

January 30, 2018

City of Yonkers Fire Department 470 Nepperhan Avenue Yonkers, NY 10701 914-377-7582

Attn: Kevin Ford

Re: Station 14 Geotechnical 2187 Central Park Avenue Yonkers, NY 10710 McLaren File No. 160616.04

Dear Chief Ford:

At the request of City of Yonkers, McLaren Engineering Group (McLaren) developed and implemented a geotechnical investigation at Fire Station 14. The project site is located at 2187 Central Park Avenue, Yonkers, New York (See Appendix A, Site Location Plan). This geotechnical investigation consisted of soil borings, a test pit, and identification and classification of soils.

The scope of this investigation was to determine the cause of the floor slab cracking in the truck bays and cracking of the interior wall of the station.

Executive Summary

A visual inspection conducted by a McLaren representative revealed concrete cracking along the left side wall and the floor slab of structure. At the entrance of the garage 3.5 inches of settlement was observed. Numerous other cracks are also present at the floor slab. A major crack is present at the interior side of the garage wall which runs along the wall in the vertical direction. Pictures and measurements were taken and are available upon request.

Three borings were performed on the site, as indicated on the Boring & Test Pit Location Plan (See Appendix B). All three borings were extended to refusal depth at top of rock, which varied from 7.5 feet to 13.5 feet below surface. (See Appendix C, Boring Logs). The soil profile below the 5.5-inch-thick concrete slab floor, consists of 8-inches fine gravel, followed by approximately seven (7) feet of dark to light gray fine silty sand, with some clay and organics. The soil layer changes to red-brown fine to medium sand, some coarse sand and gravel until the maximum depth of exploration. This layer was saturated as the ground water table was encountered at 7.5 feet to 8 feet.

A test pit was performed outside of the fire station adjacent to the garage wall. Asphalt pavement covered the surface with a thickness of 10 inches. Below the surface, the soil consists of light brown

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e-mail: mgmclaren@mgmclaren.com On the web: www.mgmclaren.com fine to medium sand. The uniform soil layer extends to maximum depth of observation of approximately 6 feet below surface. At a depth of three feet, a 6-inch diameter clay pipe believed to be a footing drain, was encountered adjacent to wall. The test pit was extended to a depth of 6 feet. The bottom of the foundation was not visible and a probe into the soil revealed that the foundation wall appears to be extended to top of rock.

To remediate the settlement that has taken place, McLaren recommends the following three options. The first option would be to remove and replace the fill soils with a structural backfill soil. The structural backfill soil should be compacted and placed in lifts of uniform thickness. The second option would be to use compaction grouting. Compaction grouting is a grouting technique that displaces and densifies below grade soils, reinforces fine grained soils and fills voids related to sinkholes or karst conditions. A third option would be to install mini piles throughout the site and replace the floor with a structural slab.

Project Scope

The City of Yonkers Fire Department has proposed the renovation of the garage of Fire Station 14 located at 2187 Central Park Avenue. The site is an active fire house, and serves as the work station of the fire fighters and where the fire trucks are stationed.

Field Investigation

Three (3) borings and one (1) test pit were performed by Soiltesting Inc. (90 Donovan Road, Oxford CT 06478) at the site. The borings were performed inside the building, in the front area of the garage. The test pit was performed outside of the structure but adjacent to the garage front wall. (See Appendix B, Boring and Test Pit Location Plan). Borings were performed with a skid-mounted rig using hollow-stem auger casing. The test pit was hand dug to full depth with shovels and cutter mattock.

Soil samples were obtained using the Standard Penetration Test (ASTM D-1586-84) using a split spoon sampler with a 2-inch O.D., and a 1³/₈₋inch I.D., that was driven into the soil with a 140-pound hammer falling freely from a height of 30-inches (See Appendix C, Boring Logs). Soil samples were field classified using the Unified Soil Classification System (USCS).

Boring B-1 was drilled to a depth of 10.5 feet. The top 7 feet of its soil profile consists of fill material. The fill is primarily dark to gray loose silty sand, with trace of organics. This layer is classified as ML. The next 3.5 feet consists of fine to medium brown sand. Weathered bedrock and rock fragments were found at the tip of the split spoon sampler. This layer is classified as SM. Bedrock was encountered at 10.5 feet. The water table was encountered at a depth of 7.5 feet to 8 feet.

Boring B-2, was the deepest boring. Top of bedrock was encountered at approximately 13.5 feet. At this location the fill also extends to 7 feet below the surface. Below the fill, a 3 feet thick layer of brown fine to medium sand was encountered. This layer transitions into a course to medium sand with some gravel and extends to top of bedrock. The ground water table was encountered at 7.5 feet.

At Boring B-3, the fill extends to approximately 2.5 feet below the surface. It consists of gray to light brown fine sand with some silt. Below the fill, light brown to red brown fine to medium sand was encountered. During the drilling operation at this location, the machine started grinding for



approximately 3.5 feet. Rock fragments were present at the tip of the sample spoon obtained in the 2-4-foot sample. The boring was terminated after the sampling refusal.

