TTCX B17 CHILD DAY-CARE CENTER

GENSLER

ISSUED FOR 100% CONSTRUCTION DOCUMENTS

DATE: 06/20/2022

STRUCTURAL DRAWING LIST

	SHEET LIST	
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S-511	TYPICAL STEEL BEAM SHEAR CONNECTIONS 3/4" DIAMETER BOLTS	
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S-602	LGMF BEARING WALL DETAILS III	

777 Old Saw Mill River Road Mount Pleasant, NY 10591

ABBREVIATION	DESCRIPTION	ABBREVIATION	DESCRIPTION
ADDL	ADDITIONAL	LL	LIVE LOAD
ADJ	ADJACENT	LLH	LONG LEG HORIZONTAL
ALT	ALTERNATE	LLV	LONG LEG VERTICAL
APPRX	APPROXIMATE	LONG	LONGITUDINAL
ARCH	ARCHITECT OR ARCHITECTURAL	LP	LOW POINT
B/	BOTTOM OF	LW	LIGHTWEIGHT
B/B	BACK TO BACK	LWC	LIGHTWEIGHT CONCRETE
BAL	BALANCE	M	MOMENT
BLDG	BUILDING	MATL	MATERIAL
BLK	BLOCK	MAX	MAXIMUM
BLKG	BLOCKING	MC	MOMENT CONNECTION(S)
BM 	BEAM	MECH	MECHANICAL
ВОТ	BOTTOM	MEP	MECHANICAL, ELECTRICAL, PLUMBING
BRDG	BRIDGING		FIRE PROTECTION
BRG	BEARING	MEZZ	MEZZANINE
BTWN	BETWEEN	MFR	MANUFACTURER
С	COMPRESSION	MID	MIDDLE
C/C	CENTER TO CENTER	MIN	MINIMUM
CIP	CAST-IN-PLACE	MISC	MISCELLANEOUS
CJP	COMPLETE JOINT PENETRATION	NIC	NOT IN CONTRACT
CL	CENTER LINE	NO	NUMBER
CLR	CLEAR OR CLEARANCE	NOM	NOMINAL
		NS	NEAR SIDE
CMU	CONCRETE MASONRY UNIT		
COL	COLUMN	NTS	NOT TO SCALE
COMP	COMPRESSION	NW	NORMAL WEIGHT
CONC	CONCRETE	NWC	NORMALWEIGHT CONCRETE
CONN	CONNECTION(S)	OC	ON CENTER
CONST	CONSTRUCTION	OD	OUTSIDE DIAMETER
CONT	CONTINUOUS	OF	OUTSIDE FACE
db	REINFORCING BAR DIAMETER	OH	OPPOSITE HAND
ab DBL	DOUBLE	OPNG(S)	OPENING(S)
		OPP	OPPOSITE
DCW	DEMAND CRITICAL WELD		
DEG	DEGREE(S)	OSL	OUTSTANDING LEG
DET	DETAIL	PC	PIECE
DIA	DIAMETER	PCY	POUNDS PER CUBIC YARD
DIAG	DIAGONAL	PERP	PERPENDICULAR
DIM(S)	DIMENSION(S)	PG	PLATE GIRDER
DL	DEAD LOAD	PJP	PARTIAL JOINT PENETRATION
DWG(S)	DRAWING(S)	PL	PLATE
DWL	` '	PRC	PRECAST
	DOWEL(S)	PRLL	PARALLEL
EA	EACH		
ECC	ECCENTRICITY	PSF	POUNDS PER SQUARE FOOT
EE	EACH END	PSI	POUNDS PER SQUARE INCH
EF	EACH FACE	PT	POINT OR POST-TENSION(ED) OR (ING
EL	ELEVATION	RAD	RADIUS
ELEC	ELECTRICAL	REF	REFERENCE
ENGR	ENGINEER	REINF	REINFORCE(D) (ING) OR (MENT)
EOD	EDGE OF DECK	REQD	REQUIRED
EOS	EDGE OF SLAB	S&T	SHRINKAGE AND TEMPERATURE
		SCHED	SCHEDULE(D)
EQ	EQUAL		` '
EQUIP	EQUIPMENT	SDL	SUPERIMPOSED DEAD LOAD
EW	EACH WAY	SECT	SECTION
EXP	EXPANSION	SER	STRUCTURAL ENGINEER OF RECORD
EXST	EXISTING	SF	SQUARE FOOT (FEET)
EXT	EXTERIOR	SFRS	SEISMIC FORCE RESISTING SYSTEM
F/F	FACE TO FACE	SHT	SHEET
FIN	FINISH(ED)	SIM	SIMILAR
	• •	SOG	SLAB ON GRADE
FLR	FLOOR	SP	SPACE SPACE
FND	FOUNDATION		
FP 	FIREPROOF(ING)	SPEC(S)	SPECIFICATION(S)
FS	FAR SIDE	STD	STANDARD
FTG	FOOTING	STL	STEEL
GA	GAGE, GAUGE	STR	STRUCTURE
GALV	GALVANIZED	STRCTL	STRUCTURAL
GB	GRADE BEAM	SYM	SYMMETRICAL
	GENERAL	T	TENSION
GEN		T&B	TOP AND BOTTOM
CB	CDADE	ועט	TOP OF
	GRADE	т/	ハンド いた
GR HK	HOOK	T/	
		TEMP	TEMPERATURE OR TEMPORARY
HK	HOOK	TEMP TEN	TEMPERATURE OR TEMPORARY TENSION
HK HORIZ	HOOK HORIZONTAL	TEMP	TEMPERATURE OR TEMPORARY
HK HORIZ HP	HOOK HORIZONTAL HIGH POINT	TEMP TEN	TEMPERATURE OR TEMPORARY TENSION
HK HORIZ HP HT ID	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER	TEMP TEN THK	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS
HK HORIZ HP HT ID IF	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE	TEMP TEN THK TYP UON	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED
HK HORIZ HP HT ID IF INFO	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION	TEMP TEN THK TYP UON V	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR
HK HORIZ HP HT ID IF INFO	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR	TEMP TEN THK TYP UON V VERT	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR VERTICAL
HK HORIZ HP HT ID IF INFO INT	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR INTERMEDIATE	TEMP TEN THK TYP UON V VERT VIF	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR VERTICAL VERIFY IN FIELD
HK HORIZ HP HT ID IF INFO	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR	TEMP TEN THK TYP UON V VERT VIF W/	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR VERTICAL VERIFY IN FIELD WITH
HK HORIZ HP HT ID IF INFO INT	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR INTERMEDIATE	TEMP TEN THK TYP UON V VERT VIF W/ W/O	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR VERTICAL VERIFY IN FIELD WITH WITHOUT
HK HORIZ HP HT ID IF INFO INT INTRM JST(S)	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR INTERMEDIATE JOIST(S)	TEMP TEN THK TYP UON V VERT VIF W/	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR VERTICAL VERIFY IN FIELD WITH
HK HORIZ HP HT ID INFO INT INTRM JST(S) JT	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR INTERMEDIATE JOIST(S) JOINT KIPS (1,000 POUNDS)	TEMP TEN THK TYP UON V VERT VIF W/ W/O	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR VERTICAL VERIFY IN FIELD WITH WITHOUT
HK HORIZ HP HT ID IF INFO INT INTRM JST(S) JT K KLF	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR INTERMEDIATE JOIST(S) JOINT KIPS (1,000 POUNDS) KIP PER LINEAR FOOT	TEMP TEN THK TYP UON V VERT VIF W/ W/O WD	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR VERTICAL VERIFY IN FIELD WITH WITHOUT WOOD
HK HORIZ HP HT ID INFO INT INTRM JST(S) JT	HOOK HORIZONTAL HIGH POINT HEIGHT INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR INTERMEDIATE JOIST(S) JOINT KIPS (1,000 POUNDS)	TEMP TEN THK TYP UON V VERT VIF W/ W/O WD WP	TEMPERATURE OR TEMPORARY TENSION THICK OR THICKNESS TYPICAL UNLESS OTHERWISE NOTED SHEAR VERTICAL VERIFY IN FIELD WITH WITHOUT WOOD WORK POINT



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Building #17 Campus Expansion Child Day-care Center

777 Old Saw Mill River Road Mount Pleasant, NY 10591 Project No. B17-DAYCARE

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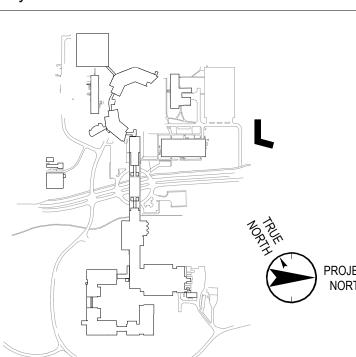
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No.	Date	Description
	05.20.2022	ISSUED FOR PERMIT
	06.20.2022	100% CONSTRUCTION DOCUMENTS
	07.01.2022	100% CONSTRUCTION DOCUMENT- 1

Plot Date: 11/10/21

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CD-9 IN CASES WHERE THE CONTRACTOR DETERMINES THAT SUSPENDED OR FLOOR MOUNTED EQUIPMENT LOADS EXIST WHICH EXCEED DESIGN LOADS INDICATED ON CONTRACT DOCUMENTS, CONTRACTOR SHALL SUBMIT LOAD DATA TO DESIGN PROFESSIONALS FOR REVIEW PRIOR TO PROCEEDING WITH WORK. CD-10 DISTRIBUTE THE MAXIMUM LOAD HUNG FROM ANY STRUCTURAL MEMBER FOR DUCTWORK, PIPING ETC OVER THE MEMBER'S TRIBUTARY AREA IN A WAY THAT THE MEP DESIGN SUPERIMPOSED DEAD LOADS LISTED IN CONTRACT DOCUMENTS ARE NOT EXCEEDED. THE CONTRACTOR SHALL COORDINATE THE LOADS OF ALL TRADES AND PROVIDE ADDITIONAL SUPPORT OR DISTRIBUTION FRAMING AS REQUIRED TO ACHIEVE THE ALLOWABLE LOAD DISTRIBUTION. CD-11 ESCALATOR SUPPORTS AND PITS ARE BASED ON ESCALATOR TYPES INDICATED ON ARCHITECTURAL CONTRACT DOCUMENTS. CONTRACTOR SHALL SUBMIT FOR REVIEW ANY PLANNED CHANGE TO ESCALATORS TO DESIGN PROFESSIONALS PRIOR TO SUBMITTING CORRESPONDING STRUCTURAL SHOP DRAWINGS CD-12 ELEVATOR GUIDERAIL SUPPORTS, MACHINE ROOMS, PITS, AND PENTHOUSES ARE BASED ON ELEVATOR TYPES INDICATED ON ARCHITECTURAL CONTRACT DOCUMENTS. CONTRACTOR SHALL SUBMIT FOR REVIEW ANY PLANNED CHANGE TO ELEVATORS TO DESIGN PROFESSIONALS PRIOR TO SUBMITTING CORRESPONDING STRUCTURAL SHOP DRAWINGS FOR ACTION. STRUCTURAL COMPONENTS ARE NOT DESIGNED FOR VIBRATING EQUIPMENT. MOUNT VIBRATING EQUIPMENT ON VIBRATION ISOLATORS SERVICEABILITY LIVE LOAD DEFLECTION IS LESS THAN L/360 LONG-TERM TOTAL DEFLECTION IS LESS THAN L/240 EXTERIOR EDGE BEAMS HAVE BEEN DESIGNED TO LIMIT LIVE LOAD MIDSPAN VERTICAL DEFLECTION TO 1/360 OF THE SPAN OR 3/8", WHICHEVER IS LESS. EXTERIOR EDGE BEAMS HAVE BEEN DESIGNED TO LIMIT DEAD PLUS SUPERIMPOSED DEAD LOAD MIDSPAN VERTICAL DEFLECTION TO 1/[XXX] OF THE SPAN OR [X"], WHICHEVER IS LESS. LATERAL DRIFT DUE TO WIND LOADS IS LESS THAN OR EQUAL TO H/400 CONNECTIONS OF SYSTEMS DESIGNED BY CONTRACTOR'S ENGINEER SUCH AS, BUT NOT LIMITED TO. CLADDING. STAIRS. ELEVATORS. ESCALATORS. PRECAST. AND MEP LOADS ARE ASSUMED TO IMPOSE VERTICAL AND/OR HORIZONTAL LOADS ON THE BASE BUILDING STRUCTURAL MEMBERS WITHOUT GENERATING TORSION IN THE SUPPORTING STRUCTURAL MEMBERS. CONTRACTOR IS RESPONSIBLE FOR FURNISHING AND INSTALLING ALL SUPPLEMENTARY BRACING MEMBERS. AS REQUIRED TO PREVENT TORSION ON THE BASE BUILDING STRUCTURE. CD-16 FOR FIRE RATING AND FIREPROOFING ASSEMBLY EVALUATIONS, CONSIDER THE FOLLOWING ASSEMBLIES RESTRAINED: COMPOSITE WIDE-FLANGE STEEL FRAMING, INTERIOR BAYS OF CONTINUOUS CAST-IN-PLACE CONCRETE CONSTRUCTION. CONSIDER ALL OTHER ASSEMBLIES UNRESTRAINED. THERE HAVE BEEN NO LOAD RESTRICTION FACTORS APPLIED TO THE STRUCTURAL DESIGN FOR THE PURPOSES OF SELECTING FIREPROOFING **ASSEMBLIES** DI DELEGATED DESIGN ITEMS DI-1 THE CONTRACTOR SHALL EMPLOY OR RETAIN A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THIS PROJECT IS LOCATED TO DESIGN AND DETAIL DELEGATED DESIGN ITEMS TO MEET THE PERFORMANCE AND DESIGN CRITERIA ESTABLISHED AS PART OF THE BASE BUILDING STRUCTURE INDICATED IN THE CONTRACT DOCUMENTS INCLUDING BUT NOT LIMITED TO: LIGHT GAGE METAL FRAMING STRUCTURAL LOAD BEARING WALL SYSTEM STRUCTURAL STEEL CONNECTIONS STEEL JOISTS, BRIDGING AND CONNECTIONS STEEL ROOF DECK SU SUBMITTALS SU-1 THE CONTRACTOR IS TO REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO DESIGN PROFESSIONALS. THE CONTRACTOR IS TO STAMP EACH SUBMITTAL VERIFYING THAT THE FOLLOWING IS ADDRESSED: THE SHOP DRAWING IS REQUESTED. THE SHOP DRAWING IS BASED ON THE LATEST DESIGN. THE DESIGN PROFESSIONALS' COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED. THE WORK IS COORDINATED AMONG ALL CONSTRUCTION TRADES. REVISIONS FROM PREVIOUS SUBMITTALS ARE CLEARLY MARKED BY CIRCLING OR CLOUDS. SUBMITTAL IS COMPLETE. SUBMITTAL DOES NOT INCLUDE SUBSTITUTION REQUEST SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL NUMBER, SPECIFICATION SECTION NUMBER. THE SER SHALL RETURN, WITHOUT COMMENT, SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS. SU-2 THE SER'S REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT. NO WORK SHALL BE STARTED WITHOUT SUCH REVIEW. SU-3 FOR COMPONENTS THAT REQUIRE ENGINEERING BY THE CONTRACTOR, PROVIDE A NOTE ON EACH SHOP DRAWING, WRITTEN AND SIGNED BY THE SUPPLIER'S ENGINEER. INDICATING THAT THE SHOP DRAWING IS IN CONFORMANCE WITH THE CALCULATIONS OF THE CONTRACTOR'S ENGINEER. SU-4 THE FOLLOWING ITEMS REQUIRE SUBMITTALS FOR STRUCTURAL REVIEW AS OUTLINED IN THE SPECIFICATIONS: 031000 CONCRETE FORMWORK CONCRETE REINFORCING LAYOUT 032000 033000 CONCRETE MIX DESIGNS CONCRETE CONSTRUCTION JOINT LAYOUT 033000 STRUCTURAL STEEL 051000 S CALC STRUCTURAL STEEL CONNECTIONS 051000 051000 SHEAR STUD LAYOUT STEEL JOISTS. BRIDGING AND CONNECTIONS S CALC STEEL ROOF DECK 054000 S CALC COLD-FORMED METAL FRAMING S = SHOP DRAWINGS REQUIRED CALC = SUPPORTING CALCULATIONS REQUIRED. SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS SU-5 THE ITEMS IN THIS SECTION REFER TO LOADS IMPOSED BY CONTRACTOR DESIGNED SYSTEMS, SPECIFICALLY: COLD-FORMED METAL FRAMING EXTERIOR CLADDING SYSTEMS METAL STAIRS ARCHITECTURAL ORNAMENTATION (FLAGPOLES, BANNERS, MASTS, ETC.) WHERE CONTRACTOR LOADS IMPOSED DO NOT EXCEED AND/OR CONNECTION CONDITIONS DO NOT DIFFER FROM WHAT IS INDICATED IN THE STRUCTURAL DRAWINGS. SUBMIT FOR RECORD A LETTER SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED STATING THE FOLLOWING: "THE CONTRACTOR DESIGNED SYSTEM HAS BEEN DESIGNED TO IMPOSE LOADS ON THE BASE BUILDING STRUCTURE THAT ARE WITHIN THE LOAD LIMITS AND AT THE LOCATIONS INDICATED ON THE STRUCTURAL DRAWINGS." WHERE CONTRACTOR LOADS IMPOSED FOR THE FOLLOWING ITEMS EXCEED AND/OR CONNECTION CONDITIONS DIFFER FROM WHAT IS SHOWN IN THE STRUCTURAL DRAWINGS, SUBMIT FOR APPROVAL TO SER LOADS IMPOSED ON THE PRIMARY STRUCTURAL FRAME DUE TO THE DEAD, LIVE, AND WIND/SEISMIC LOADS INDICATED ON THE CONTRACT DOCUMENTS. SUBMITTAL SHALL LIST THE DESIGN LOADS USED AND BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. SUBMITTAL SHALL INCLUDE LOCATION, MAGNITUDE AND DIRECTION OF UNFACTORED IMPOSED LOADS, GRAPHICALLY REPRESENTED IN THEIR APPROPRIATE LOCATIONS ON A COPY OF THE CONTRACT DOCUMENT STRUCTURAL FRAMING PLANS OR ELEVATIONS AS APPROPRIATE. DETAIL REFERENCES IN THE CONNECTIONS APPLICABLE AT EACH LOCATION SHALL BE NOTED ON THE SUBMITTAL DRAWINGS. FOR EXTERIOR WALL ASSEMBLIES, THE LOADS IMPOSED SUBMITTAL SHALL BE COMPREHENSIVE INDICATING THE LOADS IMPOSED ON THE BASE BUILDING STRUCTURE AND SHALL INCLUDE THE REACTIONS BASED ON THE ACTUAL LOADS OF THE ENTIRE ASSEMBLY, INCLUDING BUT NOT LIMITED TO GLAZING. CLADDING, METAL STUD BACKUP, AND MULLIONS. FOR MEP SYSTEMS. THE LOADS IMPOSED SUBMITTAL SHALL BE COMPREHENSIVE INDICATING THE LOADS IMPOSED ON THE BASE BUILDING STRUCTURE AND SHALL INCLUDE THE REACTIONS BASED ON THE ACTUAL LOADS OF THE ENTIRE MECHANICAL, ELECTRICAL, PLUMBING, AND FIRE PROTECTION SYSTEM, INCLUDING BUT NOT LIMITED TO PIPING, DUCTS, ELECTRICAL RACEWAYS, AND EQUIPMENT WEIGHTS. A SUBSTITUTION REQUEST MAY BE REQUIRED WHERE CONTRACTOR LOADS IMPOSED EXCEED AND/OR CONNECTION CONDITIONS DIFFER FROM THE BASIS OF FN FOUNDATIONS FN-1 THE FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL REPORT BY CARLIN SIMPSON ASSOCIATES DATED JULY 14, 2021. FN-2 FOUNDATIONS HAVE BEEN DESIGNED BASED ON THE FOLLOWING DESIGN VALUES FROM THE GEOTECHNICAL REPORT (SERVICE LEVEL): BEARING STRATUM VIRGIN SOIL/NEW COMPACTED FILL **NET ALLOWABLE BEARING CAPACITY:** SEE GEOTECHNICAL REPORT FOR ADDITIONAL REQUIREMENTS AND INFORMATION. DESIGN VALUES SHALL BE FIELD VERIFIED BY QUALIFIED GEOTECHNICAL ENGINEER RETAINED BY THE OWNER. FN-3 THE CONTRACTOR SHALL VERIFY FOUNDATION INSTALLATION AND CONSTRUCTION IS IN CONFORMANCE WITH THE RECOMMENDATIONS OUTLINED IN THE GEOTECHNICAL REPORT. FN-4 CONTRACTOR SHALL BE RESPONSIBLE TO ADEQUATELY PROTECT ALL EXCAVATION. WHERE NECESSARY, SHEET AND SHORE THE EXCAVATION WITH ALL REQUIRED TIEBACKS AND BRACING AS DETERMINED BY CONTRACTOR'S ENGINEER. CM CONCRETE MATERIALS CM-1 CONCRETE STRENGTH SHALL MEET THE FOLLOWING 28-DAY COMPRESSIVE STRENGTHS (f' c), UON:

FOOTINGS. PILE CAPS AND PIERS 4.000 PSI GRADE BEAMS 4,000 PSI FOUNDATION WALLS, PILASTERS, BUTTRESSES 4,000 PSI 8.000 PSI NON-SHRINK GROUT 4.000 PSI SLAB ON GRADE CONCRETE HOUSEKEEPING PADS, AND FILL SLABS 4,000 PSI LIGHTWEIGHT CM-2 PROVIDE NORMALWEIGHT CONCRETE WITH CURED DENSITY OF 145 +/- 5 PCF, AND AGGREGATE CONFORMING TO ASTM C33, UON. WHERE INDICATED, PROVIDE LIGHTWEIGHT CONCRETE WITH CURED DENSITY OF 112+/-3 PCF AND AGGREGATE CONFORMING TO ASTM C330. CM-3 THE USE OF CALCIUM CHLORIDE AND OTHER CHLORIDE CONTAINING AGENTS IS PROHIBITED. THE USE OF RECYCLED CONCRETE IS PROHIBITED. PLACEMENT WITHIN AND CONTACT BETWEEN ALUMINUM ITEMS, INCLUDING ALUMINUM CONDUIT, AND CONCRETE IS PROHIBITED. CM-4 ALL CAST-IN-PLACE CONCRETE WILL EXPERIENCE DIFFERING VARIATIONS OF CRACKING. ANY ELEMENT EXPOSED TO DIRECT WEATHER AND/OR TEMPERATURE VARIATIONS DURING CONSTRUCTION OR IN THE FINAL CONDITION IS TO BE TREATED AND REGULARLY MAINTAINED TO PREVENT PROPAGATION OF CRACKS AND

WATER PENETRATION. THE CONTRACTOR SHALL DEVELOP A REGULAR MAINTENANCE PROGRAM AND SUBMIT IT TO THE OWNER.

RE CONCRETE REINFORCEMENT

EPOXY COATED DEFORMED BARS:

WELDED WIRE REINFORCEMENT

RE-1 ALL CONCRETE SHALL INCLUDE REINFORCEMENT. IF REINFORCEMENT IS NOT SPECIFICALLY INDICATED ON THE DRAWINGS VERIFY WITH THE

ASTM A615 / A775

ASTM A1064 / A884

ASTM A1064

RE-2 REINFORCEMENT SHALL CONFORM TO THE FOLLOWING STANDARDS AND MATERIAL PROPERTIES UON: ASTM A615 GRADE 60 **DEFORMED BARS:** WELDABLE DEFORMED BARS: ASTM A706

EPOXY COATED WELDED WIRE REINFORCEMENT RE-3 DETAIL REINFORCEMENT BASED ON THE PROJECT REQUIREMENTS, ACI-318 AND ACI-315, UON.

RE-5 DOWELS SHALL MATCH SIZE AND SPACING OF MAIN REINFORCEMENT UON

RE-4 WHERE A 90-DEG. 135 -DEG OR 180-DEG HOOK IS GRAPHICALLY INDICATED, PROVIDE CORRESPONDING ACI STANDARD HOOKS UON.

RE-6 REINFORCEMENT SHALL HAVE CONCRETE PROTECTION (CLEAR COVER) PER ACI 318 UNLESS OTHERWISE INDICATED ON THE DRAWINGS.

RE-7 LAP REINFORCEMENT ONLY AT LOCATIONS AS SPECIFICALLY DETAILED ON THE DRAWINGS EXCEPT REINFORCEMENT MARKED AS CONTINUOUS CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR USING TENSION LAP SPLICES (LTS). SEE LAP SPLICE AND EMBEDMENT SCHEDULE.

RE-8 UNLESS OTHERWISE NOTED ALL LAP SPLICES ARE TO BE TENSION LAP SPLICES PER LAP SPLICE AND EMBEDMENT SCHEDULE RE-9 PROVIDE MECHANICAL SPLICES FOR BARS LARGER THAN #11 OR WHERE INDICATED. PROVIDE TENSILE, PRE-QUALIFIED, WELDED OR THREADED MECHANICAL SPLICES UON.

RE-10LAP WELDED WIRE REINFORCEMENT TWO PANEL SPACINGS, UON

RE-11PROVIDE LAP SPLICE LOCATIONS AS FOLLOWS. UON: GRADE BEAM / WALL (TOP HORIZONTAL REINFORCEMENT): AT CENTER OF SPAN

GRADE BEAM / WALL (BOTTOM HORIZONTAL REINFORCEMENT): AT SUPPORTS WALL INSIDE FACE (VERTICAL REINFORCEMENT): AT SUPPORT WALL OUTSIDE FACE (VERTICAL REINFORCEMENT): AT STORY MIDHEIGHT OF WALL FOR BELOW GRADE FOUNDATION WALLS, AT SUPPORT

UNLESS OTHERWISE NOTED TERMINATE BARS AT DISCONTINUOUS ENDS WITH STANDARD HOOKS.

RE-12PROVIDE EPOXY COATED REINFORCEMENT AND ACCESSORIES IN AREAS OF DIRECT EXPOSURE TO THE ENVIRONMENT, CHEMICALS, OR DE-ICING FOR THE AREAS INDICATED ON THE DRAWINGS.

CJ CONCRETE CONSTRUCTION JOINTS

CJ-1 PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI-318. SUBMIT SHOP DRAWINGS SHOWING PROPOSED CONSTRUCTION JOINT LOCATIONS, DETAILS AND THE PLACEMENT SEQUENCE FOR THE SER'S APPROVAL PRIOR TO PROCEEDING WITH WORK.

CJ-2 UNLESS SPECIFICALLY SHOWN ON THE DRAWINGS, HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE PERMITTED IN FOOTINGS, PILE CAPS, MAT FOUNDATIONS, GRADE BEAMS, BEAMS, UPTURNED BEAMS, SLABS, AND WALLS WITHOUT PRIOR WRITTEN APPROVAL FROM THE SER BEFORE CONSTRUCTION.

CJ-3 PLACE VERTICAL CONSTRUCTION JOINTS TO PROVIDE A 60 FT MAXIMUM LENGTH OF CONCRETE PLACEMENT AND LOCATE AS FOLLOWS: A. FOUNDATION WALLS: MINIMUM OF 8 FT FROM ANY WALL INTERSECTION, PILASTER, PIER, OR WALL OPENING B. BEAMS AND GRADE BEAMS: WITHIN THE MIDDLE THIRD OF THE CLEAR SPAN AVOIDING LAP SPLICES. SUBJECT TO SER APPROVAL.

CJ-4 PROVIDE CONTINUOUS WATERSTOPS AT ALL CONSTRUCTION JOINTS EXPOSED TO SOIL OR WATER, AS DESCRIBED IN THE SPECIFICATIONS AND WHERE INDICATED IN THE ARCHITECTURAL DOCUMENTS.

SS STRUCTURAL STEEL

SS-1 STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS OTHERWISE NOTED ON THE CONTRACT DOCUMENTS. SEE ADDITIONAL MATERIAL REQUIREMENTS RELATED TO MATERIAL TOUGHNESS BELOW:

ASTM A6 ROLLED W SHAPES AND CHANNELS: ASTM A572 OR A992, MINIMUM YIELD STRENGTH 50 KSI

ANGLES FOR TRUSSES AND BRACES: ASTM A572 OR A529. MINIMUM YIELD STRENGTH 50 KSI

MISCELLANEOUS ANGLES: ASTM A36. MINIMUM YIELD STRENGTH 36 KSI ASTM A500 GRADE B. MINIMUM YIELD STRENGTH 42 KSI FOR ROUND AND 46 KSI FOR RECTANGULAR HSS HOLLOW STRUCTURAL SECTIONS: ASTM A572 OR A529. MINIMUM YIELD STRENGTH 50 KSI

SS-2 CONNECTION MATERIAL SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS OR AS NEEDED FOR CONNECTION DESIGN:

ASTM A572 OR A529. MINIMUM YIELD STRENGTH 50 KSI UON ASTM A572 OR A992, MINIMUM YIELD STRENGTH 50 KSI ASTM A572 OR A529. MINIMUM YIELD STRENGTH 50 KSI UON ASTM F3125 GRADES A325 AND F1852 OR A490 AND F2280 OR AS INDICATED IN DETAILS

NUTS: ASTM A563 ASTM F436

ANCHOR RODS: ASTM F1554 GRADE 55 WITH WELDABILITY SUPPLEMENT S1

HEADED STUDS ASTM A108. GRADE 1010 THROUGH 1020 HEADED STUD TYPE, COLD-FINISHED CARBON STEEL, AWS D1.1, TYPE B 3/4" DIAMETER WELD ELECTRODES: MINIMUM TENSILE STRENGTH 70 KSI

SS-3 WHERE NO CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION SS-4 SPLICES SHALL BE ALLOWED ONLY AT LOCATIONS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS UNLESS APPROVED OTHERWISE

BY THE SER IN WRITING. SS-5 FOR STEEL MEMBERS AND EMBEDMENTS EXPOSED TO WEATHER, PROVIDE HOT-DIPPED GALVANIZED FINISH OR APPROVED ZINC RICH

EXTERIOR COATING SYSTEM.

SS-6 PROVIDE HOLES IN ALL STEEL AS REQUIRED TO PREVENT ANY ACCUMULATION OF WATER. ALL PENETRATIONS THROUGH MAIN MEMBERS SHALL NOT EXCEED 1 1/8" DIA. AND SHALL BE GROUND SMOOTH. THESE DRAINS MUST BE KEPT CLEAN AND OPEN.

SS-7 SHOW ALL COPES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF

OTHER TRADES ON THE SHOP DRAWINGS FOR APPROVAL BY THE DESIGN PROFESSIONALS.

SS-8 FIELD MODIFICATION OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE DESIGN PROFESSIONALS.

SS-9 THE FOLLOWING STRUCTURAL STEEL SHAPES AND ELEMENTS MUST MEET ADDITIONAL MINIMUM MATERIAL TOUGHNESS REQUIREMENTS THROUGH CHARPY V-NOTCH (CVN) IMPACT TESTING VALUES AND ADDITIONAL MATERIAL PROPERTIES INDICATED:

ASTM A6 HOT-ROLLED W SHAPES ALL STRUCTURAL STEEL INCLUDED IN THIS CATEGORY TO BE FULLY KILLED AND PRODUCED TO A FINE GRAIN PRACTICE PER SUPPLEMENTARY REQUIREMENT S28. AND SURFACES OF RE-ENTRANT CORNERS/INTERNAL RADII ARE TO BE GROUND TO BRIGHT METAL, SEE PROJECT

SPECIFICATIONS.

ALL OTHER ASTM A6 HOT-ROLLED SHAPES

USED AS HANGERS WITH FLANGE THICKNESS EXCEEDING 1 1/2 INCHES REGARDLESS OF MEANS OF CONNECTION: - WITH SERVICE TEMPERATURES MAINTAINED ABOVE 50 DEG F, 30 FT-LB @ 40 DEG F - EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG F, 40 FT-LB @ LAST BUT NOT HIGHER THAN 40 DEG F - TESTING TO BE IN ACCORDANCE WITH SUPPLEMENTARY REQUIREMENT S5 WITH CVN IMPACT TEST LOCATION IN THE FLANGE OR

HSS SHAPES

USED AS TRUSS MEMBERS OR HANGERS REGARDLESS OF THICKNESS OR MEANS OF CONNECTION:

- WITH SERVICE TEMPERATURES MAINTAINED ABOVE 50 DEG F, 25 FT-LB @ 40 DEG F EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG F, 40 FT-LB @ LAST BUT NOT HIGHER THAN 40 DEG F

- EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG F. 40 FT-LB @ LAST BUT NOT HIGHER THAN 40 DEG F.

LEG PER ASTM A673 EXCEPT THE SPECIMEN SHALL BE LOCATED AT THE MID-DEPTH OF THE MATERIAL THICKNESS.

PLATES AND SHAPES BUILT-UP FROM PLATES

ALL PLATE TESTING TO BE PERFORMED IN ACCORDANCE WITH WITH ASTM A673 AND PERFORMED AT FREQUENCY P. ALL STRUCTURAL STEEL

INCLUDED IN THIS CATEGORY TO BE FULLY KILLED AND PRODUCED TO A FINE GRAIN PRACTICE PER SUPPLEMENTARY REQUIREMENT S28.

1. COLUMN BASE PLATES WITH THICKNESS EXCEEDING 2 INCHES THAT ARE CONNECTED TO COLUMNS IN THE SEISMIC FORCE RESISTING SYSTEM WITH DEMAND CRITICAL WELDS: - WITH SERVICE TEMPERATURES MAINTAINED ABOVE 50 DEG F, 30 FT-LB @ 40 DEG F

WELD METAL

WELD METAL USED AS PART OF THE SEISMIC FORCE RESISTING SYSTEM WITH SERVICE TEMPERATURES MAINTAINED ABOVE 50 DEG F, 20 FT-LB @ -20 DEG F AND 50 FT-LB @ 0 DEG F

EXPOSED TO TEMPERATURES IN SERVICE BELOW 50 DEG F, 20 FT-LB @ -40 DEG F AND 60 FT-LB @ 0 DEG F

WELD METAL USED FOR CJP WELDS: WITH SERVICE TEMPERATURES MAINTAINED ABOVE 50 DEG F, 40 FT-LB @ 40 DEG F AND 20 FT-LB @ 0 DEG F - EXPOSED TO IN SERVICE TEMPERATURES BELOW 50 DEG F, 50 FT-LB @ 40 DEG F AND 30 FT-LB @ 0 DEG F

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Key Plan

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Vendor Name: GENSLER Vendor Project No.: 006.3608.000 Drawn By: Author Discipline: **GENERAL NOTES**

Scale: Floor:

SC STRUCTURAL STEEL CONNECTIONS

SC-1 ALL STEEL DETAILS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", AISC-LOAD AND RESISTANCE FACTOR DESIGN.

SC-2 ALL CONNECTIONS, UNLESS INDICATED AS BEING COMPLETELY DESIGNED ON THE STRUCTURAL DRAWINGS, SHALL BE DESIGNED AND DETAILED BY A **PROFESSIONAL** ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE DESIGN AND DETAILING SHALL COMPLY WITH ALL APPLICABLE CODES AND SPECIFICATION SECTIONS.

SC-3 UNLESS INDICATED AS BEING COMPLETELY DESIGNED, DETAILS ON DRAWINGS INDICATE GENERAL CRITERIA FOR DESIGN AND DETAILING OF CONNECTIONS AND ARE NOT INTENDED TO CONVEY COMPLETE CONNECTOR SIZES, PLATE SIZES, WELD SIZES, NUMBER OF BOLTS, OR ANY OTHER SPECIFIC INFORMATION THAT IS OBTAINED THROUGH DESIGNING OF AN INDIVIDUAL CONNECTION FOR A GIVEN SET OF LOADS. DETAILS DO NOT SHOW ERECTION AIDS. PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM AFTER WORK IS COMPLETE.

SC-4 SUBMIT CONNECTIONS NOT SPECIFICALLY INDICATED AS COMPLETELY DESIGNED ON THE DRAWINGS TO THE SER FOR REVIEW PRIOR TO REVIEW OF SHOP DRAWINGS. FOR BIDDING PURPOSES, WHERE NO MOMENT IS INDICATED ON DRAWINGS PROVIDE FULL MOMENT CAPACITY OF MEMBER (.9 Fy Z) AND WHERE NO VERTICAL SHEAR IS INDICATED ON DRAWINGS PROVIDE FULL SHEAR CAPACITY (.54 Fy d tw).

SC-5 ALTERNATE CONNECTIONS TO THOSE SHOWN ON DRAWINGS WILL BE CONSIDERED AS A SUBSTITUTION REQUEST. SEE PROJECT SPECIFICATIONS.

SC-6 FOR CONNECTION DESIGN AND DETAILING, SET CONNECTION WORK POINT AT INTERSECTION OF MEMBER CENTERLINES, UON.

SC-7 DESIGN ALL CONNECTIONS FOR FORCES INDICATED ON THE DRAWINGS. CONNECTION DESIGN FORCES INDICATED ON THE DRAWINGS ARE FACTORED PER LRFD DESIGN BASIS UON. SC-8 USE NO MORE THAN TWO BOLT DIAMETERS, ALL BOLTS OF THE SAME DIAMETER SHALL BE OF THE SAME GRADE, SKIP ONE SIZE BETWEEN

DIAMETERS. BOLTS FOR THIS PROJECT SHALL BE: 3/4" DIAMETER F3125 GRADE A325 OR F1852 OR 1" DIAMETER F3125 GRADE A490 OR F2280

SC-9 BEAM CONNECTION DESIGN NOTES:

SEE PLANS AND ELEVATIONS FOR BEAM REACTIONS AND MOMENTS.

DEVELOP THE LARGER OF THE BEAM SHEAR REACTION SHOWN ON PLANS OR ELEVATIONS. IF NO SHEAR REACTIONS ARE SHOWN ON PLANS OR ELEVATIONS THEN ALLOW FOR SHEAR CONNECTION WITH FULL SHEAR CAPACITY (.54 Fy d tw).

DEVELOP THE LARGER OF THE MOMENT SHOWN ON PLANS OR ELEVATIONS. IF NO MOMENT REACTIONS ARE SHOWN ON PLANS OR ELEVATIONS THEN ALLOW FOR MOMENT CONNECTION THAT DEVELOPS THE FULL BEAM SECTION MOMENT CAPACITY (.9FyZ).

DEVELOP THE LARGER OF THE AXIAL FORCE DENOTED AS P OR TF SHOWN ON PLANS OR ELEVATIONS. SEE STEEL BEAM LEGEND.

ALL BEAM REACTIONS, AXIAL FORCES AND MOMENTS SHOWN ACT CONCURRENTLY. UON, BEAM REACTIONS ACT IN GRAVITY DIRECTION WHILE AXIAL FORCES AND MOMENTS ARE TO BE CONSIDERED REVERSIBLE. WHERE NO AXIAL FORCE IS SHOWN, ALL BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM AXIAL FORCE EQUAL TO 5% OF THE

FACTORED DEAD LOAD PLUS LIVE LOAD VERTICAL BEAM SHEAR. FOR THE PURPOSES OF DESIGNING FOR THIS MINIMUM AXIAL FORCE: THE VERTICAL BEAM SHEAR AND CORRESPONDING MINIMUM AXIAL FORCE NEED NOT BE CONSIDERED TO ACT CONCURRENTLY AND BEARING BOLTS IN CONNECTIONS WITH SHORT SLOTTED HOLES PARALLEL TO THE AXIAL FORCE ARE PERMITTED. SHEAR CONNECTIONS INDICATED AS COMPLETELY DESIGNED IN THESE DRAWINGS HAVE BEEN DESIGNED TO MEET THESE MINIMUM AXIAL FORCE REQUIREMENTS.

EXCEPT WHERE "SNUG TIGHT" INSTALLATION IS SPECIFICALLY PERMITTED ON DRAWINGS OR "SLIP CRITICAL" DETAILING IS REQUIRED. ALL HIGH STRENGTH BOLTS SHALL BE INSTALLED AS FULL PRETENSIONED BOLTS.

AT A MINIMUM ALL BOLTED MOMENT AND AXIAL CONNECTION SHALL HAVE PRETENSIONED BOLTS IN STANDARD HOLES.

BOLTED MOMENT CONNECTIONS AT CANTILEVERS AND BACKSPANS SHALL USE SLIP CRITICAL BOLTS.

DO NOT USE OVERSIZED OR SLOTTED HOLES FOR ANY CONNECTIONS UNLESS SPECIFICALLY INDICATED ON THE DRAWINGS OR APPROVED IN WRITING BY THE SER.

SC-10ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE STRUCTURAL WELDING CODE, ANSI/AWS D1.1, LATEST EDITION. ALL WELD SIZES SHALL BE THE LARGER OF THE SIZE REQUIRED BY CONNECTION FORCES, THE MINIMUM SIZE PER ANSI/AWS D1.1, OR 3/16 INCH MINIMUM FILLET WELD UON. ANY WELD SIZES SHOWN ON THE DESIGN DRAWINGS ARE CONSIDERED EFFECTIVE WELD SIZES AND SHALL BE INCREASED IN ACCORDANCE WITH AWS AS REQUIRED BY GAPS OR SKEWS BETWEEN COMPONENTS.

SC-11 USE RUNOFF TABS AT ALL BEVEL AND COMPLETE JOINT PENETRATION WELDS. REMOVE RUNOFF TABS BY NEAT CUTS AFTER WELD IS COMPLETED. GRIND SMOOTH WHERE REQUIRED BY DETAIL.

SC-12WHERE REQUIRED BY DETAIL REMOVE WELD BACK UP BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED.

SC-13DESIGN, DETAIL, FURNISH AND INSTALL STIFFENERS, CONTINUITY PLATES, DOUBLER PLATES, OR OTHER NECESSARY ADDITIONAL LOCAL STRENGTHENING MEASURES AS REQUIRED. MEMBER SIZES INDICATED ON THE DRAWINGS ARE BASED ON MEMBER BEHAVIOR AWAY FROM CONNECTIONS.

SJ OPEN WEB STEEL JOISTS AND JOIST GIRDERS

SJ-1 DESIGN, MANUFACTURE, AND ERECT JOISTS AND BRIDGING IN ACCORDANCE WITH THE "STANDARD SPECIFICATION FOR OPEN WEB JOISTS" OF THE STEEL JOIST INSTITUTE (SJI), CURRENT EDITION, AS A MINIMUM.

