drawings. Fabricate plank to a length tolerance of $\pm 1/2$ inch.
ons are complete by either water g compound in accordance with	1.	based on the geotechnical engineering investigation prepared by Terracon, Project No. J6205175 dated 11/30/2020.	6.	Provide openings in precast for all penetrations. No field cutting of plank for openings will be permitted. For openings up to 8 inches in diameter field drilling or coring may be allowed provided
ours of finishing and to a minimum	2.	Soil bearing capacity shall be field verified by an approved soil testing agency and documented in writing to the COR.		that all such openings be marked and approved by the precast manufacturer prior to any drilling or coring. All openings over 8 inches in diameter must be shop-formed or framed.
ed wire fabric). Provide joints at a ation plan to the COR for	3.	If soil of design capacity is not encountered at footing elevations shown, excavate to a depth necessary to attain design capacity and extend foundations as required. Obtain COR approval	7.	Design reinforcement for any plank with field-drilled or cored openings to support all dead and live loads with any one strand cut.
n with a minimum shore A hardness	Λ	prior to lowering the foundations. The bottoms of all exterior footings shall extend a minimum of 3'-0" below finished grade unless	8.	Supply all headers required for plank support around openings as required.
labs for a minimum of seven (7)	4. 5.	noted otherwise.	META	L DECKING
adjacent slab sections.	5.	Excavate all foundations to reasonable exact outline and depth avoiding over-excavation and cave-in of surrounding material. Bottoms of all foundations shall be dry, soil proof-rolled, and rock	1.	Installation of all decking shall be in strict accordance with the Steel Deck Institute (SDI) specifications and with manufacturer's recommendations. Steel deck shall be manufactured by a
he COR Contractor is juirements.	6.	surfaces leveled and cleared of loose debris prior to pouring. Neatly level and trim all foundation excavations prior to setting steel.	2.	member of the Steel Deck Institute. All floor deck shall be galvanized and all roof deck shall be painted, U.N.O. Composite metal deck
	7.	Pipes extending under footings shall be placed before footing is placed and the void produced in laying the pipe shall be filled with 3,000 psi concrete.		for floor slabs shall have a min. yield strength = 40 KSI. Deck shall be a minimum 3-span continuous where possible.
MAX. TYPE 1 SLUMP	8. o	Thoroughly compact all foundation and slab subgrade material prior to placement of stone bases or concrete.	3.	Deck shall be attached to the steel structure using #12 screws at 12" o.c. with sidelaps fastened with #10 TEK screws at 12" O.C., U.N.O. Alternate fasteners must be approved by the COR.
W/C PORTLANDCE RANGE RATIO MENT	9.	Provide weathertight covering for all foundations after excavation; do not expose to rainwater or freezing.	4. 5.	Store and handle decking so as to prevent damage. Submit shop drawings to the COR for approval prior to installation.
	10. 11.	Where foundations are formed, backfill edges immediately after form removal. No fill or backfill shall be placed against retaining or foundation walls until grout or concrete has	5. 6. 7.	Provide a 1/4" thick bent plate or angle at all floor/roof edges U.N.O. Provide accessories as needed to provide a complete system. Contractor shall refer to
0.53 5 BAGS 3" - 6"		attained design strength and supporting members are in place, unless prior written approval is obtained from the COR.		Manufacturer's specifications for accessories for specialty deck types (e.g. Epicore, Arch Deck, etc.).
0.45 6 BAGS 3" - 6"	12.	All backfill shall be granular material approved by the COR. Backfill shall be deposited and machine compacted in 8-inch maximum layers. Compaction shall be a minimum of 95% of the	8.	etc.). All floor or roof deck penetrations > 6" (square or dia.) shall be properly reinforced with angle frames or slab reinforcing as specified by the Engineer.
0.10 0000 0-0	]	maximum dry weight density at the optimum moisture content in accordance with ASTM D1557 (Modified Proctor) as verified by a testing laboratory.	9.	All welding shall be performed in accordance with the requirements of the AWS Code, D1.3, Structural Welding Code - Sheet Steel, latest edition, and shall be performed only by qualified
	13.	All backfill adjacent to retaining structures shall be compacted using hand operated equipment; no heavy equipment shall be allowed within 5 feet of any wall.		welders.
finished structure. Unless	14.	For all foundation construction that will be exposed to freezing temperatures during construction, the bottom of all footings (interior and exterior) shall extend a minimum of 3'-0" below grade (at no		
f construction. r persons during construction		additional cost to the owner). Contractor shall contact the COR for approval to lower foundations		