A test pit was performed adjacent to the exterior garage wall, five feet from the front of the garage. The test pit was performed in order to visually inspect the integrity of the foundation in the area of the crack in the tile façade along the interior garage wall. (See Appendix B, Boring and Test Pit Location Plan). Test Pit 1 (TP-1) was approximately 5 feet long by 4.5 feet wide by 6 feet deep. At the center line of the test pit on the inside wall, there was a vertical crack present. This crack was not visible on the outside wall.

Visual inspection of the property was performed by a McLaren representative. Multiple cracks were present on floor and on the left side wall. There was also an indication of possible differential settlement present at the site. The floor slab settled approximately 3.5 inches at the front of the garage. The settlement problem can most likely be attributed to the weight of the fire trucks and the unsuitable fill soil that is present at the site. It is to our knowledge that the wall with the large crack was hit by a fire truck, which could have caused substantial damaged to the structure. The crack in the wall can possibly be attributed to the settlement that has been occurring on the site or from the impact by the fire truck.

RECOMMENDATIONS

Based on the geotechnical investigation and observation that occurred during the field work, the following recommendations are forwarded for your use:

Remediation of Settlement

Based on the soil encountered at all three borings, the differential settlement is being caused by poorly compacted soil and possible decay of the organic soil used as fill. The soil is not suitable to support the loading exerted by fire trucks on the loose soil.

The first remediation option is to replace the existing 7 feet of fill soil, with suitable backfill soil. The backfill material should be NYS DOT Item 304 or similar compacted to a density of 95%. This will achieve an allowable bearing pressure of 6 kips per square foot. Backfill material shall be of uniform soil characteristic and moisture content and shall be placed in lifts of less than 12 inches. This operation must be done under the supervision of a geotechnical engineer or a qualified inspector with third party testing. A subgrade modulus of 250 kcf should be used for design of the any exterior slabs and sidewalks.

The second remediation option is compaction grouting for remediation of the soils below surface of the existing Fire House Station. Compaction grouting is a grouting technique that displaces and densifies below grade soils, reinforces fine grained soils and fills voids related to sinkholes or karst conditions. For the proposed soil remediation, the injection pipe should be lowered to a depth of 8 feet, and then injected in lifts as the pipe is slowly extracted. By implementing this method in a grid pattern across the proposed area, it will create columns of grout bulbs which will reinforce the surrounding soil. The grid pattern of grout columns should be sequenced in primary and secondary locations for maximum effectiveness.



Final design of the grout mix and sequencing shall be signed and sealed by an Engineer licensed in the state of New York. Soil in areas designated as proposed planters or trees should not be grouted Compaction grouting locations should be designed with a maximum spacing of 5 feet.

The third remediation option is installation of a structural slab with mini-piles. These mini-piles shall be spaced uniformly across the proposed area and shall be driven to bedrock. The existing slab shall be removed and replaced with a new structural slab supported by the mini-piles. These mini-piles will help to support the structural slab and prevent any settlement issues in the future. An 8 inch mini pile with steel reinforcement should be designed to support a load of 20 tons. Piles over 40 tons will have to be tested as per NYS Building Code.

Settlement of Foundation Wall

It is believed that the foundation wall is bearing on bedrock for the structure. Based on the observation of the foundation wall by the test pit, it does not appear the crack on the interior wall has originated from settlement of the foundation. Verification of this must be performed in the field when the existing floor slab is removed since the foundation wall was observed from the exterior and the crack is present on the interior. Viewing the foundation wall from the interior will give a clear indication of whether settlement of the foundation has occurred. Based on the current observation that McLaren has done, it seems the large crack on the interior wall was caused by the impact of a fire truck into the wall.

Site Preparation

The Contractor shall be responsible at all times for conducting all earthwork operations in a safe and prudent manner such that all workmen and the general public will be protected from hazards. The Contractor shall observe all applicable local, State and/or Federal requirements. The Contractor should protect any utilities that are present under the new slab.

Dewatering

Groundwater was encountered at a depth of 7.5 feet to 8 feet below surface elevation in both borings B-1 and B-2. At location B-3, ground water was not observed. This water elevation may fluctuate seasonally or with rain events. Dewatering may be required if excavations are greater than 7 feet.



This report is respectfully submitted in accordance with our contract, and is to the best of our knowledge accurate and complete. Any questions regarding its content may be directed to the undersigned.