SJ-2 JOISTS AND JOIST GIRDERS SHALL BE DESIGNED AND PROVIDED BY CONTRACTOR PER THE SJI SPECIFICATIONS AS INDICATED ON THE DRAWINGS. SEE DRAWINGS FOR JOIST SPACING, LOAD CRITERIA, AND DEPTH LIMITATIONS.

SJ-3 BRIDGING SHALL BE DESIGNED AND PROVIDED BY THE CONTRACTOR PER THE SJI SPECIFICATIONS.

SJ-4 BEFORE STEEL DECK IS PLACED, ATTACH ALL BRIDGING TO THE JOISTS AND ANCHOR ALL BRIDGING TERMINATING AT WALLS OR BEAMS TO THE WALLS OR BEAMS. WELD OR BOLT ALL BRIDGING CONNECTIONS TO STEEL JOISTS AND BEAMS.

SJ-5 DESIGN AND DETAIL STEEL JOISTS AND JOIST CONNECTIONS TO CARRY THE MOST SEVERE COMBINATION OF DIAPHRAGM FORCES, KICKER FORCES, GRAVITY LOADS, SNOW LOADS, AND WIND UPLIFT FORCES SHOWN ON THE DRAWINGS. IT IS NOT ACCEPTABLE TO DESIGN JOISTS FOR SJI STANDARD LOADS IN LIEU OF THE LOADS SHOWN ON THE DRAWINGS. IN ADDITION TO THE LOADS SHOWN ON THE DRAWINGS, JOISTS SHALL BE DESIGNED FOR:

A. A MINIMUM NET UPLIFT FORCE OF 10 PSF (SERVICE LEVEL), UON B. ADDITIONAL SERVICE POINT LOAD AT ANY PANEL POINT OF 300 LBS FOR K-SERIES JOISTS AND 700 LBS FOR LH AND DLH-SERIES

SJ-6 DESIGN JOISTS TO LIMIT DEFLECTION UNDER TOTAL LOAD TO SPAN LENGTH DIVIDED BY 240, UON. DESIGN JOISTS TO LIMIT DEFLECTION UNDER LIVE LOAD TO SPAN LENGTH DIVIDED BY 360, UON.

SJ-7 CAMBER JOISTS PER SJI STANDARDS, UON.

SJ-8 PROVIDE DOUBLE ANGLE TOP AND BOTTOM CHORDS.

SJ-9 HANGING AND POINT BEARING LOADS AT JOISTS SHALL ONLY BE PERMITTED AS INDICATED ON THE DRAWINGS. DESIGN JOIST FOR HANGING AND POINT BEARING LOADS AT ANY ADJACENT PANEL POINT. COORDINATE HANGING AND POINT BEARING LOADS WITH ARCHITECTURAL AND MEP DRAWINGS.

SJ-10 EXTEND BOTTOM CHORDS OF JOISTS AND JOIST GIRDERS AT COLUMNS, SEE JOIST DETAILS FOR ADDITIONAL INFORMATION. COORDINATE BOTTOM CHORD EXTENSIONS WITH ARCHITECTURAL DRAWINGS.

SJ-11 AT EDGE CONDITIONS EXTEND JOIST TOP CHORDS BEYOND SUPPORTING BEAMS TO PERIMETER ANGLE OR BENT PLATE. UNO.

SJ-12 JOIST SERIES, SEAT, AND SUPPORT INFORMATION SHOWN ON THE DRAWINGS IS A MINIMUM. JOIST DESIGN ENGINEER TO REVIEW ALL SJI REQUIREMENTS AND NOTIFY ENGINEER OF RECORD IF SELECTED SYSTEM DIFFERS FROM CONTRACT DOCUMENTS PRIOR TO FABRICATION OF JOISTS. CONTRACTOR IS RESPONSIBLE FOR COORDINATION BETWEEN SELECTED JOIST SYSTEM AND OTHER TRADES.

SD STEEL DECK GENERAL REQUIREMENTS

SD-1 THE DESIGN, MANUFACTURE AND ERECTION OF STEEL DECK AND ITS ANCHORAGE SHALL, AT A MINIMUM, BE IN ACCORDANCE WITH "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS" OF THE STEEL DECK INSTITUTE (SDI), CURRENT EDITION AND "SPECIFICATIONS" FOR DESIGN OF LIGHT GAGE COLD FORMED STEEL STRUCTURAL MEMBERS" AS PUBLISHED BY THE AMÉRICAN IRON AND STEEL INSTITUTE (AISI),

SD-2 CONFIGURE ALL STEEL DECK USING THREE SPAN CONTINUOUS LAYOUTS WHEREVER POSSIBLE.

SD-3 CONFIGURE ALL STEEL DECK AS SHOWN ON THE DRAWINGS.

RD STEEL ROOF DECK

RD-1 STEEL ROOF DECK SHALL CONFORM TO THE FOLLOWING STANDARDS AND MATERIAL PROPERTIES:

STEEL FOR DECK ASTM A653, MINIMUM YIELD STRENGTH OF 33 KSI HOT-DIP GALVANIZING ASTM A653 G60

ROOF DECK SHALL BE HOT-DIP GALVANIZED, UON

RD-2 DESIGN AND DETAIL ROOF DECK AND ITS ANCHORAGE TO SUPPORTING MEMBERS TO SUPPORT SCHEDULED DESIGN LOADS. INDICATED DIAPHRAGM SHEAR, AND INDICATED ROOF UPLIFT. ASSUME ROOF DIAPHRAGM LOADS AND ROOF UPLIFT LOADS TO BE APPLIED SIMULTANEOUSLY.

RD-2 STEEL ROOF DECK AND ITS ANCHORAGE TO STRUCTURAL FRAMING SHALL BE CAPABLE OF WITHSTANDING THE FOLLOWING MINIMUM LOADING REQUIREMENTS (SERVICE LEVEL): A GRAVITY LOAD

DIAPHRAGM SHEAR DESIGN FORCE 400 PLF REF. TO LOADING DIAG. ON S-003 NET UPLIFT FORCE ASSUME ROOF DIAPHRAGM LOADS AND ROOF UPLIFT LOADS TO BE APPLIED SIMULTANEOUSLY.

RD-3 ROOF DECK AND ITS ANCHORAGE TO SUPPORTING MEMBERS SHALL MEET THE FOLLOWING MINIMUM FASTENING REQUIREMENTS A. AT ENDS OF UNITS AND AT ALL INTERMEDIATE SUPPORTS: BY PUDDLE WELDS NOT LESS THAN 5/8 INCH DIAMETER SPACED NOT MORE THAN 12 INCHES ON CENTER MAX. B. SIDE LAPS OF ADJACENT UNITS: SHALL BE FASTENED BY SIDE SEAM WELDING OR SIDELAP SCREWS SPACED PER MANUFACTURERS ENGINEERED CALCULATIONS WITH A MAXIMUM SPACING OF 24 INCHES ON CENTER. ARC SEAM WELDS SHALL BE A MINIMUM OF 1-1/2 INCH BY 1/2 INCH.

RD-4 NO LOADS SHALL BE HUNG DIRECTLY FROM STEEL ROOF DECK WITHOUT PRIOR WRITTEN APPROVAL OF THE DECK SUPPLIER AND REVIEW BY THE SER.

RD-5 DECKING CONTRACTOR SHALL COORDINATE DECK OPENING SIZES AND LOCATIONS FROM ARCHITECTURAL AND MEP CONTRACT DOCUMENTS, PROVIDE HEADER MEMBERS OR REINFORCEMENT AS REQUIRED BY TYPICAL DETAILS EVEN IF NOT SHOWN ON THE PLANS, AND SUBMIT PROPOSED OPENINGS THROUGH SLAB/DECK FOR REVIEW BY THE DESIGN PROFESSIONALS.

SI SPECIAL INSPECTIONS

SI-1 THE FOLLOWING STRUCTURAL ITEMS REQUIRE SPECIAL TESTING AND/OR INSPECTIONS:

CAST-IN-PLACE CONCRETE CONCRETE REINFORCEMENT CONCRETE FORMWORK STRUCTURAL STEEL DETAILS STEEL DECK

PA POST-INSTALLED ANCHORS

PA-1 ADHESIVE ANCHOR SYSTEMS USED FOR DESIGN:

SEISMIC DESIGN CATEGORY A - F

ADHESIVE: HILTI HIT-HY 200

THREADED ROD: HILTI HAS OR

THREADED ROD: HILTI HIT-Z

OVERHEAD AND/OR CONSTANT TENSION ADHESIVE ANCHOR INSTALLATIONS NOT SHOWN ON THE DRAWINGS SHALL NOT BE PERMITTED UNLESS.

PA-2 PROOF TESTING OF ADHESIVE ANCHORS SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. UNLESS NOTED OTHERWISE, ADHESIVE ANCHOR PROOF TENSION LOADS SHALL BE PER THE ADHESIVE ANCHOR PROOF SCHEDULES. PA-3 FIELD DRILLED EXPANSION ANCHOR SYSTEMS USED FOR DESIGN:

HILTI KWIK BOLT TZ2

COURSE BELOW THE ANCHOR, UON.

PA-4 PROOF TESTING OF EXPANSION ANCHORS SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. UNLESS NOTED OTHERWISE, EXPANSION ANCHOR PROOF TORQUE LOADS SHALL BE PER THE EXPANSION ANCHOR PROOF SCHEDULES.

PA-5 FIELD DRILLED THREADED SCREW ANCHOR SYSTEMS USED FOR DESIGN:

EACH CONDITION IS REVIEWED AND APPROVED IN WRITING BY THE SER.

HILTI KH-EZ

PA-6 ALTERNATIVE SYSTEM EQUIVALENT TO OR EXCEEDING THE PROPERTIES OF THE SYSTEMS ABOVE WILL BE CONSIDERED AS A SUBSTITUTION REQUEST. SEE PROJECT SPECIFICATIONS.

PA-7 ANCHORS ARE TO BE MINIMUM [3/4]" DIAMETER WITH A MINIMUM EMBEDMENT OF [6]", UON.

PA-8 INSTALL ANCHORS TO MEET THE REQUIREMENTS INDICATED IN THE CONTRACT DOCUMENTS AND THE CURRENT MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS (MPII).

PA-9 LOCATE, BY NON-DESTRUCTIVE MEANS, AND AVOID ALL EXISTING REINFORCEMENT PRIOR TO INSTALLATION OF ANCHORS. IF EXISTING REINFORCING LAYOUT PROHIBITS THE INSTALLATION OF ANCHORS AS INDICATED ON THE DRAWINGS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE DESIGN PROFESSIONALS.

PA-10 INSTALL ANCHORS IN SOLID MASONRY OR IN HOLLOW MASONRY THAT HAS BEEN GROUTED SOLID AT LEAST ONE COURSE ABOVE TO ONE

PA-11SEE PROJECT SPECIFICATIONS FOR POST-INSTALLED ANCHOR INSPECTION REQUIREMENTS.



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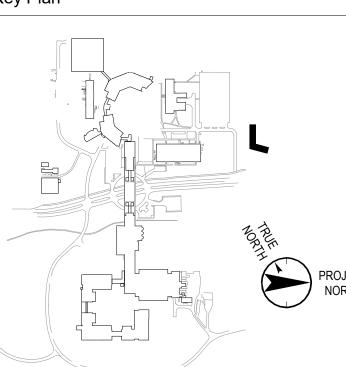
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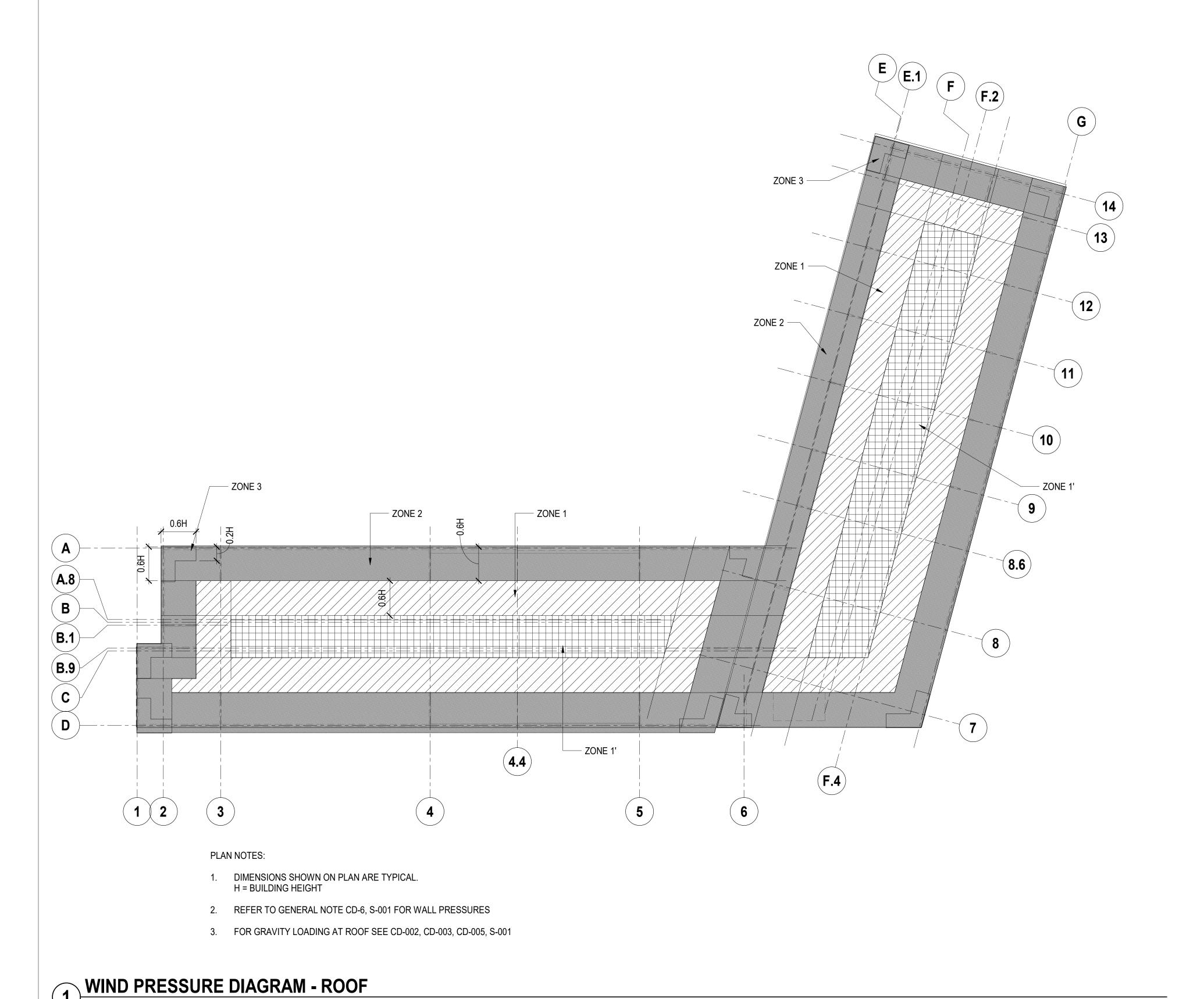
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Floor:



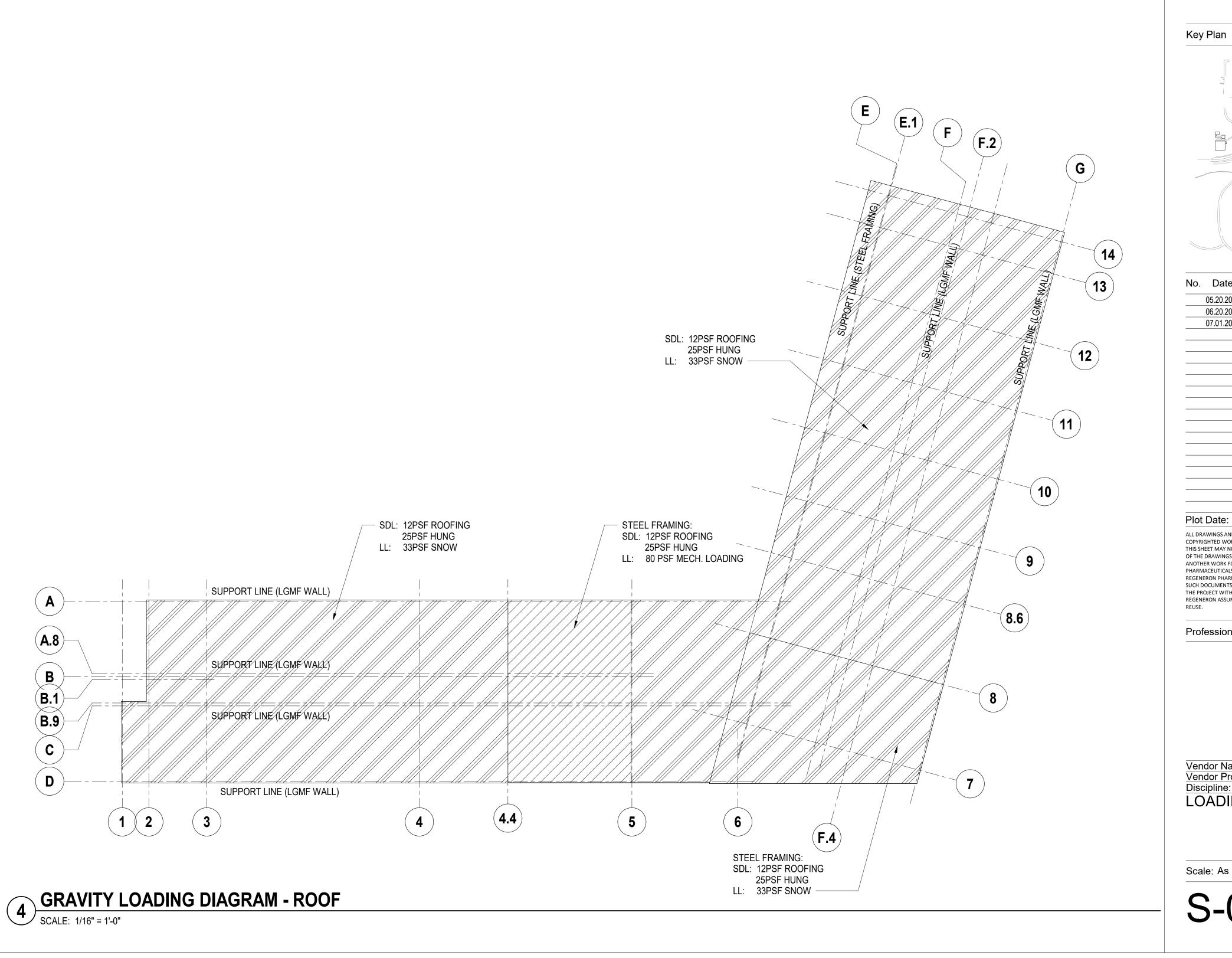
EFFECTIVE AREA (FT^2)	AREA 1' (PSF)	AREA 1 (PSF)	AREA 2 (PSF)	AREA 3 (PSF)
10	+16/-32	+16/-56	+16/-74	+16/-101
20	+16/-27.1	+16/-52	+16/-69	+16/-91
50	+16/-32	+16/-48	+16/-63	+16/-79
100	+16/-32	+16/-44	+16/-58	+16/-69
500	+16/-22	+16/-35	+16/-47	+16/-47

TABLE 1 NOTES:

- (-) NEGATIVE PRESSURE ACT OUTWARD FROM BUILDING
- (+) POSITIVE PRESSURES ACT INWARD TOWARD THE BUILDING
 THESE PRESSURES REPRESENT THE EFFECTS OF THE WIND ON THE TOP SURFACES OF THE ROOFS.
 THE VALUES SHOWN MAY BE USED TO ASSESS PRESSURES FOR:
- A. ROOFING ASSEMBLIES
 B. STRUCTURAL STEEL FRAMING
- C. OPEN-WEB JOISTS

 5. PORTIONS OF THE LIGHT GUAGE TRUSSES THAT DO NOT HAVE A BOTTOM CHORD EXPOSURE.
- WIND PRESSURE SCHEDULE

 SCALE: 12" = 1'-0"



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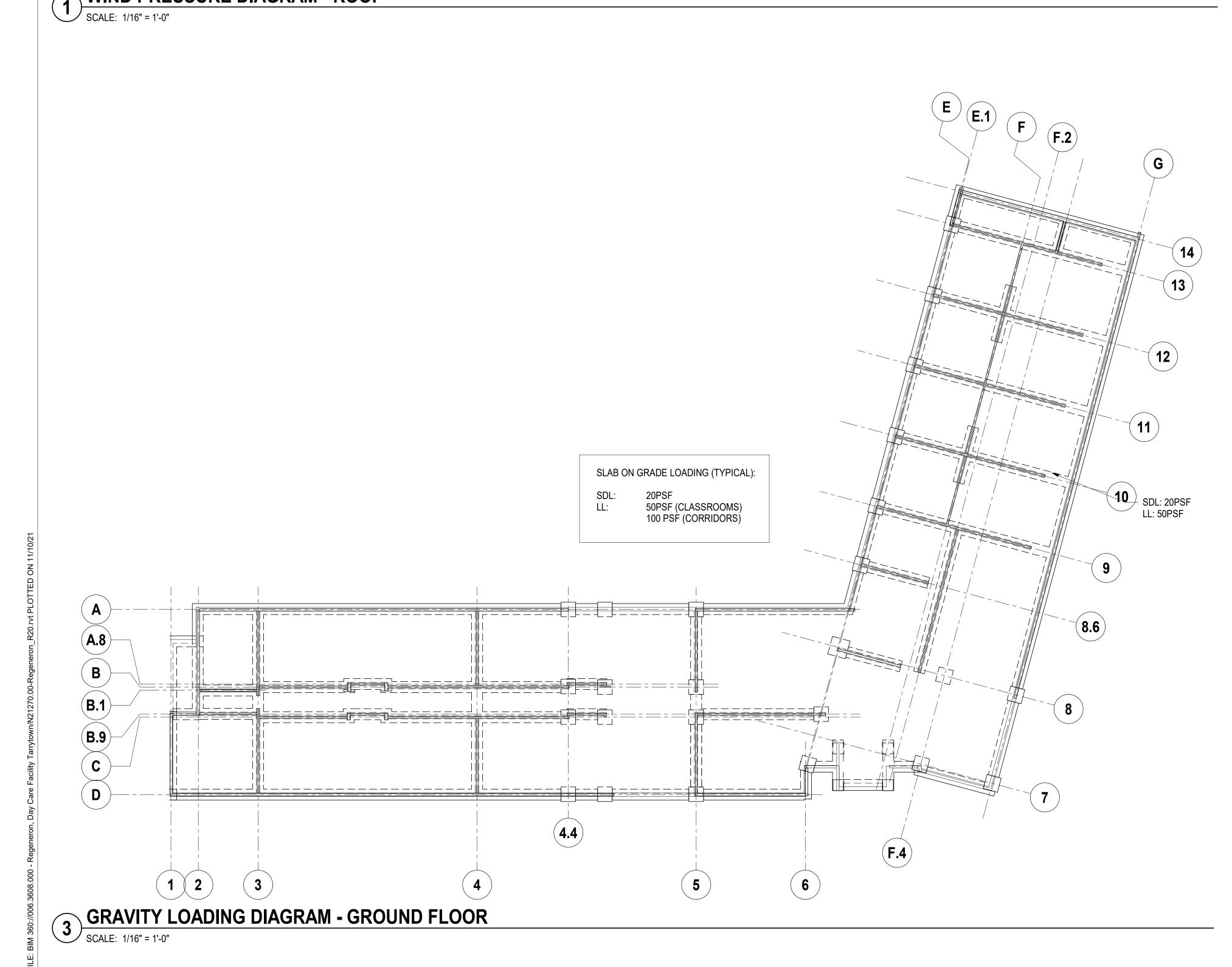
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Vendor Name: GENSLER Vendor Project No.: 006.3608.000 LOADING DIAGRAMS



					RCEMEN E (INCHE		SEE NOTE 5
BAR	MINIMUM BAR			TENSIO	N (LTS)		
SIZE	SPACING (INCHES)	f'c = 3 KSI	f'c = 4 KSI	f'c = 5 KSI	f'c = 6 KSI	f'c = 7 KSI	f'c = 8 KSI
#4	5.500	22	19	17	16	14	14
#5	5.375	32	28	25	23	21	20
#6	5.250	43	37	34	31	28	27
#7	5.125	69	60	54	49	46	43
#8	5.000	86	74	67	61	56	53

					DE	VE	LO	PN	1EN	IT	LEN	IGT	Ή ;	SCI	HEI	DU	LE	(IN	СН	ES)								SE	EE NO	TE {
											TEN	SIOI	N												CON	IPR	ESS	1018	I		
DAD	MINIMUM BAR			NC	OTED A	AS Ld	ON D	RAWI	NGS					NOT	ED AS	S Ldh	ON D	RAWI	NGS					NOT	ED AS	S Ldc	ON D	RAWI	NGS		
BAR	SPACING					f'c	(PSI)									f'c (PSI)									f'c (PSI)				
SIZE	(INCHES) [MAX(1",db) + db] NOTE 2	3000	4000	2000	6000	7000	8000	0006	10,000	11,000	12,000	3000	4000	2000	0009	7000	8000	0006	10,000	11,000	12,000	3000	4000	2000	6000	2000	8000	0006	10,000	11,000	12 000
#4	1.500	22	19	17	16	15	14	13	12	12	12	11	10	9	8	8	7	7	6	6	6	11	10	9	9	9	9	9	9	9	9
#5	1.625	28	24	22	20	18	17	16	15	15	15	14	12	11	10	9	9	8	8	8	8	14	12	12	12	12	12	12	12	12	12
#6	1.750	33	29	26	24	22	21	19	18	18	18	17	15	13	12	11	11	10	9	9	9	17	15	14	14	14	14	14	14	14	14
#7	1.875	48	42	38	34	32	30	28	27	27	27	20	17	15	14	13	12	12	11	11	11	20	17	16	16	16	16	16	16	16	16
#8	2.000	55	48	43	39	36	34	32	30	30	30	22	19	17	16	15	14	13	12	12	12	22	19	18	18	18	18	18	18	18	18
#9	2.375	62	54	48	44	41	38	36	34	34	34	25	22	20	18	17	16	15	14	14	14	25	22	21	21	21	21	21	21	21	2
#10	2.625	70	61	54	50	46	43	41	39	39	39	28	25	22	20	19	18	17	16	16	16	28	25	23	23	23	23	23	23	23	23
#11	2.875	78	67	60	55	51	48	45	43	43	43	31	27	24	22	21	19	18	17	17	17	31	27	26	26	26	26	26	26	26	26

DEVELOPMENT LENGTH SCHEDULE NOTES:

- 1. WHERE MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST BELOW THE DEVELOPMENT LENGTH, MULTIPLY Ld BY 1.3.
- 2. WHERE STIRRUPS OR TIES ARE NOT PRESENT THROUGHOUT Ld, MINIMUM BAR SPACING MUST BE INCREASED TO [MAX(1", db) + 2db] FOR SCHEDULED VALUES TO BE APPLICABLE.

LAP SPLICE NOTES:

- 1. TABULATED VALUES ARE PER ACI 318-11 REQUIREMENTS FOR NORMALWEIGHT CONCRETE. THE VALUES ON THIS SHEET DO NOT APPLY TO LIGHTWEIGHT CONCRETE
- 2. SEE TYPICAL DETAILS FOR CLEAR COVER
- 3. MINIMUM BAR SPACING DIAGRAM "S"

 ∞ ∞ 8 8

- FIRST BAR O SECOND BAR PLACED OR SPLICE BAR
- 4. WHERE ACTUAL CONDITIONS DIFFER FROM THE CLEAR COVER SHOWN ON THE TYPICAL DETAILS OR DIFFER FROM PROVIDED SCHEDULED BAR SIZE MINIMUM SPACING AND/OR f'c, LENGTHS SHALL BE ADJUSTED ONLY WITH THE APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
- TABULATED VALUES ARE FOR NON-EPOXY COATED GRADE 60 REINFORCEMENT IN NORMALWEIGHT CONCRETE

FOR EPOXY COATED REINFORCEMENT: MULTIPLY Ld BY 1.5 MULTIPLY Ldh BY 1.2

Ldc IS NOT AFFECTED MULTIPLY LTS BY 1.3 FOR "TOP BARS" MULTIPLY LTS BY 1.5 FOR ALL OTHER REINFORCEMENT

FOR GRADE 75 REINFORCEMENT: MULTIPLY Ld, Ldh, Ldc, AND LTS BY 1.25

MULTIPLY Lcs BY 1.45

- WHERE BARS OF DIFFERENT SIZES ARE LAP SPLICED IN TENSION, THE TENSION LAP SPLICE LENGTH (LTS) SHALL BE THE LARGER OF THE TENSION DEVELOPMENT LENGTH (Ld) OF THE LARGER BAR AND THE TENSION LAP SPLICE LENGTH OF THÉ SMALLER BAR.
- 7. WHERE BARS OF DIFFERENT SIZES ARE LAP SPLICED IN COMPRESSION, THE COMPRESSION LAP LENGTH (LCS) SHALL BE THE LARGER OF THE COMPRESSION DEVELOPMENT LENGTH (Ldc) OF THE LARGER BAR OR THE COMPRESSION LAP SPLICE LENGTH OF THE SMALLER BAR.
- 8. "TOP BARS" ARE DEFINED AS HORIZONTAL REINFORCEMENT PLACED SUCH THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST BELOW THE DEVELOPMENT LENGTH OR SPLICE "OTHER BARS" ARE ALL BARS FOR WHICH THIS DOES NOT APPLY

						DE BEAN PLICE LE									SEE NOTE 5
BAR	MINIMUM BAR							TENSIO	N (LTS)						
SIZE	SPACING (INCHES)	f'c = 4	4 KSI	f'c =	5 KSI	f'c =	6 KSI	f'c = '	7 KSI	f'c =	8 KSI	f'c =	9 KSI	f'c = 1	0 KSI
	of Adillo (INDITES)	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER
#4	1.500	33	25	29	23	27	21	25	19	23	18	22	17	21	16
#5	1.625	41	31	36	28	33	26	31	24	29	22	27	21	26	20
#6	1.750	49	37	44	34	40	31	37	28	35	27	33	25	31	24
#7	1.875	71	54	63	49	58	45	54	41	50	39	47	36	45	35
#8	2.000	81	62	72	56	66	51	61	47	57	44	54	42	51	39
#9	2.375	91	70	81	63	74	57	69	53	64	50	61	47	58	44
#10	2.625	102	79	92	71	84	64	77	60	72	56	68	53	65	50
#11	2.875	114	87	102	78	93	71	86	66	80	62	76	58	72	55

						OOTING/I				ES)					SEE NOTE 5
BAR	MINIMUM BAR							TENSION I	LAP (LTS)						
SIZE		f'c = 4	4 KSI	f'c = 5	KSI	f'c = 6	KSI	f'c = 7	' KSI	f'c = 8	KSI	f'c = 9	KSI	f'c = 1	0 KSI
SIZE	SPACING (INCHES)	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER
#4	5.500	20	15	18	14	16	13	15	12	14	12	13	12	13	12
#5	5.375	25	19	22	17	20	16	19	14	18	14	17	13	16	12
#6	5.250	29	23	26	20	24	19	22	17	21	16	20	15	19	15
#7	5.125	43	33	38	29	35	27	32	25	30	23	29	22	27	21
#8	5.000	49	37	44	34	40	31	37	28	35	27	33	25	31	24
#9	4.875	63	49	57	44	52	40	48	37	45	35	42	33	40	31
#10	4.750	82	63	74	57	67	52	62	48	58	45	55	42	52	40
#11	4.625	104	80	93	72	85	65	79	61	74	57	69	54	66	51

		FOUND				EMENT -		AL INSIDI HES)	E BARS		SEE NOTE 5
BAR	MINIMUM BAR				T	ENSION (LT	S)				COMPRESSION
SIZE	SPACING (INCHES)	f'c = 4 KSI	f'c = 5 KSI	f'c = 6 KSI	f'c = 7 KSI	f'c = 8 KSI	f'c = 9 KSI	f'c = 10 KSI	f'c = 11 KSI	f'c = 12 KSI	(LCS)
#4	5.500	15	14	13	12	12	12	12	12	12	15
#5	5.375	23	20	18	17	16	15	14	14	14	19
#6	5.250	31	28	25	23	22	21	20	20	20	23
#7	5.125	50	45	41	38	35	33	32	32	32	27
#8	5.000	62	56	51	47	44	42	39	39	39	30
#9	4.875	76	68	62	57	54	51	48	48	48	34
#10	4.750	92	82	75	69	65	61	58	58	58	39
#11	4.625	108	97	89	82	77	72	69	69	69	43

		FOUND		ALL REIN				TAL INSII HES)	DE BARS		SEE NOTE 5
BAR	MINIMUM BAR				T	ENSION (LT	S)				COMPRESSION
SIZE	SPACING (INCHES)	f'c = 4 KSI	f'c = 5 KSI	f'c = 6 KSI	f'c = 7 KSI	f'c = 8 KSI	f'c = 9 KSI	f'c = 10 KSI	f'c = 11 KSI	f'c = 12 KSI	(LCS)
#4	5.500	20	18	16	15	14	13	13	13	13	15
#5	5.375	25	22	20	19	18	17	16	16	16	19
#6	5.250	29	26	24	22	21	20	19	19	19	23
#7	5.125	48	43	39	36	34	32	31	31	31	27
#8	5.000	61	54	50	46	43	41	39	39	39	30
#9	4.875	75	67	61	57	53	50	47	47	47	34
#10	4.750	91	82	75	69	65	61	58	58	58	39
#11	4.625	109	97	89	82	77	73	69	69	69	43

		FOUND	ATION W LAP		NFORCE LENGTH			_	DE BARS		SEE NOTE 5
BAR	MINIMUM BAR				TI	ENSION (LT	S)				COMPRESSION
SIZE	SPACING (INCHES)	f'c = 4 KSI	f'c = 5 KSI	f'c = 6 KSI	f'c = 7 KSI	f'c = 8 KSI	f'c = 9 KSI	f'c = 10 KSI	f'c = 11 KSI	f'c = 12 KSI	(LCS)
#4	5.500	15	14	13	12	12	12	12	12	12	15
#5	5.375	19	17	16	14	14	13	12	12	12	19
#6	5.250	23	20	19	17	16	15	15	15	15	23
#7	5.125	33	29	27	25	23	22	21	21	21	27
#8	5.000	37	34	31	28	27	25	24	24	24	30
#9	4.875	49	44	40	37	35	33	31	31	31	34
#10	4.750	63	57	52	48	45	42	40	40	40	39
#11	4.625	EOUNDA'	72 TΙΟΝ WΔ	II RFINI	FORCEM	57 FNT - HC	PIZONT	51 ΔΙ OUTS	IDF RAR	51 S	43
#11			TION WA	LL REIN	· · · · · · · · · · · · · · · · · · ·	ENT - HC	RIZONT	AL OUTS	-	L	
#11			TION WA	LL REIN	FORCEM LENGTH	ENT - HC	RIZONTA JLE (INC	AL OUTS	-	L	SEE NOTE 5
	F		TION WA	LL REIN	FORCEM LENGTH	ENT - HC	RIZONTA JLE (INC	AL OUTS	-	L	SEE NOTE 5
BAR	MINIMUM BAR	OUNDA	TION WA	LL REINI SPLICE	FORCEM LENGTH	ENT - HC SCHEDU ENSION (LT	RIZONTA JLE (INC S)	AL OUTS HES)	IDE BAR	S	SEE NOTE 5
BAR SIZE	MINIMUM BAR SPACING (INCHES)	Fc = 4 KSI	TION WA LAP	SPLICE	FORCEM LENGTH TI	ENT - HC SCHEDU ENSION (LT	PRIZONTA JLE (INC S)	AL OUTS HES)	f'c = 11 KSI	S f'c = 12 KSI	SEE NOTE 5 COMPRESSION (LCS)
BAR SIZE	MINIMUM BAR SPACING (INCHES) 5.500	F'c = 4 KSI 20	TION WA LAP f'c = 5 KSI 18	SPLICE f'c = 6 KSI 16	FORCEM LENGTH TI f'c = 7 KSI	ENT - HC SCHEDU ENSION (LT fc = 8 KSI	PRIZONTA JLE (INC S) fc = 9 KSI	AL OUTS HES) fc = 10 KSI	f'c = 11 KSI 13	f 'c = 12 KSI 13	SEE NOTE 5 COMPRESSION (LCS) 15
BAR SIZE #4 #5	MINIMUM BAR SPACING (INCHES) 5.500 5.375	F'c = 4 KSI 20 25	TION WA LAP f'c = 5 KSI 18 22	SPLICE f'c = 6 KSI 16 20	FORCEM LENGTH TI fc = 7 KSI 15 19	ENT - HC SCHEDU ENSION (LT f'c = 8 KSI 14 18	PRIZONTA JLE (INC S) f'c = 9 KSI 13 17	AL OUTS HES) f'c = 10 KSI 13 16	f'c = 11 KSI 13 16	f'c = 12 KSI 13 16	SEE NOTE 5 COMPRESSION (LCS) 15 19
BAR SIZE #4 #5 #6	MINIMUM BAR SPACING (INCHES) 5.500 5.375 5.250	F'c = 4 KSI 20 25 29	TION WA LAP f'c = 5 KSI	## C = 6 KSI	FORCEM LENGTH TI fc = 7 KSI 15 19 22	ENT - HC SCHEDU ENSION (LT f'c = 8 KSI 14 18 21	PRIZONTA JLE (INC S) f'c = 9 KSI 13 17 20	AL OUTS HES) f'c = 10 KSI 13 16 19	f'c = 11 KSI 13 16 19	f 'c = 12 KSI 13 16 19	SEE NOTE 5 COMPRESSION (LCS) 15 19 23
BAR SIZE #4 #5 #6 #7	MINIMUM BAR SPACING (INCHES) 5.500 5.375 5.250 5.125	F'c = 4 KSI 20 25 29 43	TION WA LAP f'c = 5 KSI	### Company of the co	FORCEM LENGTH TI f'c = 7 KSI 15 19 22 32	ENT - HC SCHEDU ENSION (LT f'c = 8 KSI 14 18 21 30	PRIZONTA JLE (INC S) f'c = 9 KSI 13 17 20 29	AL OUTS HES) f'c = 10 KSI 13 16 19 27	f'c = 11 KSI 13 16 19 27	f'c = 12 KSI 13 16 19 27	SEE NOTE 5 COMPRESSION (LCS) 15 19 23 27
#4 #5 #6 #7 #8	MINIMUM BAR SPACING (INCHES) 5.500 5.375 5.250 5.125 5.000	F'c = 4 KSI 20 25 29 43 49	TION WA LAP f'c = 5 KSI	Tc = 6 KSI 16 20 24 35 40	FORCEM LENGTH TI f'c = 7 KSI 15 19 22 32 37	ENT - HC SCHEDU ENSION (LT f'c = 8 KSI 14 18 21 30 35	PRIZONTA JLE (INC S) f'c = 9 KSI 13 17 20 29 33	AL OUTS HES) f'c = 10 KSI 13 16 19 27 31	f'c = 11 KSI 13 16 19 27 31	f'c = 12 KSI 13 16 19 27 31	SEE NOTE 5 COMPRESSION (LCS) 15 19 23 27 30



REGENERON

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Building #17 Campus Expansion Child Day-care Center

777 Old Saw Mill River Road Mount Pleasant, NY 10591 Project No. B17-DAYCARE

Gensler 1700 Broadway, Suite 400 New York, NY 10019 (212) 492-1400 Phone

(212) 492-1472 Fax

Structural Engineer

Thornton Tomasetti 120 Broadway, 15th Floor New York, NY 10271 (917) 661-7800 Phone (917) 661-7801 Fax

MEP / IT / Security Engineer

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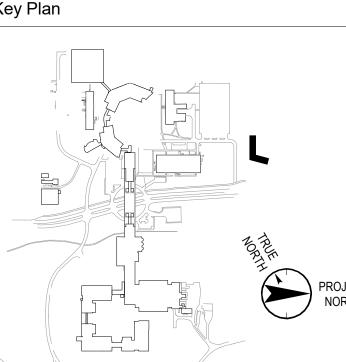
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120 Bedford Road Armonk, NY 10504 (914) 273-5225 Phone (914) 273-2102 Fax

Landscape Architect

Langan 21 Penn Plaza, 360 West 31st Street, 8th Floor New York, NY 10001 (212) 479-5400 Phone (212) 479-5444 Fax

Key Plan



No. Date Description 05.20.2022 ISSUED FOR PERMIT 06.20.2022 100% CONSTRUCTION DOCUMENTS 07.01.2022 100% CONSTRUCTION DOCUMENT- 1

Plot Date: 04/27/10

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Vendor Name: GENSLER
Vendor Project No.: 006.3608.000
Discipline: Drawn By: Author
GENERAL LAP SPLICE
SCHEDULES

Scale: NOT TO SCALE Floor:

- 1. SEE GENERAL NOTES FOR CONCRETE COMPRESSIVE STRENGTH
- 2. T/FOOTING = EL -2'-6" BELOW FF ELEVATION, UON
- 3. TOP OF SLAB EL +217'-3" UON THUS:
- X" X INDICATES CHANGE IN STRUCTURAL SLAB ELEVATION
- 4. SLAB TO BE 6" THICK NORMALWEIGHT CONCRETE SLAB ON GRADE, UON. REINFORCED WITH WWF6X6-W2.9XW2.9
- 5. SEE GENERAL NOTES FOR CONCRETE COMPRESSIVE STRENGTH
- 6. FOR ADDITIONAL INFORMATION REFER TO THE FOLLOWING DRAWINGS: DRAWING LISTS, GENERAL NOTES AND LOADING DIAGRAMS S0 SERIES DRAWINGS

TYPICAL FOUNDATION DETAILS

S2 SERIES DRAWINGS

LATERAL SYSTEM ELEVATIONS, CONNECTION FORCES AND DETAILS

STEEL SUPERSTRUCTURE

SCHEDULES AND DETAILS

S3 SERIES DRAWINGS

S5 SERIES DRAWINGS

INDICATES LGMF LOAD BEARING WALL (BY OTHERS)

REFER TO ARCHITACTURL DRAWINGS FOR FINAL WALL LAYOUTS, LOCATIONS AND GEOMETERY. GC TO COORDINATE WITH LGMF SUB-CONTRACTOR

INDICATES LGMF SHEAR WALL (BY OTHERS)

8. SEE CIVIL DRAWINGS FOR UNDERSLAB DRAINAGE PIPING. ELEVATIONS RANGE FROM EL 215' TO 215.5'. GC TO CONFIRM/COORDINATE PROVIDE PENETRATIONS THRU. KNEE WALLS WHERE REQUIRED. SEE 3/S-202 FOR WALL PENETRATION DETAILS

9. SEE MEP DRAWINGS FOR UNDERSLABD PIPING INFORMATION. ELEVATIONS RANGE FROM EL 215.5' TO 216'. GC TO CONFIRM/COORDINATE. PROVIDE VERTICAL PENETRATIONS THRU. SLABS (REF. 6/S-201) AND HORIZONTAL PENETRATIONS THRU. KNEE WALLS (REF. 3/S-202). VÉRTICAL PENETRATIONS THRU. KNEE WALLS ARE NOT PERMITTED.