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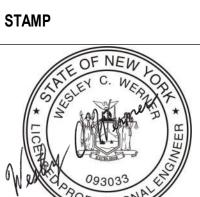
4

ARCHITECT/ENGINEER OF RECORD

<u>A/E:</u> TRIPLE C - The A/E Group 201 E. Jefferson Street, Suite 200 Syracuse, NY 13202 (315) 484-5958 Mat Perkins

and obtain concrete pier sizes, if necessary.





License valid

6



A MULTI-DISCIPLINE COMPANY

5

5/09/2022 through 11/30/2024

VA U.S. Depart of Veterans

CONCRETE MASONRY	CONSTRUCTION

7

6

All concrete masonry shall conform to the requirements of ASTM C90 unless noted otherwise. 1. Concrete masonry construction shall conform to the requirements of ACI 530/ASCE 5/TMS 402, building code requirements for Masonry Structures, latest edition and ACI 530.1/ASCE 6/TMS 602, Specifications for Masonry Structures, latest edition, including all supplements. 28 day design compressive strength of masonry (f'm) used is as follows: a.) Standard Masonry = 2. 2,000 psi. b.) Ivany = 2,800 psi The Contractor shall be responsible for the temporary bracing of all masonry construction. Bracing 3.

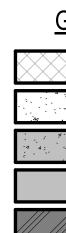
8

- shall remain in place until masonry has had sufficient time to gain design strength. Where masonry attaches to floor and/or roof systems, bracing shall remain in place until the diaphragm has been anchored in its final position. Grout lifts for reinforced masonry walls shall not exceed 5' -4" in height. High lift grout procedures 4.
- may be used only after written approval is provided by the Engineer. All mortar shall conform to ASTM C270. Mortar shall be Type S (2/1/9 mix ratio of 5. cement/lime/sand) with an average 28 day compressive strength of 1,800 psi for above grade walls (except Ivany Block). Mortar shall be Type M (4/2/12 mix) with an average 28 day compressive strength of 2,500 psi for Ivany walls, walls below grade, or walls exposed to earth.
- Grout shall conform to the requirements of ASTM C476. Fill grouted cores, bond beams, and wall 6. reinforcement where shown on plans with 3,000 psi small aggregate concrete mix with 8"-11" slump. Grout samples shall be tested per the Cast-in-place Concrete Construction specifications. All steel lintels for exterior masonry shall be galvanized.
- 8. Provide vertical control joints in above grade masonry walls so that the distance between joints does not exceed the lesser of 1 1/2 times the overall wall height, or 25 feet. Control joints shall not be placed greater than 8" from a bearing plate or jamb of an opening. A control joint shall be placed at one side of an opening less than 6'-0" wide and at both sides of openings over 6'-0" wide. Provide vertical control joints at intersections of reinforced & unreinforced walls, U.N.O. Submit a shop drawing showing control joint locations to the Engineer for approval prior to construction. Coordinate exact control joint & expansion joint locations with architectural drawings.
- Horizontal joint reinforcement shall consist of truss type ASTM A1064 cold drawn No. 9 gauge 9. galvanized wire steel, located 16" o.c. Cross rods to be welded to side rods at 16" o.c. to form a truss. Lap splices 6" minimum. Provide horizontal reinforcing in the two courses above and below openings and at the top of walls or partitions. 10. Masonry cores shall be grouted solid.
- Head joints shall be mortared solid the full depth of the face shell. Webs shall be fully mortared at 11. grouted cores. 12. A minimum of one course of masonry shall be grouted solid below transitions in masonry
- thickness. 13. Cold Weather Masonry Construction shall conform to the requirements of AC1 530.1/ASCE 6/TMS602, latest edition.
- Masonry stair shafts and elevator shafts shall be grouted solid, unless noted otherwise. 14. 15. Reinforcing steel shall be deformed, billet-steel conforming to ASTM A615 - Grade 60. Welding of reinforcing steel must be approved by the Engineer and shall be performed in accordance with the requirements of the AWS Code, D1.4, Structural Welding Code - Reinforcing Steel latest edition and be performed only by qualified welders. Shop drawings for the fabrication and placement of reinforcing steel shall be submitted to and approved by the Engineer before fabrication commences.
- Submittals required to be reviewed and approved by the Engineer prior to concrete masonry 16. construction include, but are not limited to: Mix designs, Reinforcement Shop Drawings, Vertical Control Joint Location Plans. Mortar shall be sampled and tested according to ASTM C 270, C 780, and C 1586.
- Wet setting of reinforcing steel is prohibited unless approved by the Engineer. 18.