Respectfully submitted by,

The Office of **McLaren Engineering Group M. G. McLAREN**, **P.C.**

Luke A Daur, P.E. Sr. Engineer

cc: SLG - Internal Project #160616.04

Appendix A – Site Location Plan Appendix B – Boring & Test Pit Location Plan Appendix C – Boring Logs Appendix D – Test Pit Logs Appendix E – Soil Profile

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Appendix A Site Location Plan



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ITS			A	LMF	017	.T.S	.04		CITY OF YONKERS,	NEW YORK		100 Snake Hill Road, West Nyack, NY 10994 Tel. (845) 353-6400 Fax. (845) 353-6509 www.mgmclaren.com

Appendix B Boring Location Plan



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		BORING AND TEST PIT	EIRE STATION 14					
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Appendix C Boring Logs



MC -	La	ren	Hist	torical Borir	ng Dat	a (195	9) BORING NUMBER B PAGE 1 OF	3-2 F 1
CLIE	ΝТ	City c	of Yon	kers Fire D	epartr	nent	PROJECT NAME Fire Station 14	
PRO.	JEC		MBER	160616.04	4		PROJECT LOCATION Yonkers, NY	
DATE	E ST	ARTE	D 12	2/14/17		CON	PLETED 12/14/17 GROUND ELEVATION 65 ft NAVD 88 HOLE SIZE 4 inches	
DRIL	LIN	G CO	NTRA	CTOR Soil	testing	g Inc.	GROUND WATER LEVELS:	
DRIL	LIN	G ME	гнор	Hollow Ste	em Au	iger 2'		
LOG	GED	ЭВΥ_	L. Mai	rtinez Frias		CHE	CKED BY L. Daur AT END OF DRILLING	
NOTE	ES _						AFTER DRILLING	
o DEPTH (ft)		SAMPLE IYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
						4 4 A	0.5 5.5 inch thick concrete slab floor	64.5
L .					GW		(GW) 6-8 inch of gravel below the floor slab, no sampling. Removed by hand	64.0
		SS 1	58	6-7-3-3 (10)	ML		(ML) 8 Inch Brown F-M sand. 6 Inch gray slity clay, loose.	62.0
5	\mathbb{N}	SS 2	75	2-3-3-5 (6)	ML		(ML) Gray fine silty sand, with trace of clay & organics.	60.0
		SS 3	75	6-14-16-10 (30)	ML		(ML) 12 inch gray fine silty sand, 6 inch brown fine sand with some silt	58.0
		SS 4	67	9-11-13-15 (24)	SM		$\overline{\nabla}$ (SM) Brown fine sand with silt	56.0
		SS 5	100	11-10-9-9 (19)	SM		(SM) Dark brown to brown coarse to medium sand, trace of silt, loose, saturated.	54.0
		SS 6	92	14-24-22- 22 (46)	SM		(SM) Brown fine-medium-coarse sand trace of gravel. Weathered rock & rock fragment at tip	52.0
	+						13.5 Auger refusal	52.0 51.5
	_						Refusal at 13.5 feet. Bottom of borehole at 13.5 feet.	

MCI -	Laren	Hist	orical Borir	ng Dat	a (195	9)	BORING NUMBER E PAGE 1 OF	}-3 F 1
CLIER	NT City c	of Yon	kers Fire D	epartr	nent		PROJECT NAME Fire Station 14	
PROJ		MBER	160616.04	4			PROJECT LOCATION Yonkers, NY	
DATE	STARTE	ED_12	/14/17		COM	PLETED 12/14/17	GROUND ELEVATION 65 ft NAVD 88 HOLE SIZE 4 inches	
DRILI		NTRA	CTOR Soil	testing	g Inc.		GROUND WATER LEVELS:	
DRILI	LING ME	THOD	Hollow St	em Au	iger 2"		AT TIME OF DRILLING	
LOGO	GED BY	L. Mai	rtinez Frias		CHE	CKED BY L. Daur	AT END OF DRILLING	
NOTE	S						AFTER DRILLING	
DEPTH (ft)	SAMPLE TYPE NUMBER	RECOVERY %	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	
Ŭ					P 4 4 4 4 7	0.5 5.5 inch thick concret	e slab floor	64.5
L -				GW		1.0 (GW) 6-8 inch gravel	below the slab, removed by hand to sample at the soil surface	DF 1
	SS 1	58	13-7-7-9 (14)	ML		(ML) Gray to light bro	wn fine sand, with some silt.	
	SS 2	75	29-100	ML		3.0 (ML) Light brown to re	ed brown fine sand. Trace of silt, rock fragments at the tip of the spoon	62.0
5	$\left(\right)$				\bigotimes	5.0 Rock fragments		60.0
	SS 3	8	8-100			7.0		58.0
	<u>r 1</u>	<u> </u>	1	1		1.0	Refusal at 7.0 feet. Bottom of borehole at 7.0 feet.	- 30.0

Appendix D Test Pit Log

K. G. McLAREN, P.C. 100 Snake Hill Road, West Nyack, NY 10994 (845) 353-6400 FAX (845) 353-6509 Excavator Foreman Inspector Date Start 12-13-17 Time Start 0.3. Elevation	Test Pit Number // Project Name ////////////////////////////////////	7 / 0 6/6.04 of
Soil Horizon/Profile: Depth Description and	d Classification	Stratum
Disht Brown Blow W/Same Silt, 3 W/Same Silt, 3 W/Same Dilt, 3	h Fine Scall 14" Stane Clay Pipe	
Site Sketch:	n Fine Sund.	
Xu in the second		
Y - Hand Dud To 6'- - Metal Bar was Zns Notes	No Foundation, ened those The so	Fouring reveul. iii, Assume Bed Rock

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Appendix E Soil Profile



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