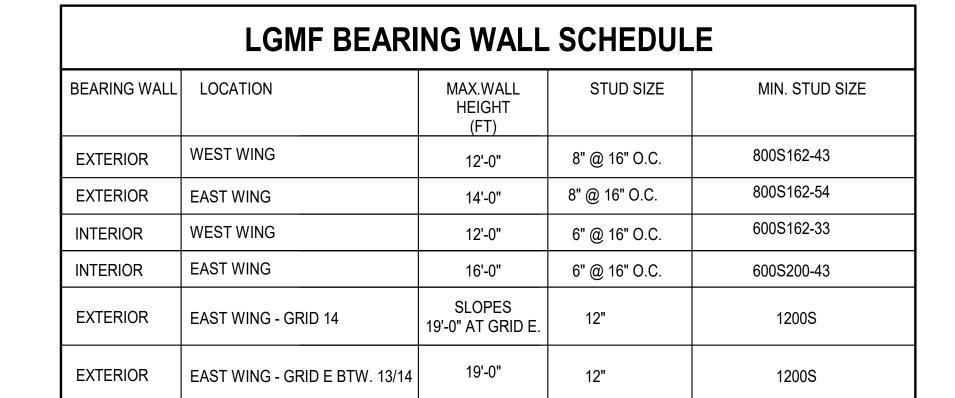
10. EXTERIOR WALL FOOTING TO STEP WHERE REQUIRED TO ACCOMODATE SEWER AND STORMWATER TIE-ING. SEE PLANS FOR LOCATIONS. GC TO COORDINATE. REF. 3/S-200 FOR STEPPED WALL FOOTING DETAILS.

	SIZE REINFORCEMENT		EMENT			
MARK	L	w	н	BOTTOM (SHORT)	TOP BARS	REMARKS
F3	3'-0"	3'-0"	1'-6"	6#5 E.W.	-	-
F4	4'-0"	4'-0"	1'-6"	9#5 E.W.	-	-
F5	5'-0"	5'-0"	1'-6"	8#5 E.W.	-	-

1. REINFORCEMENT SHALL NOT BE SPLICED WITHOUT PRIOR WRITTEN APPROVAL FROM SER.

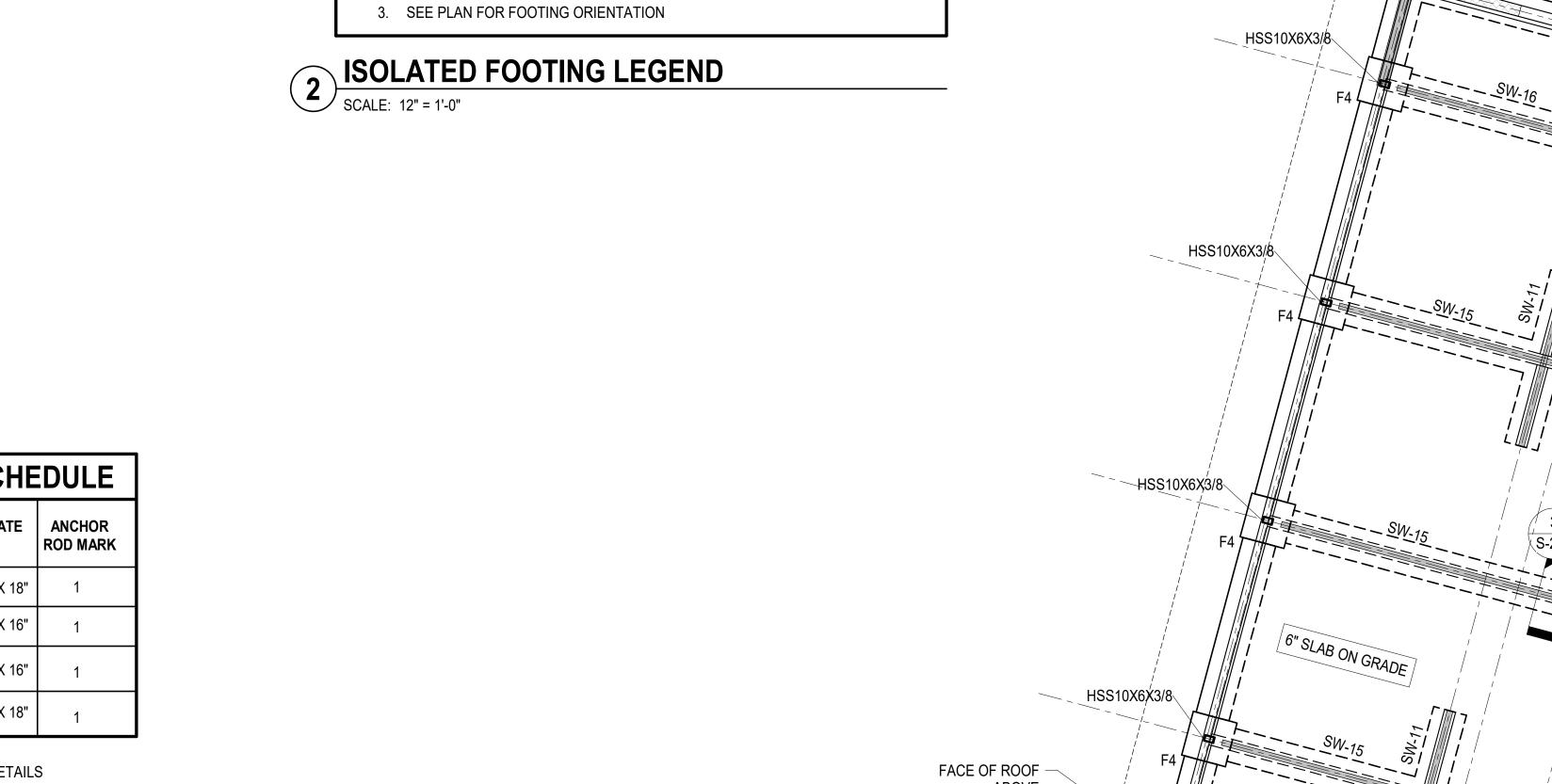
BASE PLA	BASE PLATE SCHE				
COLUMN SIZE	BASE PLATE SIZE	ANCHOR ROD MARK			
HSS10X6	3/4" X 18" X 18"	1			
HSS10X8	3/4" X 16" X 16"	1			
HSS8X8	3/4" X 16" X 16"	1			
HSS10X4	3/4" X 18" X 18"	1			

SEE S-500 FOR COLUMN BASE DETAILS



NOTE: FINAL STUD SIZE AND SPACING TO BE VERIFIED BY SUB-CONTRACTOR BASED ON INFORMATION PROVIDED AND

PROJECT LOADING CRITERIA NOTE IN S-000 SERIES. MINIMUM DEPTH OF STUDS TO BE AS NOTED IN TABLES ABOVE



FOOTING MARK

T/FOOTING EL

THAN TYPICAL

WHERE DIFFERENT

ISOLATED FOOTING LEGEND

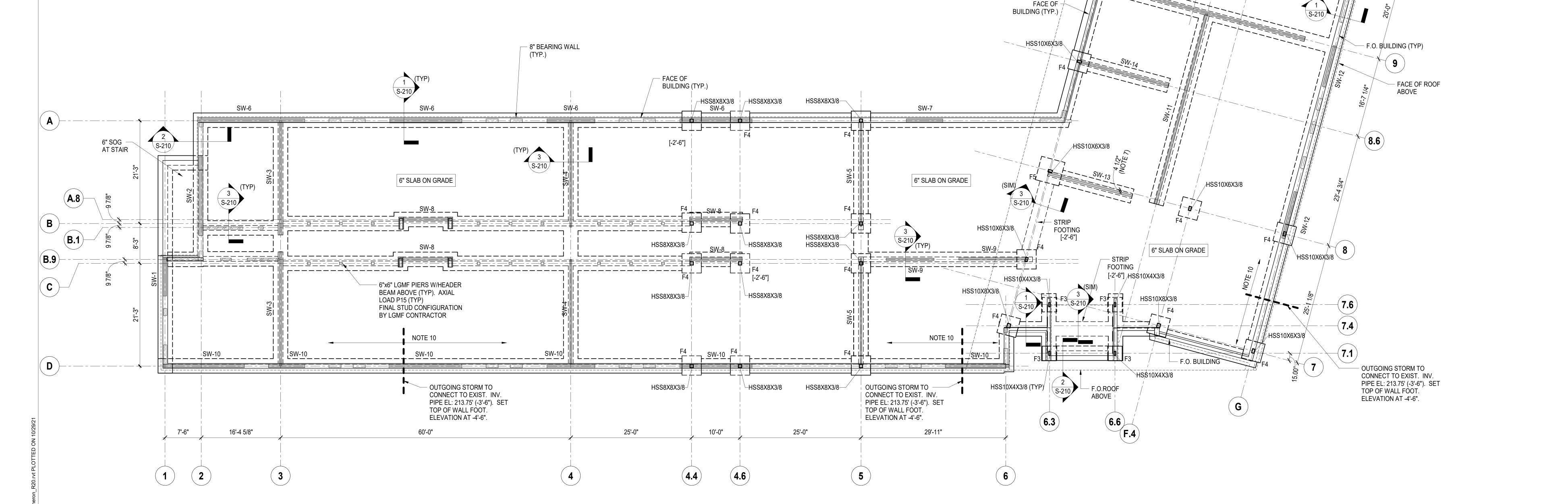
STEEL OR CONCRETE

COLUMN

NOTES:

1. T/FOOTING EL SEE SHEET NOTES

2. SEE TYPICAL ISOLATED FOOTING SCHEDULE AND DETAILS





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New York, NY 10019
(212) 492-1400 Phone

(212) 492-1472 Fax Structural Engineer

Thornton Tomasetti 120 Broadway, 15th Floor New York, NY 10271 (917) 661-7800 Phone

- OUTGOING STORM TO

TOP OF WALL FOOT.

ELEVATION AT -4'-6".

CONNECT TO EXIST. INV.

PIPE EL: 213.75' (-3'-6"). SET

(917) 661-7801 Fax MEP / IT / Security Engineer

Cosentini Associates 498 Seventh Avenue New York, NY 10018 (212) 615-3600 Phone (212) 615-3700 Fax

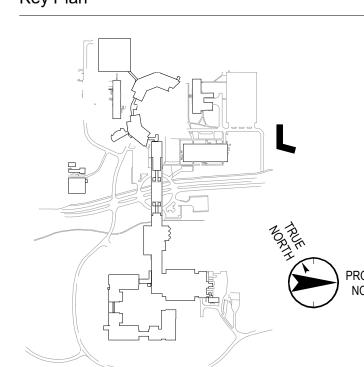
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Landscape Architect

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Key Plan



No. Date Description 05.20.2022 ISSUED FOR PERMIT 06.20.2022 100% CONSTRUCTION DOCUMENTS 07.01.2022 100% CONSTRUCTION DOCUMENT-

Plot Date: 10/29/21

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Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 **GROUND FLOOR PLAN**

1. T/STRUCTURAL SLAB EL+15'-0" UON THUS:

X' X - INDICATES CHANGE IN T/STRUCTURAL SLAB ELEVATION

2. T/STEEL EL+14'-5 3/4" UON

LOADING DIAGRAMS

3. INDICATES DECK SPAN DIRECTION

4. FOR ADDITIONAL INFORMATION REFER TO THE FOLLOWING DRAWINGS: DRAWING LISTS, GENERAL NOTES, AND S0 SERIES DRAWINGS

TYPICAL FOUNDATION DETAILS S2 SERIES DRAWINGS LATERAL SYSTEM ELEVATIONS, CONNECTION S3 SERIES DRAWINGS FORCES, AND DETAILS

STEEL SUPERSTRUCTURE SCHEDULES AND S5 SERIES DRAWINGS

6. SYMBOLS:

INDICATES MOMENT CONNECTION

INDICATES FULL HEIGHT FITTED STIFFENER CONNECTION SEE DETAIL 7/S-501

7. NOTATIONS:

SW INDICATES SHEAR WALL (SEE FOUNDATION PLAN FOR TYP. AND S-300 FOR LOADING) SEE BRACED FRAME ELEVATIONS FOR BEAM INFORMATION AND CONNECTION FORCES CA/CB INDICATES COLUMN ABOVE/BELOW. COMPRESSION AXIAL FORCE IN

COLUMN INDICATED WHERE APPLICABLE. SEE DETAIL 8/S-502 INDICATES TORSIONAL CONNECTION FOR STAIR. SEE DETAIL 8/S-501

8. JOIST ENDS AT SPANDRELS SHALL BE DESIGNED TO BRACE TOP AND BOTTOM FLANGES OF SPANDRELS. SEE TYPICAL DETAILS ON 2/S03.07 FOR DETAILS

9. JOIST BRIDGING SHALL BE DESIGNED FOR TF1.5 TO BRACE TOP AND BOTTOM FLANGES OF WIDE FLANGE AT INDICATED LOCATIONS

10. INDICATES LGMF LOAD BEARING WALL (BY OTHERS) INDICATES LGMF SHEAR WALL (BY OTHERS)

11. PROVIDE CONNECTION AT JOISTS FOR V10, U.O.N ON PLAN. SEE PLAN FOR CONNECTION FORCES AT STEEL FRAMING

BOX BE	AM HEADER SCH	EDULE
HEADER MARK	MAX. LENGTH	MINIMUM SIZE
H1	7'-0"	8000S162-68
H2	12'-0"	1400S162-118
H3	19'-0"	1600S162-118

STAIR (BY OTHERS) - GC TO -

ALONG 2-LINE

COORDINATE BETWEEN TRADES

STAIR LOADS ON LGMF WALLS

(C.3)

	ROOF DECK SCHEDULE						
ROOF DECK MARK	ROOF DECK (MINIMUM)	UNFACTORED DIAPHRAGM SHEAR (PLF)	NOTES				
_ R1	1 1/2"-20 GA TYPE B		UNFACTORED SHEAR CAN BE CALCULATED BY DIVIDING FACTORED SHEAR BY 1.6				

NUMBER OF HEADED STUDS EQUALLY SPACED BETWEEN BEAMS REQUIRED CAMBER (NONE IF OMITTED)					
•					
DENOTES BRACED FRAME(SEE BRACED FRAME ELEVATIONS)					
COLLECTOR CONNECTION					
X'-XX" DEVIATION FROM TYPICAL T/STEEL					
V INDICATES FACTORED VERTICAL SHEAR IN KIPS M INDICATES FACTORED END MOMENT IN KIP-FEET H INDICATES FACTORED HORIZONTAL SHEAR FORCE IN KIPS T INDICATES TORSIONAL MOMENT IN KIP-FEET P INDICATES FACTORED MEMBER AXIAL FORCE IN KIPS TF INDICATES FACTORED AXIAL FORCE TRANSFERRED THROUGH					
F					

BOTTOM CHORD UPLIFT

BRIDGING AT FIRST PANEL

POINT, EACH END (TYP, ALL

EQ

- TOP AND BOTTOM CHORD

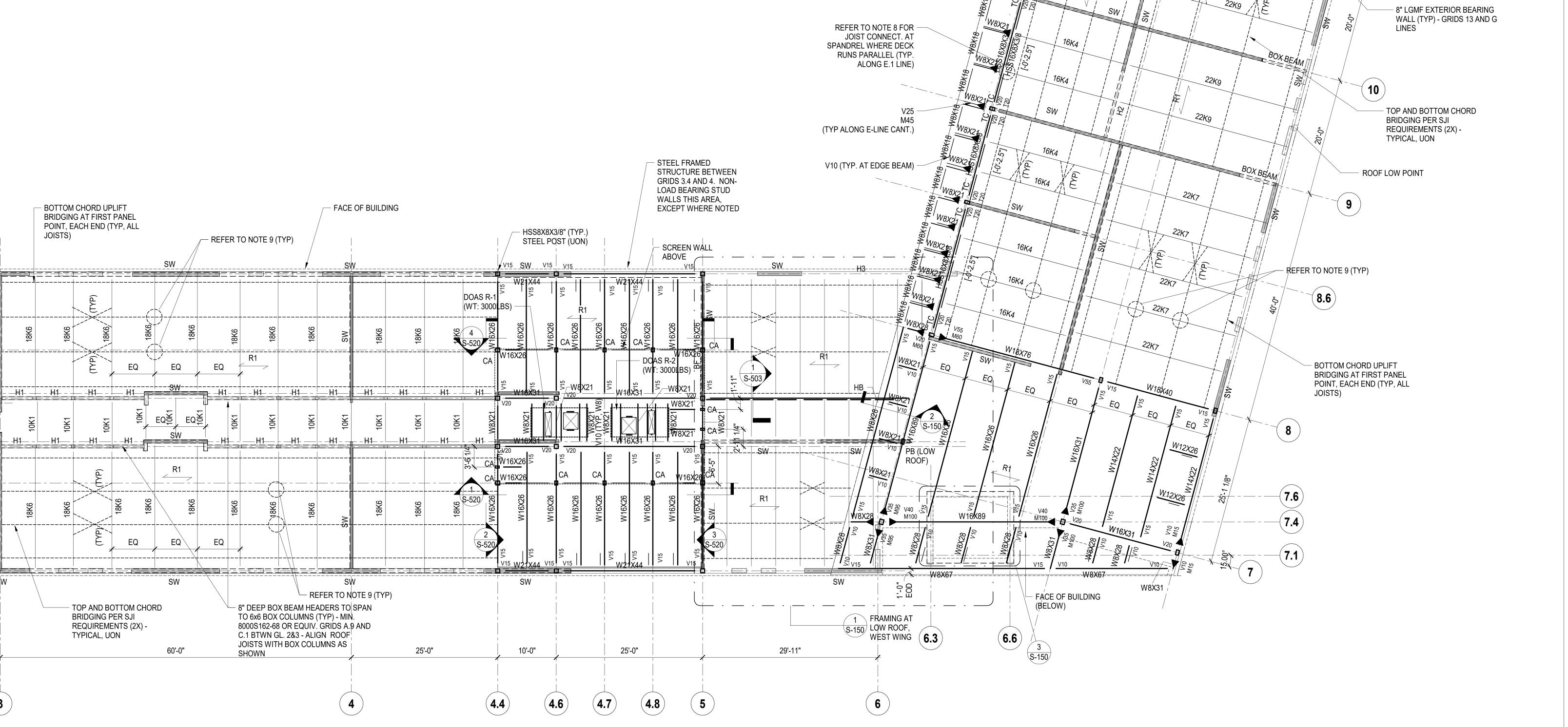
BRIDGING PER SJI

TYPICAL, UON

REQUIREMENTS (2X) -

EQ

EQ\



8" DEEP — CANT. BOX

F.O.BUILDING BELOW

EDGE OF CANOPY -

WT2.5x8 (TYP. AT W8 -

CANT. ALONG E.1 LINE)

LGMF SHEAR WALL TO -

BRACE COLUMN (TYP)

ROOF HIGH POINT

F.O.BUILDING BELOW

22K9



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Gensler 1700 Broadway, Suite 400 New York, NY 10019

TOP AND BOTTOM CHORD

REQUIREMENTS (1X) - THIS

BRIDGING PER SJI

LOCATION ONLY

(13)

12)

(212) 492-1400 Phone (212) 492-1472 Fax Structural Engineer

Thornton Tomasetti 120 Broadway, 15th Floor New York, NY 10271 (917) 661-7800 Phone (917) 661-7801 Fax

MEP / IT / Security Engineer

Cosentini Associates 498 Seventh Avenue New York, NY 10018 (212) 615-3600 Phone (212) 615-3700 Fax

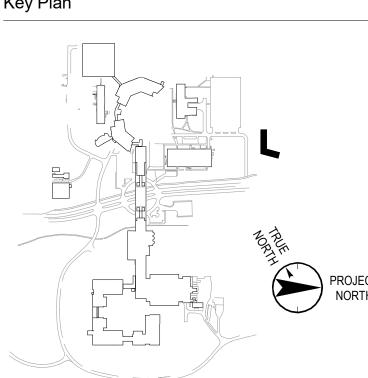
Civil Engineer

120 Bedford Road Armonk, NY 10504 (914) 273-5225 Phone (914) 273-2102 Fax

Landscape Architect

Langan 21 Penn Plaza, 360 West 31st Street, 8th Floor New York, NY 10001 (212) 479-5400 Phone (212) 479-5444 Fax

Key Plan



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Plot Date: 10/29/21

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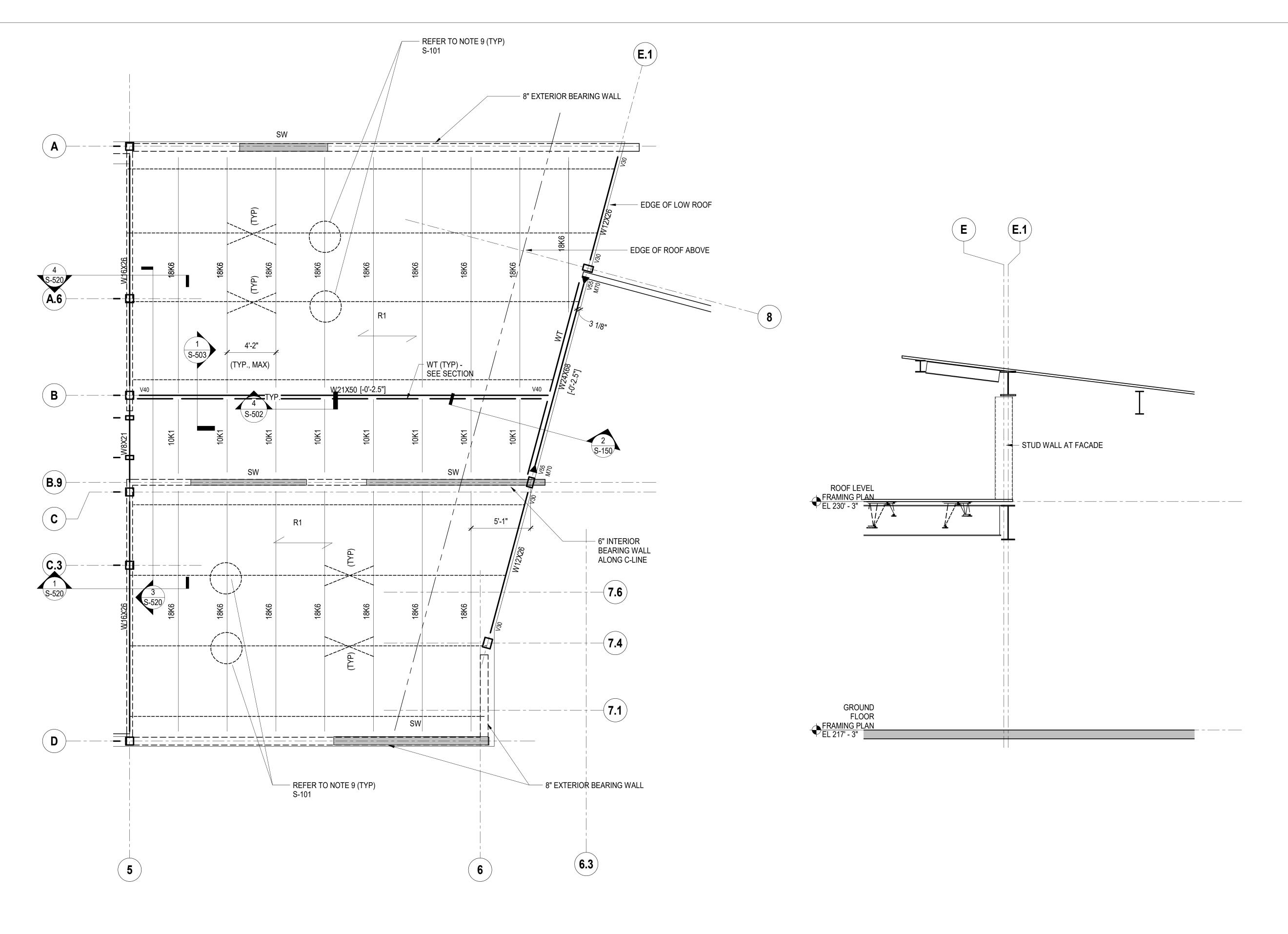
Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 Drawn By: Author **ROOF PLAN**

Scale: As indicated Floor:

ROOF LEVEL FRAMING PLAN

7'-6"

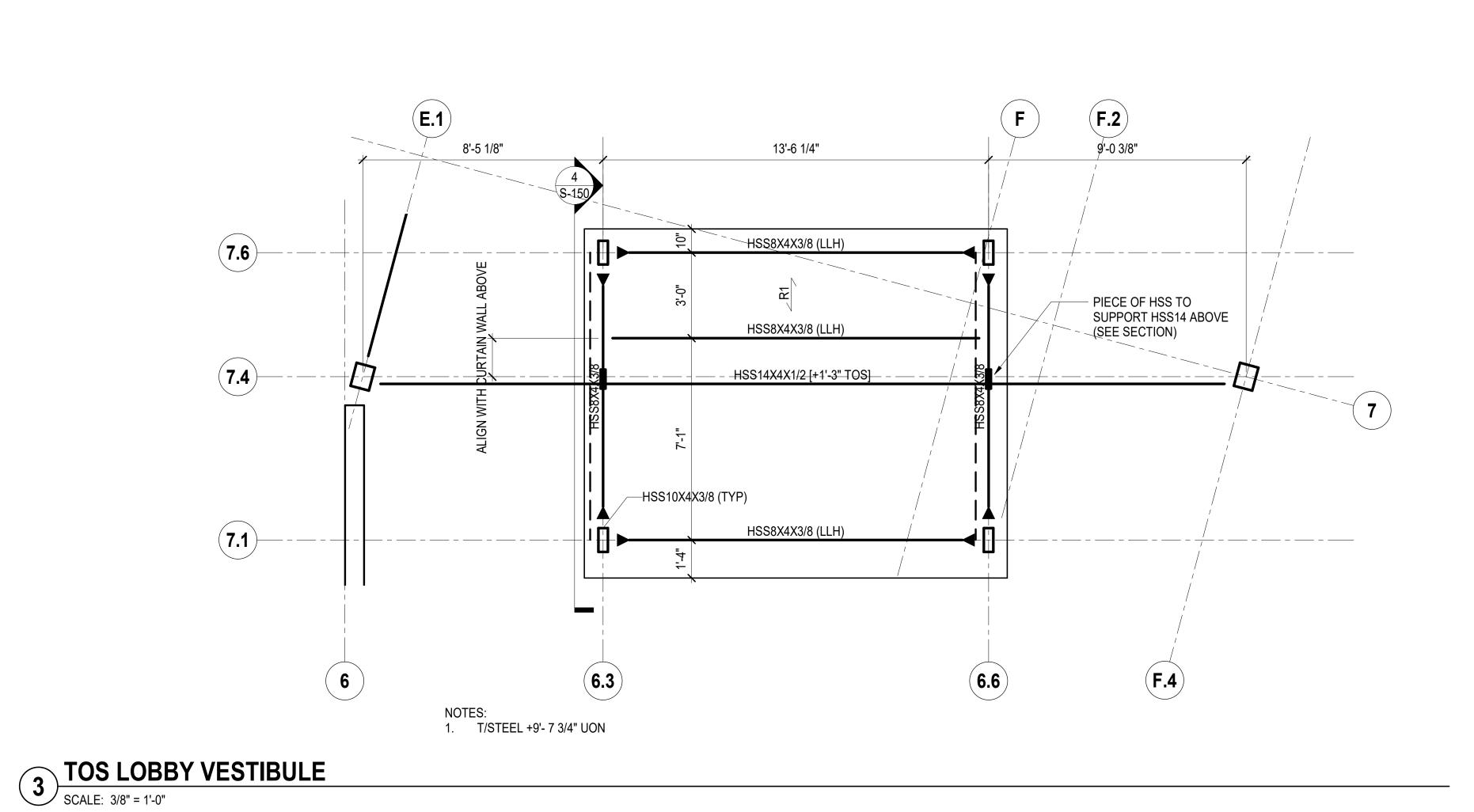


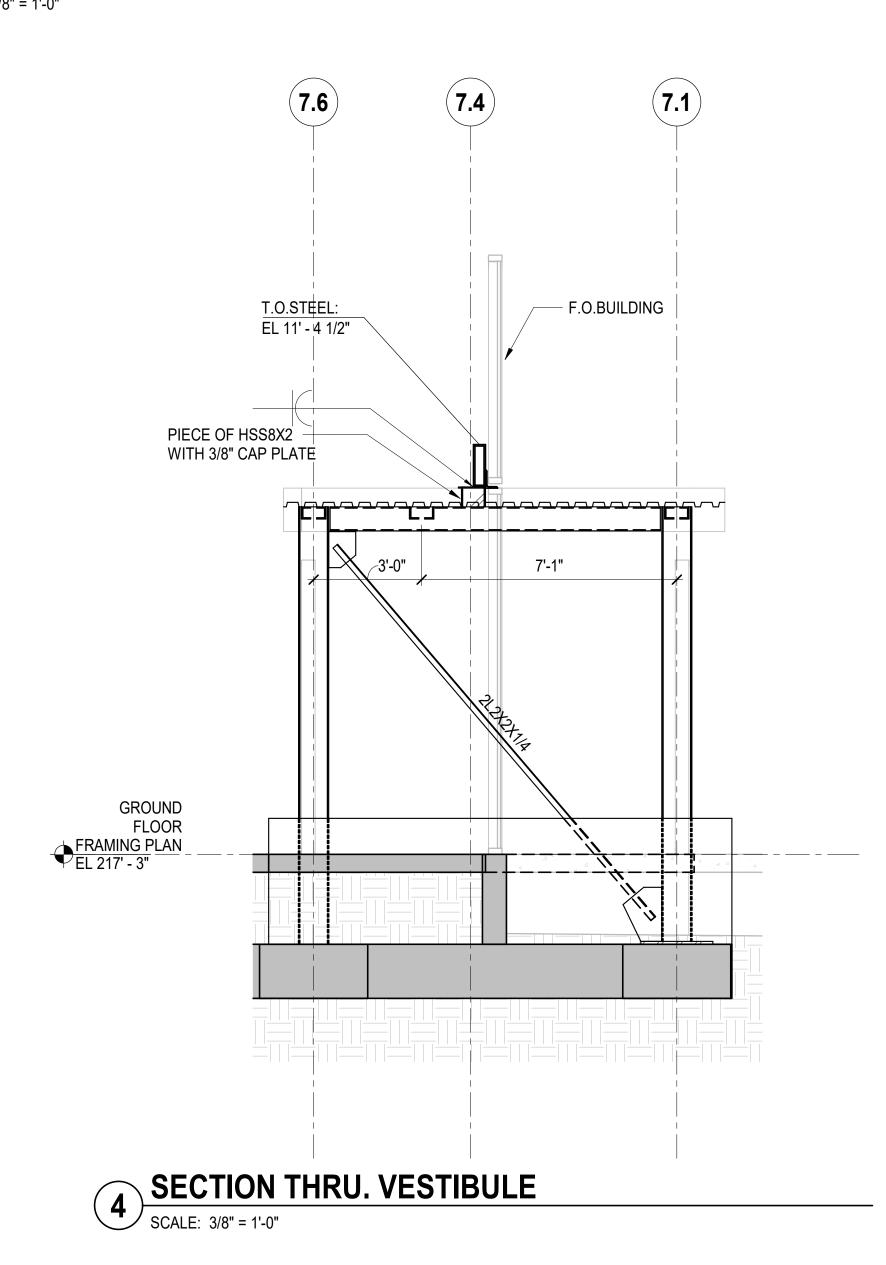
PART PLAN AT LOW ROOF_WEST WING

SCALE: 1/4" = 1'-0"

SECTION THRU. EAST - WEST WINGS

SCALE: 3/8" = 1'-0"





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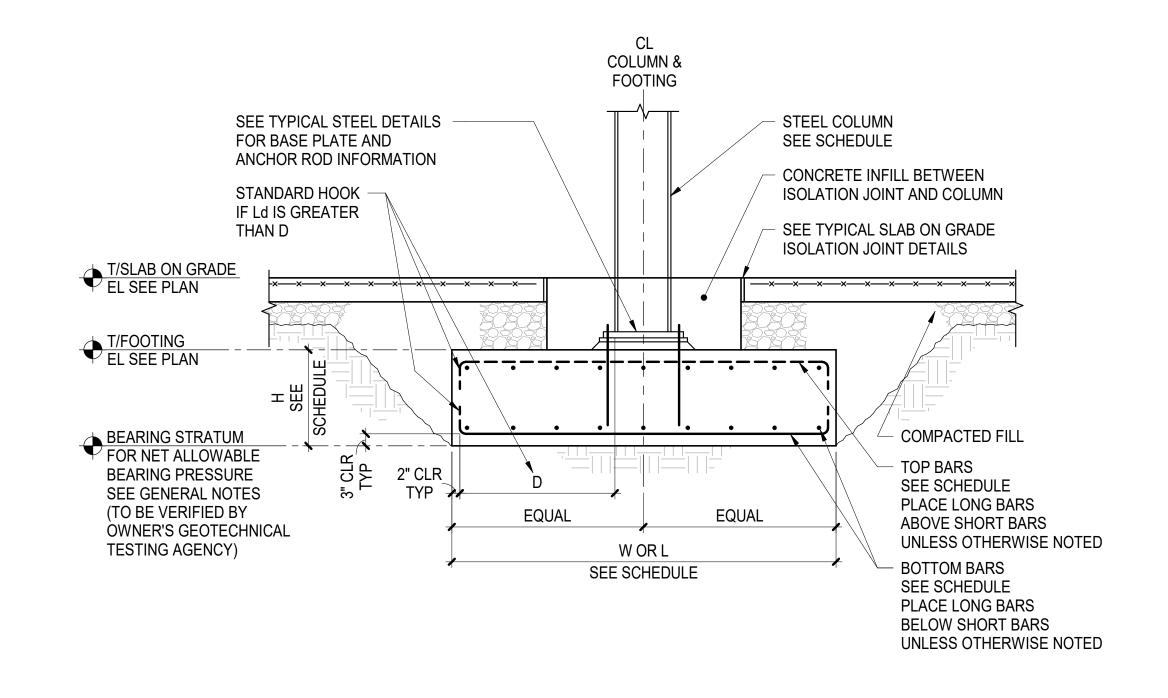
Plot Date: 03/08/22

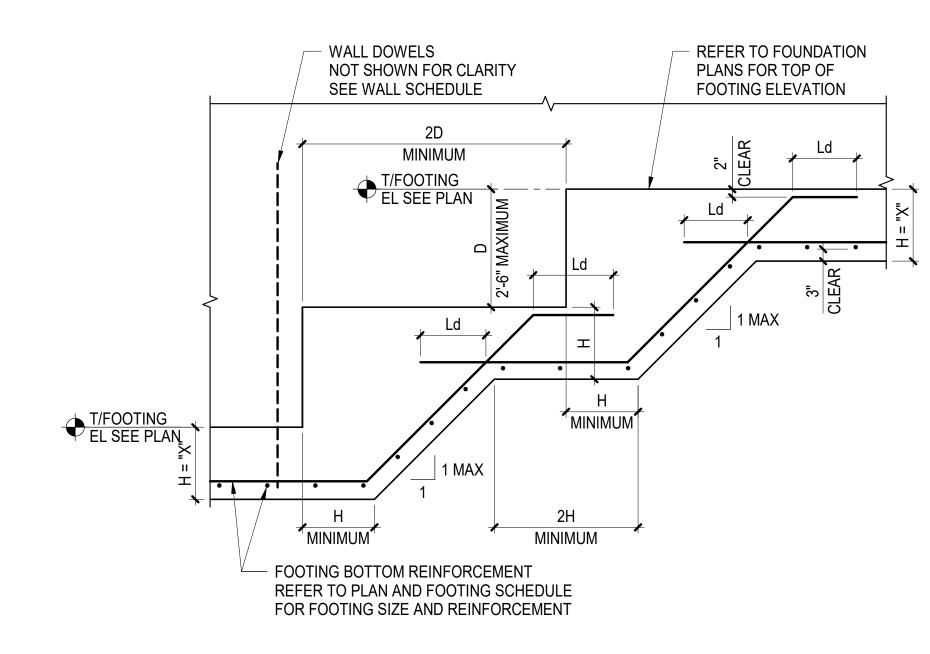
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Professional Seal and Signature

Vendor Name: GENSLER
Vendor Project No.: 006.3608.000
Discipline: Drawn
PART PLANS Drawn By: Author

Scale: As indicated Floor:





TYPICAL ISOLATED FOOTING - BASE PLATE WITHIN CONCRETE INFILL
NOT TO SCALE

2 TYPICAL STEPPED WALL FOOTING DETAIL
SCALE: NOT TO SCALE



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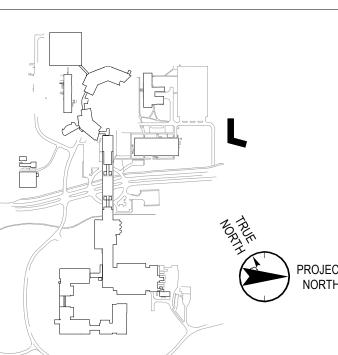
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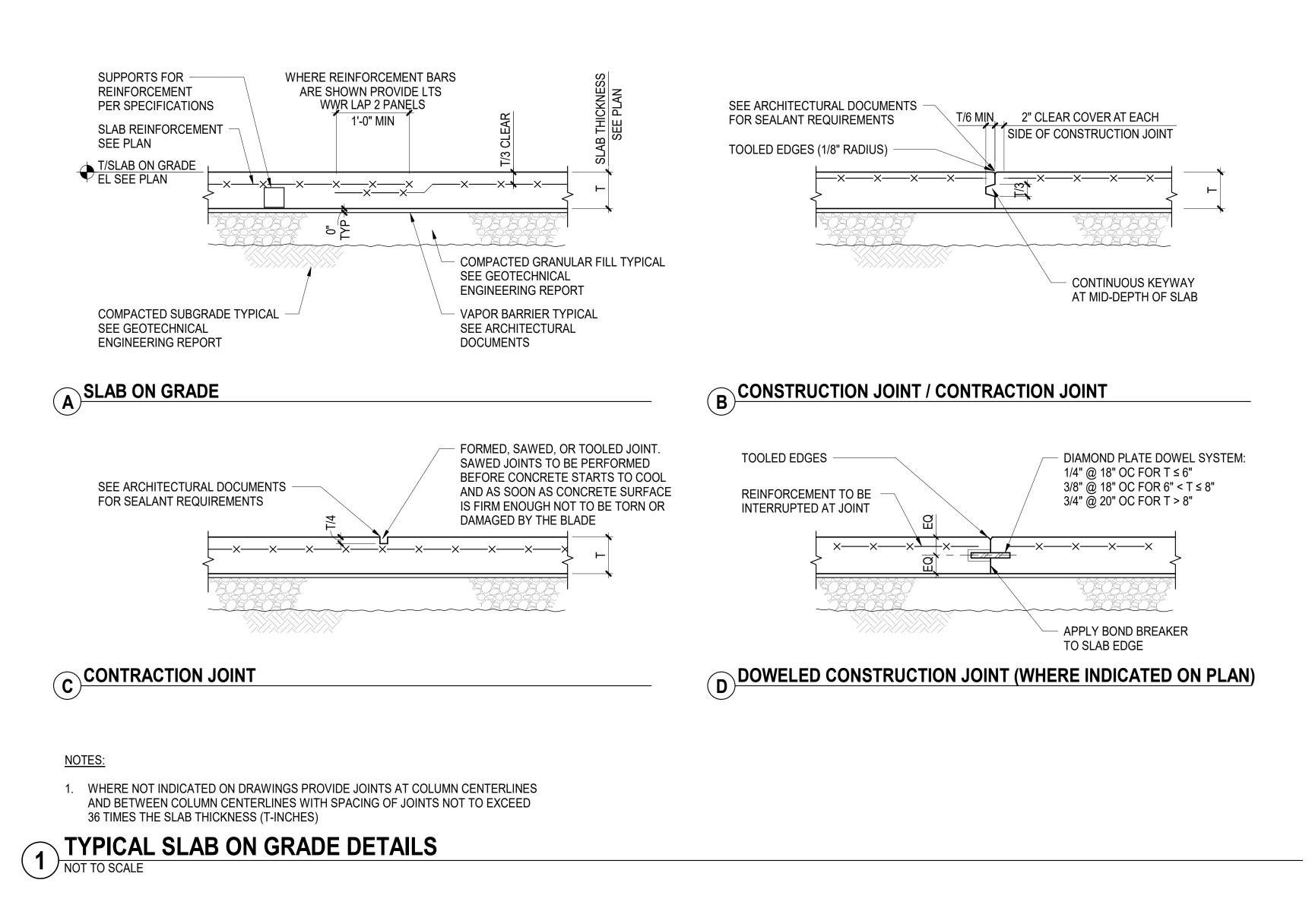
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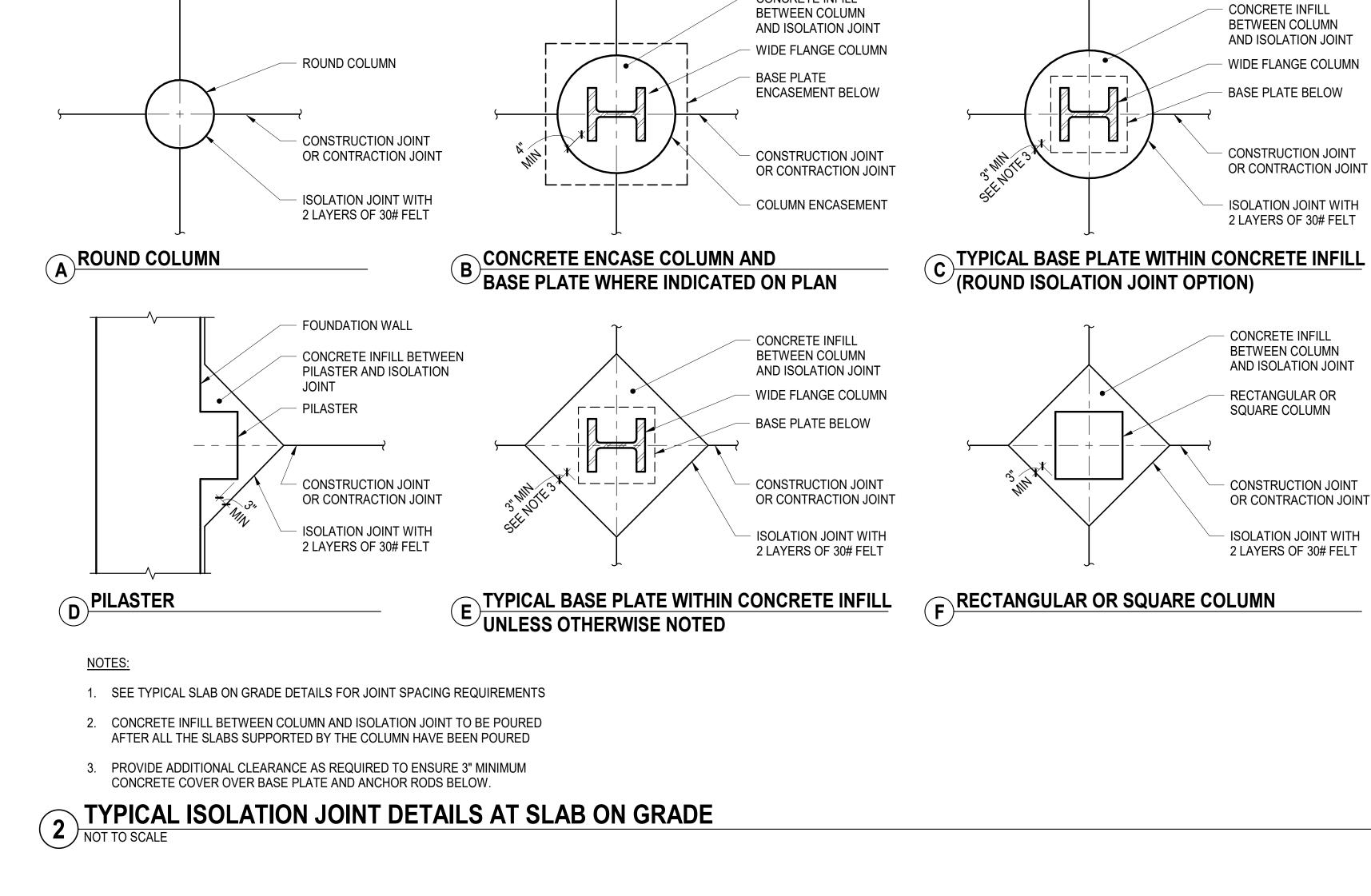
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Vendor Name: GENSLER Vendor Project No.: 006.3608.000 Discipline: Drawn By: Author
TYPICAL FOOTING DETAILS