	LEGEND OF ACR	ONYMS &	ABBRE
&	USE FOR "AND" WHEN APPROPRIATE	H.M.	HOLLOW
@	USE FOR "AT" WHEN APPROPRIATE	HORIZ.	HORIZO
A.C.T.	ACOUSTICAL CEILING TILE	HVAC	HEATING
ADDL.	ADDITIONAL	I.F.	INSIDE F
A.F.F.	ABOVE FINISHED FLOOR	INSUL.	INSULAT
ALUM.	ALUMINUM	INT.	INTERIO
ARCH.	ARCHITECTURAL	L.	LENGTH
BL	BUILDING LINE	L.L.H.	LONG LE
BLDG.	BUILDING	L.L.V.	LONG LE
BLK.	BLOCK	MAX.	MAXIMU
B.O.F.	BOTTOM OF FOOTING	MET.	METAL
BRG.	BEARING	MFG.	MANUFA
BTWN.	BETWEEN	MIN.	MINIMUN
C.	CARPET	MISC.	MISCELL
C.I.P.	CAST-IN-PLACE	N.I.C	NOT IN C
CL	CENTERLINE	NO.	NUMBEF
CLG.	CEILING	O.C.	ON CEN
CLR.	CLEAR	0.F.	OUTSID
CMU	CONCRETE MASONRY UNIT	PennDOT	
COL.	COLUMN	PL.	PLATE
CONC.		PLF	POUNDS
CONT.	CONTINUOUS	PNT.	PAINT
D. OR DP.		PWD.	POWDE
db	REINFORCING BAR DIAMETER	PSF	POUNDS
DIA.	DIAMETER	PSI	POUNDS
DIM.	DIMENSION	RAD.	RADIUS
DN.	DOWN	REQ.	REQUIR
DWG.	DRAWING	REINF.	REINFO
EA.	EACH	RET.	RETURN
E.F.	EACH FACE	SC	SLIP CR
ENG.	ENGINEER(S) / ENGINEERING	SECT.	SECTION
ELEC.	ELECTRIC / ELECTRICAL	SIM.	SIMILAR
ELEV.	ELEVATOR	SLR.	SEALER
EQ.	EQUAL	SPC.	SPACE
EQUIP.	EQUIPMENT	S.S.	STAINLE
E.W.	EACH WAY	STL.	STEEL
EX.	EXISTING	T&B	TOP ANI
EXT.	EXTERIOR	THK.	THICK
F.E.	FIRE EXTINGUISHER	T.O.F.	TOP OF
F.E./C	FIRE EXTINGUISHER WITH CABINET	T.O.S.	TOP OF
F.F.	FINISHED FLOOR	T.O.W.	TOP OF
F.G.	FINISHED GRADE	TYP.	TYPICAL
FIB.	FIBERGLASS	U.N.O.	UNLESS
FIN.	FINISH(ED)	V.	VINYL
FLR.	FLOOR	VAR.	VARIES
FTG.	FOOTING	V.C.T.	VINYL C
GA.	GAUGE	VERT.	VERTICA
GALV.	GALVANIZED	W.	WIDE / W
GYP.	GYPSUM	W/	WITH
GPDW	GYPSUM DRYWALL	WD.	WOOD
HGT.	HEIGHT	W.W.R	WELDED