Scale: NOT TO SCALE Floor:

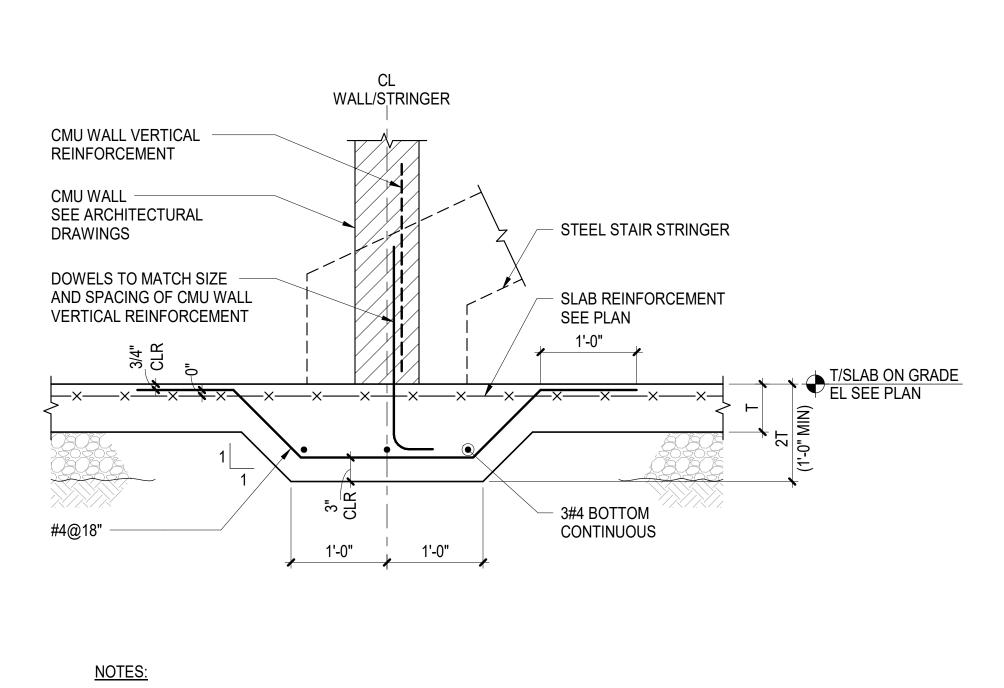


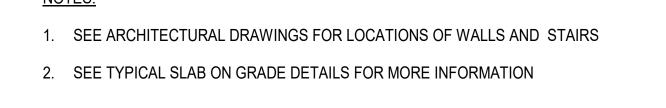


CONCRETE INFILL

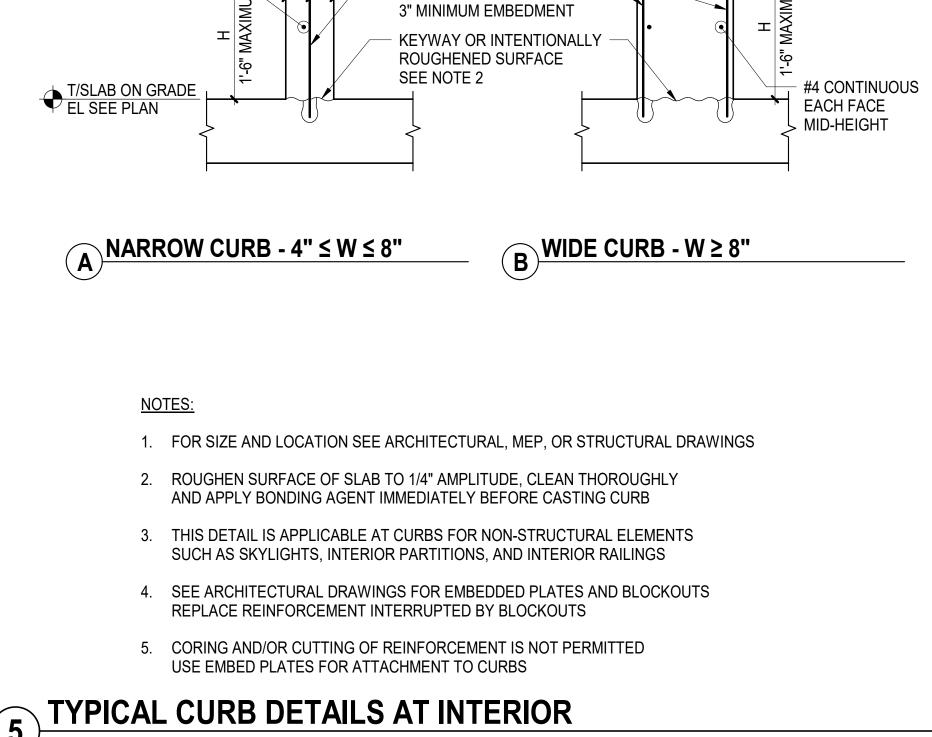
#4 CONTINUOUS EACH FACE

AT TOP





TYPICAL THICKENED SLAB ON GRADE AT NON-BEARING CMU WALL OR STAIR STRINGER NOT TO SCALE



ADHESIVE ANCHOR WITH

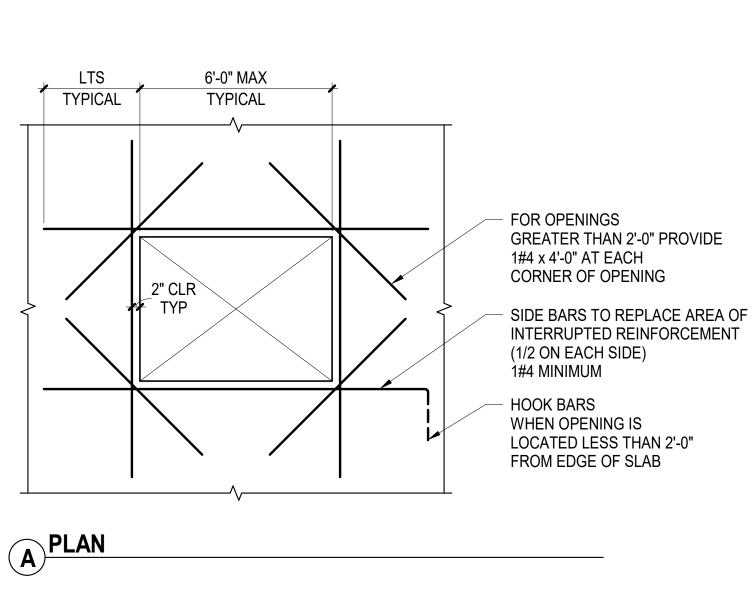


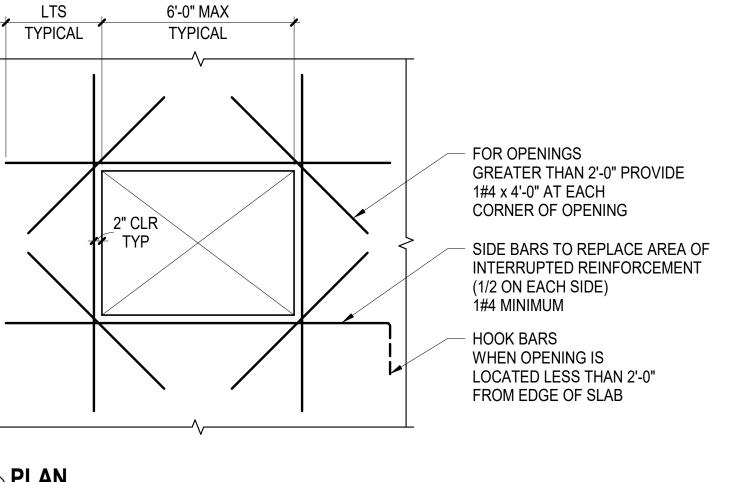
#4 CONTINUOUS

#4 CONTINUOUS

AT MID-HEIGHT

AT TOP





SLAB REINFORCEMENT

SEE PLAN

<u>N</u>	10	<u>ITES:</u>
1		FOR SIZE OF OPENINGS SEE ARCHITECTURAL DRAWINGS
2)	ADDITIONAL REINFORCEMENT IS NOT REQUIRED AT OPENINGS THAT DO NOT INTERRUPT THE TYPICAL REINFORCEMENT

CROSS WIRE AT END OF WWR

1. SEE TYPICAL SLAB ON GRADE DETAILS FOR ADDITIONAL INFORMATION

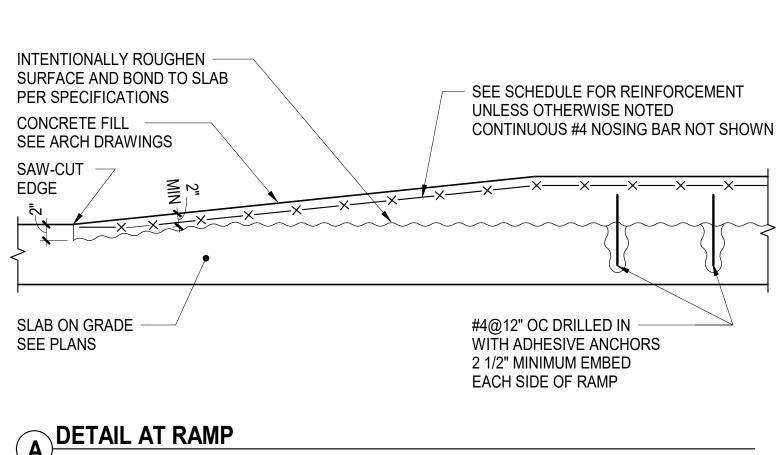
TYPICAL SLAB ON GRADE STEP 6" MAXIMUM

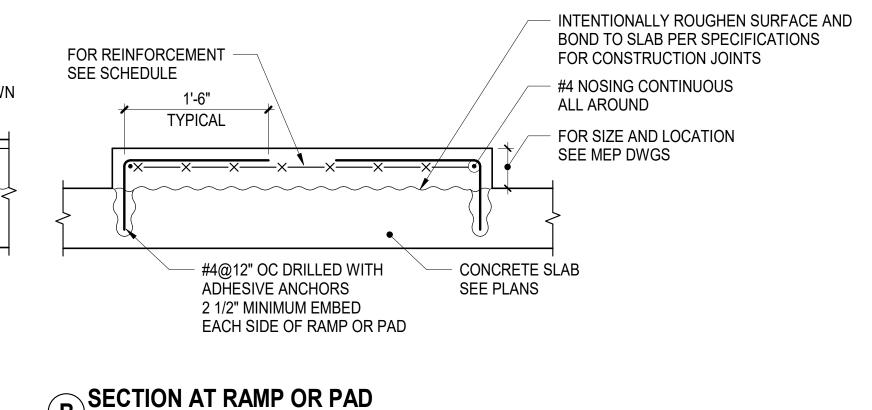
NOT TO SCALE

T/SLAB ON GRADE EL SEE PLAN

TYPICAL SLAB ON GRADE DETAIL OF ADDITIONAL REINFORCEMENT AT ANY OPENINGS OR AT STEP GREATER THAN 6" UP TO 3'-0"

NOT TO SCALE





THICKNESS	REINFORCEMENT
≤ 3"	WWR 6x6 - W2.9xW2.9
≤ 4"	WWR 6x6 - W4.0xW4.0
≤ 6"	#4 @ 12" TOP EACH WAY
≤ 12"	#4 @ 12" TOP & BOTTOM EACH WAY

140	<u>OTEO.</u>	
1.	THIS DETAIL IS NOT APPLICABLE TO GENERAL RAISED SLAB AREA HIGHER THAN 4" OTHER THAN MECHANICAL PADS AND HOUSEKEEPING PADS	

TYPICAL DETAIL OF CONCRETE FILL HOUSEKEEPING PAD / MECHANICAL PAD / RAMP
NOT TO SCALE

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Key Plan

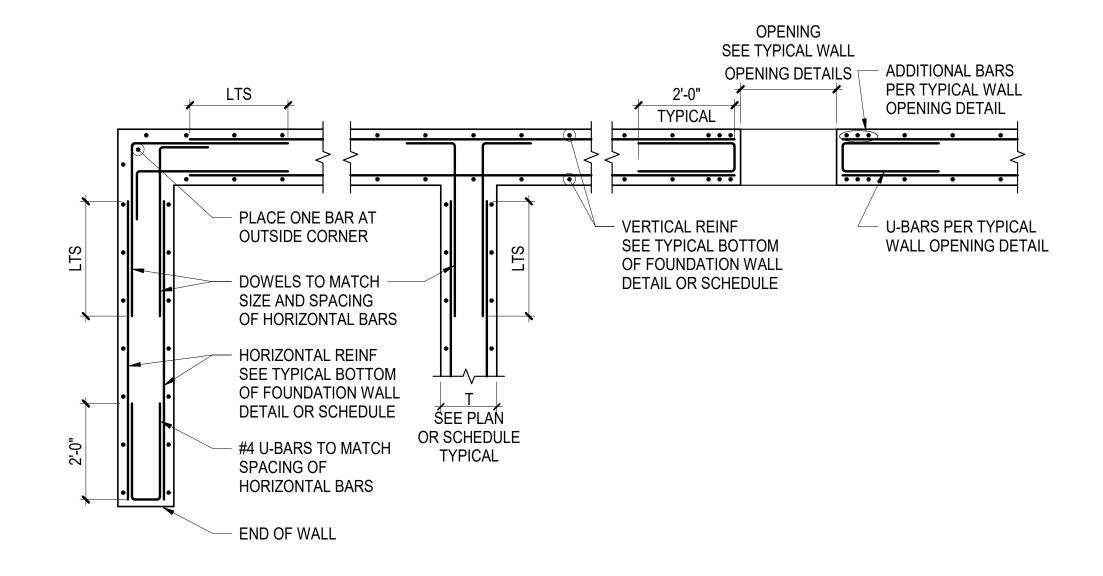
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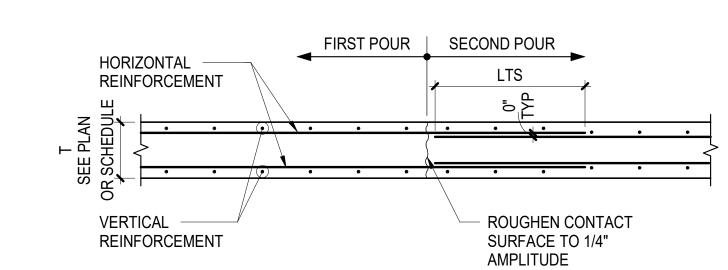
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Vendor Name: GENSLER Vendor Project No.: 006.3608.000 TYPICAL SLAB ON GRADE **DETAILS**

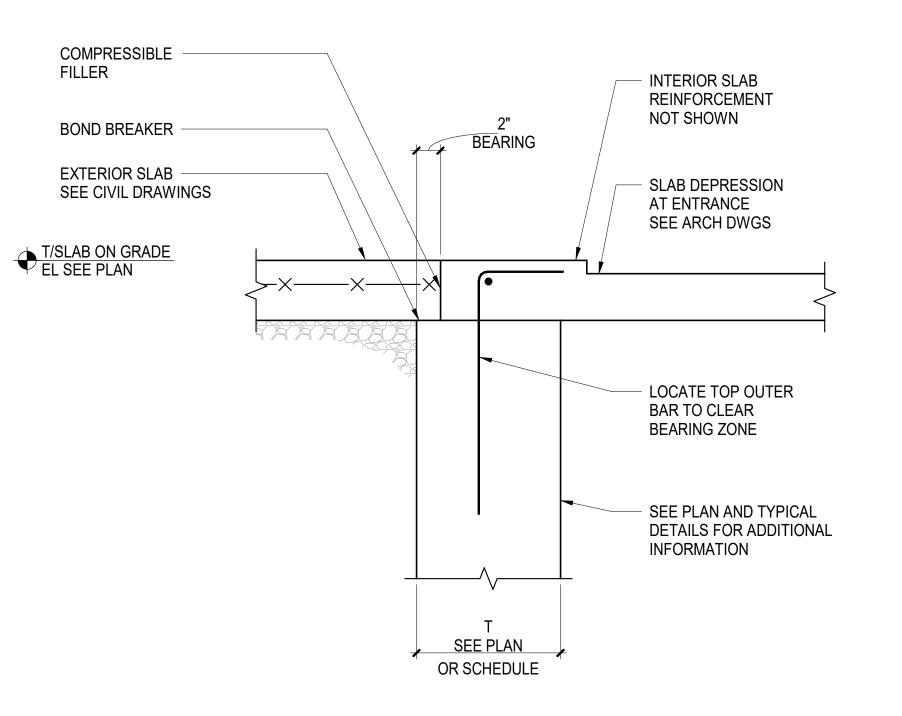




- 1. SEE ARCHITECTURAL DOCUMENTS FOR WATERSTOP REQUIREMENTS
- 2. SEE GENERAL NOTES FOR CONSTRUCTION JOINT MAXIMUM SPACING

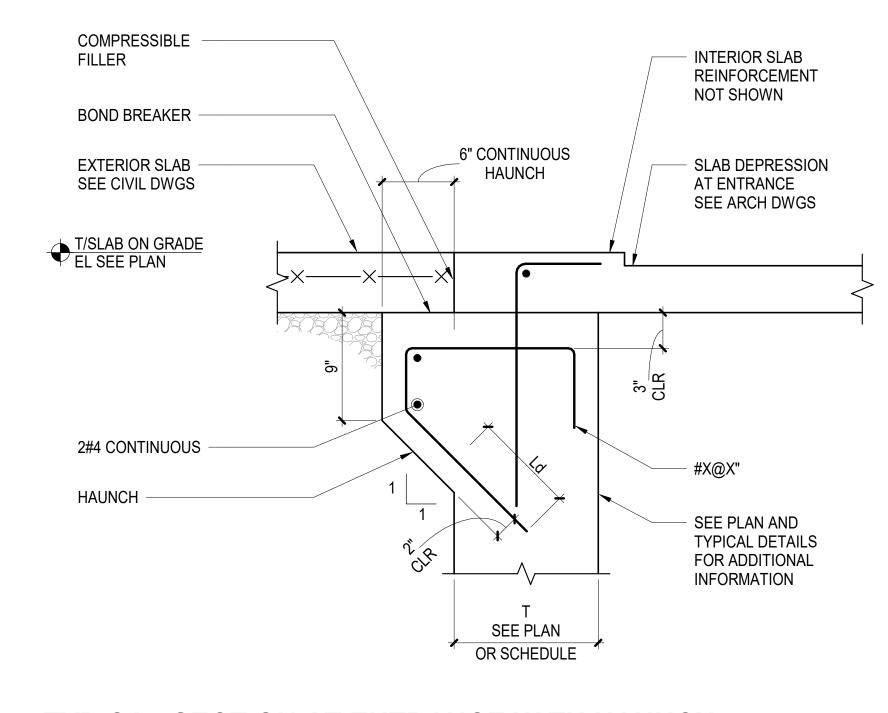
TYPICAL VERTICAL CONSTRUCTION JOINT

2 IN CONCRETE WALL
NOT TO SCALE



TYPICAL FOUNDATION WALL DETAIL - PLAN

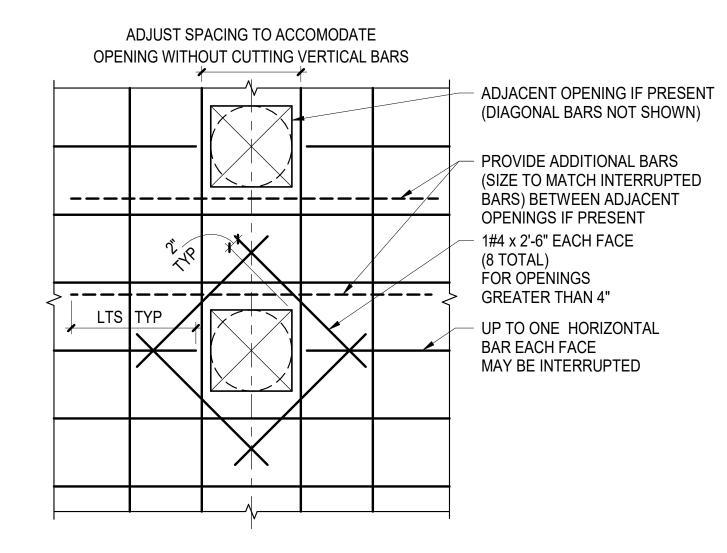
NOT TO SCALE

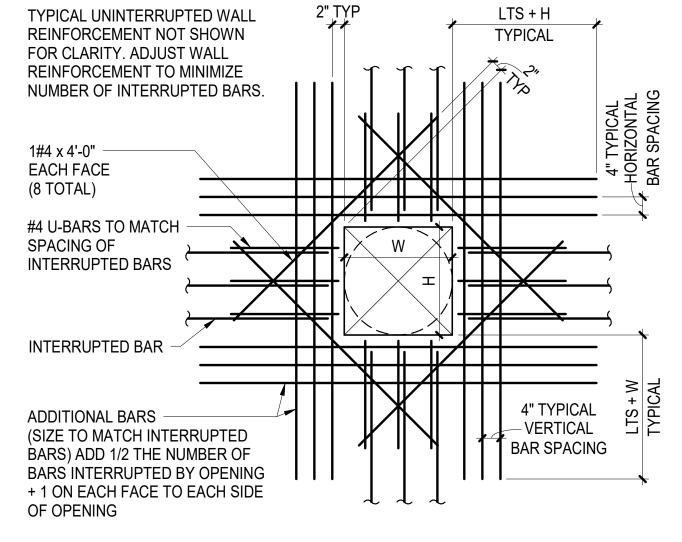


TYPICAL SECTION AT ENTRANCE

SCALE: NOT TO SCALE

5 TYPICAL SECTION AT ENTRANCE WITH HAUNCH SCALE: NOT TO SCALE





A OPENING LESS THAN 10"

B OPENING 10" TO 30"

NOTES:

- 1. MINIMUM CLEAR DISTANCE BETWEEN OPENINGS IS 2 TIMES MAXIMUM OPENING SIZE
- 2. FOR OPENINGS NOT SHOWN ON STRUCTURAL DRAWINGS, CONTRACTOR TO SUBMIT LOCATIONS AND SPACING TO STRUCTURAL ENGINEER FOR WRITTEN APPROVAL

3 TYPICAL WALL OPENING DETAILS
NOT TO SCALE



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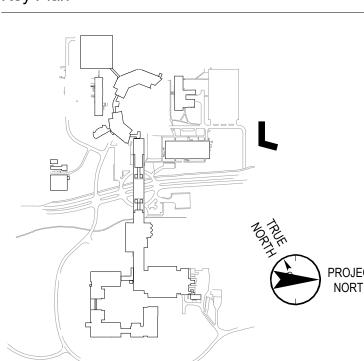
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Key Plan

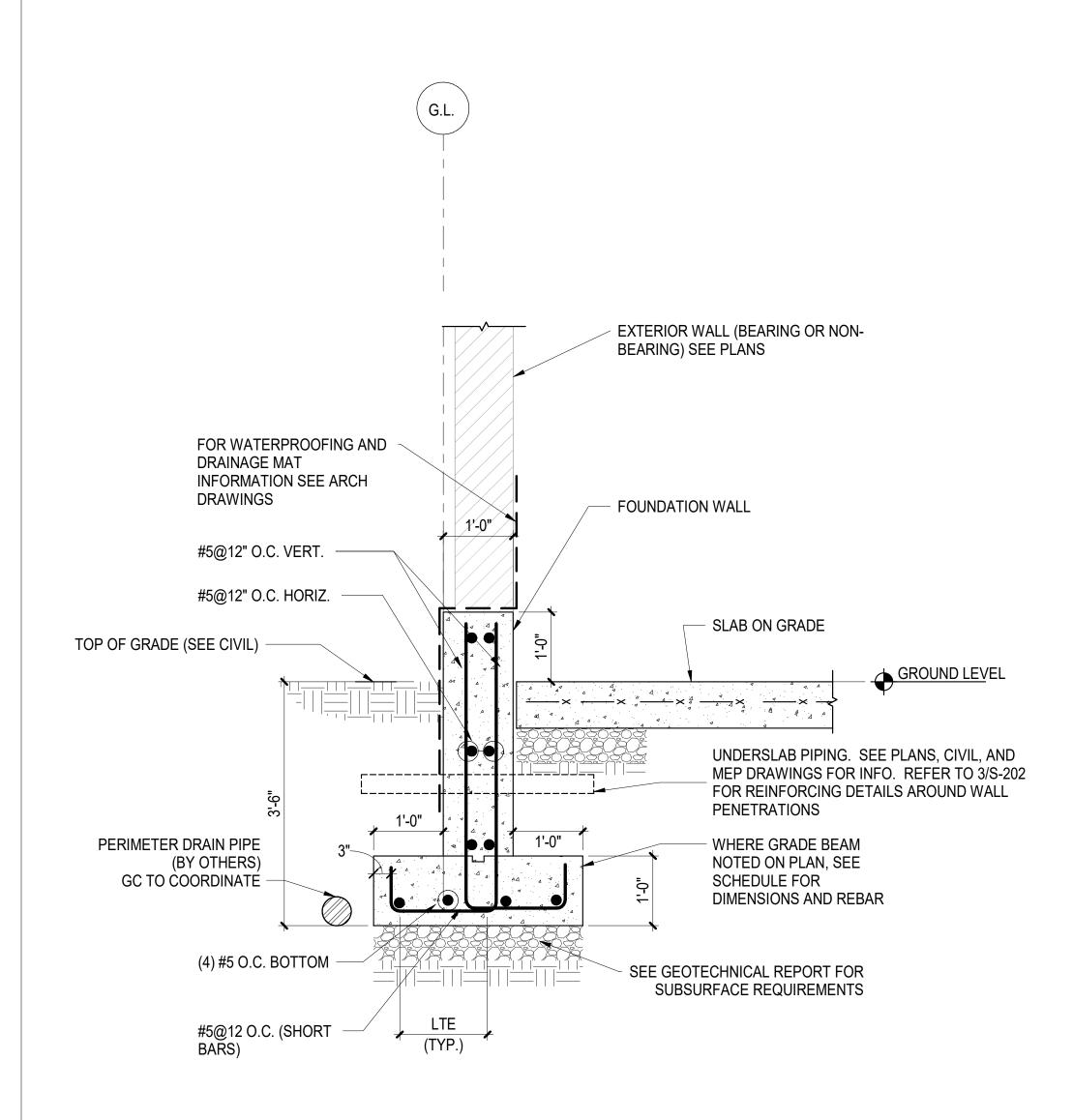


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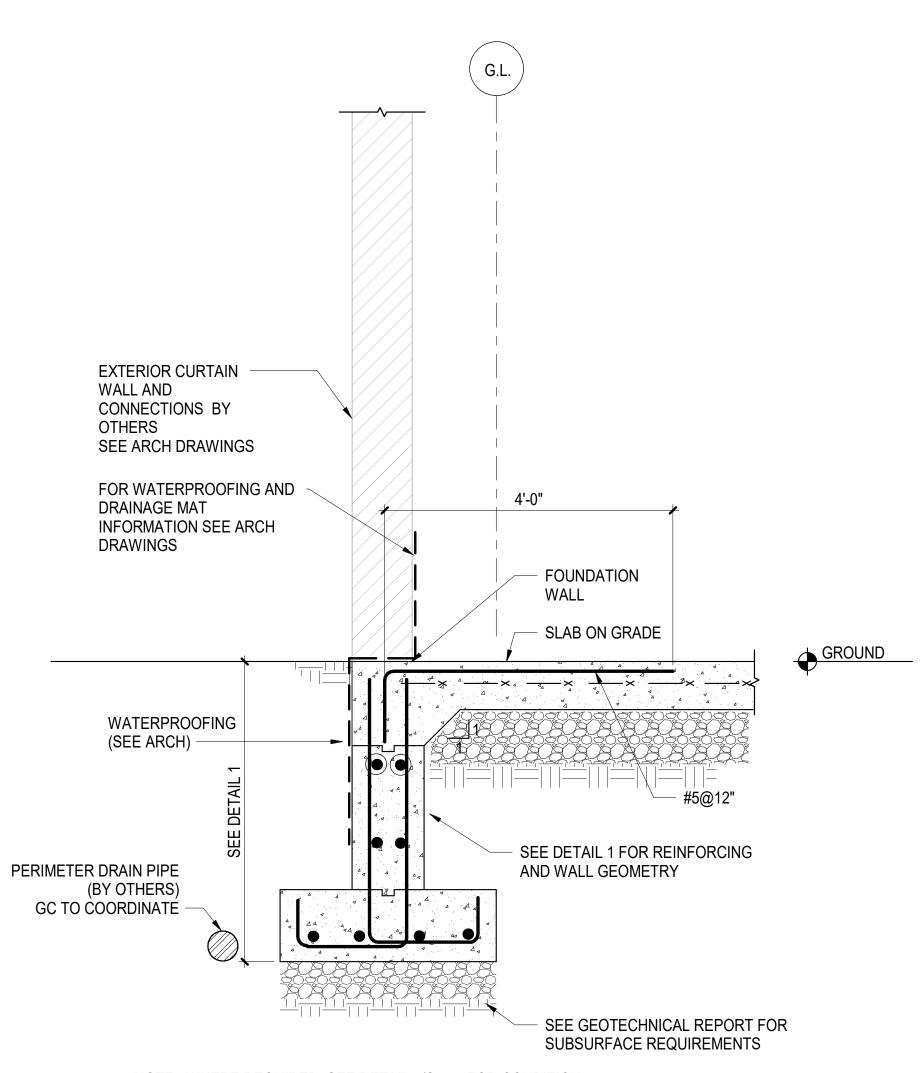
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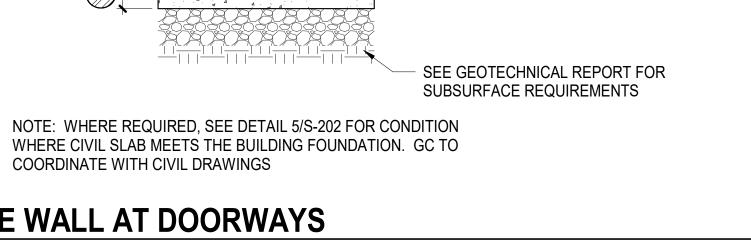
Vendor Name: GENSLER TYPICAL FOUNDATION WALL **DETAILS**

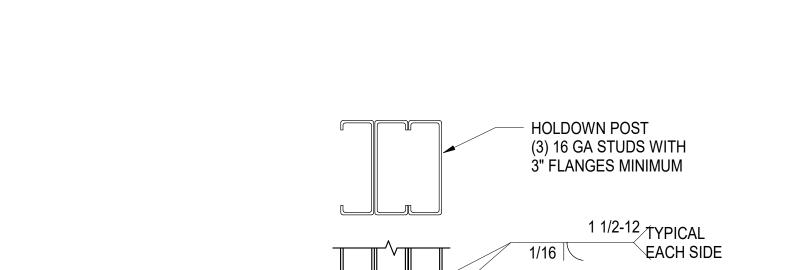


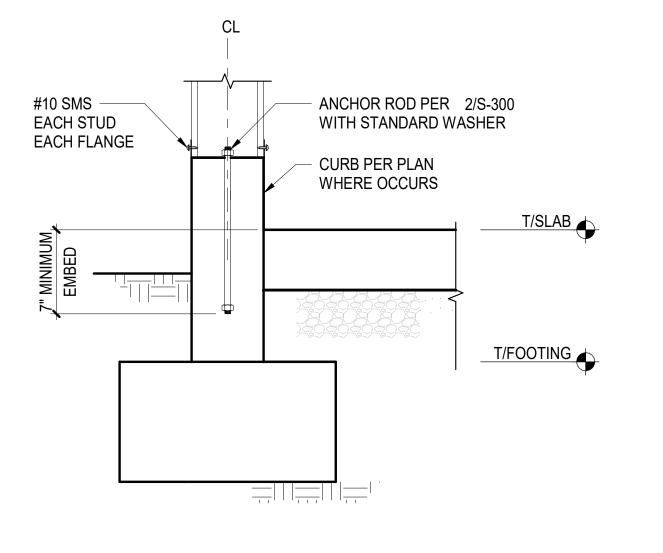




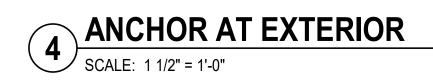
KNEE WALL AT DOORWAYS SCALE: 3/4" = 1'-0"

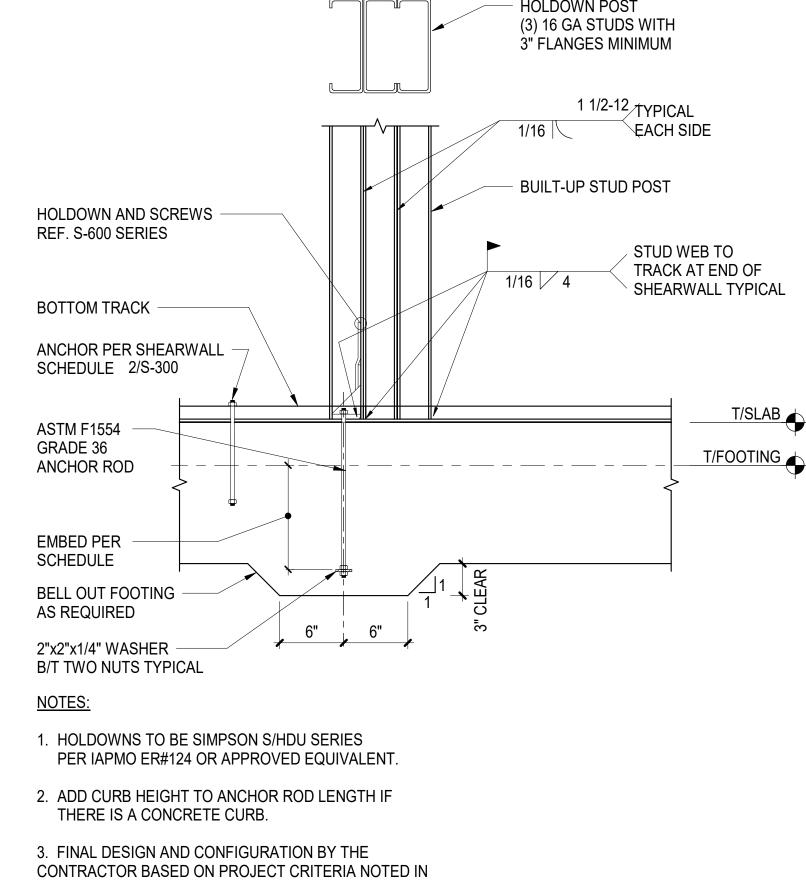






DETAIL FOR WALL BASE CONNECTION ONLY, REFER TO DETAILS 1, 2, 3 FOR FOUNDATION SECTIONS AND DETAILS

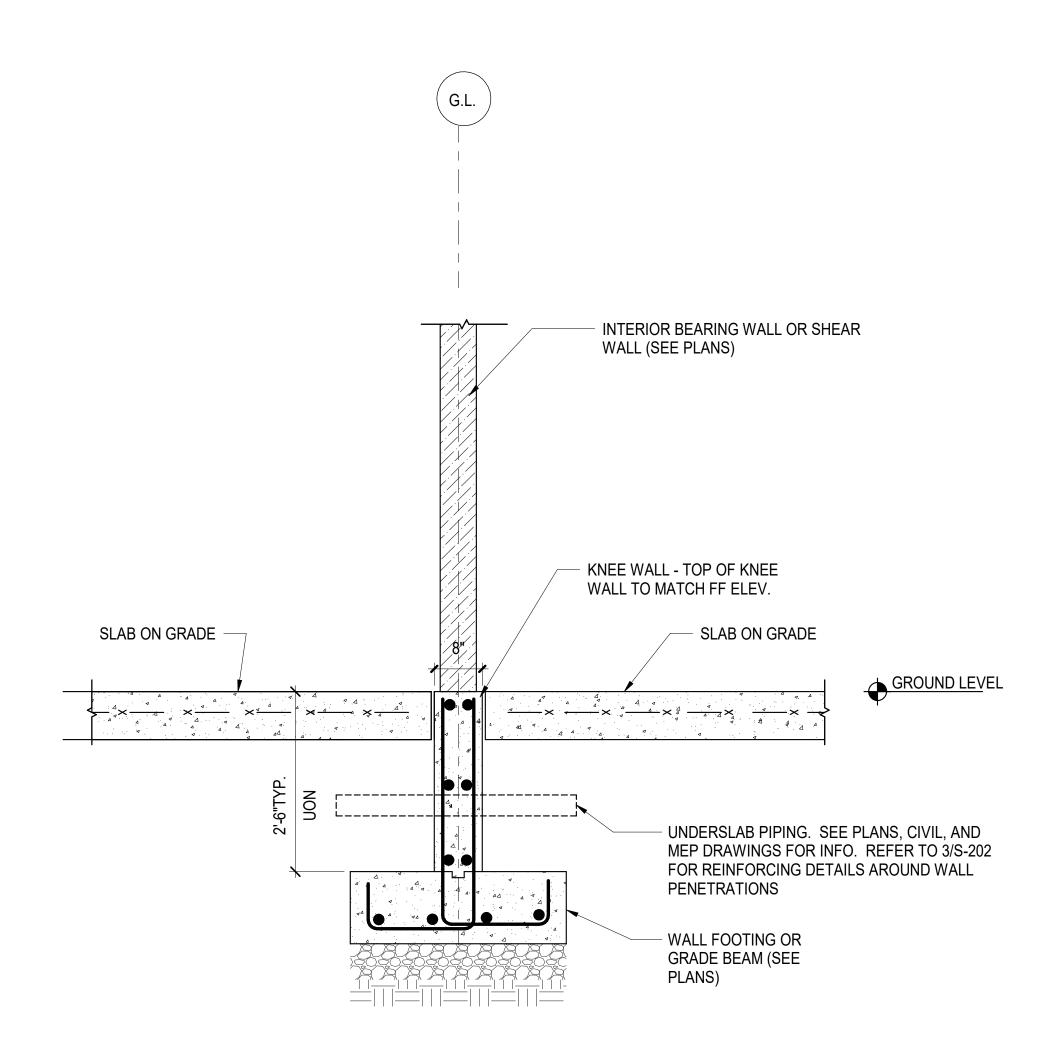




5 HOLDOWN DETAIL

SCALE: 1" = 1'-0"

CONTRACT DOCUMENTS



3 TYPICAL INTERIOR BEARING/SHEAR WALL FOOTING
SCALE: 3/4" = 1'-0"



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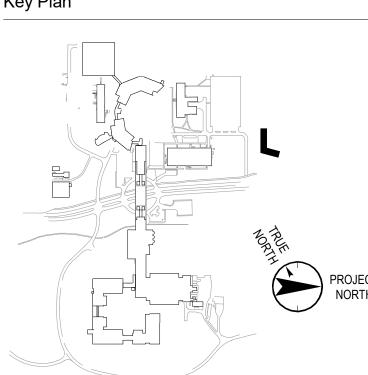
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Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 FOUNDATION SECTIONS AND **DETAILS**

SHEAR WALL LOAD SCHEDULE						
SHEAR WALL MARK	GRIDLINE	SHEAR LOAD (KIPS) - ULTIMATE	TENSION FORCE (KIPS)			
SW-1	1	3.7	5.0			
SW-2	2	2.6	1.5			
SW-3	3	5.8	2.5			
SW-4	4	8.6	3.5			
SW-5	5	7.2	3.0			
SW-6	А	1.1	1.0			
SW-7	A, 5/6	4.0	5.0			
SW-8	A.9, C	2.2	2.0			
SW-9	C, 5/6	3.6	3.5			
SW-10	D	1	1.0			
SW-11	F	4.2	1.5			
SW-12	G	1.2	1.5			
SW-13	8	13	9.5			
SW-14	8.5	9.1	3.0			
SW-15	9, 10, 11, 12	8.6	3.0			
SW-16	13	6.5	2.5			
SW-17	14	1.1	1.0			

DESIGN LOADS SHALL BE APPLIED AT THE TOP OF THE WALL. SEE STRUCTURAL PLANS AND ARCH DRAWINGS FOR WALL LOCATIONS AND FINAL

SHEAR WALL SCHEDULE SCALE: 3/4" = 1'-0" CL WALL 16 GA TRACK BLOCKING ALONG ENTIRE WALL JOISTS SEE PLAN ALIGN WITH WALL STUDS — BOX BEAM AT TOP OF WALL WHERE REQUIRED. SHEAR WALL SHEATHING NOTE: WALLS TO BE DESIGNED FOR SHEAR FORCES NOTED IN SCHEDULE ABOVE PLUS TRANSFER AND/OR CHORD FORCES AS

- CUT FLANGES OF TRACK

AND BEND WEB

- METAL STUDS

SEE PLANS

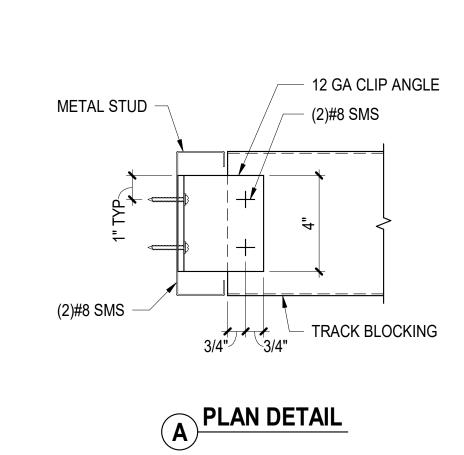
TRACK BLOCKING WITH

1 1/2" FLANGES TO MATCH GAGE OF STUDS

6 BLOCKING DETAIL

WALL PERPENDICULAR TO JOISTS SCALE: 1" = 1'-0"

REQUIRED, SEE PLANS



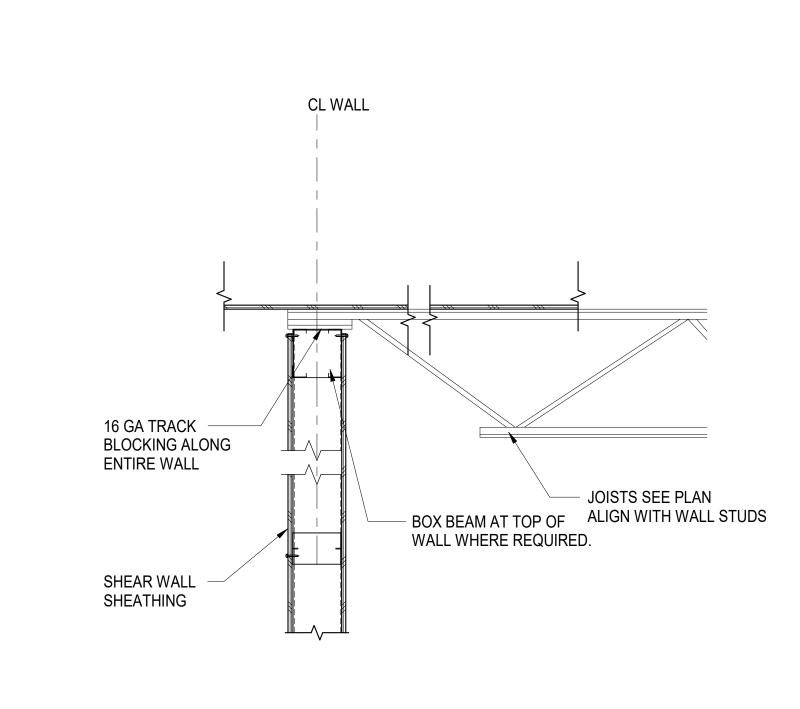
EDGE SCREW -TO TOP PLATE SEE 8/S-601 FOR LEDGER SPLICE BLOCKING AT ADJOINING PANEL TYPICAL SEE 4/S-300 FOR DIAPHRAM CONNECTION - STAGGER HORIZONTAL PANEL JOINTS OR USE FULL HEIGHT SHEETS ₩INIMUM SEE TABLE PARTIAL SHEETS -WHERE REQUIRED USE FULL FIELD SCREWS AT SHEETS WHERE 12" OC TYPICAL POSSIBLE TYPICAL STUD FRAMING EDGE SCREW ALL SEE PLANS PANEL EDGES FOR PLYWOOD SHEATHING EDGE SCREW PLYWOOD FACE GRAIN
PARALLEL TO STUDS STAGGER BETWEEN BOTH STUDS EDGE SCREW TO SOLE PLATE

T/SLAB HD AT POST PER PLAN AND 5/S-210 SEE 2/S-300 SEE DETAIL 5A/ S-300 10" MAXIMUM TO FOR SILL ANCHOR FOR 14" MAXIMUM OPENING FIRST ANCHOR BOLT ONE 14" WIDE OPENING SPACING AT SHEAR WALL PER 8' OF WALL WITHOUT APPROVAL FROM SEOR

> 1. DETAIL PROVIDED FOR INFORMATION. FINAL SHEAR WALL DESIGN BY THE LGMF CONTRACTOR AS PART OF DELEGATED DESIGN, BASED ON PROJECT CRITERIA NOTED IN CONTRACT DOCUMENTS. 2. WHERE CROSS BRACING STRAPS ARE REQUIRED, SEE S-301 FOR

SHEAR WALL ELEVATION

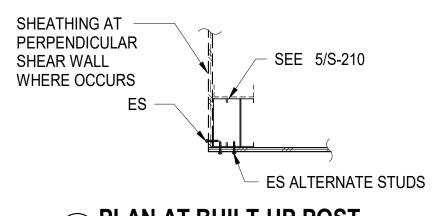
SCALE: 3/8" = 1'-0"



NOTE: WALLS TO BE DESIGNED FOR SHEAR FORCES NOTED IN SCHEDULE ABOVE PLUS TRANSFER AND/OR CHORD FORCES AS REQUIRED, SEE PLANS

WALL PARALLEL TO JOISTS

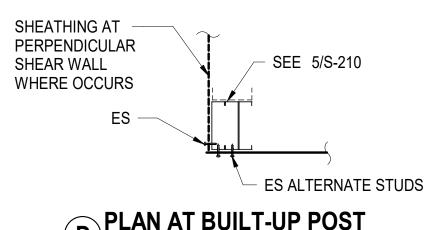




C PLAN AT BUILT-UP POST

- 1. FOR PLYWOOD ATTACHED TO METAL STUDS SEE 1/S-300 FOR INFORMATION NOT NOTED TYPICAL.
- 2. TYPICAL FIELD SCREWS TO BE #8 SMS @ 12" OC AT INTERMEDIATE MEMBERS.
- 3. USE #8 FLAT HEAD SELF-DRILLING TAPPING SCREW WITH A MINIMUM HEAD DIAMETER OF 0.285.
- 4. MINIMUM EDGE DISTANCE FOR SCREWS IN RECEIVING MEMBERS & SHEATHING SHALL BE 3/8".





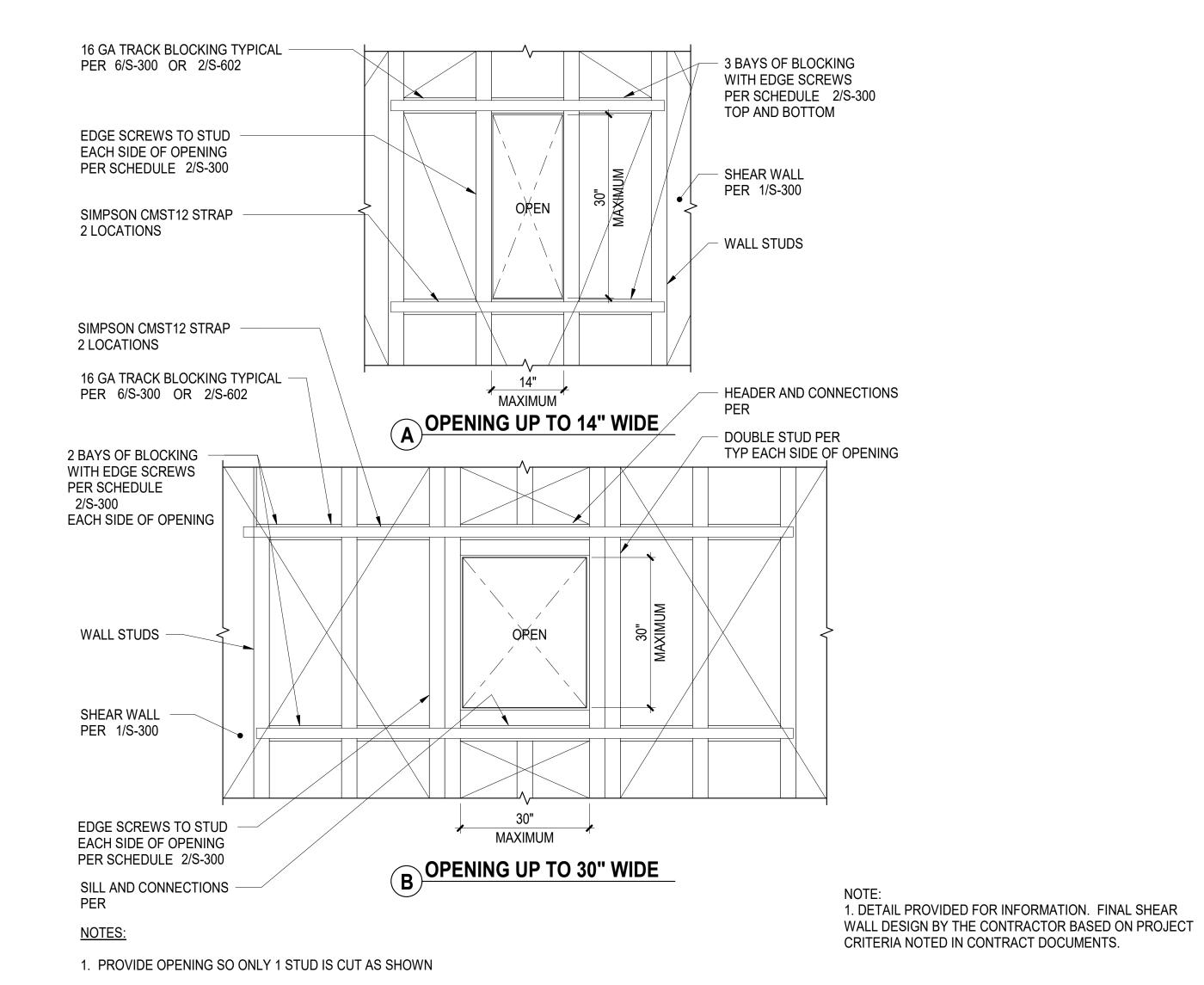
PLAN AT BUILT-UP POST

- 1. FOR STEEL SHEET ATTACHED TO METAL STUDS SEE 1/S-300 FOR INFORMATION NOT NOTED TYPICAL. 2. TYPICAL FIELD SCREWS TO BE #8 SMS @ 12" OC AT INTERMEDIATE MEMBERS.
- 3. USE SELF-DRILLING/SELF TAPPING #8 BUGLE HEAD SCREWS WITH MINIMUM 0.315" HEAD DIAMETER.
- 4. MINIMUM EDGE DISTANCE FOR SCREWS IN RECEIVING MEMBERS & SHEATHING SHALL BE 3/8".

B STEEL SHEET SHEATHING

1. DETAIL PROVIDED FOR INFORMATION. FINAL SHEAR WALL DESIGN BY THE CONTRACTOR BASED ON PROJECT CRITERIA NOTED IN CONTRACT DOCUMENTS.

SHEAR WALL SCHEDULE



TYPICAL OPENING IN SHEAR WALL DETAIL

SCALE: 3/4" = 1'-0"

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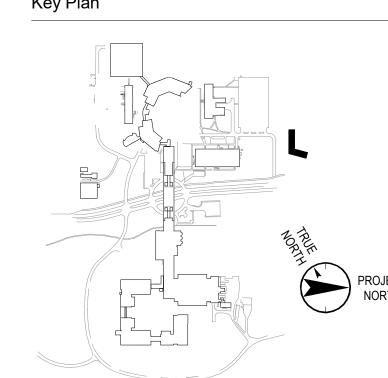
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Key Plan



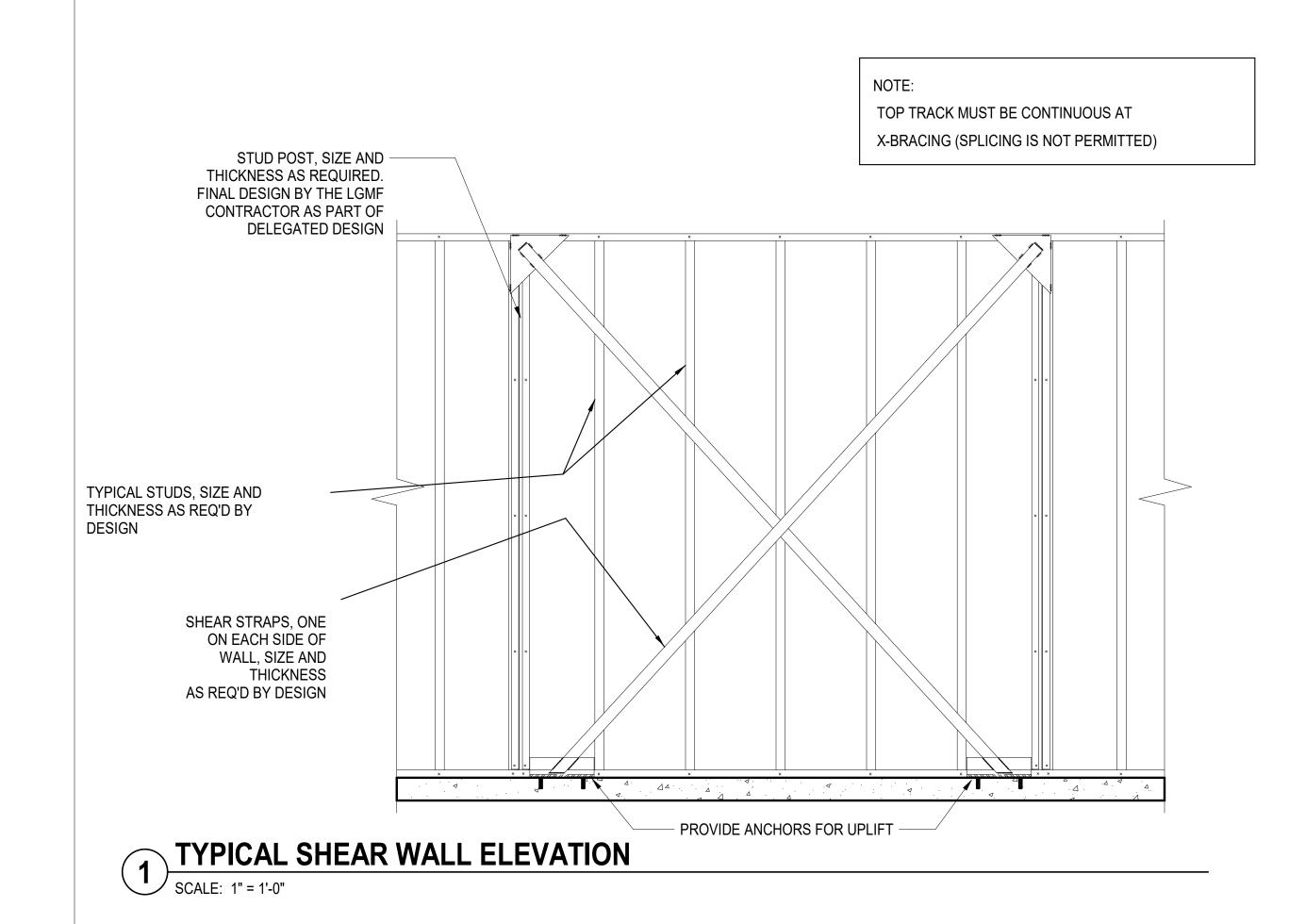
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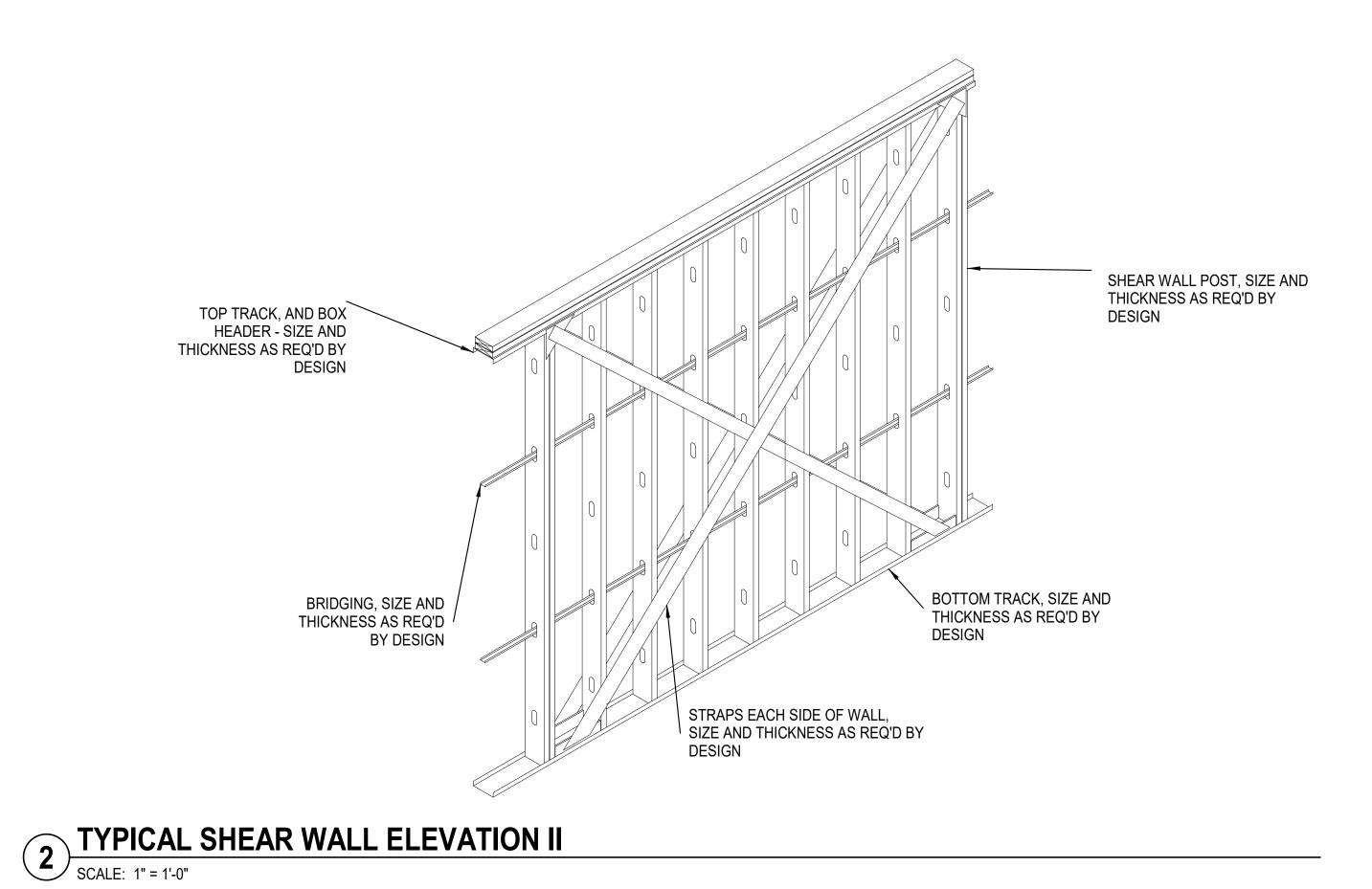
Plot Date: 02/24/22

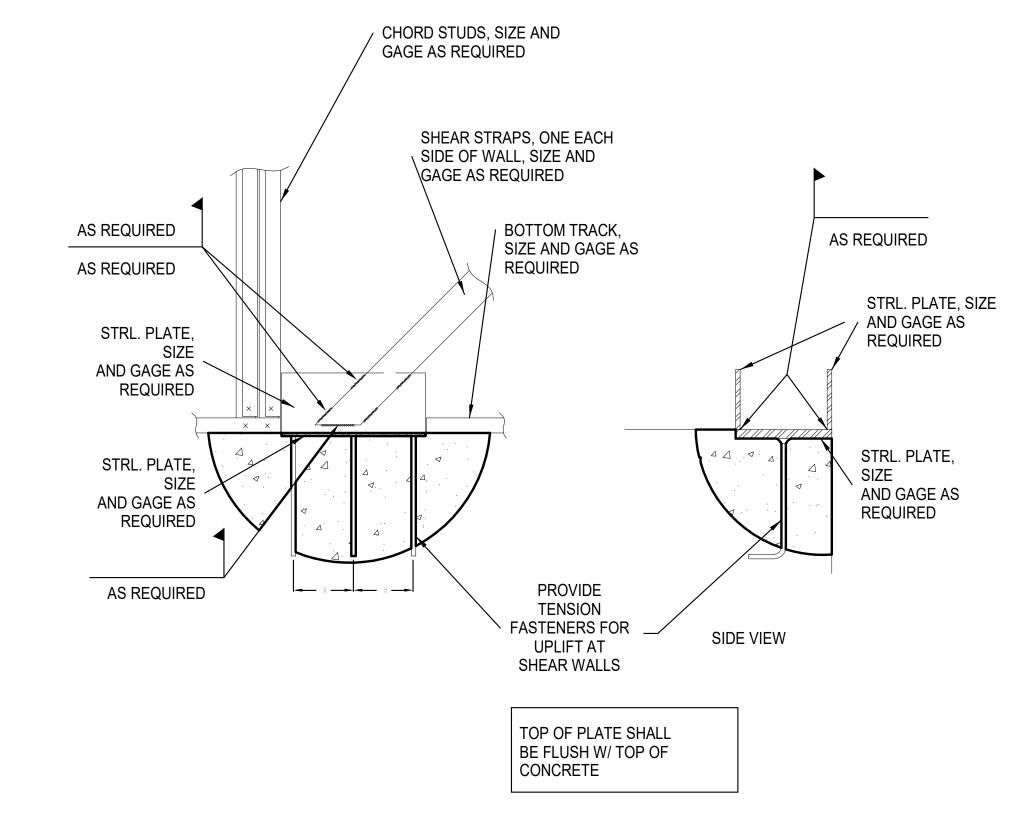
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Professional Seal and Signature

Vendor Name: GENSLER SHEAR WALL LOADS AND **DETAILS**







3 TYPICAL SHEAR WALL STRAP AT BASE

SCALE: 1" = 1'-0"

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Cosentini Associates 498 Seventh Avenue New York, NY 10018 (212) 615-3600 Phone (212) 615-3700 Fax

JMC 120 Bedford Road

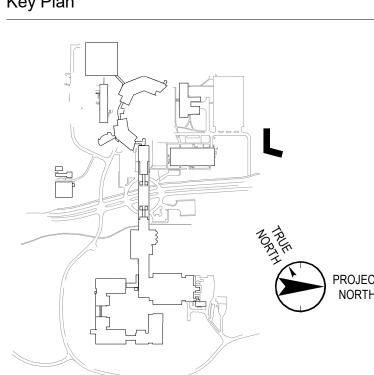
Civil Engineer

Armonk, NY 10504 (914) 273-5225 Phone (914) 273-2102 Fax

Landscape Architect

Langan 21 Penn Plaza, 360 West 31st Street, 8th Floor New York, NY 10001 (212) 479-5400 Phone (212) 479-5444 Fax

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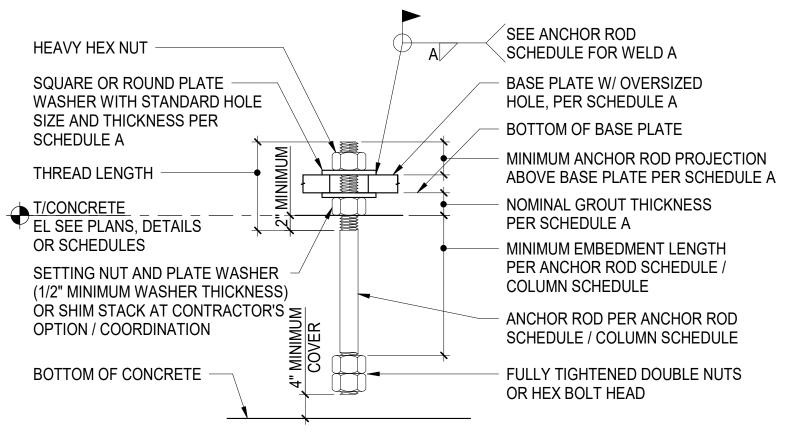
Plot Date: 03/11/22

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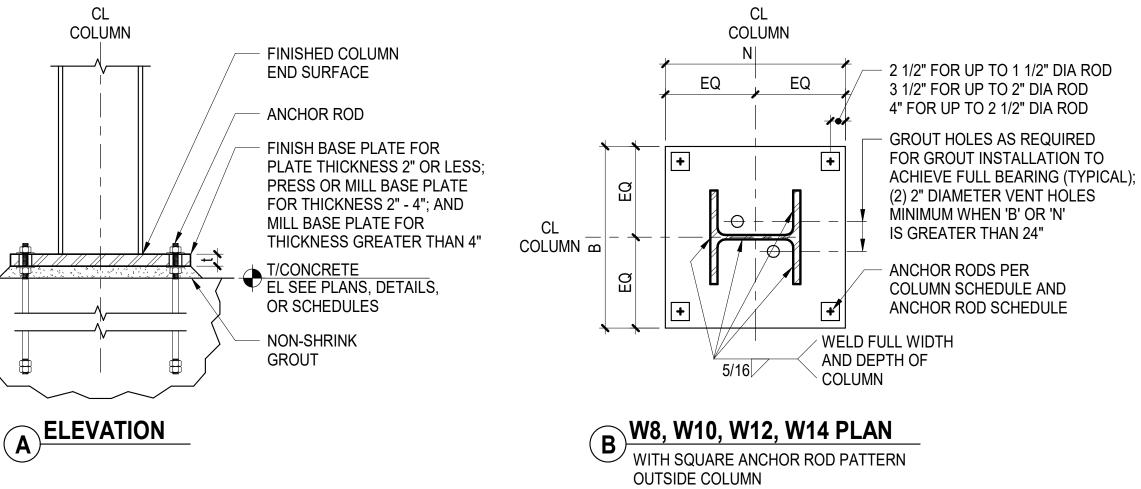
Professional Seal and Signature

Vendor Name: GENSLER
Vendor Project No.: 006.3608.000
Discipline: Drawn By: Author
SHEAR WALL DETAILS

Scale: 1" = 1'-0" Floor:



SCHEDULE A						
ANCHOR ROD DIAMETER	BASE PL HOLE DIA	MIN WASHER SIZE	MIN WASHER t	MIN PROJ ABOVE BASE PL	NOMINAL GROUT THICKNESS	
3/4"	1-5/16"	2"	1/4"	3"	2"	
1"	1-13/16"	3"	3/8"	3-1/2"	2"	
1-1/4"	2-1/16"	3"	1/2"	4"	3"	
1-1/2"	2-5/16"	3-1/2"	1/2"	4"	3"	
1-3/4"	2-3/4"	4"	3/4"	5"	3"	
2"	3-1/4"	5"	3/4"	5"	4"	
2-1/2"	3-3/4"	5-1/2"	1"	5-1/2"	4"	



COLUMN ∣ N EQ EQ SIN SIN NOTES:



SHAPE

W8

W10

W12

W14

APPLICABLE TO W8

NOTES FOR TABLE:

S IN FOR SQUARE ANCHOR ROD

PATTERN INSIDE COLUMN

1. INSIDE ANCHOR ROD CONFIGURATION NOT APPLICABLE TO ROD DIAMETERS GREATER

2. INSIDE ANCHOR ROD CONFIGURATION NOT

THAN 1 1/4" FOR W14, 1" FOR W12, AND 3/4"

FOR W10. INSIDE BOLT CONFIGURATION NOT

APPLICABLE TO COLUMNS HEAVIER THAN W14X426

SIN

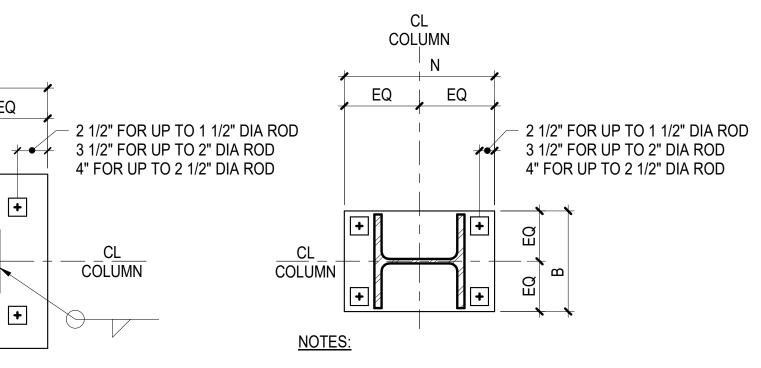
2 1/2"

D ROUND OR RECTANGULAR HSS PLAN W10, W12, W14 PLAN WITH SQUARE ANCHOR ROD PATTERN INSIDE COLUMN - SEE TABLE

COLUMN

___C<u>L</u> COLUMN

EQ



1. SEE ANCHOR ROD PATTERN OUTSIDE COLUMN FOR INFORMATION NOT SHOWN

E RECTANGULAR BASE PLATE AT WIDE FLANGE COLUMN WITH RECTANGULAR ANCHOR ROD PATTERN OUTSIDE COLUMN

NOTES:

- 1. SEE COLUMN SCHEDULE FOR BASE PLATE SIZE, ORIENTATION AND THICKNESS
- BASE PLATE THICKNESS SHOWN ON SCHEDULE IS A MINIMUM. DIMENSION AFTER ALL MILLING IS COMPLETED
- 3. COLUMN STABILITY DURING ERECTION IS RESPONSIBILITY OF CONTRACTOR
- 4. SEE ANCHOR ROD SCHEDULE AND TYPICAL ANCHOR ROD DETAIL FOR
- ADDITIONAL INFORMATION 5. CONTRACTOR'S OPTION TO FIELD WELD COLUMNS TO BASEPLATES FOR HEAVY
- BASEPLATES
- 6. ANCHOR ROD CONFIGURATION IS TO USE SQUARE PATTERN OUTSIDE COLUMN. IF SPECIFIED BASE PLATE SIZE DOES NOT PERMIT OUTSIDE PLACEMENT WHERE NOTED

L
WH
-

ANCHOR ROD SCHEDULE						
ANCHOR		ANCHOR RODS	ANCHOR RODS			
ROD MARK	NUMBER	TYPE	EMBED LENGTH	WELD A	REMARKS	
1	4	1" DIA ASTM F1554 GR 55 (SUPP S1)	1'-3"			

1 TYPICAL ANCHOR ROD DETAIL

NOT TO SCALE



 NOT TO SCALE

	ANCHOR ROD SCHEDULE									
)R		ANCHOR RODS								
(NUMBER	ТҮРЕ	EMBED LENGTH	WELD A	REMARKS					
	4	1" DIA ASTM F1554 GR 55 (SUPP S1)	1'-3"							

NOTES:

- 1. SEE TYPICAL ANCHOR ROD AND BASE PLATE DETAILS
- 2. WHERE WELD 'A' IS NOT SHOWN, TACK-WELD AS REQUIRED FOR ERECTION



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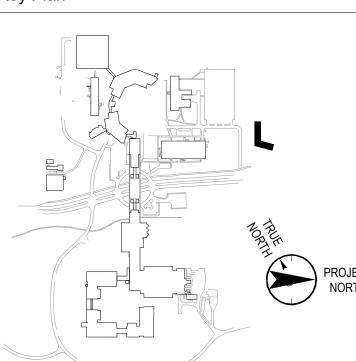
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Key Plan



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Plot Date: 03/03/09

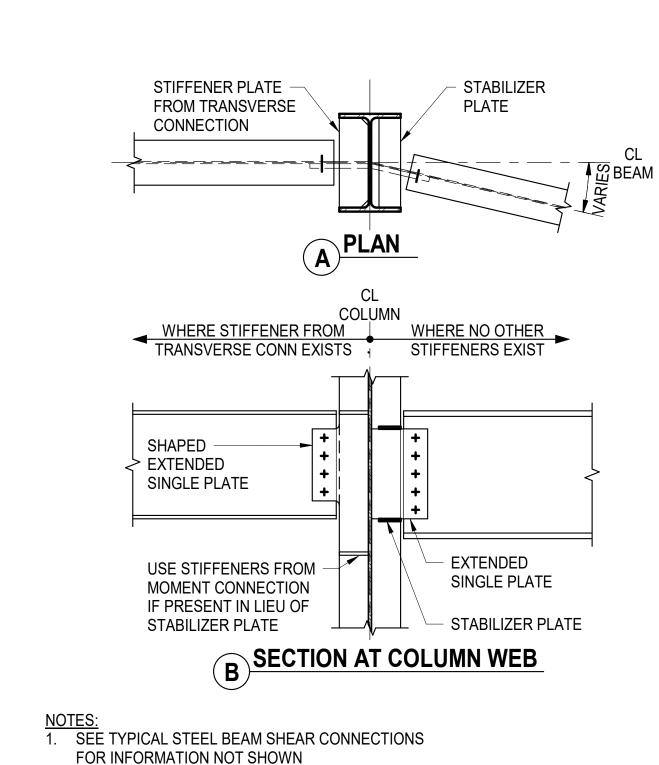
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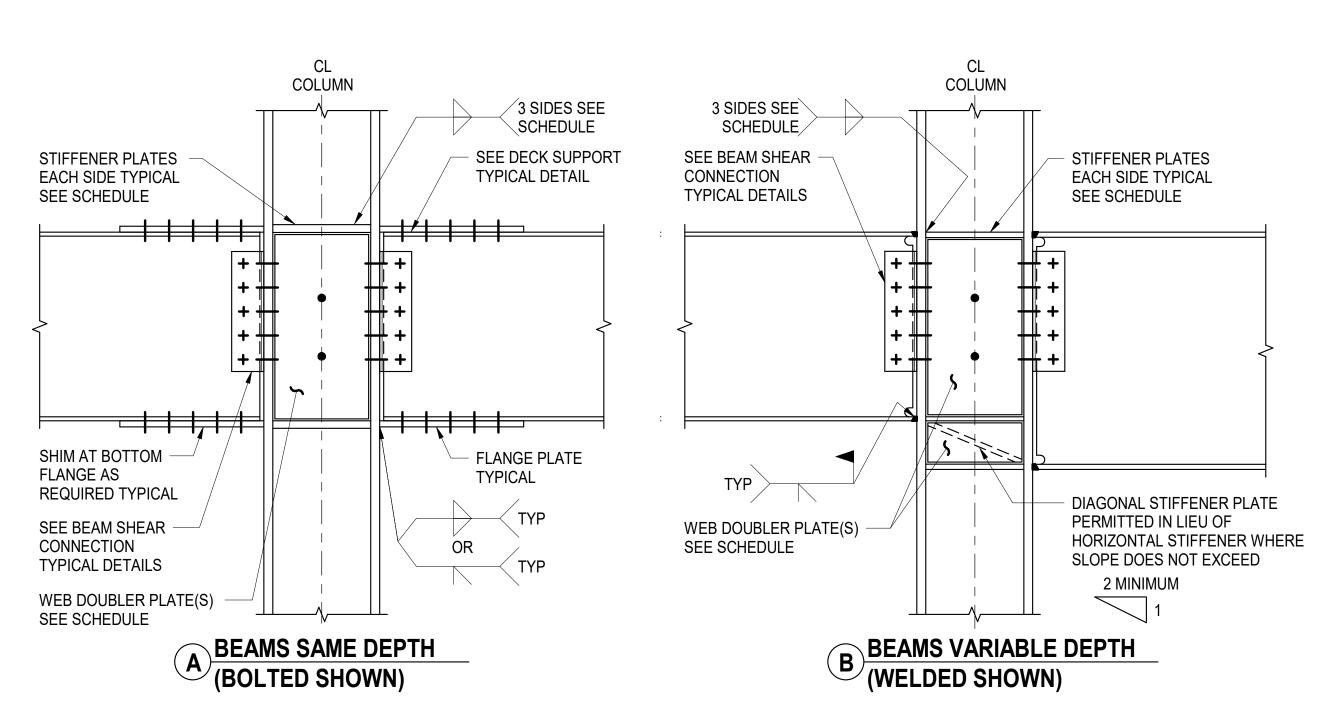
Vendor Name: GENSLER Vendor Project No.: 006.3608.000 **DETAILS**

- 1. SUPPORTED BEAMS PRIMARILY SUPPORT DISTRIBUTED LOADS FROM SLABS OR DECKING
- 2. SUPPORTING BEAMS SUPPORT SIGNIFICANT POINT LOADS FROM ONE OR MORE SUPPORTED BEAMS OR FROM COLUMNS BEING TRANSFERRED. SUPPORTING BEAMS MAY BE SUPPORTED BY COLUMNS OR BY OTHER SUPPORTING BEAMS
- 3. FOR SHEAR CONNECTIONS AT SUPPORTED BEAM ENDS, DOUBLE ANGLE, SINGLE PLATE OR SINGLE ANGLE MAY BE USED UNLESS OTHERWISE NOTED
- 4. SEE TYPICAL STEEL BEAM SHEAR CONNECTIONS FOR INFORMATION NOT SHOWN

TYPICAL BEAM TO BEAM SHEAR CONNECTION (3 TYPES) NOT TO SCALE

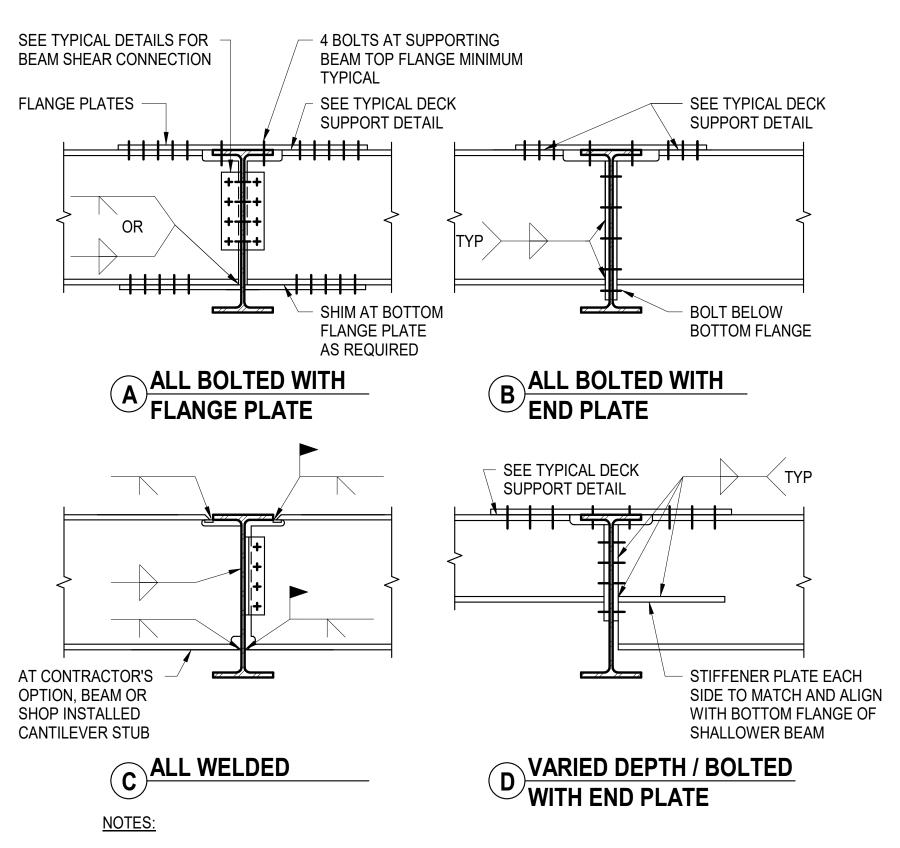


TYPICAL EXTENDED SINGLE PLATE BEAM TO **COLUMN WEB SHEAR CONNECTION**



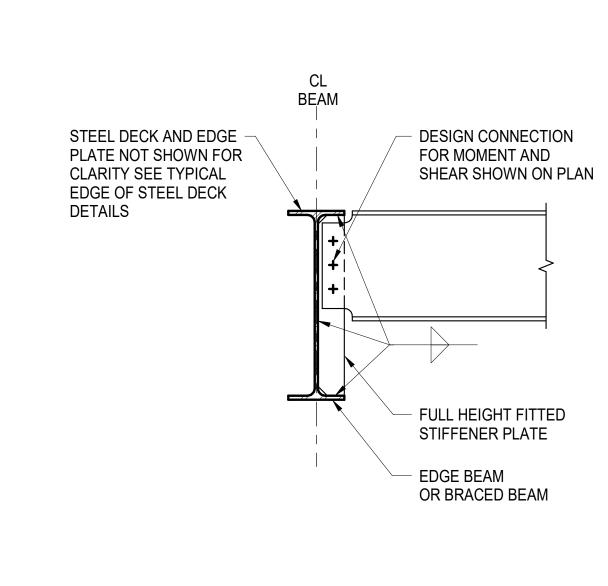
- 1. ALL BOLTED MOMENT AND AXIAL CONNECTIONS AT A MINIMUM SHALL HAVE PRETENSIONED BOLTS IN STANDARD HOLES AT FLANGES AND WEBS UNLESS OTHERWISE NOTED
- 2. BOLTED MOMENT CONNECTIONS AT CANTILEVERS AND BACKSPANS SHALL USE SLIP CRITICAL BOLTS
- 3. AT CONTRACTOR'S OPTION, WEB DOUBLER PLATES CAN TERMINATE OUTSIDE STIFFENER PLATE. CONTRACTOR'S ENGINEER MUST FULLY DESIGN THE CONNECTION