9

DRAWING LIST				
SHEET NUMBER	SHEET NAME			
S-001	GENERAL NOTES			
S-002	WIND UPLIFT & SNOW DRIFT			
S-101	FOUNDATION PLAN			
S-102	ENLARGED FOUNDATION PLANS			
S-103	TUNNEL FRAMING PLAN			
S-104	ALTERNATE FOUNDATION PLAN			
S-105	FLOOR FRAMING & LOW ROOF FRAMING PLAN			
S-106	FLOOR FRAMING & LOW ROOF FRAMING PLAN			
S-107	ENLARGED FRAMING PLANS			
S-108	ENLARGED FRAMING PLANS			
S-109	PATIENT HOIST PLAN			
S-110	ROOF FRAMING PLAN			
S-111	ENLARGED FRAMING PLANS			
S-112	ISOMETRIC OF STRUCTURAL STEEL			
S-201	FOUNDATION DETAILS			
S-202	FOUNDATION DETAILS			
S-203	FOUNDATION SECTIONS & DETAILS			
S-204	FOUNDATION SECTIONS & DETAILS			
S-205	FOUNDATION SECTIONS & DETAILS			
S-301	FRAMING DETAILS			
S-302	FRAMING DETAILS			
S-303	FRAMING SECTIONS			
S-304	FRAMING SECTIONS			
S-305	FRAMING SECTIONS			
S-306	FRAMING SECTIONS			
S-307	FRAMING SECTIONS			
S-308	FRAMING SECTIONS			
S-309	FRAMING SECTIONS			
S-310	FRAMING SECTIONS			
S-401	CONNECTION INFORMATION			
S-501	LIGHT GAUGE STUD DETAILS			
S-502	LIGHT GAUGE STUD DETAILS			



"TEXT" (-2'-9") (-4'-9")

ce of ruction acilities	Drawing Title GENERAL NOTES	Phase ISSUED FOR CONSTRUCTION		Project Title NEW COMMUNITY LIVING CENTER			Project Number 620-334 Building Number CLC Drawing Number S-001	
gement Department eterans Affairs	Approved:	FULLY SPRINKLERED		Location2094 Albany Post Road, Montrose, NY 10548Issue Date05/09/2022WCWSIR				
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VIATIONS
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### OW METAL

ZONTAL ING / VENTILATION / AIR CONDITIONING E FACE ATION LEG HORIZONTAL LEG VERTICAL IM JFACTURER

#### /UM ELLANEOUS I CONTRACT

ENTER SIDE FACE ISYLVANIA DEPT. OF TRANSPORTATION IDS PER LINEAL FOOT

#### IDS PER SQUARE FOOT DS PER SQUARE INCH

IRED ORCING CRITICAL

## NLESS STEEL

AND BOTTOM F FOOTING STEEL

### WALL

SS NOTED OTHERWISE

COMPOSITION TILE

/ WIDTH

DED WIRE REINFORCING

# **GENERAL LEGEND**

$\mathbf{X}$	- REINFORCED MASONRY WALL
,	- CONCRETE
**************************************	- 2" DEPRESSED SLAB; REFER TO ARCH
	- 5" CONCRETE SLAB W/ TURNDOWN EDGES
	- SHEAR WALL LOCATIONS
	- PROPOSED CONDITIONS
	- EXISTING CONDITIONS
SIM	- SECTION MARK
-	- MOMENT CONNECTION

- INDICATES STEP FOOTING LOCATION

> - INDICATES DEDUCTIVE ALTERNATE ITEM