TYPICAL BEAM TO COLUMN FLANGE MOMENT CONNECTION - BOLTED/WELDED NOT TO SCALE



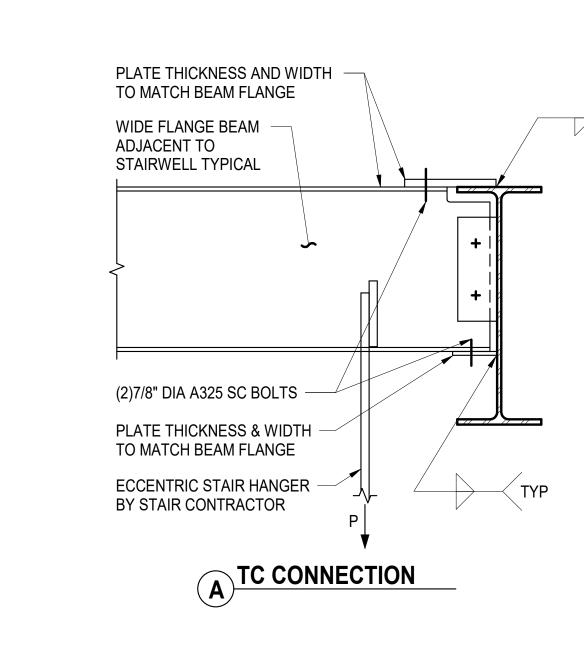
- 1. ALL BOLTED MOMENT AND AXIAL CONNECTIONS AT A MINIMUM SHALL HAVE PRETENSIONED BOLTS IN STANDARD HOLES AT FLANGES AND WEBS UNLESS
- 2. BOLTED MOMENT CONNECTIONS AT CANTILEVERS AND BACKSPANS SHALL USE SLIP CRITICAL BOLTS

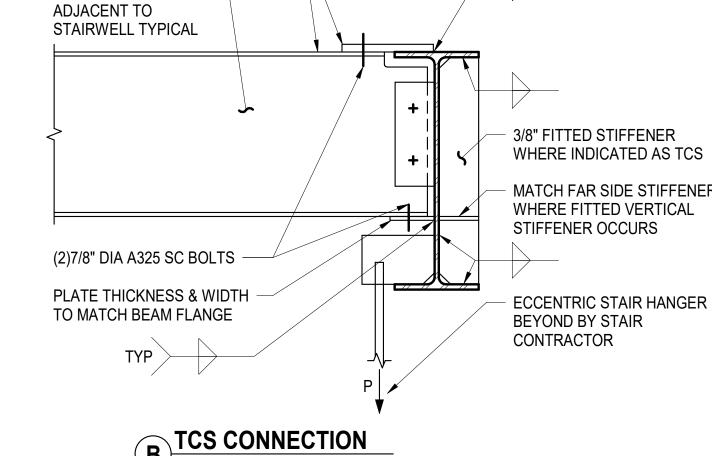
6 TYPICAL BEAM TO BEAM MOMENT CONNECTION NOT TO SCALE



1. SEE PLAN FOR LOCATIONS

TYPICAL FULL HEIGHT FITTED STIFFENER AT **EDGE BEAM OR BRACED BEAM CONNECTION**





NOTES: 1. SEE TYPICAL PLAN DETAILS FOR STAIRWELLS

SCALE: NOT TO SCALE

2. SUBMIT SHOP DRAWINGS THAT SHOW THE MAGNITUDES, DIRECTIONS, LOCATIONS, AND CONNECTION CONDITIONS OF ALL LOADS IMPOSED ON THE SUPPORTING STRUCTURE BY STAIR CONTRACTOR

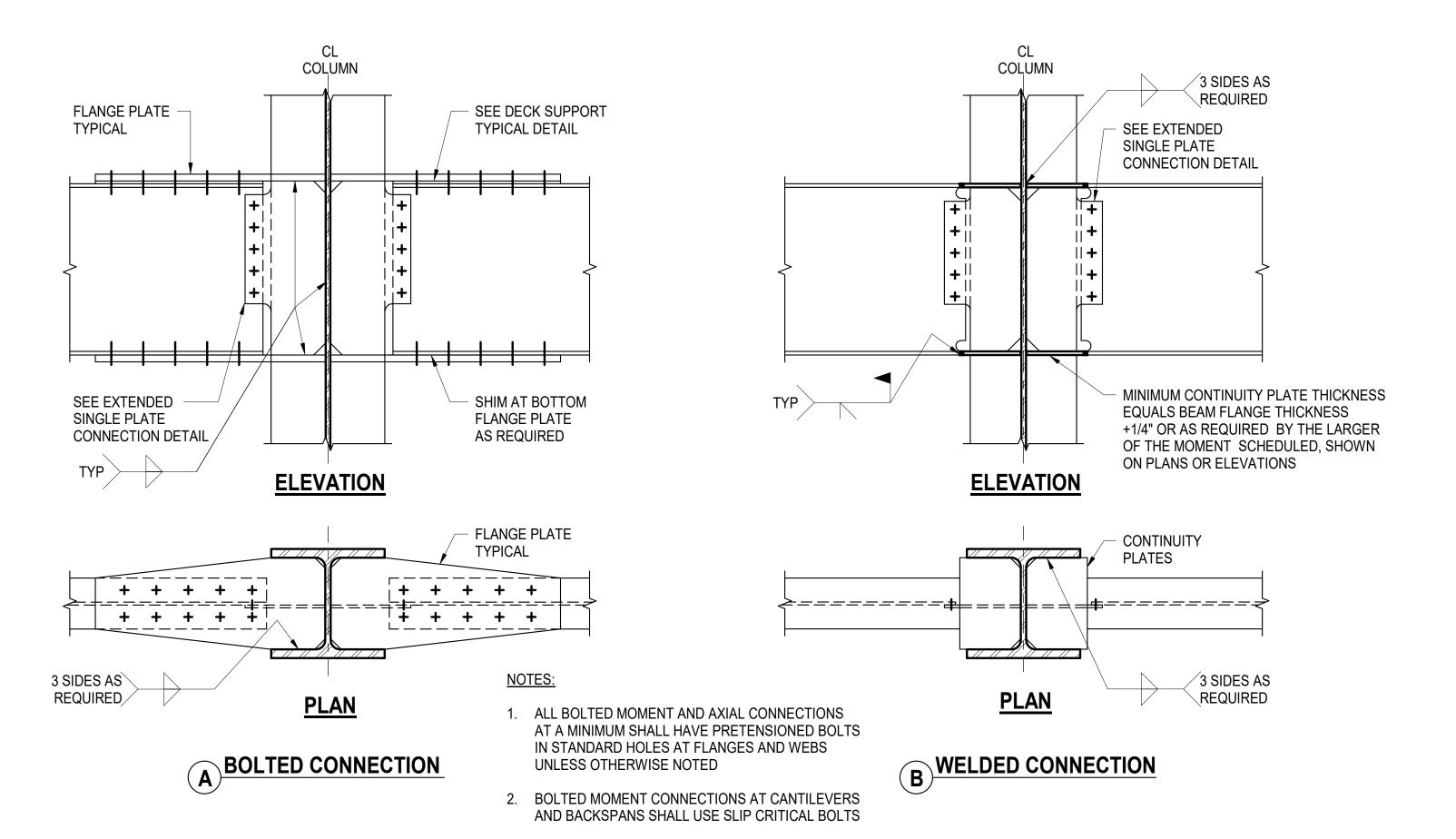
TYPICAL TORSIONAL CONNECTION AT STAIRWELL / TC AND TCS LOCATIONS

WELD BEAM
TOP FLANGE TOP FLANGE SKEWED SINGLE PLATE SKEWED SHEAR END PLATE - BENT PLATE BENT PLATE - WELDED OR BOLTED CONNECTION TOP FLANGE MINIMIZE © BENT PLATE (D) SECTION

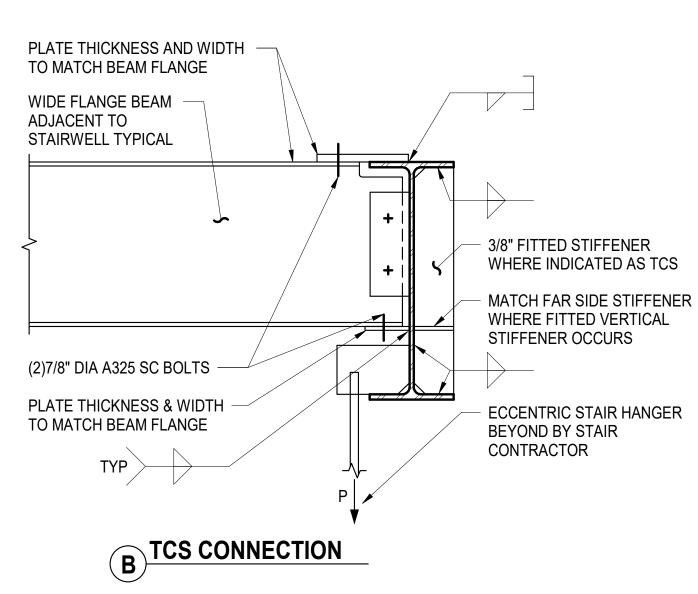
NOTES:

- 1. SEE TYPICAL STEEL BEAM SHEAR CONNECTIONS FOR ADDITIONAL INFORMATION FOR DETAIL A.
- 2. DETAILS B AND C ARE CONCEPTUAL (NOT COMPLETELY DESIGNED) AND ARE INTENDED FOR FRAMING CONDITIONS OUTSIDE THE LIMITS OF COMPLETELY DESIGNED TYPICAL STEEL BEAM SHEAR CONNECTIONS IN THESE DRAWINGS

TYPICAL SKEWED BEAM SHEAR CONNECTION NOT TO SCALE



TYPICAL BEAM TO COLUMN WEB MOMENT CONNECTION - BOLTED / WELDED NOT TO SCALE



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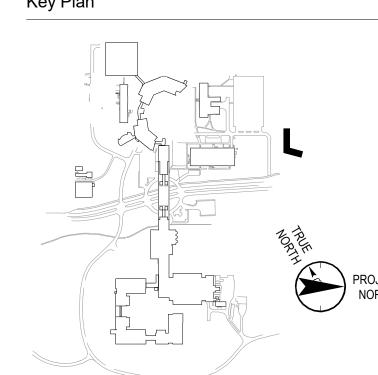
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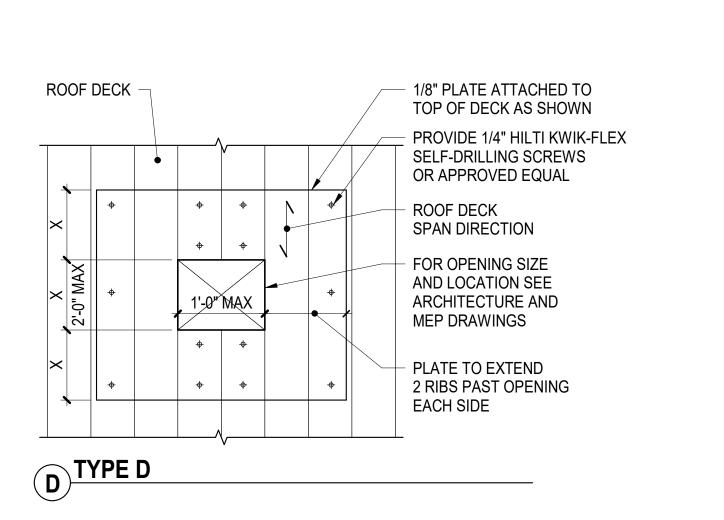
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Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 TYPICAL BEAM DETAILS

Scale: As indicated Floor:



SCALE: NOT TO SCALE

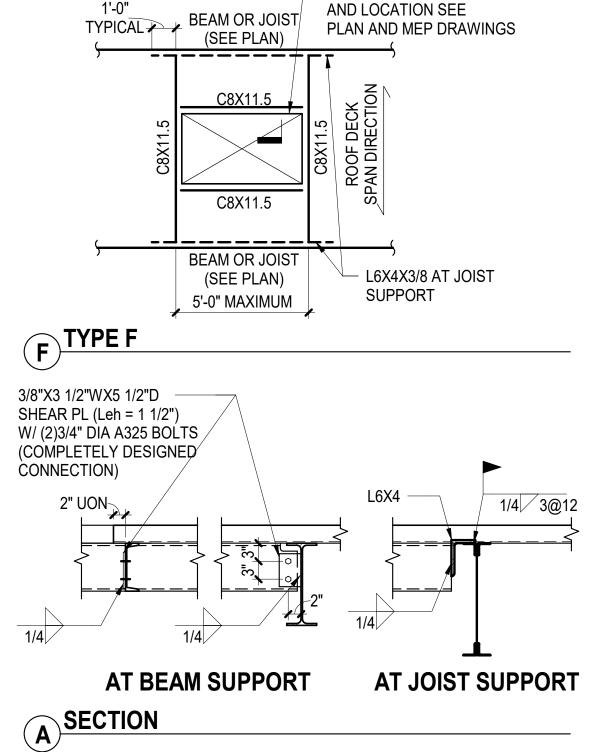
- 1. FOR EDGE OVERHANG GREATER THAN 2" SEE TYPICAL ROOF DECK AT EDGE 2. NO REINFORCEMENT REQUIRED IF OPENING IS 6"x6" OR SMALLER PROVIDED
- ONLY ONE RIB IS INTERRUPTED 3. CLUSTERED OPENINGS WITH CLEAR SPACE LESS THAN 1'-0" SHALL BE TREATED

AS ONE LARGE OPENING AND PROVIDE THE REINFORCEMENT OR CHANNEL

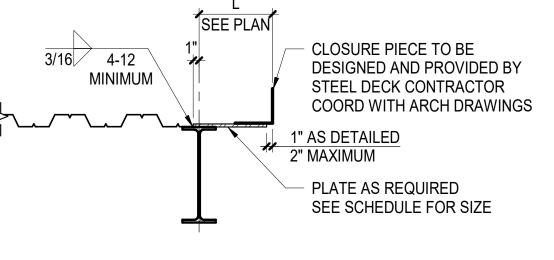
- FRAMING AS PER DETAIL ABOVE 4. ATTACH DECK TO CHANNELS TYPICAL
- 5. TYPE D SHALL NOT BE USED TO SUPPORT MECHANICAL EQUIPMENT

TYPICAL ROOF DECK AT OPENING

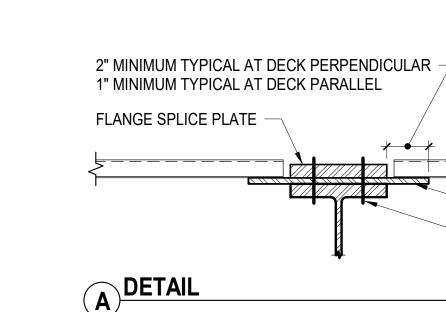
6. TYPE F FRAMING SHALL BE COORDINATED WITH EQUIPMENT SUPPORT FRAMING



FOR OPENING SIZE



THICKNESS OF PLATE OR CLOSURE
1/4"
10 GAGE
12 GAGE



1/4 3 - 12 1/4 3 - 12

" MINIMUM

TYPICAL

1/4 / 3-12

L3X2X3/8

MINIMUM LLH

NOTES:

1. AT CONTRACTOR'S OPTION SINGLE BENT PLATE (OR EQUIV ANGLE) MAY BE USED PROVIDED IT IS FIELD INSTALLED AND ACCOMMODATES SLAB EDGE TOLERANCES

2 TYPICAL ROOF DECK AT EDGE SCALE: NOT TO SCALE

3 TYPICAL DECK SUPPORT AT MOMENT CONNECTION SCALE: NOT TO SCALE

1/4 / 3-12

WELDED TO

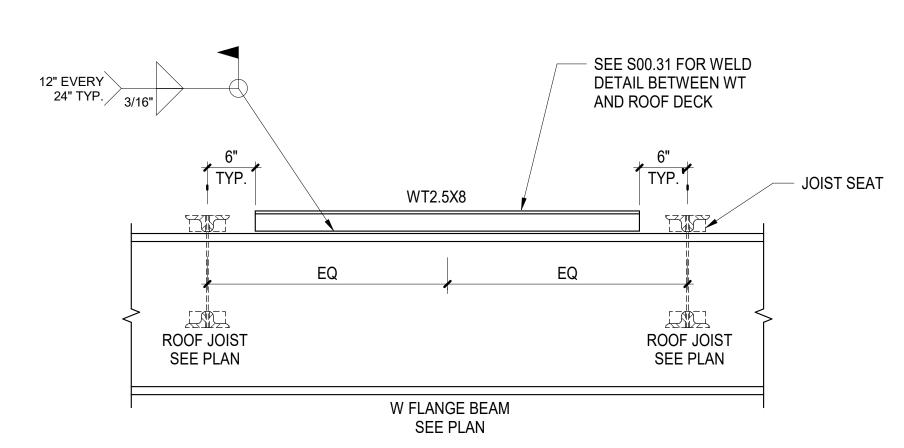
MOMENT PLATE

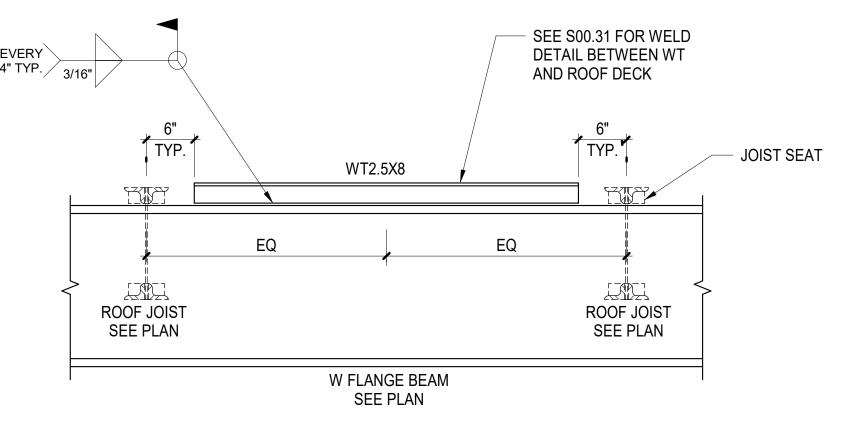
— STEEL DECK

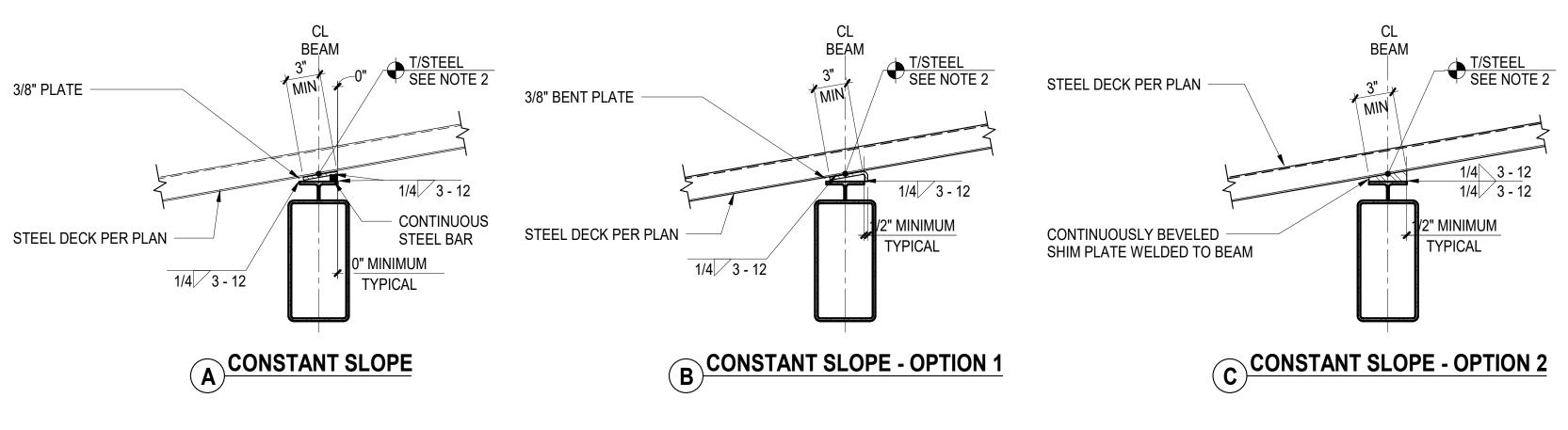
SEE PLAN FOR ORIENTATION

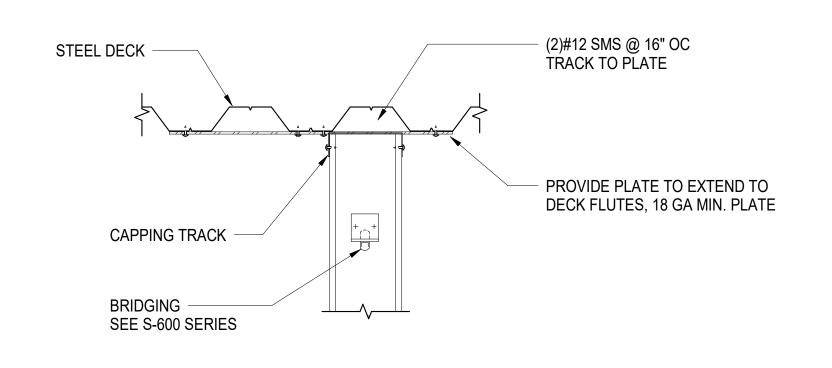
BOLTS AS REQUIRED

FOR MOMENT CONNECTION





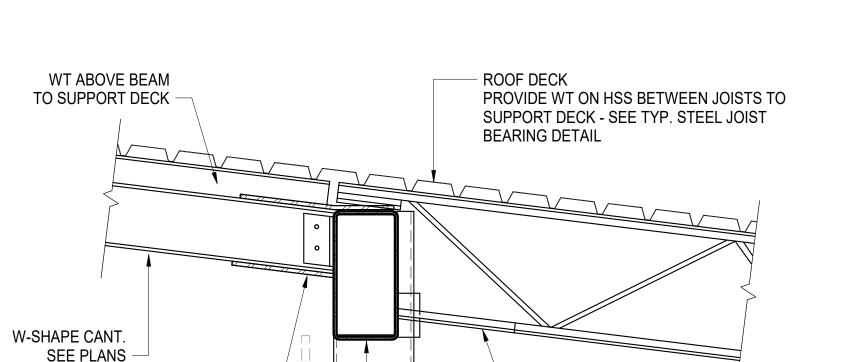




1. TOP OF STEEL ELEVATIONS INDICATED ON PLAN OR ELEVATION IS AT BOTTOM OF DECK AND CENTERLINE OF BEAM CONTRACTOR TO COORDINATE DETAILED TOP OF STEEL ELEVATION WITH SELECTED METHOD OF DECK SUPPORT

TYPICAL SUPPORT OF STEEL DECK INCLINED OVER 5 BEAM SCALE: NOT TO SCALE

TYPICAL DETAIL AT DECK CONNCETION TO BEARING 6 WALL WHEN WALL IS BETWEEN FLUTES SCALE: 1 1/2" = 1'-0"



TYPICAL CONNECTION OF WT WITH WIDE FLANGE

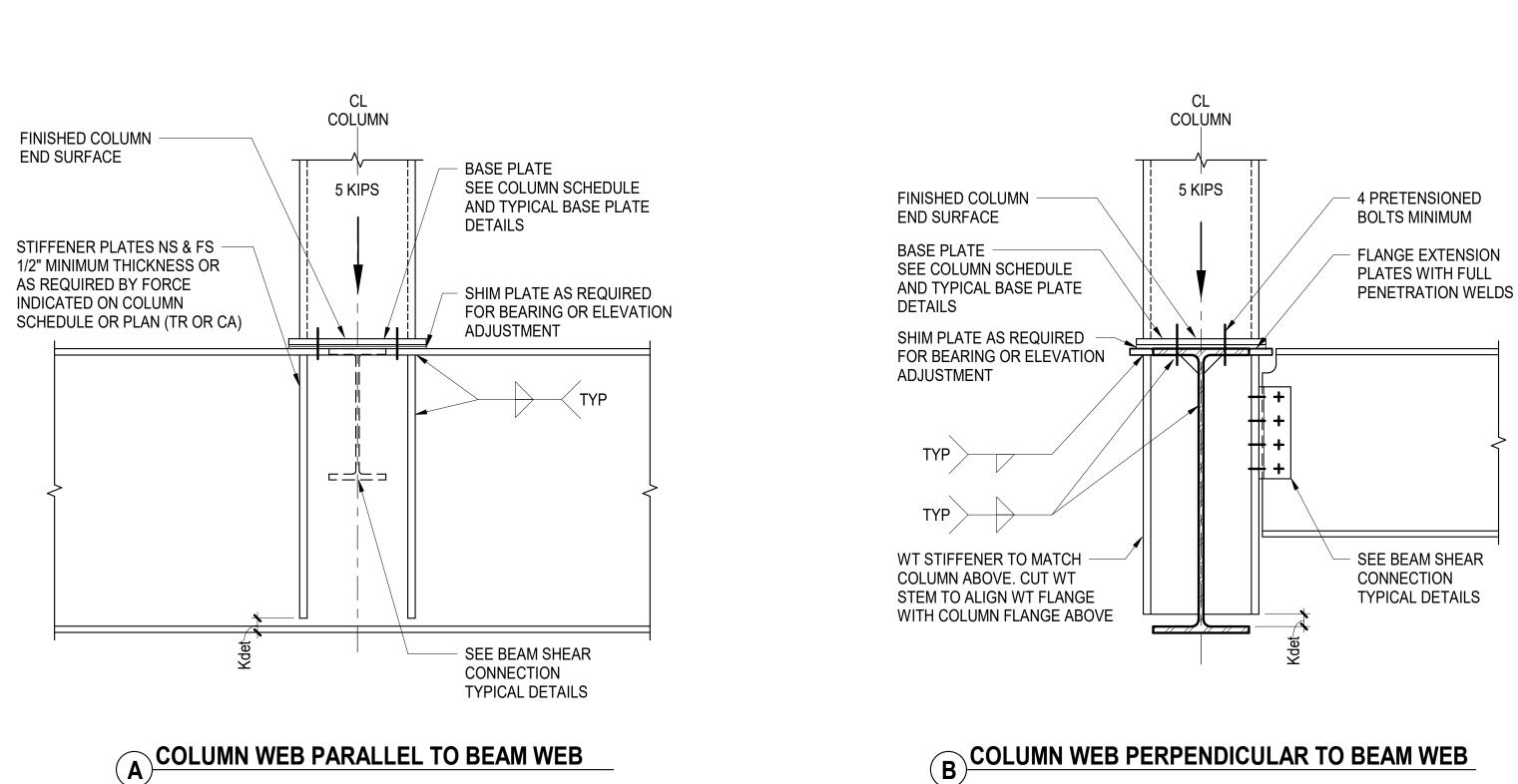
BEAM WITH JOISTS PERPENDICULAR

SCALE: 1" = 1'-0"

SEE PLANS -PROVIDE SHEAR AND -MOMENT CONNECTION FOR JOIST FRAMING, SEE PLANS. EXTENT BOTTOM CHORD AND CONNECT TO SPANDREL TO FORCES SHOWN ON PLAN **BRACE BEAM** - HS\$ SPANDREL, SEE PLANS FACADE SYSTEM ABOVE (SEE ARCH - COLUMN SEE PLAN TUBE ELEVATION SEE ARCH.

NOTE: SEE TYPICAL DETAIL FOR SUPPORT AT INCLINDED BEAM FOR ADD'L INFORMATION SECTION THROUGH GRID E.1 AT ROOF OVERHANG

FACADE SYSTEM



TYPICAL COLUMN BEARING ON BEAM (COMPRESSION

8 ONLY)
SCALE: NOT TO SCALE

Key Plan

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120 Broadway, 15th Floor

MEP / IT / Security Engineer

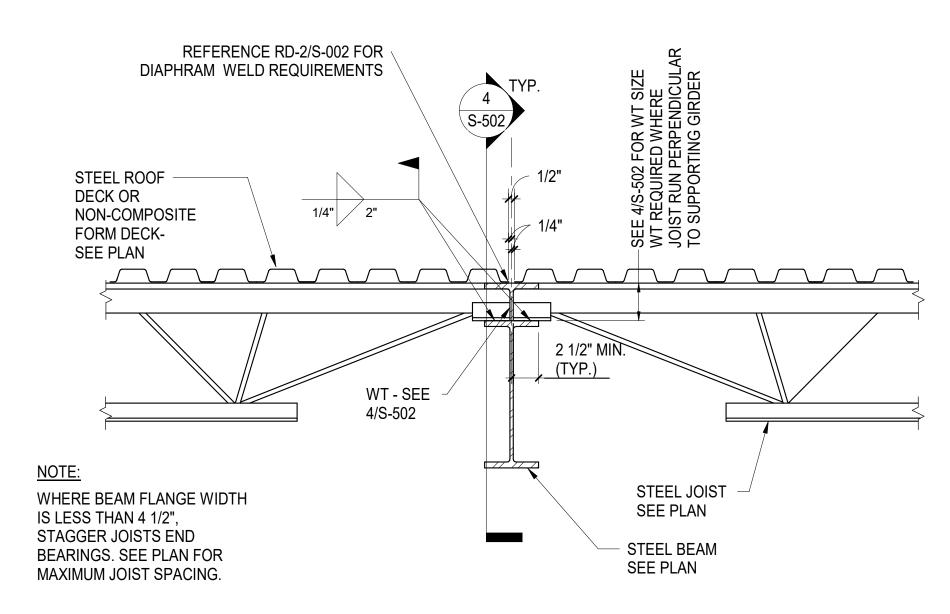
No. Date Description 05.20.2022 ISSUED FOR PERMIT 06.20.2022 100% CONSTRUCTION DOCUMENTS 07.01.2022 100% CONSTRUCTION DOCUMENT- 1

Plot Date: 10/14/20

OF THE DRAWINGS OR WRITTEN MATERIALS APPEARING WITHIN, BE INCORPORATED INTO ANOTHER WORK FOR ANY REASON WITHOUT THE WRITTEN CONSENT OF REGENERON PHARMACEUTICALS. THIS SHEET MUST BE RETURNED UPON THE REQUEST OF SUCH DOCUMENTS OR INFORMATION PRODUCED BY DESIGN PROFESSIONAL OUTSIDE OF THE PROJECT WITHOUT THE EXPRESS WRITTEN CONSENT OF DESIGN PROFESSIONAL, REGENERON ASSUMES FULL RESPONSIBILITY FOR ANY AND ALL RISKS INVOLVED IN SUCH

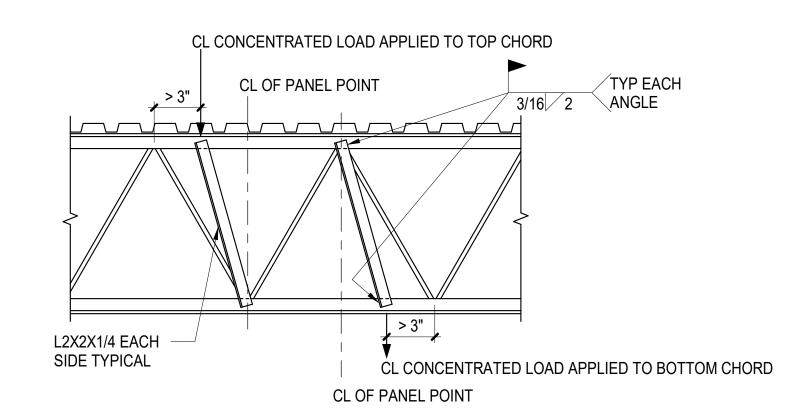
Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 TYPICAL SUPERSTRUCTURE **DETAILS**



TYPICAL STEEL JOIST BEARING DETAIL

SCALE: 3/4" = 1'-0"



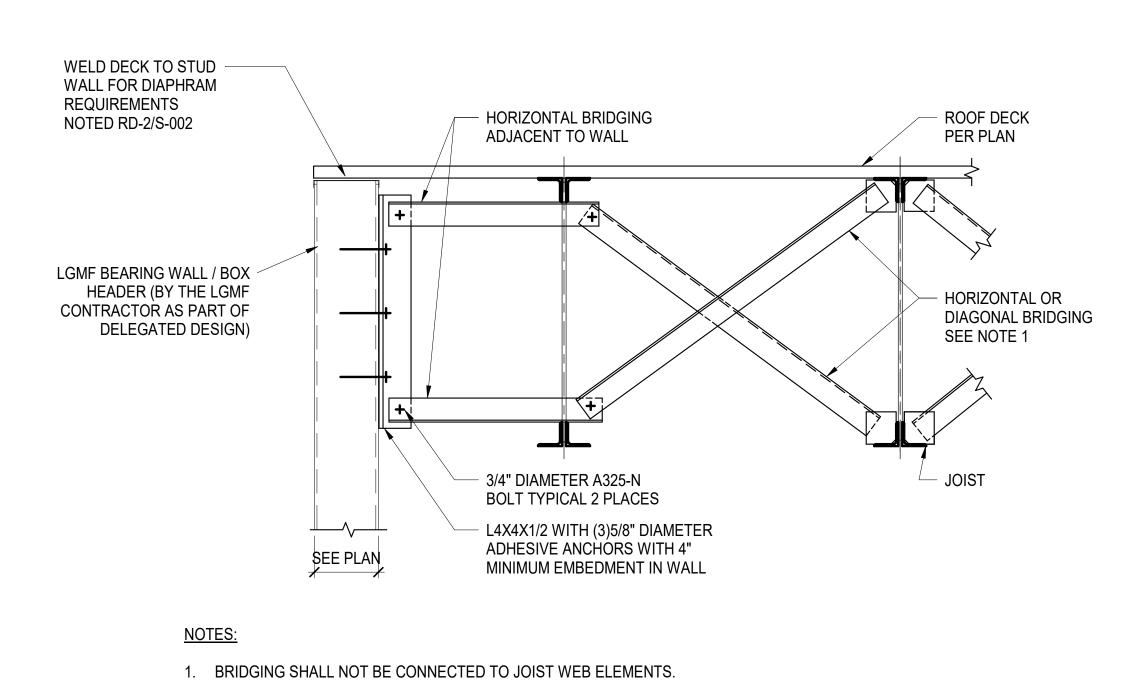
NOTES:

- 1. THE TOTAL SUPERIMPOSED LOAD ON ANY JOIST SHALL NOT EXCEED THE LOADING SHOWN ON THE STRUCTURAL DRAWINGS INCLUDING CONCENTRATED AND UNIFORM LOADS. CONTRACTOR SHALL COORDINATE ALL SUBCONTRACTORS HANGING METHODS, LOCATIONS, AND LOADS WITH LIMITS PROVIDED IN STRUCTURAL DRAWINGS.
- 2. HANGING CONCENTRATED LOADS SHALL BE APPLIED CONCENTRIC TO THE JOIST CHORD.
- 3. JOIST REINFORCEMENT IS NOT REQUIRED FOR CONCENTRATED LOADS LESS THAN 100 POUNDS.
- 4. CONCENTRATED LOADS LOCATED WITHIN 3" OF THE PANEL POINT CAN BE CONSIDERED TO ACT AT THE PANEL POINT.
- 5. FOR CONCENTRATED LOADS THAT DO NOT MEET THE CRITERIA OF NOTES 3 AND 4, PROVIDE JOIST REINFORCEMENT PER THIS DETAIL. TOTAL LOAD APPLIED TO THE JOIST MUST SATISFY NOTE 1.

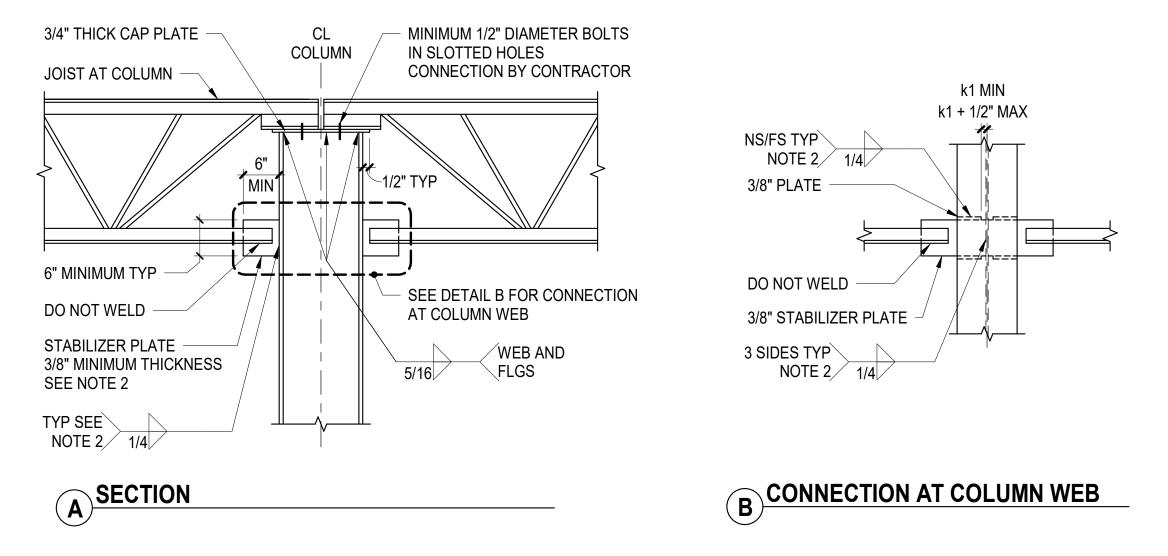
TYPICAL JOIST REINFORCEMENT AT CONCENTRATED

4 LOAD

SCALE: NOT TO SCALE



TYPICAL JOIST BRIDGING TO BEARING WALL



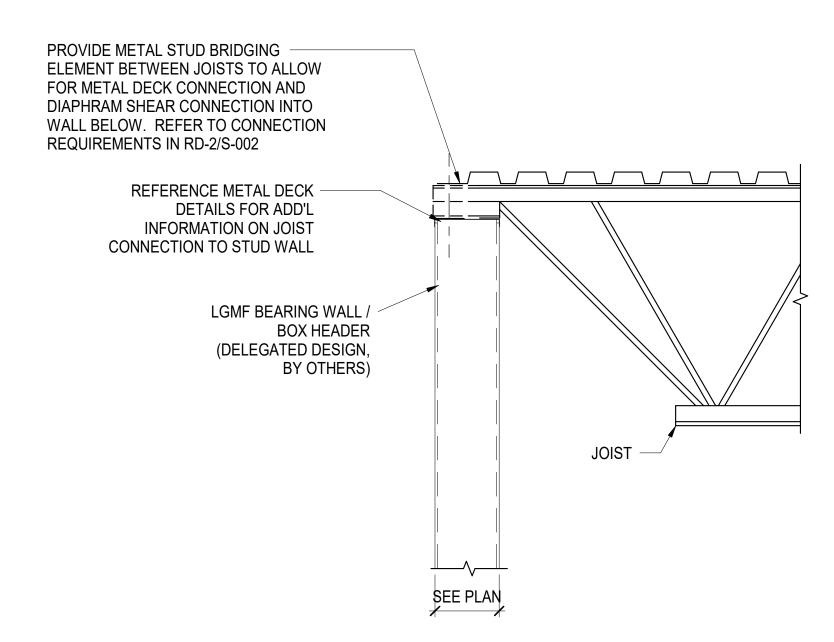
NOTES:

SCALE: NOT TO SCALE

- 1. PROVIDE SIMILAR CONNECTION AT HSS COLUMNS.
- 2. IF TEMPORARY ERECTION BRACING IS ATTACHED TO THE BOTTOM CHORD STABILIZER PLATE,
- THE CONNECTION OF THE STABILIZER PLATE TO THE COLUMN MUST BE DESIGNED BY THE CONTRACTOR FOR THE ERECTION STABILITY LOADS AND THE REQUIRED PLATE AND WELDS MAY EXCEED THAT SHOWN IN THIS DETAIL.

TYPICAL JOIST TO TOP OF COLUMN DETAILS

SCALE: NOT TO SCALE



NOTES:

- JOIST BEARING PLATE IS CENTERED ON BEARING WALL BELOW CONTRACTOR SHALL ACCOUNT FOR BEARING LOCATION IN DESIGN OF JOIST SEAT.
- 3 TYPICAL JOIST TO BEARING WALL DETAIL SCALE: NOT TO SCALE



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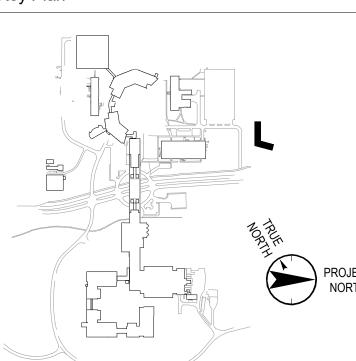
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Key Plan



No. Date Description

05.20.2022 ISSUED FOR PERMIT

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07.01.2022 100% CONSTRUCTION DOCUMENT- 1

Plot Date: 03/03/22

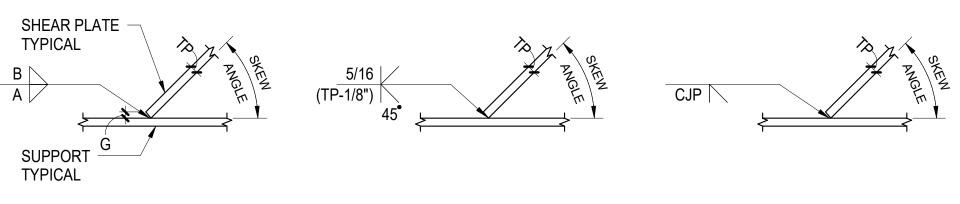
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Professional Seal and Signature

Vendor Name: GENSLER
Vendor Project No.: 006.3608.000
Discipline: Drawn By: A
TYPICAL JOIST DETAILS

Scale: As indicated Floor:

- 1. ALL SHEAR CONNECTIONS PROVIDED ON THIS SHEET ARE COMPLETELY DESIGNED REFER TO STRUCTURAL GENERAL NOTES FOR ADDITIONAL REQUIREMENTS
- 2. ALL CONNECTION CAPACITIES PROVIDED IN SCHEDULES ARE STRENGTH LEVEL PER AISC-LOAD AND RESISTANCE FACTOR DESIGN. SELECT CONNECTION CAPACITY BASED ON FACTORED SHEAR REACTIONS SHOWN ON PLAN AND REQUIREMENTS OF DETAILS.
- 3. SINGLE PLATE, SINGLE ANGLE, OR DOUBLE ANGLE SHEAR ONLY CONNECTIONS MAY BE USED AT THE CONTRACTOR'S OPTION UNLESS OTHERWISE NOTED IN THE CONSTRUCTION DOCUMENTS
- 4. ALL CONNECTIONS SHOWN ARE DESIGNED UTILIZING BOLTS INDICATED
- 5. THE FOLLOWING MINIMUM BOLT EDGE DISTANCES AND SPACING SHALL BE USED: HORIZONTAL EDGE DISTANCE = 2 x BOLT DIAMETER VERTICAL EDGE DISTANCE = 1-1/2" SPACING = 3"
- 6. ALL CONNECTIONS SHOWN ARE DESIGNED UTILIZING PLATE MATERIAL CONFORMING TO ASTM A572 OR A529 GRADE 50
- 7. PROVIDE STANDARD HOLES IN BEAM. STANDARD OR HORIZONTAL SHORT SLOTTED HOLES ARE PERMITTED IN THE SINGLE PLATE
- 8. TYPICAL SINGLE PLATE SHEAR CONNECTION DETAILS SHOWN ARE FOR PERPENDICULAR FRAMING CONDITIONS. AT SKEWED FRAMING CONDITIONS, PROVIDE WELDS AS INDICATED IN TYPICAL SKEWED SINGLE PLATE WELD DETAILS.



A FILLET WELD





- 1. DETAIL NOTATIONS: TP INDICATES THICKNESS OF PLATE
- D INDICATES FILLET WELD SIZE AS SPECIFIED FOR PERPENDICULAR FRAMING CONDITION
- (OTHER DETAILS THIS SHEET) G INDICATES GAP IF SQUARE CUT PLATE IS USED
- 2. FILLET WELDS: FOR G ≤ 3/16" USE SQUARE CUT PLATE
- FOR G > 3/16", BEVEL PLATE SUCH THAT G = 0 (TOLERANCE = +1/16", -0) FOR (D + G) > 3/4", BEVEL PLATE SUCH THAT G = 0 (TOLERANCE = +1/16", -0)
- 3. PJP AND CJP WELDS:

WELD PREPARATIONS NOT SHOWN 2 TYPICAL SKEWED SINGLE PLATE WELD DETAILS NOT TO SCALE

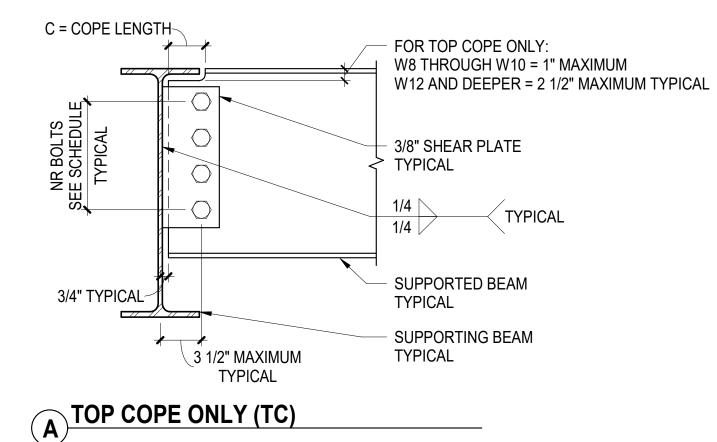
TYPE SP1 SINGLE PLATE

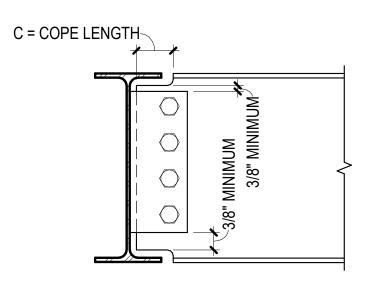
SHEAR CONNECTION SCHEDULE:

3/4" DIAMETER ASTM F3125-N BOLTS

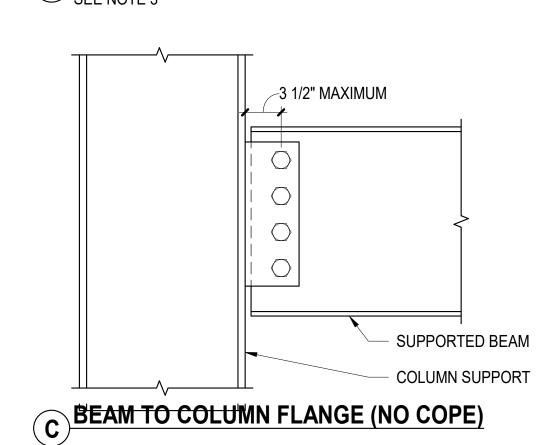
SKEWED SINGLE PLATE WELD SCHEDULE **SKEW ANGLE** WELD B (DEGREES) CJP WELD PJP WELD > 45 - 50 (D*1.31) + G > 50 - 55 > 55 - 60 > 60 - 65 > 65 - 70 (D*1.16) + G(D*1.12) + G > 80 - 85 (D*1.08) + G

1 SINGLE PLATE SHEAR CONNECTION NOTES





B TOP AND BOTTOM COPE (TBC)



CONNECTION CAPACITY (KIPS)									
RF	AM SIZE	NR	NO		: 4" C =		6"		- 8"
		BOLTS	COPE	TC	TBC	TC	TBC	TC	TBC
W8	x10	2	23	20	9	14	6	11	4
	x13-24	2	23	23	13	19	9	14	7
	x28+	2	23	23	16	23	11	20	9
W10	x12	2	23	23	11	23	7	19	5
	x15-26	2	23	23	13	23	9	23	7
	x30+	2	23	23	17	23	12	23	9
W12	x14-16	2	23	23	11	23	8	20	6
		3	42	40	25	28	16	20	11
	x19-30	2	23	23	13	23	9	23	7
		3	42	42	30	37	20	28	14
	x35+	2	23	23	17	23	12	23	9
		3	42	42	38	42	27	41	20
W14	x22-30	3	42	42	29	42	19	37	14
	x34-48	3	42	42	36	42	26	42	19
	x53+	3	42	42	42	42	33	42	26
W16	x26-31	3	42	42	32	42	22	42	16
		4	62	62	57	62	37	54	26
	x36-40	3	42	42	38	42	27	42	20
		4	62	62	62	62	47	62	33
	x45+	3	42	42	42	42	31	42	24
		4	62	62	62	62	55	62	42
W18	x35-50	3	42	42	38	42	27	42	20
		4	62	62	62	62	48	62	34
		5	81	81	81	81	73	81	52
	x55+	3	42	42	42	42	35	42	27
		4	62	62	62	62	62	62	48
		5	81	81	81	81	81	81	75
W21	x44-63	4	62	62	62	62	56	62	43
		5	81	81	81	81	81	81	64
		6	85	85	85	85	85	85	85
	x68+	4	62	62	62	62	62	62	53
		5	81	81	81	81	81	81	81
		6	85	85	85	85	85	85	85
W24 - 4	4 MIN	4	62	62	62	62	62	62	49
		5	81	81	81	81	81	81	76
		6	85	85	85	85	85	85	85
W27, V	V30 - 5 MIN	5	81	81	81	81	81	81	81
		6	85	85	85	85	85	85	85
		7	104	104	104	104	104	104	104
		8	123	123	123	123	123	123	123
	/40 - 7 MIN	7	104	104	104	104	104	104	104

- 1. SEE TYPICAL SINGLE PLATE SHEAR CONNECTION GENERAL NOTES FOR ADDITIONAL INFORMATION
- 2. SELECT CONNECTION CAPACITY BASED ON COPED CONDITION OF BEAM
- 3. TBC VALUES IN TABLE ALSO APPLY TO BOTTOM COPE ONLY CONDITION
- TYPICAL SINGLE PLATE SHEAR CONNECTION AT BEAM OR COLUMN FLANGE (TYPE SP1)

 NOT TO SCALE

	5/16 TYPICAL TO COLUMN FLANGE
DITS EDULÉ	1/2" STABILIZER PLATE TOP AND BOTTO OR CONTINUITY PLATES AS OCCUR SEE NOTE 4 5/16 TYPICAL TO SHEAR PLATE
NR BOLTS SEE SCHEDUL	1/2" SHEAR PLATE 5/16 5/16
SEE SCHEDULE 2 COLUMNS	

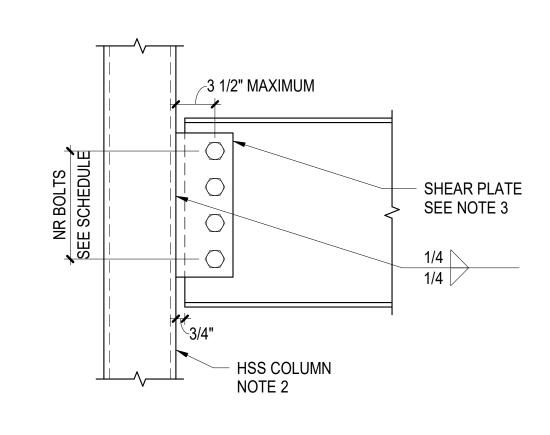
SHEAR CONNECTION SCHEDULE: 3/4" DIAMETER ASTM F3125-N BOLTS GRADES A325 OR F1852								
BEAM SIZE NR CONNECTION CAPACITY (KIPS)								
DEAIVI SIZE	BOLTS	A ≤ 6"	A ≤ 8"	A ≤ 10 1/2"	A ≤ 12"			
W8 - W12	2	13	10	8	8			
W12 - W18	3	30	24	20	18			
W16 - W24	4	54	44	36	32			
W18 - W30	5	80	66	54	48			
W21 - W40	6	110	91	75	68			
W27 - W44	7	142	120	99	89			
W30 - W44	8	177	151	125	114			
W33 - W44	9	214	184	154	140			
W36 - W44	10	251	219	186	169			

TYPE SP2 SINGLE PLATE

NOTES:

- 1. SEE TYPICAL SINGLE PLATE SHEAR CONNECTION GENERAL NOTES FOR ADDITIONAL INFORMATION
- 2. SELECT CONNECTION CAPACITY BASED ON DETAILING DIMENSION "A"
- 3. MINIMUM COLUMN WEB THICKNESS SHALL BE 0.29" (INCLUDES: W8x31+, W10x33+, W12x40+, W14x43+)
- 4. STABILIZER PLATES ONLY REQUIRED FOR SHADED CAPACITY CASES
- 5. SEE TYPICAL DETAIL FOR ADDITIONAL INFORMATION ON SINGLE PLATE CONNECTIONS TO COLUMN WEB WHEN STIFFENERS FROM OTHER CONNECTIONS ARE PRESENT

TYPICAL SINGLE PLATE SHEAR CONNECTION AT COLUMN WEB (TYPE SP2) NOT TO SCALE



TYPE SP3 SINGLE PLATE SHEAR CONNECTION SCHEDULE: 3/4" DIAMETER ASTM F3125-N BOLTS GRADES A325 OR F1852									
CONNECTION CAPACITY (KIPS)									
BEAM SIZE	NR BOLTS	HSS WALL THICKNESS ≥ 3/8" SHEAR PLATE THICKNESS = 3/8"	HSS WALL THICKNESS = 1/4" or 5/16" SHEAR PLATE THICKNESS = 1/4"						
W8 - W12	2	23	23						
W12 - W18	3	42	42						
W16 - W24	4	62	62						
W18 - W30	5	81	78						
W21 - W40	6	85	85						
W27 - W44	7	104	104						
W30 - W44	8	123	123						
W33 - W44	9	142	142						
W36 - W44	10	161	161						

NOTES:

- 1. SEE TYPICAL SINGLE PLATE SHEAR CONNECTION GENERAL NOTES FOR ADDITIONAL INFORMATION
- 2. DETAIL APPLIES TO SQUARE, RECTANGULAR, OR ROUND HSS COLUMNS 3. SINGLE PLATE THICKNESS BASED ON HSS WALL THICKNESS PER SCHEDULE
- TYPICAL SINGLE PLATE SHEAR CONNECTION AT HSS COLUMN (TYPE SP3)
 NOT TO SCALE

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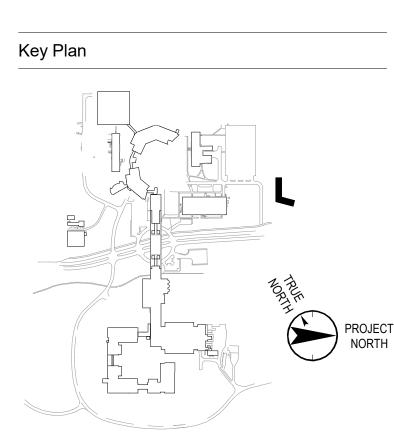
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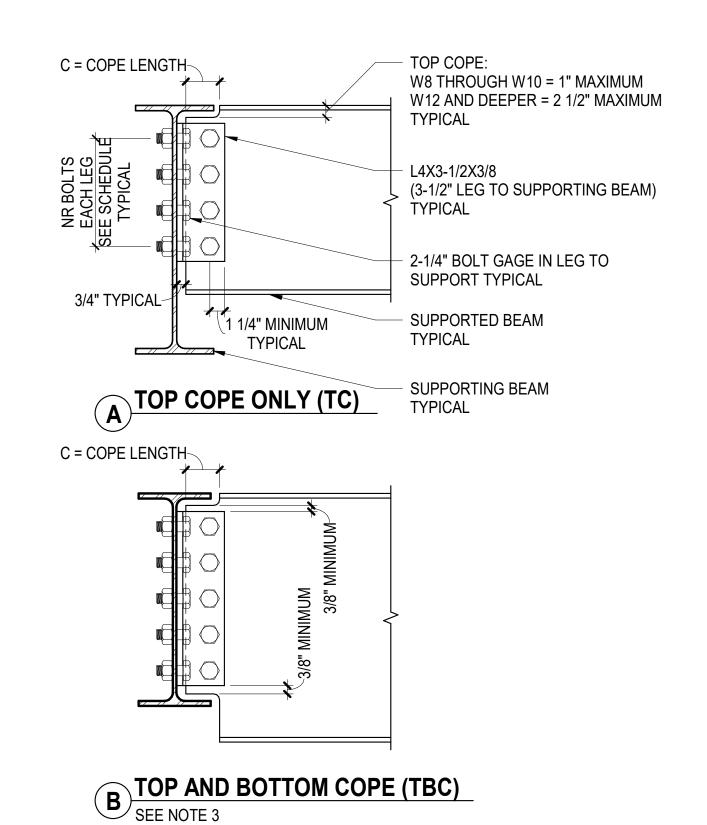
Plot Date: 09/13/18 ANOTHER WORK FOR ANY REASON WITHOUT THE WRITTEN CONSENT OF REGENERON PHARMACEUTICALS. THIS SHEET MUST BE RETURNED UPON THE REQUEST OF SUCH DOCUMENTS OR INFORMATION PRODUCED BY DESIGN PROFESSIONAL OUTSIDE OF THE PROJECT WITHOUT THE EXPRESS WRITTEN CONSENT OF DESIGN PROFESSIONAL, REGENERON ASSUMES FULL RESPONSIBILITY FOR ANY AND ALL RISKS INVOLVED IN SUCH

Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 TYPICAL STEEL BEAM SHEAR CONNECTIONS 3/4" DIAMETER BOLTS

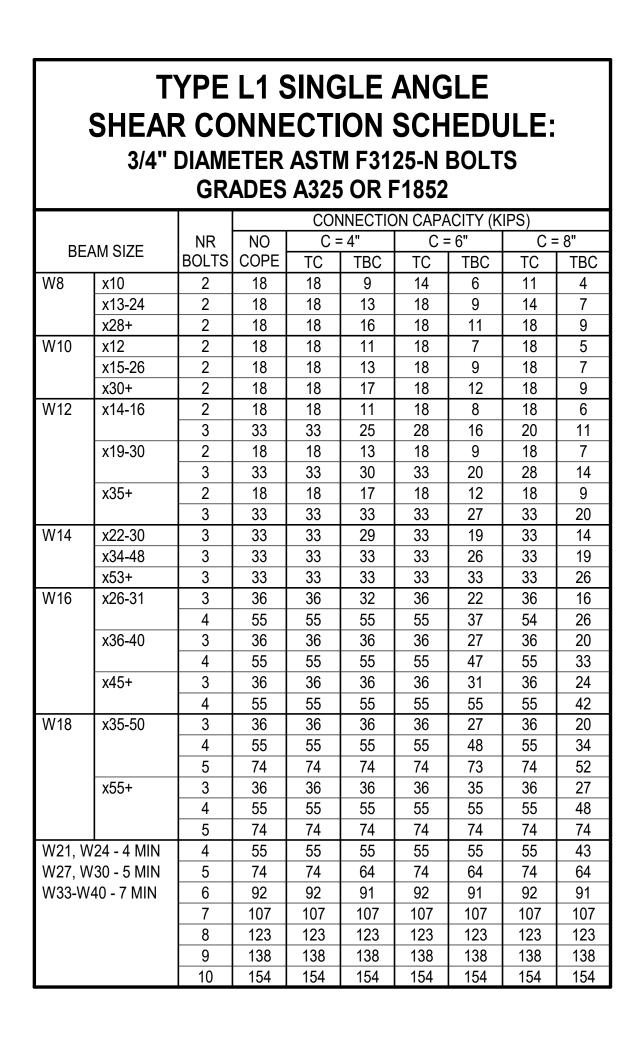
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- 3. SINGLE PLATE, SINGLE ANGLE, OR DOUBLE ANGLE CONNECTIONS MAY BE USED AT THE CONTRACTOR'S OPTION UNLESS OTHERWISE NOTED IN THE CONSTRUCTION DOCUMENTS
- 4. ALL CONNECTION SHOWN ARE DESIGNED UTILIZING BOLTS INDICATED
- 5. THE FOLLOWING MINIMUM BOLT EDGE DISTANCES AND SPACING SHALL BE USED: HORIZONTAL EDGE DISTANCE = 1-1/2" UON VERTICAL EDGE DISTANCE = 1-1/2"
- 6. ALL CONNECTIONS SHOWN ARE DESIGNED UTILIZING ANGLE MATERIAL CONFORMING TO ASTM A572 OR A529 GRADE 50
- 7. WHERE SINGLE ANGLE OR DOUBLE ANGLE CONNECTIONS ARE ALIGNED EACH SIDE OF THE SUPPORTING BEAM WEB (SHARED BOLTS), MINIMUM SUPPORTING BEAM WEB THICKNESS SHALL BE 0.25" FOR W8-W14 CONNECTIONS AND 0.35" FOR W16 AND DEEPER CONNECTIONS. CONNECTION SHALL BE DETAILED TO PERMIT ERECTION OF BEAMS EACH SIDE PER OSHA STANDARDS
- 8. NOTATIONS: INDICATES SHORT SLOTTED HOLES TRANSVERSE TO LINE OF FORCE

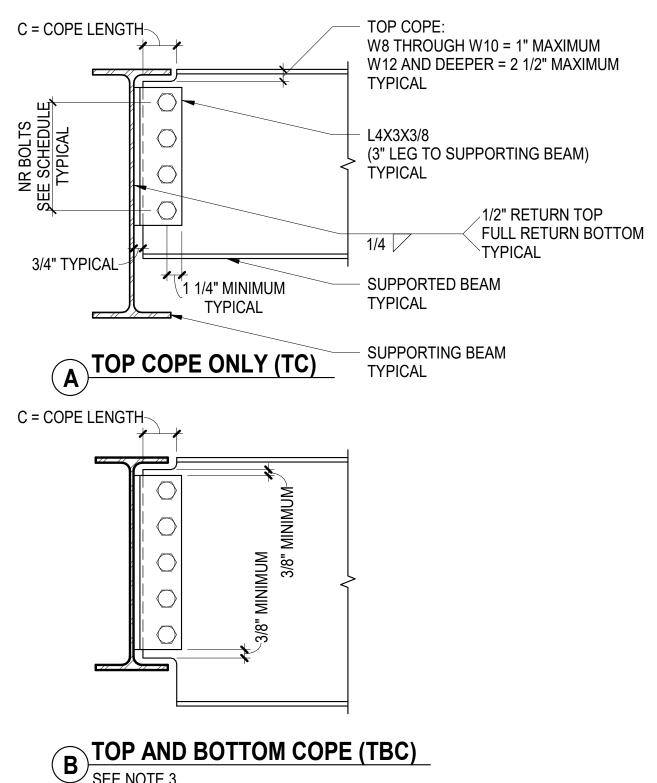
ANGLE SHEAR CONNECTION NOTES



- 1. SEE TYPICAL ANGLE SHEAR CONNECTION GENERAL NOTES FOR ADDITIONAL INFORMATION
- 2. SELECT CONNECTION CAPACITY BASED ON COPED CONDITION OF BEAM
- 3. TBC VALUES IN TABLE ALSO APPLY TO BOTTOM COPE ONLY CONDITION
- 4. SSLT HOLES IN ANGLE LEG TO SUPPORTED BEAM, STANDARD HOLES IN ANGLE LEG TO SUPPORT
- TYPICAL SINGLE ANGLE SHEAR CONNECTION AT BEAM (TYPE L1) BOLTED-BOLTED

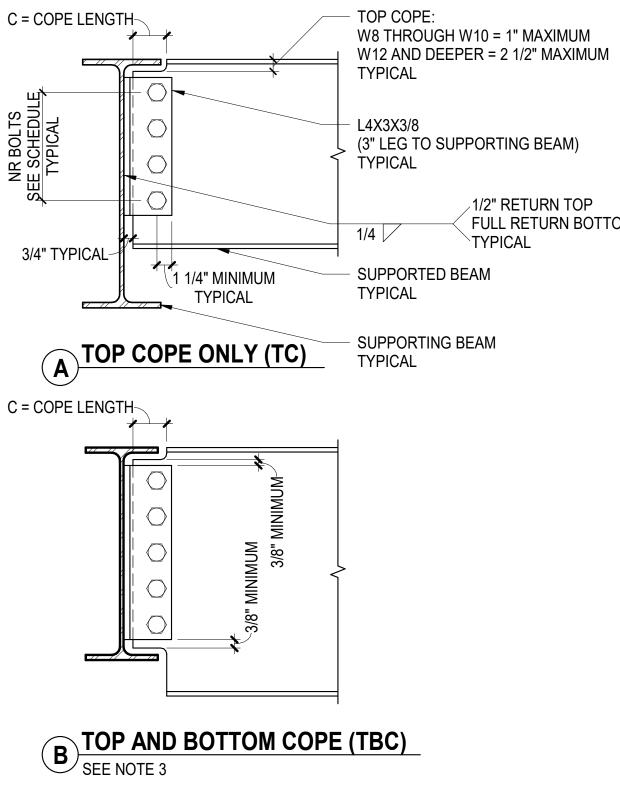
 NOT TO SCALE





- 1. SEE TYPICAL ANGLE SHEAR CONNECTION GENERAL NOTES FOR ADDITIONAL INFORMATION
- 2. SELECT CONNECTION CAPACITY BASED ON COPED CONDITION OF BEAM
- 3. TBC VALUES IN TABLE ALSO APPLY TO BOTTOM COPE ONLY CONDITION
- TYPICAL SINGLE ANGLE SHEAR CONNECTION AT BEAM (TYPE L2) BOLTED-WELDED

 NOT TO SCALE 4. SSLT HOLES IN ANGLE LEG TO SUPPORTED BEAM, STANDARD HOLES IN ANGLE LEG TO SUPPORT



TYPE L2 SINGLE ANGLE

SHEAR CONNECTION SCHEDULE:

3/4" DIAMETER ASTM F3125-N BOLTS

GRADES A325 OR F1852

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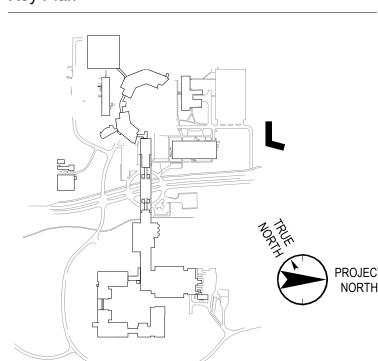
Civil Engineer

JMC 120 Bedford Road Armonk, NY 10504 (914) 273-5225 Phone (914) 273-2102 Fax

Landscape Architect

Langan 21 Penn Plaza, 360 West 31st Street, 8th Floor New York, NY 10001 (212) 479-5400 Phone (212) 479-5444 Fax

Key Plan



No. Date Description 05.20.2022 ISSUED FOR PERMIT 06.20.2022 100% CONSTRUCTION DOCUMENTS 07.01.2022 100% CONSTRUCTION DOCUMENT- 1

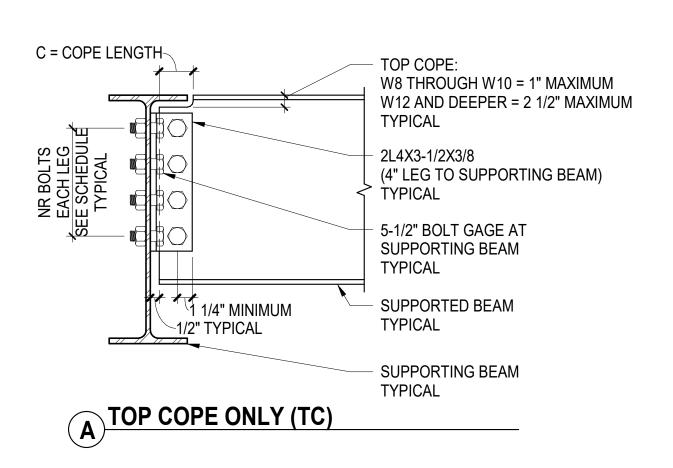
Plot Date: 09/19/18

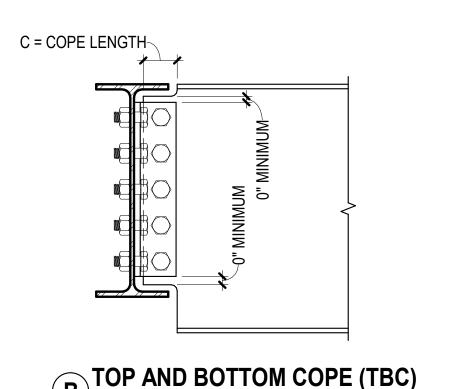
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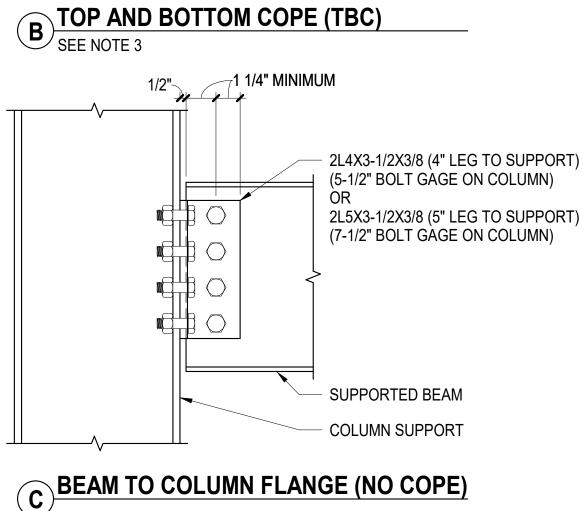
Professional Seal and Signature

Vendor Name: GENSLER
Vendor Project No.: 006.3608.000 Discipline: Drawn By: TLL
TYPICAL STEEL BEAM SHEAR CONNECTIONS 3/4" DIAMETER BOLTS

Scale: NOT TO SCALE Floor:





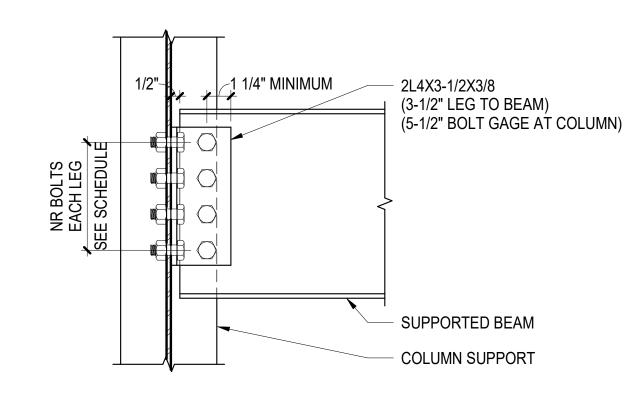


TYPE 2L1 DOUBLE ANGLE SHEAR CONNECTION SCHEDULE:									
	3/4"	DIAMI GR		_	M F31: 5 OR F	_	_	S	
					NECTIO				
BE	AM SIZE	NR BOLTS	NO COPE	C =	= 4" TBC	C =	= 6" TBC	C =	= 8" TBC
W8	x10	2	25	20	9	14	6	11	4
	x13-24	2	34	27	13	19	9	14	7
	x28+	2	43	37	16	26	11	20	9
W10	x12	2	28	28	11	24	7	19	5
	x15-26	2	34	34	13	30	9	23	7
	x30+	2	44	44	17	44	12	37	9
W12	x14-16	2	30	30	11	28	8	20	6
		3	48	40	25	28	16	20	11
	x19-30	2	35	35	13	35	9	28	7
		3	56	49	30	37	20	28	14
	x35+	2	44	44	17	44	12	41	9
		3	66	65	38	54	27	41	20
W14	x22-30	3	55	53	29	49	19	37	14
	x34-48	3	66	66	36	66	26	51	19
	x53+	3	66	66	47	66	33	66	26
W16	x26-31	3	59	58	32	58	22	54	16
		4	81	71	57	71	37	54	26
	x36-40	3	70	68	38	68	27	68	20
		4	96	85	67	85	47	70	33
	x45+	3	82	80	44	80	31	80	24
		4	112	101	78	101	55	85	42
W18	x35-50	3	71	69	38	69	27	69	20
		4	98	88	68	88	48	88	34
		5	124	95	93	95	73	88	52
	x55+	3	92	90	50	90	35	90	27
		4	123	115	89	115	62	115	48
		5	154	128	121	128	98	126	75
W21	x44-63	4	114	103	80	103	56	103	43
		5	145	125	109	125	88	125	64
		6	175	133	131	133	126	133	91
	x68+	4	123	123	98	123	69	123	53
		5	154	153	134	153	108	153	83
		6	184	168	160	168	155	168	119
W24 - 4	4 MIN	4	123	116	90	116	63	116	49
W27, W30 - 5 MIN		5	154	141	123	141	99	141	76
W33-W	/40 - 7 MIN	6	184	165	147	165	142	165	108
		7	215	215	200	215	200	215	174
		8	246	246	234	246	234	246	232
		9	276	276	276	276	276	276	276
		10	307	307	307	307	307	307	307

- 1. SEE TYPICAL ANGLE SHEAR CONNECTION GENERAL NOTES FOR ADDITIONAL INFORMATION
- 2. SELECT CONNECTION CAPACITY BASED ON COPED CONDITION OF BEAM
- 3. TBC VALUES IN TABLE ALSO APPLY TO BOTTOM COPE ONLY CONDITION
- 4. SSLT HOLES IN ANGLE LEG TO SUPPORT
- 5. MINIMUM COLUMN FLANGE WIDTH SHALL BE 8" (INCLUDES: W10x33+, W12x40+, W14x43+)

TYPICAL DOUBLE ANGLE SHEAR CONNECTION AT BEAM OR COLUMN FLANGE (TYPE 2L1) - BOLTED-BOLTED

NOT TO SCALE

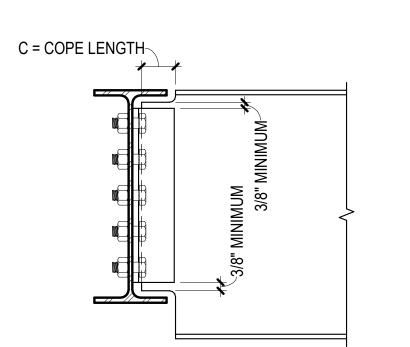


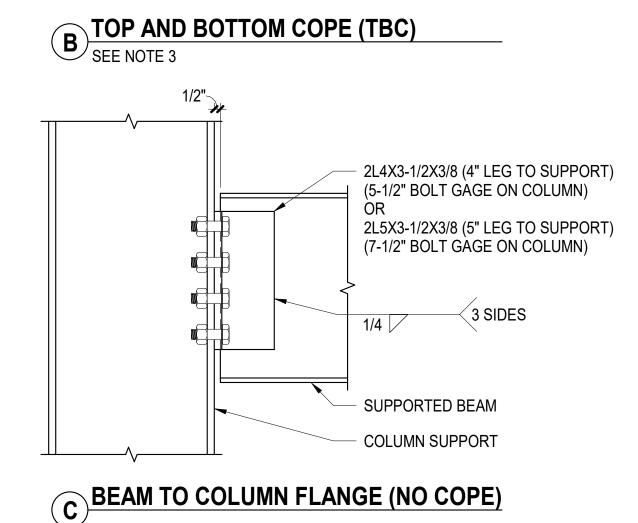
TYPE 2L3 DOUBLE ANGLE SHEAR CONNECTION SCHEDULE: 3/4" DIAMETER ASTM F3125-N BOLTS										
	GRADES A325 OR F1852									
BEAM SIZE	BOLTS	CONNECTION CAPACITY (KIPS)								
W8 - W12	2	25								
W12	3	48								
W14 - W18	3	55								
W16 - W24	4	81								
W18 - W30	5	124								
W21 - W40	6	153								
W27 - W44	7	178								
W30 - W44	8	204								
W33 - W44	9	229								
W36 - W44	10	254								

- 1. SEE TYPICAL ANGLE SHEAR CONNECTION GENERAL NOTES FOR ADDITIONAL INFORMATION
- 2. SSLT HOLES IN ANGLE LEG TO COLUMN SUPPORT
- 3. PROVIDE ERECTION SEAT PER FABRICATOR WHEN SAME DEPTH CONNECTION IS REQUIRED ON BOTH SIDES AND BOLT STAGGERING IS NOT POSSIBLE
- 4. MINIMUM COLUMN WEB THICKNESS SHALL BE 0.29" (INCLUDES: W8x31+, W10x33+, W12x40+, W14x43+)
- TYPICAL DOUBLE ANGLE SHEAR CONNECTION AT COLUMN WEB (TYPE 2L3) BOLTED-BOLTED

 NOT TO SCALE

SE SCHEDULE TYPICAL TYPICAL	TOP COPE: W8 THROUGH W10 = 1" MAXIMUM W12 AND DEEPER = 2 1/2" MAXIMUM TYPICAL 2L4X3-1/2X3/8 (4" LEG TO SUPPORTING BEAM) TYPICAL 5-1/2" BOLT GAGE AT
1/2" TYPICAL	SUPPORTING BEAM TYPICAL 3 SIDES 1/4 TYPICAL SUPPORTED BEAM TYPICAL
TOP COPE ONLY (TC)	SUPPORTING BEAM TYPICAL





X35+		TY	PE 2	2L2 [DOU	BLE	AN	GLE	! 	
Search S		SHEAF	R CO	NNE	ECTI	ON	SCH	EDL	JLE:	
Search S		3/4" DIAMETER ASTM F3125-N ROLTS								
BEAM SIZE										
BEAM SIZE			GRA	ADES						
BEAM SIZE										
W8	BE	AM SIZE		_						<u> </u>
X13-24	14/0	1 40			-					
X28+	W8									
W10										
X15-26 2	14/40									
X30+ 2	VV 10									
W12										
Name	11110									
X19-30	w12	X14-16						_		
Name		40.00								
X35+		x19-30								
W14					<u> </u>			l		14
W14		x35+						l		
X34-48		1								20
X53+ 3 66 66 47 66 33 66 26	W14									14
W16								l	_	19
A										26
X36-40	W16	x26-31						 		16
4 123 119 67 91 47 70 33 x45+ 3 92 92 44 92 31 85 24 W18 x35-50 3 92 92 38 92 27 88 20 4 123 123 68 114 48 88 34 5 154 137 107 114 73 88 52 x55+ 3 92 92 50 92 35 92 27 4 123 123 89 123 62 123 48 5 154 154 139 154 98 126 75 W21 x44-63 4 123 123 80 123 56 123 43 5 154 154 139 154 98 126 75 W21 x44-63 4 123 123 80 123 56 123 43 5 154 154			1				72		_	26
x45+ 3 92 92 44 92 31 85 24 W18 x35-50 3 92 92 38 92 27 88 20 4 123 123 68 114 48 88 34 5 154 137 107 114 73 88 52 x55+ 3 92 92 50 92 35 92 27 4 123 123 89 123 62 123 48 5 154 154 139 154 98 126 75 W21 x44-63 4 123 123 80 123 56 123 43 5 154 154 139 154 98 126 75 W21 x44-63 4 123 123 80 123 56 123 43 5 154		x36-40					91	!		20
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W18 x35-50 3 92 92 38 92 27 88 20 4 123 123 68 114 48 88 34 5 154 137 107 114 73 88 52 x55+ 3 92 92 50 92 35 92 27 4 123 123 89 123 62 123 48 5 154 154 139 154 98 126 75 W21 x44-63 4 123 123 80 123 56 123 43 5 154 154 154 154 88 143 64 6 184 184 179 184 126 143 91 x68+ 4 123 123 98 123 69 123 53 5 154 154 154		x45+	3	92	92	44	92	31	85	24
4 123 123 68 114 48 88 34 5 154 137 107 114 73 88 52 x55+ 3 92 92 50 92 35 92 27 4 123 123 89 123 62 123 48 5 154 154 139 154 98 126 75 W21 x44-63 4 123 123 80 123 56 123 43 5 154 154 124 154 88 143 64 6 184 184 179 184 126 143 91 x68+ 4 123 123 98 123 69 123 53 5 154 154 153 154 108 154 83 6 184 184 184 184 184 184 119 W24 - 4 MIN 4 123 123 90 123<				123	123	78	111	55	85	42
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4 123 123 89 123 62 123 48 5 154 154 139 154 98 126 75 W21 x44-63 4 123 123 80 123 56 123 43 5 154 154 124 154 88 143 64 6 184 184 179 184 126 143 91 x68+ 4 123 123 98 123 69 123 53 5 154 154 153 154 108 154 83 6 184 184 184 184 184 155 184 119 W24 - 4 MIN 4 123 123 90 123 63 123 49 W27, W30 - 5 MIN 5 154 154 140 154 99 154 76 W33-W40 - 7 MIN 6 184 184 184 184 184 142 184 108 <t< td=""><td></td><td></td><td>5</td><td>154</td><td>137</td><td>107</td><td>114</td><td>73</td><td>88</td><td>52</td></t<>			5	154	137	107	114	73	88	52
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6 184 184 184 184 155 184 119 W24 - 4 MIN 4 123 123 90 123 63 123 49 W27, W30 - 5 MIN 5 154 154 140 154 99 154 76 W33-W40 - 7 MIN 6 184 184 184 184 142 184 108 7 215 215 215 215 215 215 215 174 8 246 246 246 246 246 246 246 232 9 276 276 276 276 276 276 276 276 276		x68+	4	123	123	98	123	69	123	53
W24 - 4 MIN 4 123 123 90 123 63 123 49 W27, W30 - 5 MIN 5 154 154 140 154 99 154 76 W33-W40 - 7 MIN 6 184 184 184 184 142 184 108 7 215 215 215 215 215 215 174 8 246 246 246 246 246 246 232 9 276 276 276 276 276 276 276 276			5	154	154	153	154	108	154	83
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W33-W40 - 7 MIN 6	W24 -	4 MIN	4	123	123	90	123	63	123	49
W33-W40 - 7 MIN 6	W27. W30 - 5 MIN			l	-			-		76
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1 19 1 001 1 001 1 001 1 001 1 001 1 001			10	307	307	307	307	307	307	307

- 1. SEE TYPICAL ANGLE SHEAR CONNECTION GENERAL NOTES FOR ADDITIONAL INFORMATION
- 2. SELECT CONNECTION CAPACITY BASED ON COPED CONDITION OF BEAM
- 3. TBC VALUES IN TABLE ALSO APPLY TO BOTTOM COPE ONLY CONDITION
- 4. SSLT HOLES IN ANGLE LEG TO SUPPORT

5. MINIMUM COLUMN FLANGE WIDTH SHALL BE 8" (INCLUDES: W10x33+, W12x40+, W14x43+)

TYPICAL DOUBLE ANGLE SHEAR CONNECTION AT BEAM OR COLUMN FLANGE (TYPE 2L2) - BOLTED-WELDED

NOT TO SCALE

BEAM SIZE

W8 - W12

W14 - W18 W16 - W24

W18 - W30

W21 - W40 W27 - W44

W33 - W44

W36 - W44

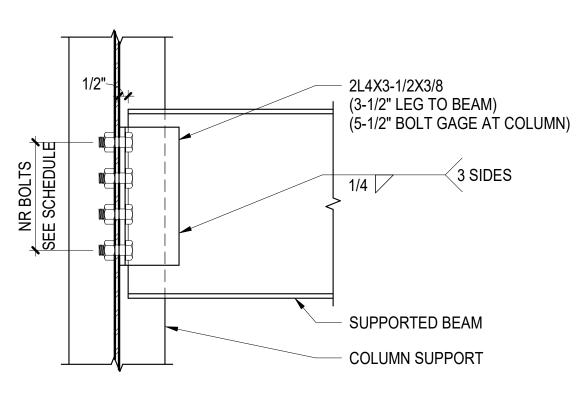
TYPE 2L4 DOUBLE ANGLE

SHEAR CONNECTION SCHEDULE:

3/4" DIAMETER ASTM F3125-N BOLTS

GRADES A325 OR F1852

CONNECTION CAPACITY (KIPS)



NOTES:

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- 2. SSLT HOLES IN ANGLE LEG TO COLUMN SUPPORT
- 3. PROVIDE ERECTION SEAT PER FABRICATOR WHEN SAME DEPTH CONNECTION IS REQUIRED ON BOTH SIDES AND BOLT STAGGERING IS NOT POSSIBLE
- 4. MINIMUM COLUMN WEB THICKNESS SHALL BE 0.29" (INCLUDES: W8x31+, W10x33+, W12x40+, W14x43+)
- TYPICAL DOUBLE ANGLE SHEAR CONNECTION AT COLUMN WEB (TYPE 2L4) BOLTED-WELDED
 NOT TO SCALE

REAL ESTATE & FACILITIES MANAGEMENT TVDE 21.2 DOLIDI E ANCI E 777 Old Saw Mill River Road Tarrytown, NY 10591-6707 T: 914.847.7400 F: 914.847.7991 www.regeneron.com Building #17

777 Old Saw Mill River Road Mount Pleasant, NY 10591

Day-care Center

Campus Expansion Child

REGENERON

Gensler 1700 Broadway, Suite 400 New York, NY 10019 (212) 492-1400 Phone (212) 492-1472 Fax

Project No. B17-DAYCARE

Structural Engineer

Thornton Tomasetti 120 Broadway, 15th Floor New York, NY 10271 (917) 661-7800 Phone (917) 661-7801 Fax

MEP / IT / Security Engineer

Cosentini Associates 498 Seventh Avenue New York, NY 10018 (212) 615-3600 Phone (212) 615-3700 Fax

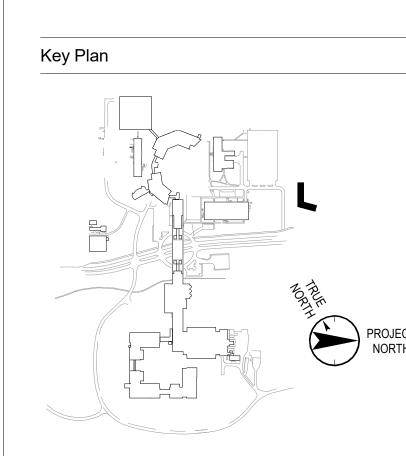
JMC 120 Bedford Road

Civil Engineer

Armonk, NY 10504 (914) 273-5225 Phone (914) 273-2102 Fax

Landscape Architect

Langan 21 Penn Plaza, 360 West 31st Street, 8th Floor New York, NY 10001 (212) 479-5400 Phone (212) 479-5444 Fax



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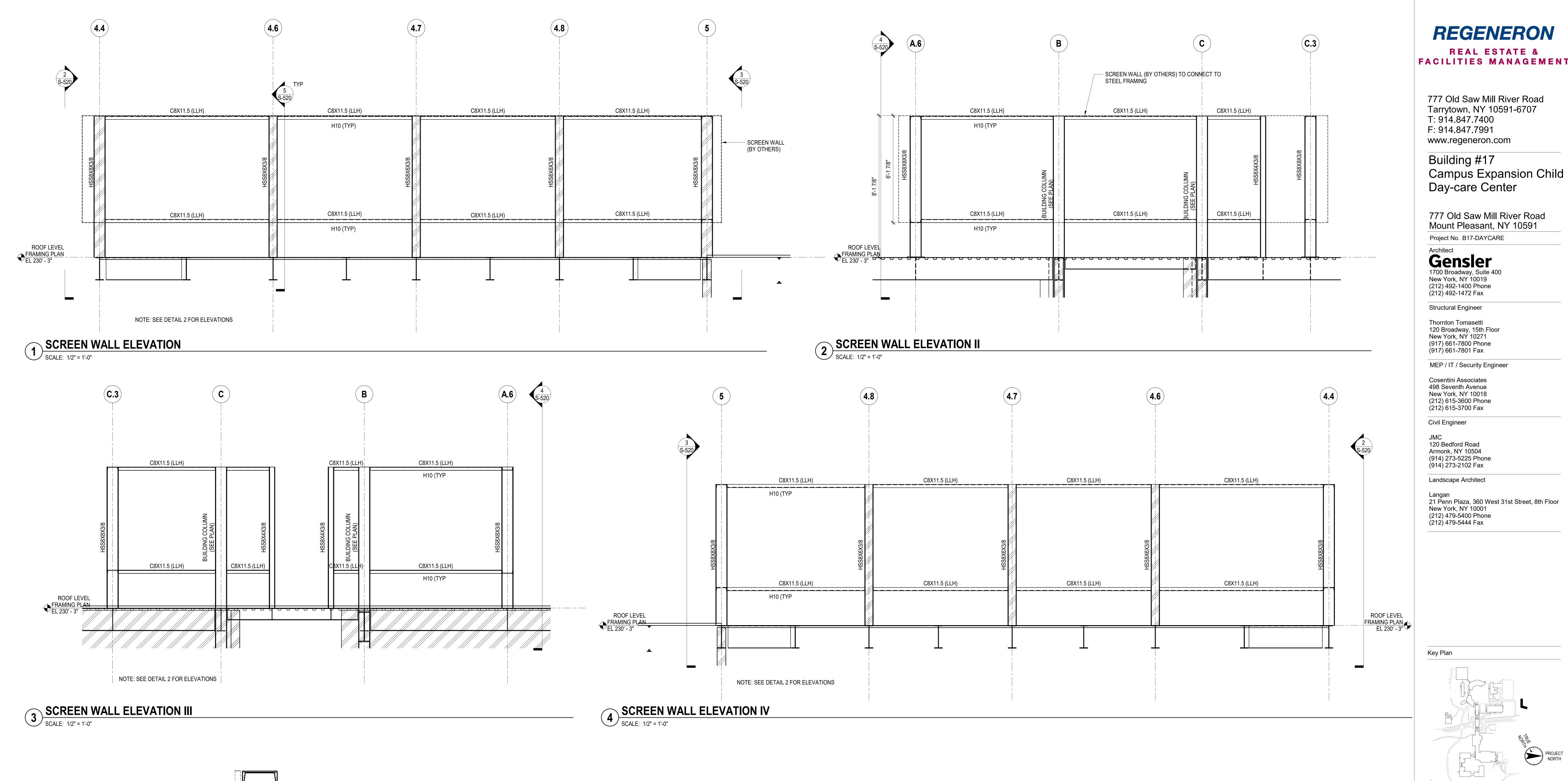
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Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 TYPICAL STEEL BEAM SHEAR CONNECTIONS 3/4" DIAMETER BOLTS

Scale: NOT TO SCALE Floor:



HSS (SEE ELEVATIONS) SCREEN WALL (BY OTHERS) C-CHANNEL (SEE ELEVATION) ____ 3/8" BASE PLATE FRAMING, WHERE SHOWN ON PLAN — 1/2" STIFF. NS/FS **UNDER SCREEN** - ROOF FRAMING, WALL POST SEE PLAN REF. TO DETAIL 8/S-502 FOR ADD'L DETAILS. BASE CONNECTION OF POST ABOVE REQUIRED TO V5, M10, P5

SECTION THRU. SCREEN WALL

SCALE: 1 1/2" = 1'-0"

REGENERON

REAL ESTATE & FACILITIES MANAGEMENT

777 Old Saw Mill River Road Tarrytown, NY 10591-6707 T: 914.847.7400 F: 914.847.7991

www.regeneron.com Building #17

777 Old Saw Mill River Road Mount Pleasant, NY 10591

Architect

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New York, NY 10019
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(212) 492-1472 Fax

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MEP / IT / Security Engineer

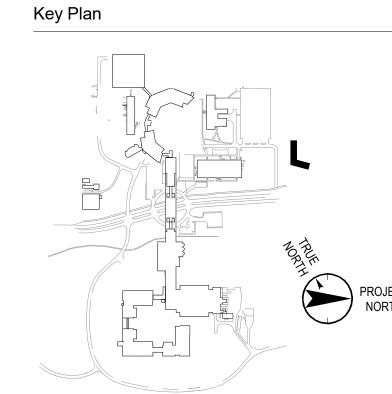
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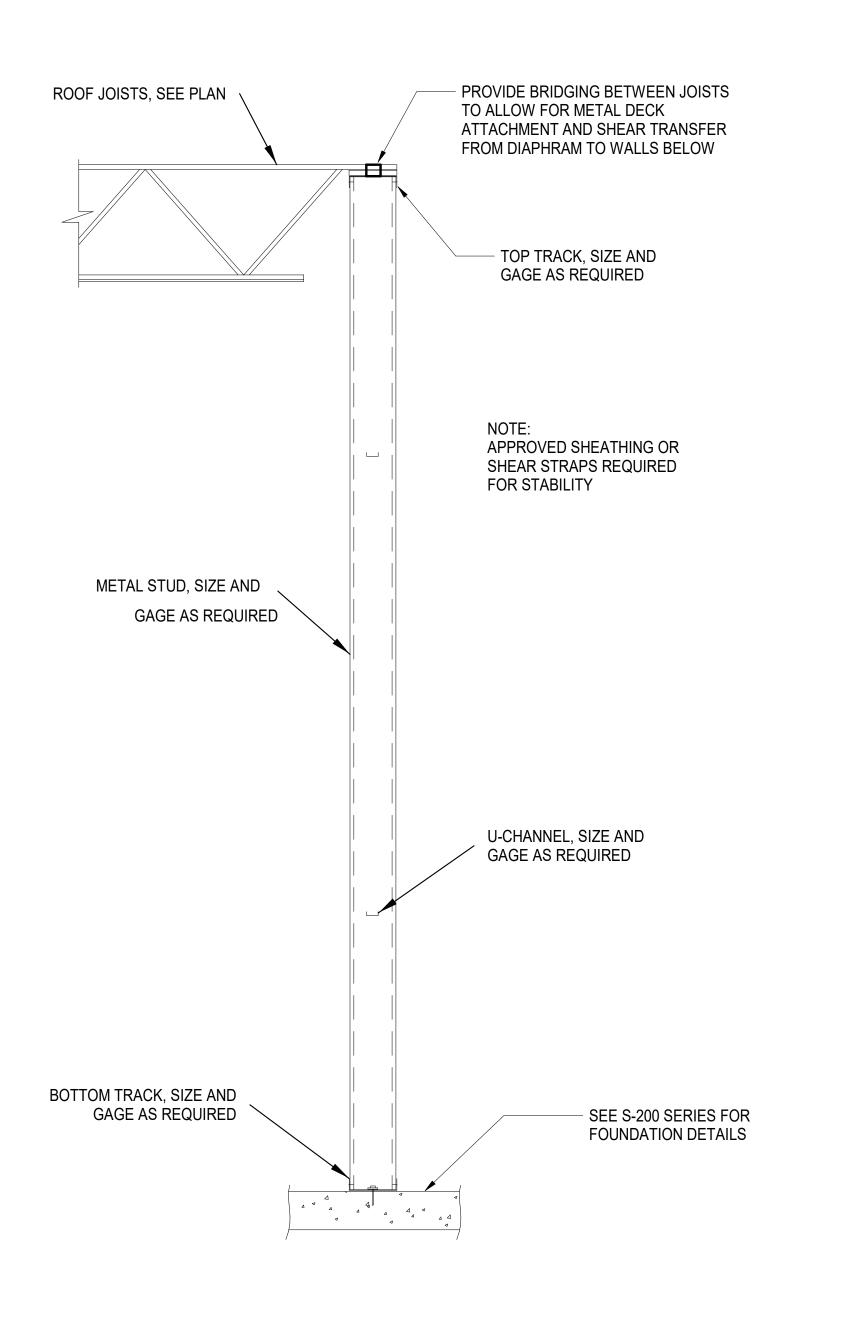
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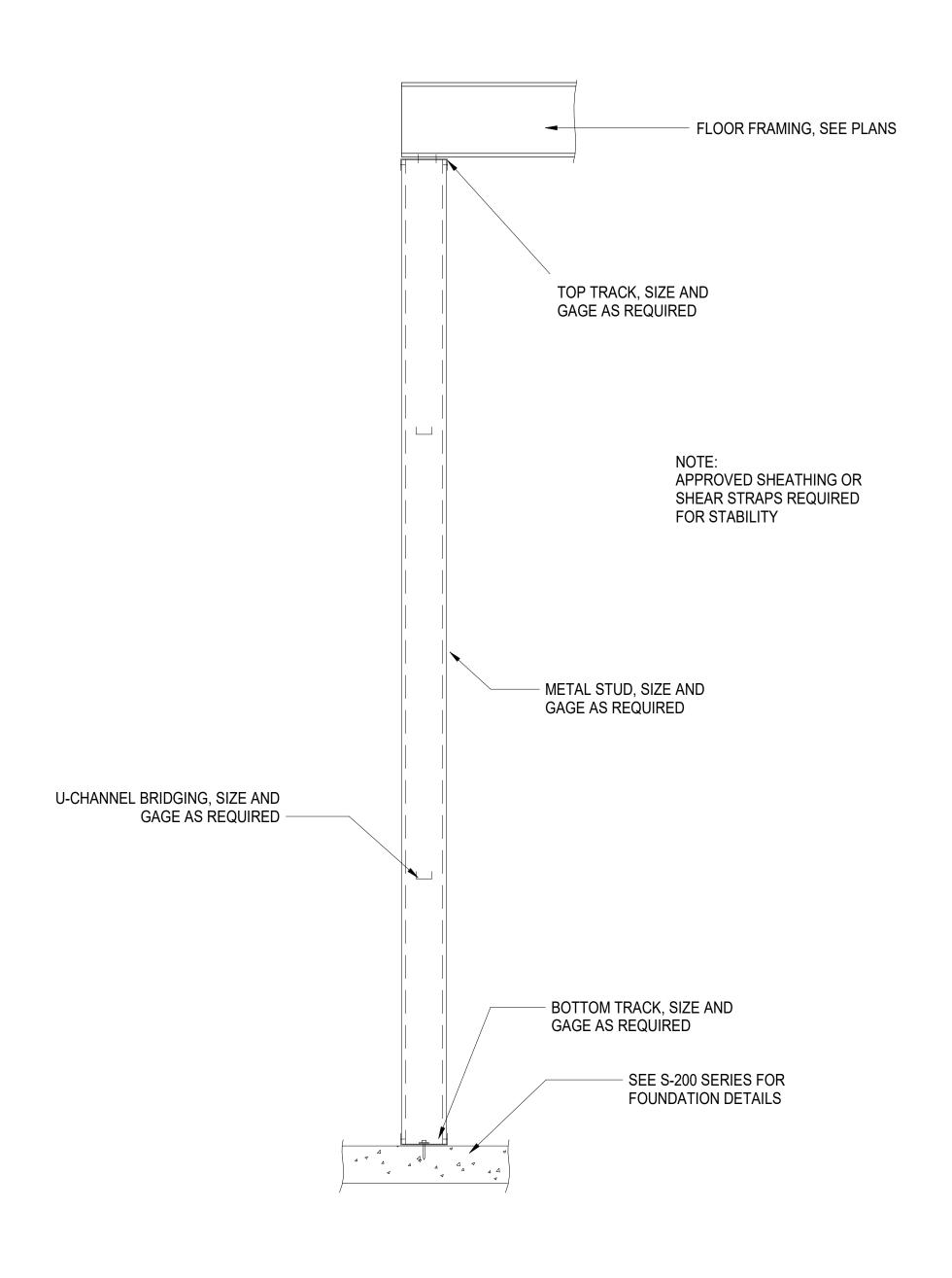
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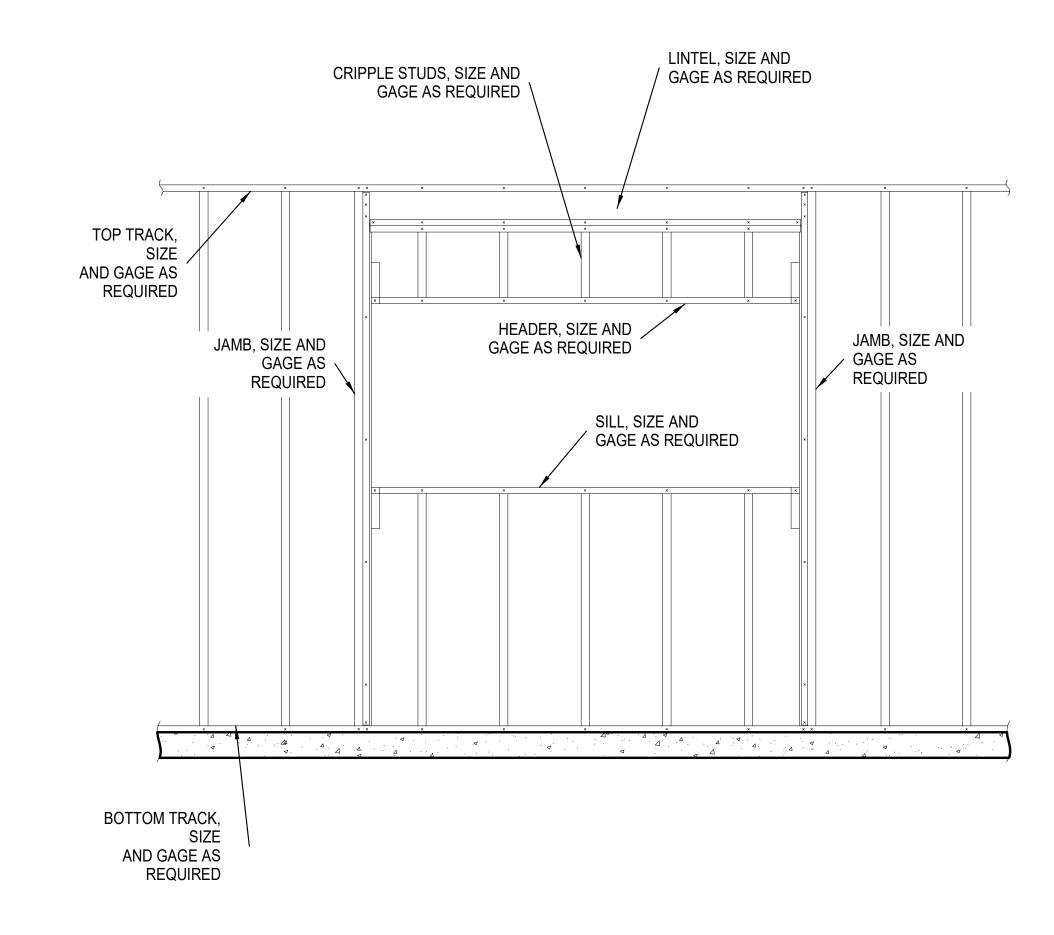
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Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 Discipline: Drawn By: Author
SCREEN WALL SECTIONS AND DETAILS







1 LOAD BEARING WALL WITH JOIST SCALE: 1 1/2" = 1'-0"

JAMB, SIZE AND GAGE AS REQUIRED

FASTEN TO LINTEL W/ SCREWS

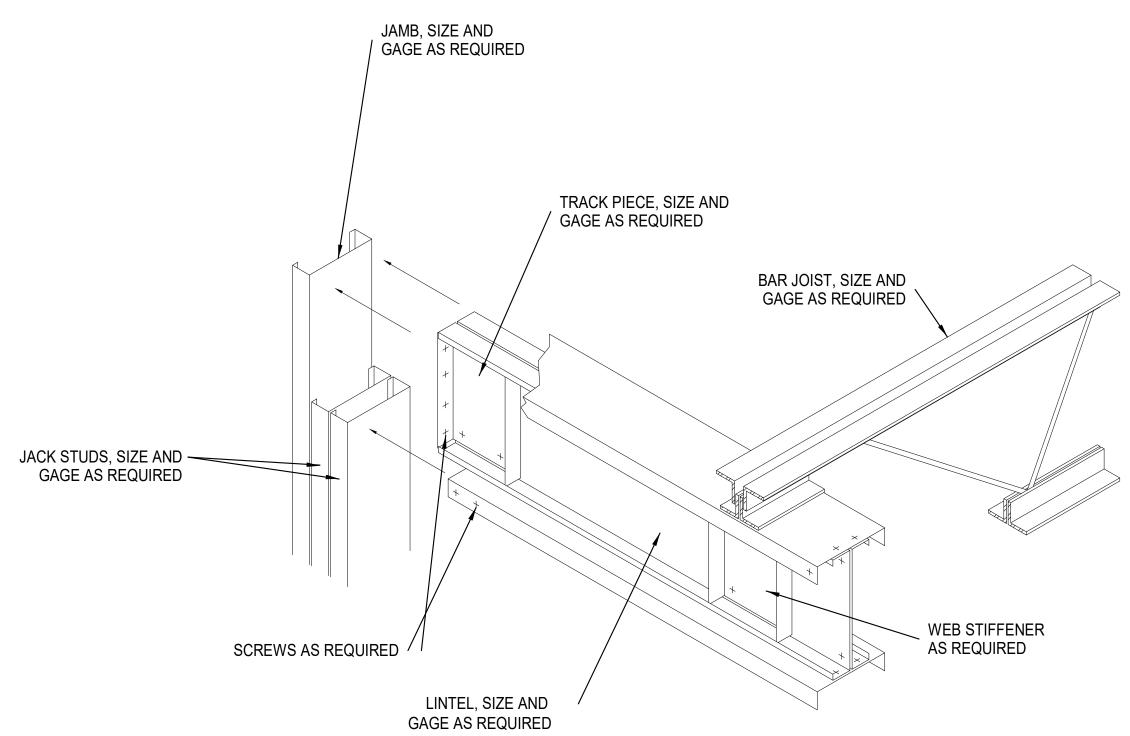
LINTEL, SIZE AND GAGE AS REQUIRED

TO EACH SIDE & SCREWS TO

JAMB AS REQUIRED

2 LOAD BEARING WALL WITH W-SHAPE

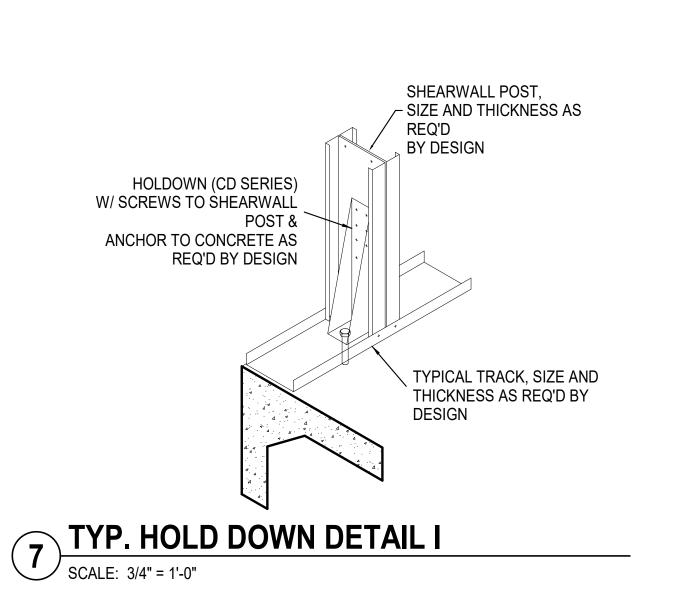
SCALE: 1 1/2" = 1'-0"

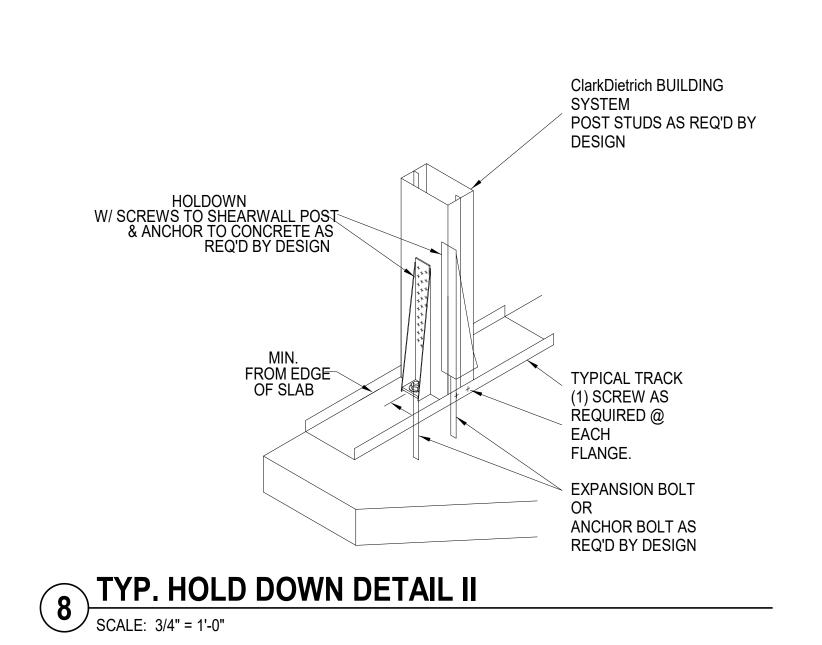


TYPICAL LOAD BEARING BOX HEADER LINTEL

SCALE: 1" = 1'-0"

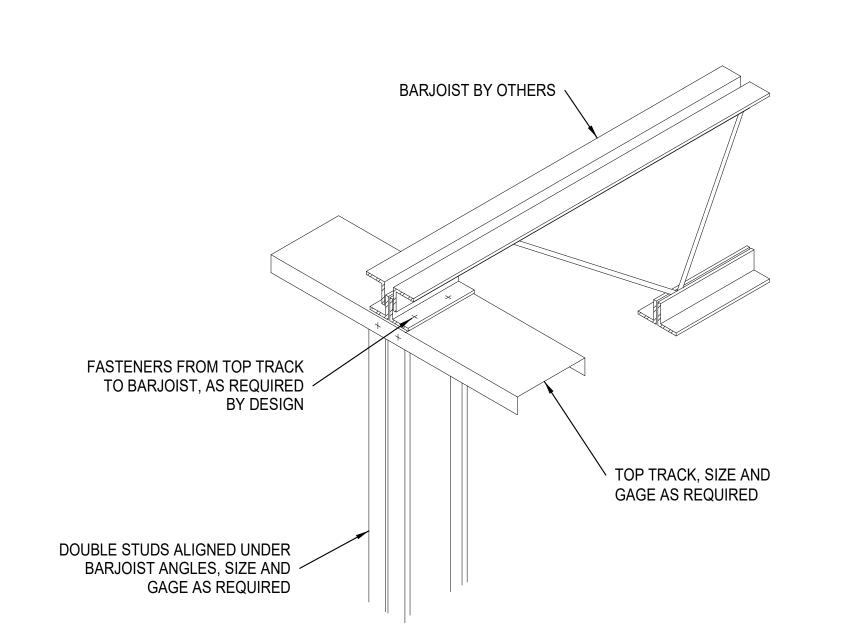
5 TYPICAL LOAD BEARING LINTEL AT BAR JOIST SCALE: 1" = 1'-0"





TYPICAL LOAD BEARING WINDOW OPENING

3 ELEVATION
SCALE: 1" = 1'-0"



6 LOAD BEARING POST AT BAR JOIST
SCALE: 1" = 1'-0"

REGENERON

REAL ESTATE & FACILITIES MANAGEMENT

777 Old Saw Mill River Road Tarrytown, NY 10591-6707 T: 914.847.7400 F: 914.847.7991 www.regeneron.com

Building #17 Campus Expansion Child Day-care Center

777 Old Saw Mill River Road Mount Pleasant, NY 10591 Project No. B17-DAYCARE

Gensler
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(212) 492-1472 Fax Structural Engineer

Thornton Tomasetti 120 Broadway, 15th Floor New York, NY 10271 (917) 661-7800 Phone (917) 661-7801 Fax

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Landscape Architect

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Key Plan

No. Date Description 05.20.2022 ISSUED FOR PERMIT 06.20.2022 100% CONSTRUCTION DOCUMENTS 07.01.2022 100% CONSTRUCTION DOCUMENT- 1

Plot Date: 03/08/22

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Vendor Name: GENSLER Vendor Project No.: 006.3608.000 LGMF BEARING WALL **DETAILS**

Scale: As indicated Floor:

TYPICAL ANCHOR ADD ANCHOR AT JAMB STUDS JAMB STUDS BOTTOM TRACK - END OF TRACK NOTES: 1. "A" DIMENSION SHALL BE 4" MINIMUM AND 12" MAXIMUM.

2. USE A MINIMUM OF 3 PAF IN EACH PIECE OF SILL TRACK.

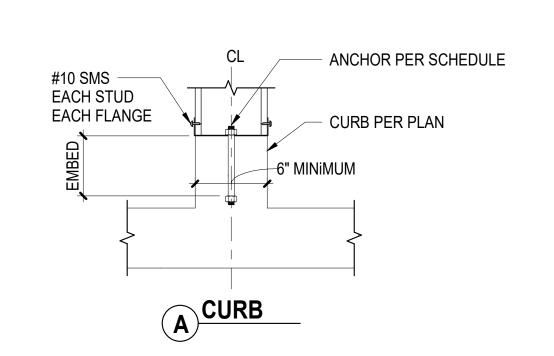
2 TYPICAL INTERIOR BOTTOM TRACK
SCALE: 1" = 1'-0"

TYPICAL INTERIOR HEADER AND JAMB CONNECTION

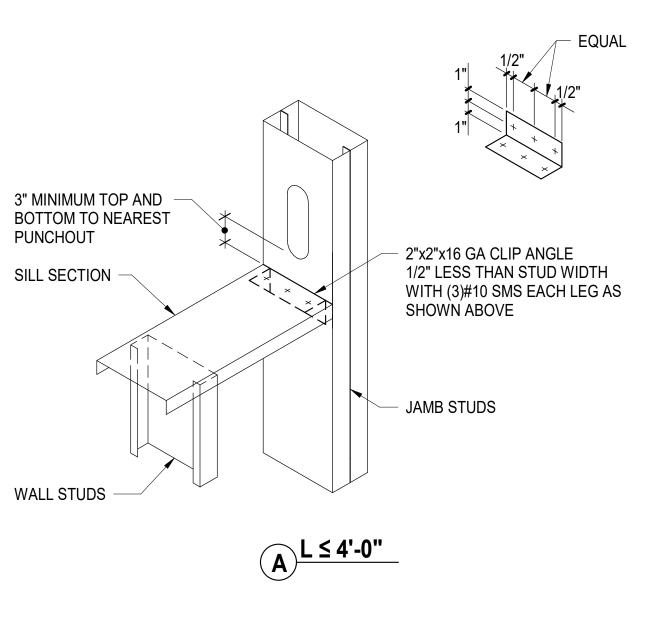
SCALE: 1" = 1'-0"

INTERIOR BOTTOM TRACK SILL ANCHOR SCHEDULE - W/ CURB ANCHOR TYPE SIZE AND SPACING (MINIMUM, OR AS REQUIRED.) 3/8"Ø @ 2'-8" OC W/ 5" EMBED ANCHOR BOLT 3/8"Ø HILTI KB-TZ @ 2'-8" OC **EXPANSION ANCHOR*** 3/8"Ø @ 2'-8" OC **SCREW ANCHOR***

*EMBED PER GENERAL NOTES

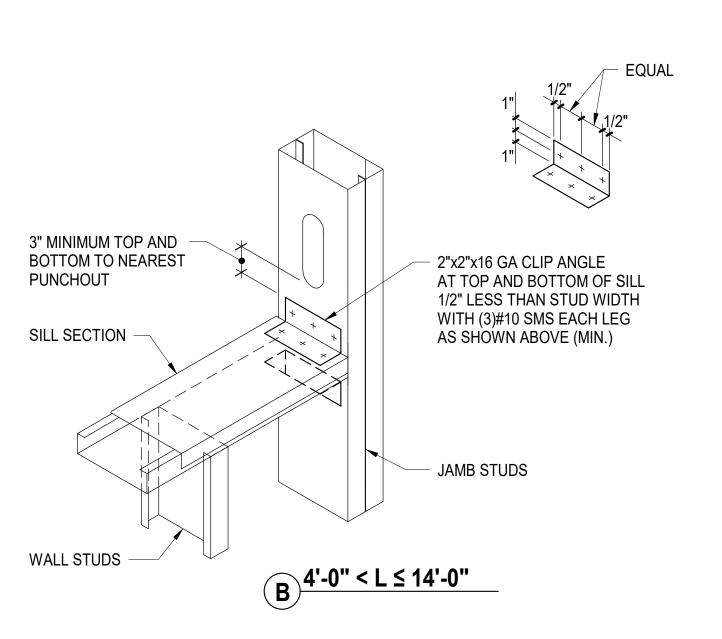


NOTES: DETAIL FOR INFORMATION ONLY. FINAL DESIGN OF LGMF SYSTEM IS DELEGATED TO THE CONTRACTOR BASED ON CONTRACT DOCUMENTS

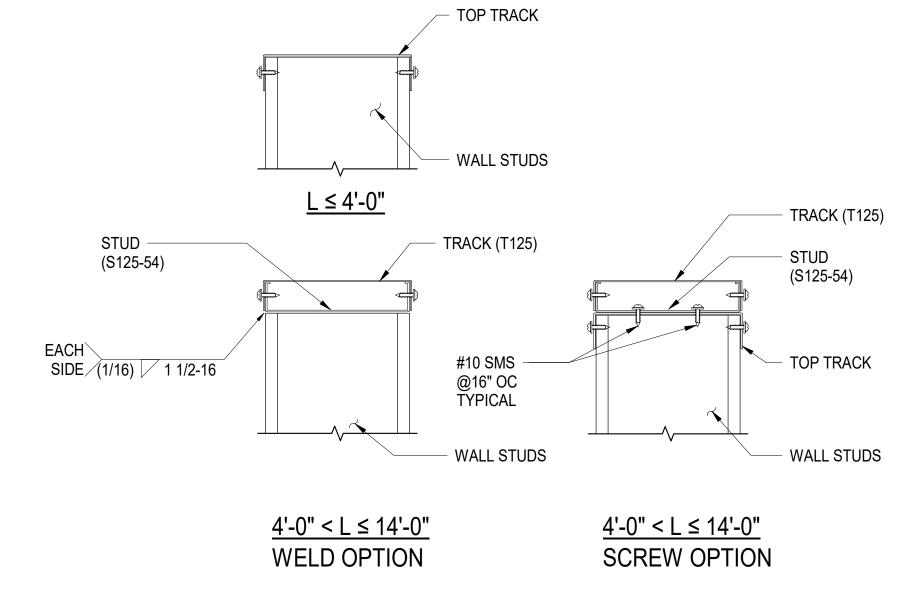


NOTES: 1. WHEN L > 14'-0", SEE -/--- FOR WALLS ON SLAB-ON-GRADE

MAIN -TRACK



2. DETAIL FOR INFORMATION ONLY. FINAL DESIGN OF LGMF SYSTEM IS DELEGATED TO THE CONTRACTOR BASED ON CONTRACT DOCUMENTS



C SILL DETAILS

BOTTOM TRACK SECTION

1 1/2" = 1'-0"



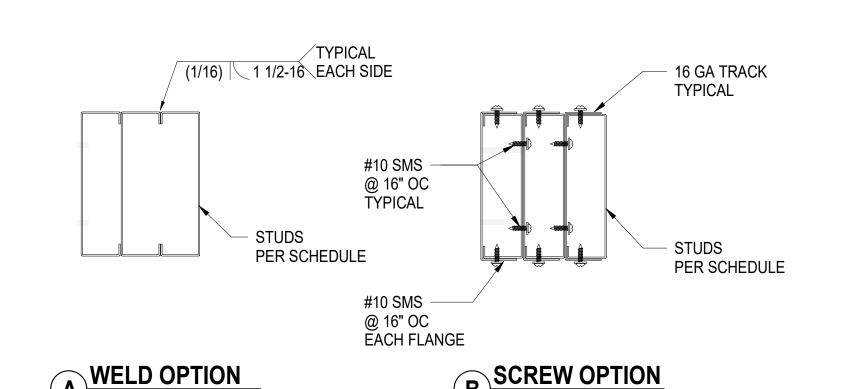
R TRACK SPLICE

12" LONG SPLICE TRACK (MIN)

TO MATCH FLANGE WIDTH

AND GAGE OF MAIN TRACK

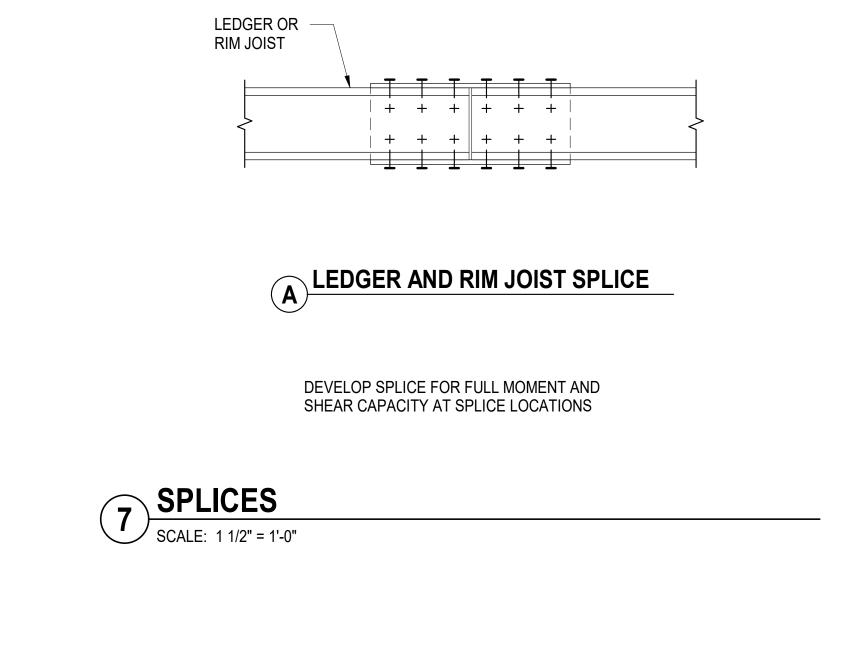
5 INTERIOR SILLS SECTION
3" = 1'-0"



JAMB STUD SCHEDUI	LE
MAX ALLOWABLE OPENING WIDTH L OR COMBINED OPENING WIDTH L1 + L2 AT ADJACENT OPENING	STUDS
11'-0"	2-16GA STUDS
18'-0"	3-16GA STUDS

1. JAMB STUDS DEPTHS SHALL MATCH TYPICAL WALL STUD DEPTHS. 2. WHEN OPENING WIDTH IS GREATER THAN 14'-0" AND STORY "HT" IS GREATER THAN 14'-0", PROVIDE TOP TRACK REINF AT JAMB STUDS PER -/---

6 TYPICAL INTERIOR JAMBS
SCALE: 3" = 1'-0"



Key Plan

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Day-care Center

Project No. B17-DAYCARE

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Civil Engineer

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Building #17

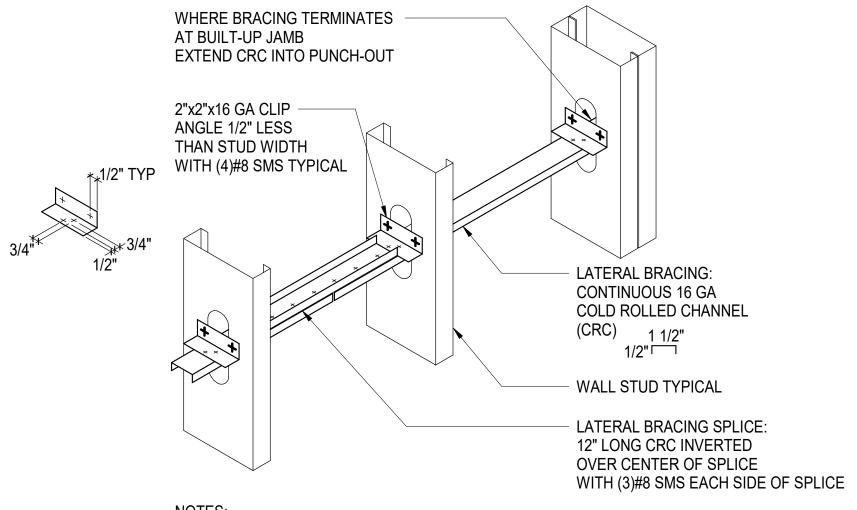
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Vendor Name: GENSLER Vendor Project No.: 006.3608.000 Drawn By: Author LGMF BEARING WALL **DETAILS II**

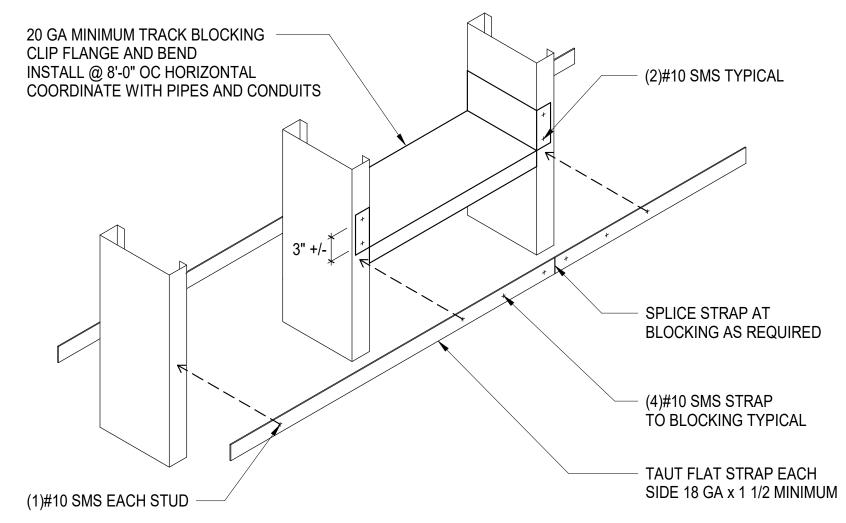


1. DETAIL 2/S-602 MAY BE USED AS AN ALTERNATE.

- 2. IF STUD DEPTH IS GREATER THAN 6", USE DETAIL 2/S-602
- 3. AT CONTRACTOR'S OPTION, IN LIEU OF 16GA CRC, USE EITHER OF (WITH SCREWS PER MANUFACTURER'S REQUIREMENTS): A. BRIDGECLIP BY THE STEEL NETWORK. B. SUBH/MSUBH BRIDGING CONNECTOR BY SIMPSON STRONG-TIE.
- 4. FINAL BRIDGING DETAILS AND CONNECTIONS TO BE ENGINEERED BY THE CONTRACTOR USING THE DESIGN CRITERIA PROVIDED IN THE CONTRACT DOCUMENTS

1 TYPICAL BRIDGING

SCALE: 1" = 1'-0"



NOTE:

- FLAT STRAP BRIDGING MAY BE OMITTED ON EITHER SIDE OF STUD WHERE GYPSUM OR LATH OR PLYWOOD IS PROVIDED AND ATTACHED WITH FASTENERS AT 12" OC MAXIMUM ALONG THE FULL HEIGHT OF EACH WALL STUD
- 2. 4. FINAL BRIDGING DETAILS AND CONNECTIONS TO BE ENGINEERED BY THE CONTRACTOR USING THE DESIGN CRITERIA PROVIDED IN THE CONTRACT DOCUMENTS

2 ALTERNATE BRIDGING
SCALE: 1" = 1'-0"

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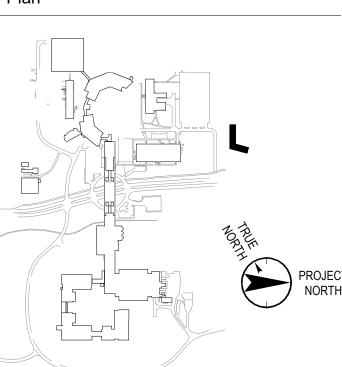
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Professional Seal and Signature

Vendor Name: GENSLER Vendor Project No.: 006.3608.000 Discipline: Drawn E

LGMF BEARING WALL **DETAILS III**

Scale: 1" = 1'-0" Floor: