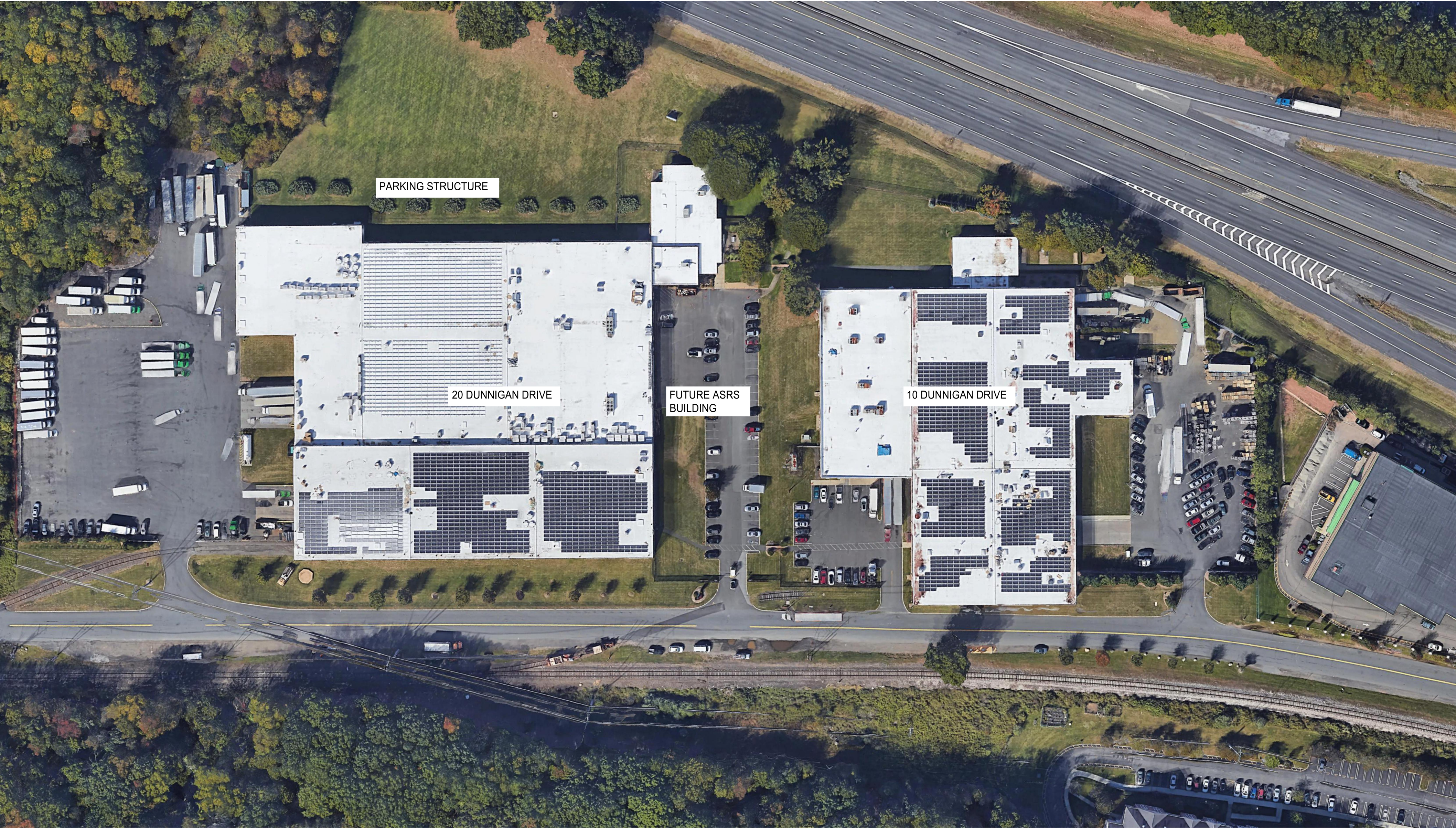


MANHATTAN BEER DISTRIBUTORS

20 DUNNIGAN DRIVE, SUFFERN, NY

STRUCTURAL REPAIRS



SITE LOCATION MAP - OVERALL LAYOUT
(NOT TO SCALE)



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GEI PROJECT No. 2101670

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COVER SHEET

STATE OF NEW YORK
PAULO ANDRES BRUNO
080328
LICENSED PROFESSIONAL ENGINEER

4/4/22

DWG NUMBER :

S-000

1 OF 61

STRUCTURAL STEEL NOTES

1. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE LATEST AISC SPECIFICATION PERMITTED BY THE NEW YORK STATE BUILDING CODE. ALL STEEL SHALL CONFORM TO:
- A. STRUCTURAL STEEL SHAPES: ASTM A 992 STEEL (GR50)

FY = 50 KSI

B. STEEL PIPE COLUMNS: ASTM A501

FY = 36 KSI

C. STRUCTURAL TUBES: ASTM A500 GRADE B (SEE NOTE 24 FOR GALVANIZING)

FY = 46 KS

D. STEEL ANGLES, PLATES & CHANNELS: ASTM A36

FY = 36 KSI

E. THREADED RODS: STAINLESS STEEL AISI TYPE 316.

FY = 64 KSI
- A.I.S.C. SPECIFICATIONS FOR DESIGN FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS SHALL APPLY, OR ANY MORE RESTRICTIVE REQUIREMENTS OF THE NEW YORK STATE BUILDING CODE.
2. CONTRACTOR SHALL PROVIDE CERTIFIED MILL REPORTS AND FABRICATOR SHALL FILE AN AFFIDAVIT OF THE PRODUCER OF THE STEEL CERTIFYING THAT THE PROVISIONS OF THE BUILDING CODE ARE MET. SUBMIT ABOVE FOR REVIEW TO STRUCTURAL ENGINEER PRIOR TO FABRICATION AND ORDERING OF MATERIALS.
3. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE, EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS. (LATEST EDITION).
4. THE CONTRACTOR SHALL COORDINATE ALL DIMENSIONS SHOWN ON THE PLANS WITH THE VARIOUS TRADES BEFORE DETAILING AND FABRICATING STEEL. REFER TO MECHANICAL AND ELECTRICAL DRAWINGS FOR ADDITIONAL STRUCTURAL WORK NOT SHOWN ON THE STRUCTURAL DRAWINGS.
5. ALL DETAILING FABRICATION AND ERECTION SHALL CONFORM TO AISC "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATIONS FOR STRUCTURAL STEEL BUILDING" OR "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN," AND AISC "CODE OF STANDARD PRACTICE," LATEST EDITION.
6. PRIOR TO SUBMITTING ANY SHOP DRAWINGS, THE CONTRACTOR SHALL SUBMIT A SHOP DRAWING SCHEDULE TO THE ENGINEER-OF-RECORD FOR APPROVAL. THE SCHEDULE SHOULD INCLUDE THE DATES, WHEN THE DRAWINGS ARE TO BE SUBMITTED, THE NUMBER OF DRAWINGS AND TYPE OF DETAILS (ERECTION PLANS, SCHEDULES, BEAMS, COLUMNS, ETC.).
7. THE FABRICATOR SHALL SUBMIT TO THE ENGINEER, FOR REVIEW AND APPROVAL, ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS. ERECTION PLANS, JOB STANDARDS, SCHEDULES, ETC. PRIOR TO THE START OF PIECE DETAILING.
8. THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS. CONNECTIONS SHOWN ON THE STRUCTURAL DRAWING ARE SCHEMATIC AND ARE ONLY INTENDED TO SHOW THE RELATIONSHIP OF MEMBERS CONNECTED. CONNECTION DETAILS INDICATED ON THE DRAWINGS SHALL BE INCORPORATED INTO FABRICATOR'S CONNECTION DESIGN. SEE SPECIFICATIONS, CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE STRUCTURE IS LOCATED. ENGINEER'S SEAL MAY BE QUALIFIED "FOR DESIGN OF CONNECTIONS ONLY."
- ENGINEER-OF-RECORD'S REVIEW OF THE SHOP DRAWINGS IS ONLY FOR THE LIMITED PURPOSE OF CHECKING FOR CONFORMANCE WITH THE DESIGN INTENT AND INFORMATION EXPRESSED IN THE CONTRACT DRAWINGS.
9. ALL COMPOSITE BEAM CONNECTIONS TO DEVELOP MINIMUM ONE-HALF OF THE TOTAL LOAD CAPACITY TABULATED IN THE AISC STEEL HANDBOOK (LATEST EDITION) TIMES 1.5 FOR THE GIVEN SHAPE AND SPAN OF THE BEAM OR THE REACTION SHOWN ON THE DRAWINGS, OR THE SCHEDULE SHOWN HEREIN, WHICHEVER IS GREATER.
- ALL BEAM CONNECTIONS TO DEVELOP MINIMUM ONE-HALF OF THE TOTAL LOAD CAPACITY TABULATED IN THE AISC STEEL HANDBOOK (LATEST EDITION) FOR THE GIVEN SHAPE AND SPAN OF THE BEAM, OR THE REACTION SHOWN ON THE DRAWING, OR THE SCHEDULE SHOWN BELOW, WHICHEVER IS GREATER.
10. FOR FRAMED BEAM CONNECTIONS, INCLUDING SINGLE PLATE CONNECTIONS, PROVIDE THE MINIMUM NUMBER OF HORIZONTAL BOLT ROWS AS SHOWN BELOW, BASED ON 3" C/C UNLESS NOTED OTHERWISE.

SHAPE	REACTION (K)	MINIMUM NUMBER OF FASTENERS
W8, C8	20	2
W10, C10	28	2
W12, C12	35	3
W14	35	3
W16	50	4
W18	65	4
W21	85	5
W24	100	6
W27	100	6
W30	120	7
W33	160	8
W36	220	9
W40	240	11
W44	260	11

PROVIDE REINFORCING AT CONNECTIONS WHERE CUTS OR COPES REDUCE THE SHEAR OR MOMENT CAPACITY BELOW THAT REQUIRED TO SUSTAIN THE REACTION. REFER TO PLAN FOR REACTIONS LARGER THAN SHOWN HERE.

11. DETAILS OF CONNECTIONS SHALL CONFORM TO AISC STANDARDS AND THE MANUAL OF STEEL CONSTRUCTION (LATEST EDITION), AND NEW YORK STATE BUILDING CODE..
- A. TYPICALLY, ALL BEAM TO COLUMN CONNECTIONS SHALL BE FRAMED BEAM CONNECTIONS.

B. TYPICALLY, BEAM TO BEAM CONNECTIONS SHALL BE FRAMED BEAM CONNECTIONS.

C. IF A ONE-SIDED CONNECTION IS USED FOR BEAMS 18" DEEP AND UNDER, THE CONNECTION SHALL BE DESIGNED IN ACCORDANCE WITH THE AISC MANUAL (LATEST EDITION), AND SHALL HAVE A MINIMUM OF TWO LINES OF BOLTS IN EACH LEG OF THE ONE-SIDED CONNECTION ANGLE.

D. WHEN THE TABULATED AISC FRAMED CONNECTION TABLES CANNOT BE USED OR WHERE THEY ARE UNABLE TO CARRY THE REQUIRED REACTIONS, SPECIAL CONNECTIONS OF ADEQUATE STRENGTH SHALL BE PROVIDED.

E. SHOP CONNECTIONS SHALL BE HIGH-STRENGTH BOLTED, OR WELDED.

F. FIELD CONNECTIONS SHALL BE HIGH-STRENGTH BOLTED, EXCEPT WHERE WELDED CONNECTIONS ARE SHOWN ON DRAWINGS.

G. ALL BEAMS CARRYING OR OVER COLUMNS, AND MOMENT SPLICES SHALL BE HIGH STRENGTH BOLTED.
12. **BOLTED CONNECTIONS**
- A. ALL BOLTS SHALL BE HIGH-STRENGTH CONFORMING TO ASTM A325 N. (A325 SLIP CRITICAL, CLASS A, FULLY TENSIONED TO COMPLY WITH AISC TABLE J 3.7).

B. ALL BOLTS SHALL BE 3/4" MINIMUM DIAMETER WITH HARDENED WASHERS UNDER THE TURNING ELEMENT (INCLUDING TURN OF THE NUT METHOD), UNLESS OTHERWISE NOTED.

C. HIGH-STRENGTH BOLTED CONNECTIONS TYPE N SHALL BE INSTALLED AND CONFORM TO AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS," APPROVED NOV. 13, 1985 AND ADOPTED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS OF THE ENGINEERING FOUNDATION.

D. ALL CONTACT SURFACES, INCLUDING SURFACES ADJACENT TO THE BOLT HEAD AND NUT, SHALL BE FREE OF SCALE, OIL, PAINT, LACQUER, AND OTHER FOREIGN MATERIAL. BURRS THAT WOULD PREVENT SOLID SEATING OF THE CONNECTED PARTS SHALL BE REMOVED. CONTACT SURFACES THAT ARE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 AND ROUGHENED BY MEANS OF HAND WIRE BRUSHING (POWER BRUSHING IS PROHIBITED) WILL BE PERMITTED.

E. ALL BOLTED CONNECTIONS SHALL MEET STRUCTURAL INTEGRITY REQUIREMENTS.

F. PROVIDE TWO BOLTS PER CONNECTION. MINIMUM CONNECTION STRENGTH SHALL BE 6 KIPS.

G. ALL HOLES SHALL BE DRILLED OR PUNCHED. NO BURNED IN HOLES SHALL BE ACCEPTABLE.

13. **WELDED CONNECTIONS**
- A. WELDING OPERATIONS AND DESIGN SHALL BE IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE AND AISC SPECIFICATIONS, INCLUDING:
1. AISC SPECIFICATION, J2.
2. STRUCTURAL WELDING CODE, AWS D1.1, AS MODIFIED BY AISC SPECIFICATION SECTION J2.

B. WELDING SHALL BE PERFORMED BY LICENSED WELDERS AND MEET THE REQUIREMENTS OF THE NEW YORK STATE BUILDING CODE AND AWS.

C. AWS CLASS E70XX SERIES LOW HYDROGEN ELECTRODES SHALL BE USED FOR WELDING NEW STRUCTURAL STEEL.

D. ALL CONTACT SURFACES AND AREAS WITHIN TWO INCHES OF WELD DURING WELDING SHALL BE FREE OF SCALE, OIL, PAINT LACQUER, GALVANIZING, AND ANY OTHER FOREIGN MATERIAL.

E. BUTT WELDS SHALL BE 100% PENETRATION WELDS AND FILLET WELDS A MINIMUM OF 1/4" UNLESS OTHERWISE NOTED.

F. ALL WELDED CONNECTIONS SHALL MEET STRUCTURAL INTEGRITY REQUIREMENTS OF THE NEW YORK STATE BUILDING CODE.

G. WELDING SHALL NOT BE PERMITTED WHEN: THE AMBIENT TEMPERATURE IS BELOW 15F; SURFACES ARE WET, EXPOSED TO RAIN, SNOW OR HIGH WIND, WELDERS ARE EXPOSED TO INCLEMENT CONDITIONS. CONTRACTOR SHALL COORDINATE WITH OWNER TO PROVIDE FOR THE SHUT-OFF OF FIRE ALARMS, FIREWATCH, AND NOTIFICATION TO THE FIRE DEPARTMENT DURING WELDING OPERATIONS.

H. ALL FILLER METAL SHALL HAVE A MINIMUM CVN (CHARPY V-NOTCH) TOUGHNESS OF 20 FT.-LBS. AT MINUS 20F.
14. CONTRACTOR SHALL SUBMIT WRITTEN WELDING PROCEDURE SPECIFICATIONS (WPS) FOR ALL WELDING ON THE JOB, INCLUDING PREQUALIFIED PROCEDURES.
15. THE CONTRACTOR MUST HAVE WPS THAT COVER EVERY WELD TO BE MADE ON THE JOB INCLUDING THOSE OF THE ERECTOR. WPS MUST BE AVAILABLE FOR TESTING LABORATORY INSPECTOR OR ENGINEER.
16. WPS MUST INCLUDE: WELDING PROCESS, BASE METAL THICKNESS AND STEEL GRADE OF BASE METAL AND BACKUP MATERIAL, JOINT TYPE, WELD TYPE, JOINT DESIGN DETAIL, BACKLOGGING, ELECTRODE DIAMETER, NUMBER AND POSITION OF WELD PASSES, VOLTAGE, POLARITY, PREHEAT, AND INSPECTION REQUIREMENTS, APPROVED WRITTEN WELDING PROCEDURES IN CLOSE PROXIMITY TO AN USED BY THE WELDERS. THE WPS MUST BE PROPERLY FOLLOWED, AND THE WELDER MUST USE THE RIGHT TOOLS (THE CORRECT TEMPERATURE AND INDICATING CRAYON). MAINTAIN THE MINIMUM SPECIFIED PREHEAT TEMPERATURE, WELD SIZE, MAXIMUM INTERPASS TEMPERATURE EXCEEDED, ELECTRODE, TRAVEL SPEED.
17. CONTRACTOR SHALL TAKE SPECIAL PRECAUTIONS DURING STEEL ERECTION TO INSURE STABILITY UNTIL STRUCTURE IS COMPLETE. ALL STRUCTURAL STEEL SHALL BE ADEQUATELY BRACED ALONG ITS MAJOR AND MINOR AXIS DURING ERECTION.
18. SHORING OF FLOOR MEMBERS TO CONTROL SLAB THICKNESS, FLOOR LEVEL TOLERANCES, AND CONCRETE PONDING IS THE CONTRACTOR'S OPTION. FLOORS TO BE POURED SO AS TO MAINTAIN UNIFORM SLAB THICKNESS ACROSS TOP OF STEEL MEMBERS.
19. STRUCTURAL STEEL CONTRACTOR TO PROVIDE DECK SUPPORT ANGLES AS REQUIRED.
20. BOLTS, NUTS AND WASHERS FOR STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE GALVANIZED.
21. STEEL, NUTS, BOLTS AND WASHERS TO BE PROVIDED BY A SINGLE DOMESTIC SOURCE.
22. ALL CUTS, HOLES, COPES REQUIRED FOR CONSTRUCTION BY OTHER TRADES, SHALL BE SHOWN ON APPROVED SHOP DRAWINGS AND MADE IN THE SHOP. FIELD BURNINGS OF CUTS OR HOLES IN THE STEEL MEMBERS WILL NOT BE PERMITTED.
23. ALL STEEL EXPOSED TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED. LINTEL ANGLES, BRACING MEMBERS OR STEEL EMBEDDED IN MASONRY SHALL RECEIVE TWO SHOP COATS OF PAINT. ALL OTHER STEEL SHALL BE LEFT UNPAINT AND GALVANIZATION FROM STEEL AREAS WHICH ARE TO BE WELDED. TOUCH UP ALL AREAS WHERE THE GALVANIZING HAS BEEN REMOVED OR DAMAGED.
- NOTE: STRUCTURAL STEEL TUBING WHICH IS EXPOSED TO THE WEATHER SHOULD BE GALVANIZED AND HAVE A RADIUS EQUAL TO THREE (3) TIMES ITS THICKNESS AS PER ASTM A143/A143M.
24. FITTED STIFFENERS SHALL BE PROVIDED IN BEAMS OVER AND UNDER ALL COLUMNS EQUAL TO COLUMNS FLANGE THICKNESS.
25. ANCHOR BOLTS TO BE ASTM F1554 (WELDABLE) FY = 55 KSI.
26. ALL GROUT UNDER BILLETS SHALL BE NON-SHRINK, PRE-MIX TYPE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5,000 P.S.I.
27. PROVIDE ADDITIONAL WIRE-MESH REINFORCEMENT, 4 X 4 -W 4 X 4 WWF X 5'-0" LONG IN TOP OF SLAB CONTINUOUSLY OVER ALL BEAMS WHICH RUN PARALLEL TO THE METAL DECK DIRECTION.
28. ALL STAIR STRUCTURES AND MISCELLANEOUS STEEL SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR. DRAWINGS AND CALCULATIONS SHALL BEAR THE SEAL AND SIGNATURE OF A NEW YORK STATE REGISTERED PROFESSIONAL ENGINEER HIRED BY THE CONTRACTOR.
29. IF FIELD BEAM SPICE IS REQUIRED, IT SHALL DEVELOP THE FULL PLASTIC MOMENT CAPACITY OF THE MEMBER. COMPLETE DETAILS SHALL BE SUBMITTED ON THE SHOP DRAWINGS
30. SHOP AND FIELD TESTING OF WELDS AND BOLTS SHALL BE AS FOLLOWS:
- A. ALL WELDS SHALL BE VISUALLY INSPECTED AND SHALL BE ACCEPTABLE IF THE CRITERIA OR AWS D1.1 TABLE 6.1 ARE SATISFIED.

B. FILLET WELDS FOR BEAM AND GIRDER SHEAR CONNECTION PLATES (15 PERCENT AT RANDOM) SHALL BE CHECKED BY MAGNETIC PARTICLE FOR FINAL PASS ONLY.

C. ULTRASONICALLY TEST 100 PERCENT OF ALL FULL PENETRATION WELDS.

D. CHECK BY CALIBRATED TORQUE WRENCH 25 PERCENT OF BOLTS IN EACH SHEAR CONNECTION, BUT NOT LESS THAN TWO (2) BOLTS PER CONNECTION.

E. THE TESTING AGENCY SHALL PERFORM ALL SHOP AND FIELD INSPECTION AND TESTING AS OUTLINED ABOVE.

F. THE STRUCTURAL STEEL FABRICATOR AND ERECTOR SHALL SCHEDULE ALL WORK TO ALLOW THE ABOVE TESTING REQUIREMENTS TO BE COMPLETED.
31. AFTER FABRICATION AND IN THE FIELD PRIOR TO THE APPLICATION OF SPRAY-ON OR TROWELED ON FIREPROOFING, ALL STEEL SHALL BE CLEANED OF ALL RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS. SEE ARCHITECTURAL DRAWING FOR FIREPROOFING REQUIREMENTS.
32. NO FIELD CUTTING OF STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT APPROVAL OF ENGINEER-OF-RECORD.
33. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY BRACING, GUYING, ETC. REQUIRED TO ERECT AND HOLD THE FRAME IN ORDER TO MAINTAIN CORRECT ALIGNMENT AND RESIST POSSIBLE COMBINATIONS OF DEAD, CONSTRUCTION, ERECTION, WIND, SEISMIC AND OTHER LATERAL LOAD COMBINATIONS.
34. PROVIDE EXISTING CONNECTION REINFORCING AT LOCATIONS SHOWN ON PLAN S101. CONTRACTOR SHALL HIRE A NYSPE TO DESIGN CONNECTION REINFORCING, CONTRACTOR SHALL SUBMIT THESE CALCULATIONS FOR REVIEW TO EOR.

STEEL JOISTS NOTES

1. ALL STEEL TRUSS GIRDERS/JOISTS, CONSTRUCTION, BRIDGING, ANCHORAGE, ETC., SHALL BE DESIGNED, DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS.
2. STEEL JOISTS SHALL BE STRUCTURAL STEEL HAVING A MINIMUM YIELD POINT OF Fy=50,000 PSI FOR CHORD SECTIONS AND A MINIMUM YIELD POINT OF Fy=36,000 PSI FOR WEB MEMBERS.
3. THE STEEL JOIST FABRICATOR SHALL SUBMIT AN AFFIDAVIT STATING COMPLIANCE WITH AISC, SJI AND AISI REQUIREMENTS. THE STEEL JOIST SUPPLIER SHALL BE A MEMBER OF SJI APPROVED FOR THE SPECIFIC JOIST

- SERIES. STEEL JOIST FABRICATOR SHALL PROVIDE AN AFFIDAVIT BY A LICENSED PROFESSIONAL ENGINEER THAT ALL WELDS HAVE BEEN MADE TO SUSTAIN AND TRANSFER THE DESIGN STRESSES TO THE JOIST MEMBERS.
4. STEEL JOIST MANUFACTURER SHALL DESIGN AND FURNISH JOISTS AND BOTTOM CHORD BRACES AND BRIDGING FOR A NET UPLIFT OF 17 PSF MINIMUM. PROVIDE POSITIVE ANCHORAGE AT END OF THE BRIDGING LINE OR TO STRUCTURAL STEEL BEAMS. CONSTRUCTION LOADS MUST NOT BE APPLIED TO JOISTS UNTIL ALL BRIDGING IS ATTACHED TO THE JOISTS AND ANCHORED AT ITS ENDS. CROSS BRIDGING REQUIRED AT END BAYS OF JOISTS.
5. TOP AND BOTTOM CHORDS SHALL BE STRUCTURAL ANGLES.
6. JOIST SHALL BE BRIDGED WITH CONTINUOUS HORIZONTAL ANGLE BRIDGING TOP AND BOTTOM. PROVIDE X-BRIDGING IN LAST PANEL, OR WHERE CONTINUOUS BRIDGING STOPS. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON JOISTS
7. INDIVIDUAL JOIST MEMBERS SHALL BE CONNECTED DIRECTLY TO ONE ANOTHER BY WELDING IN ACCORDANCE WITH THE LATEST AWS CODE. ALL WELDERS SHALL BE CERTIFIED WELDERS AND LICENSED.
8. FOR ALL JOISTS FORTY(40) FEET AND LONGER STRICTLY FOLLOW MANUFACTURER'S INSTRUCTIONS REGARDING ERECTION.
9. JOISTS SHALL HAVE MINIMUM 4" BEARING ON MASONRY OR CONCRETE, AND 2-1/2" ON STEEL.
10. ALL STEEL JOISTS SHALL BE THOROUGHLY INSPECTED BEFORE SHIPMENT TO INSURE COMPLIANCE IN MATERIALS AND WORKMANSHIP.
11. ALL JOISTS TO RECEIVE ONE SHOP COAT OF APPROVED PAINT.
12. DESIGN OF JOISTS SHALL BE BY MANUFACTURER'S ENGINEER REGISTERED IN THE STATE OF NEW YORK FOR ALL LOADING CONDITIONS. SHOP DRAWINGS SHALL BEAR THE ENGINEERS SEAL AND SIGNATURE. SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION.
13. JOIST CONTRACTOR SHALL COORDINATE WITH ALL TRADES IN ORDER TO PROVIDE ALL ROOF OPENINGS WHERE REQUIRED.
14. WHEN HORIZONTAL BRIDGING INTERFERES WITH MECHANICAL PENETRATIONS THROUGH THE JOIST SYSTEM, THEN INSTALL TWO (2) ADJACENT BAYS OF BOLTED OR WELDED X-BRIDGING EACH SIDE OF THE PENETRATION. METAL DECKING SHALL BE SECURED IN PLACE AND BRIDGING INSTALLED PRIOR TO CUTTING AND REMOVING HORIZONTAL BRIDGING.
15. ALL CONCENTRATED LOADS SUCH AS FROM ROOFTOP UNITS, PIPING, SUSPENDED EQUIPMENT, ETC. SHALL BE SUPPORTED AT PANEL POINTS OF THE JOIST. IF LOADS NEED TO BE SUPPORTED BETWEEN PANEL POINTS PROVIDE ADDITIONAL REINFORCEMENT (SEE TYPICAL DETAILS).
16. PROVIDE CEILING EXTENSIONS WHERE REQUIRED BY ARCHITECTURAL DRAWINGS OR SPECIFICATIONS.
17. BRIDGING SHALL BE CONNECTED TO THE JOISTS WITH 1/8-INCH FILLET WELD 1-INCH LONG (MINIMUM) OR EQUIVALENT. HORIZONTAL BRIDGING SHALL HAVE A STANDARD 3-INCH LAP AND BE WELDED WITH 2-1/8 INCH FILLET WELDS 1-INCH LONG OR EQUIVALENT.
18. DEFLECTION DUE TO DESIGN LIVE LOAD SHALL NOT EXCEED 1/360 OF SPAN.
19. PROVIDE UPLIFT BRIDGING AT JOIST ENDS AT ALL ROOF JOISTS.

METAL DECKING

1. STEEL DECKING SHALL BE CORRUGATED STEEL SHEETS, AND SHALL CONFORM TO THE STEEL DECK INSTITUTE SPECIFICATIONS, AND MEET THE REQUIREMENTS SPECIFIED HEREIN. SEE DRAWINGS FOR SIZE, GAGE AND TYPE. STEEL SHALL MEET ASTM A-653, GRADE 33, MINIMUM YIELD OF 33 KSI. GALVANIZING SHALL CONFORM TO REQUIREMENTS OF ASTM A-653 COATING CLASS G90. STEEL FOR PAINTED METAL DECK SHALL CONFORM TO ASTM A1008 GRADE.
2. **INSTALLATION:**
- A. GENERAL: DECKING SHALL BE COMPLETE, INCLUDING ALL SHAPING, CUTTING, FITTING, DRILLING, WELDING, FASTENINGS, COLUMN CLOSURES, RIDGE AND VALLEY PLATES, SUMP AND CANT STRIPS AND ALL OTHER ACCESSORIES AND MISCELLANEOUS PIECES NECESSARY FOR PROPER EDGING INSTALLATION. CLOSURE ELEMENTS FOR CONTAINMENT OF CONCRETE DURING POUR DECKING SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. END LAPS SHALL BE TWO INCHES MINIMUM, SHALL OCCUR OVER SUPPORTS, AND SHALL BE CENTERED THEREON. ADJACENT ROWS SHALL BE SIDE LAP ONE CORRUGATION WITH PREVIOUSLY PLACED ROW.

B. WELDING: SHALL CONFORM TO APPLICABLE REQUIREMENTS OF AMERICAN IRON AND STEEL INSTITUTE'S "SPECIFICATION FOR THE DESIGN OF LIGHT GAGE COLD-FORMED STEEL STRUCTURAL MEMBERS." WORK SHALL BE PERFORMED BY QUALIFIED WELDERS. SHEETS SHALL BE WELDED TO SUPPORTS IMMEDIATELY AFTER ALIGNMENT, AND ALL SHEETS SHALL BE WELDED BEFORE END OF WORKING DAY. WELDING SHALL MEET FOLLOWING MINIMUM REQUIREMENTS. USE ELECTRODE AWS D1.3 AND WELD TO ALL SUPPORTING MEMBERS IN ACCORDANCE WITH AWS REQUIREMENTS.
1) A MINIMUM VISIBLE 3/4" DIAMETER ARC PUDDLE WELD SHALL BE USED. WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL, AND SHALL HAVE GOOD FUSION TO SUPPORTING MEMBERS.
2) EXCEPT AS NOTED, DECK SHALL BE FASTENED BY 3/4" DIAMETER FUSION WELDS AT 12" O.C. AT ENDS AND INTERIOR SUPPORTS PERPENDICULAR TO DECK SPAN AND AT EDGE AND INTERIOR SUPPORTS PARALLEL TO THE DECK SPAN.
3) SIDE LAPS TO BE WELDED OR SCREWED WITH #12 SELF DRILLING SCREWS WITH MAXIMUM SPACING AT 3'-0" O.C.
3. METAL DECK TO BE INSTALLED CONTINUOUSLY OVER SUPPORTING MEMBERS WITH A MINIMUM OF THREE CONTINUOUS SPANS AND SHALL BEAR AT LEAST TWO INCHES MINIMUM BEARING AT FLOOR DECK SUPPORTS AND THREE INCH MINIMUM BEARING AT ROOF DECK SUPPORTS.
4. SHEAR STUDS SHALL BE CONSIDERED TO REPLACE WELDS.
5. FOR ONE OR TWO SPAN CONDITIONS, THE CONTRACTOR SHALL PROVIDE SHORING AS REQUIRED OR FURNISH HIGHER GAGE AS REQUIRED TO SUPPORT ALL APPLICABLE LOADS.
6. THE CONTRACTOR SHALL DEPOSIT ALL CONCRETE DURING PLACING IN SUCH A MANNER AS NOT TO OVERLOAD THE METAL DECK.
7. THE CONTRACTOR SHALL CALCULATE AND INCLUDE ALL ADDITIONAL CONCRETE THAT MAY BE REQUIRED DURING PLACING DUE TO DEFLECTION OF STRUCTURE.
8. USE ELECTRODE AWS D1.3 AND WELD TO ALL SUPPORTING MEMBERS IN ACCORDANCE WITH AWS REQUIREMENTS. ALL WELDING SHALL BE PERFORMED BY LICENSED WELDERS AND MEET THE REQUIREMENTS OF THE AWS AND THE LOCAL BUILDING CODE. WELDING SHALL NOT BE PERFORMED AT TEMPERATURES BELOW 32 DEGREES F.
9. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS WHICH INDICATE ALL INFORMATION NECESSARY FOR A COMPLETE FABRICATION AND ERECTION OF THE METAL WORK. DRAWINGS SHALL INCLUDE ALL SHAPING, CUTTING, FITTING, DRILLING, WELDING, CLOSURE STRIPS, CLOSURE PLATES, SUMP PANS, FILLERS, FASTENINGS, METAL LATH COLUMN CLOSURES AND ALL OTHER ACCESSORIES AND MISCELLANEOUS PIECES NECESSARY FOR PROPER INSTALLATION. TYPE, NUMBER OF WELDS, LOCATION AND METHOD OF WELDING SHALL BE INDICATED ON SHOP DRAWINGS.
10. STANDARD CLOSURES AND EDGE ANGLE POUR STOPS (SCREED ANGLES) SHALL BE GALVANIZED AND CONFORM TO ASTM A653, CLASS G60, AND PROVIDED AS REQUIRED.
11. BOTTOM OF METAL DECK MUST BE FREE OF DUST, LOOSE SCALE AND OIL PRIOR TO APPLYING ANY CEMENTITIOUS MIXTURE.
12. FLOOR DECK SHALL BE COMPOSITE TYPE AND MANUFACTURED WITH INTEGRAL HANGER TABS FOR VENTING PURPOSES ONLY.
13. PROVIDE ADDITIONAL ANGLE SUPPORTS UNDER DECK AT ALL COLUMNS.
14. NO LOADS SUCH AS MECHANICAL EQUIPMENT, PIPING, DUCTS, LIGHTING, ETC., SHALL BE HUNG DIRECTLY FROM METAL DECK. HANGERS FOR DUCTWORK, PIPING, ETC. SHALL BE HUNG DIRECTLY FROM STRUCTURAL STEEL MEMBER OR SUPPLEMENTARY MEMBERS OR ANCHORS EMBEDDED IN CONCRETE. SUBMIT HANGER LOAD DETAILS FOR REVIEW.
15. DECK CONTRACTOR SHALL COORDINATE OPENING SIZES AND LOCATIONS IN FLOORS AND ROOF FROM ARCHITECTURAL AND MECHANICAL DRAWINGS. HE SHALL PROVIDE ANY HEADER MEMBERS OR REINFORCEMENT AS REQUIRED EVEN IF NOT SHOWN ON PLANS.
16. CONSTRUCTION JOINTS FOR SLABS ON METAL DECK SHALL BE LOCATED MIDWAY BETWEEN BEAMS WHERE THE JOINT IS PARALLEL TO THE BEAM SPAN. WHERE THE JOINT IS PERPENDICULAR TO THE BEAM SPAN THE JOINTS SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF SPAN.

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DATE :	9/10/2021
SCALE :	AS SHOWN

DRAWING TITLE :

NOTES

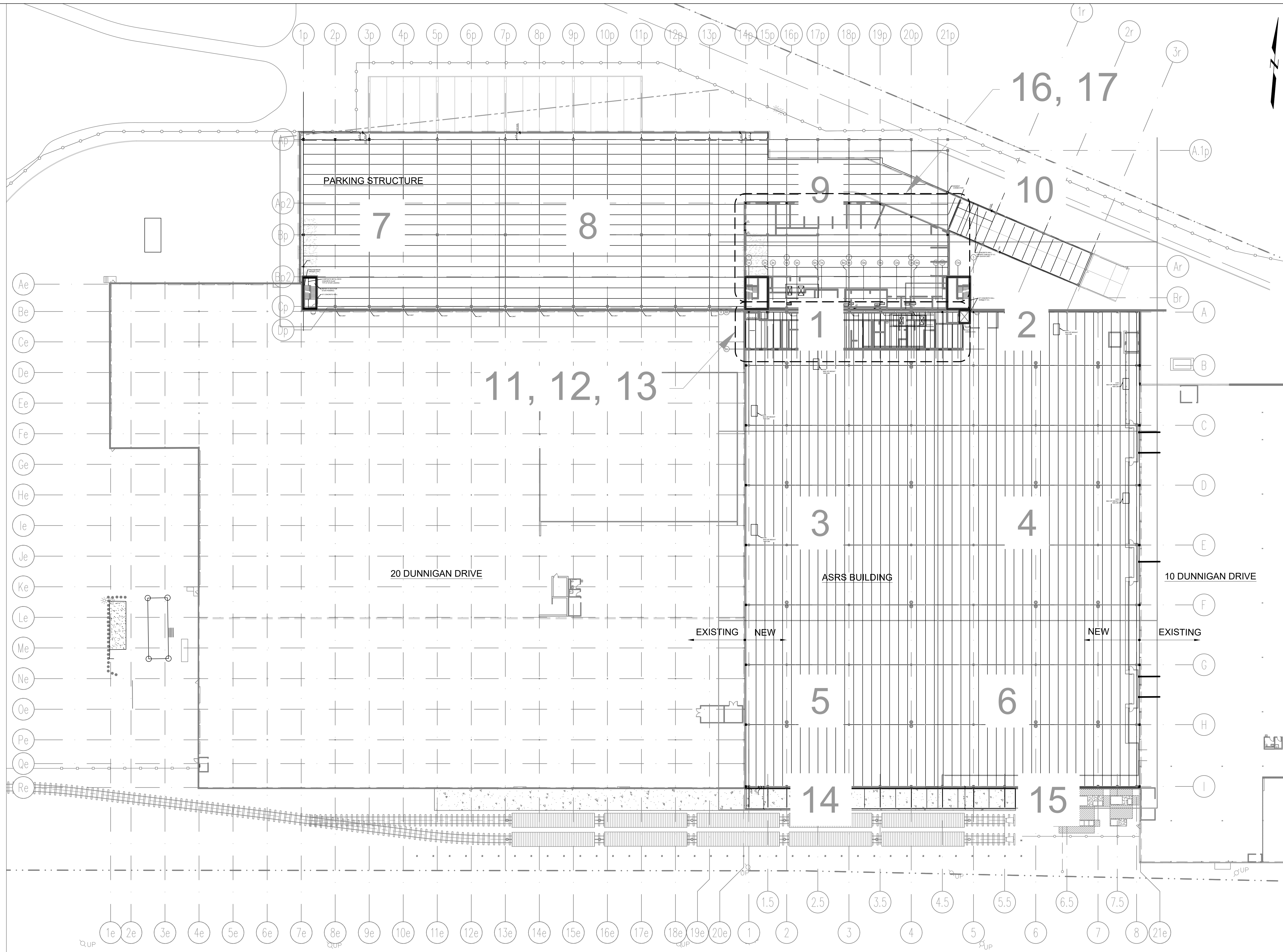
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FRAMING KEY PLAN
 SCALE: 1/32" = 1'-0"

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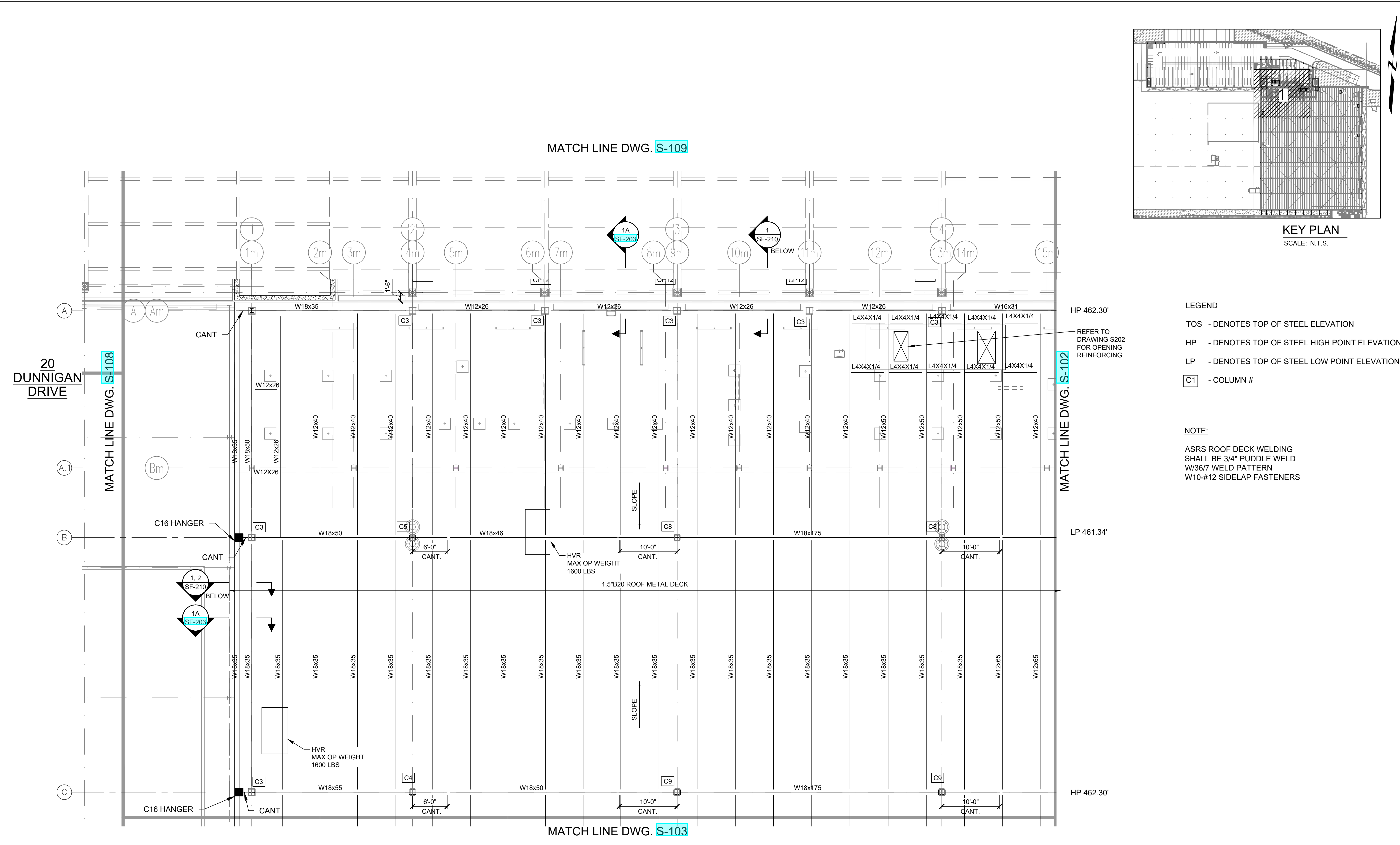
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FRAMING - KEY PLAN



DWG NUMBER :
S-100

5 OF 61



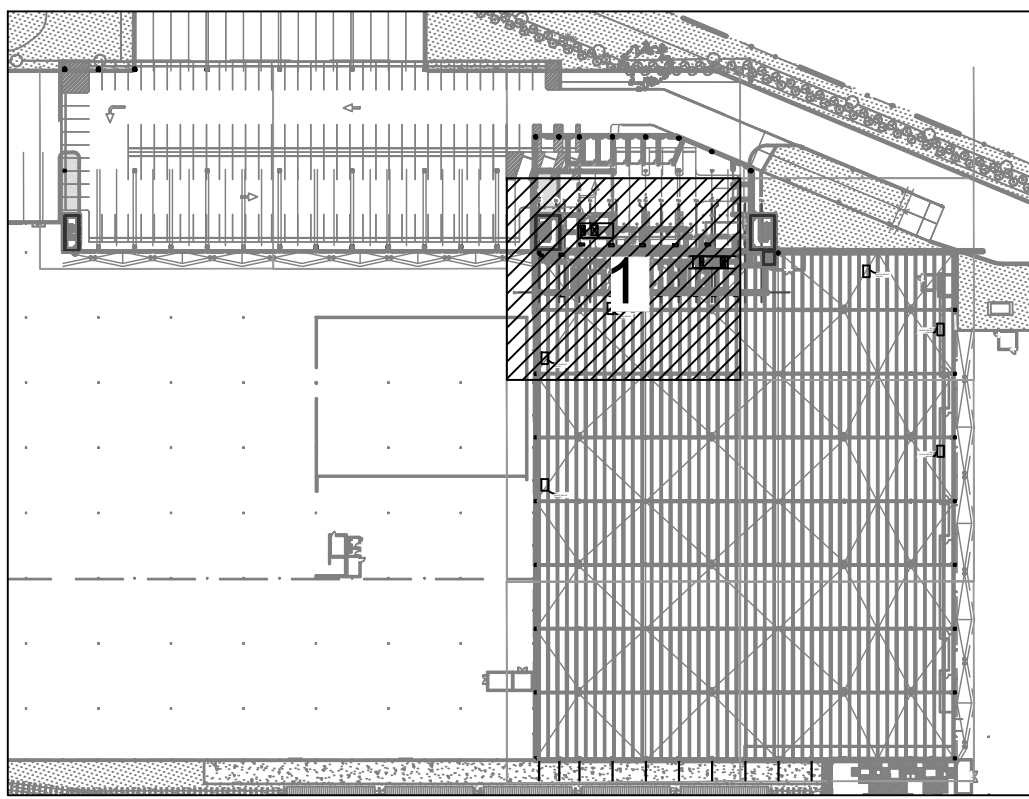
MATCH LINE DWG. S-109

MATCH LINE DWG. S-108

MATCH LINE DWG. S-102

MATCH LINE DWG. S-103

1 PART PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN



KEY PLAN
SCALE: N.T.S.

- LEGEND
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #

NOTE:
ASRS ROOF DECK WELDING
SHALL BE 3/4" PUDDLE WELD
W/36/7 WELD PATTERN
W10-#12 SIDELAP FASTENERS

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AS/RS FRAMING PART PLAN



DWG NUMBER :
S-101
6 OF 61



TOS - DENOTES TOP OF STEEL ELEVATION

HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION

LP - DENOTES TOP OF STEEL LOW POINT ELEVATION

C1 - COLUMN #

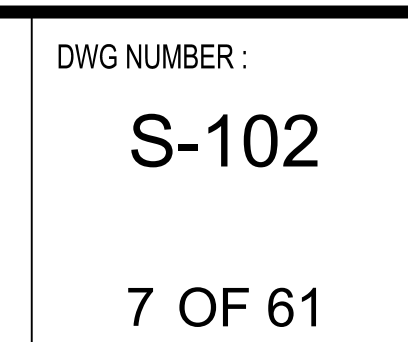


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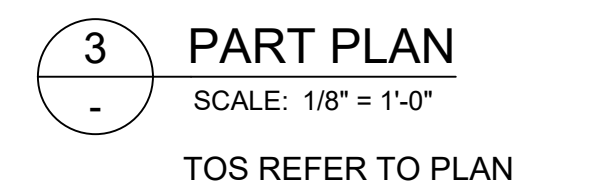
SCALE: N.T.S.

TOS - DENOTES TOP OF STEEL ELEVATION

HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION

LP - DENOTES TOP OF STEEL LOW POINT ELEVATION

C1 - COLUMN #



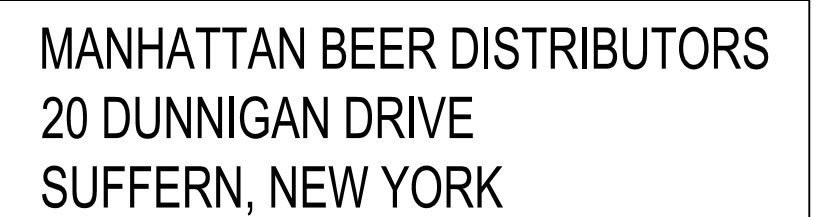
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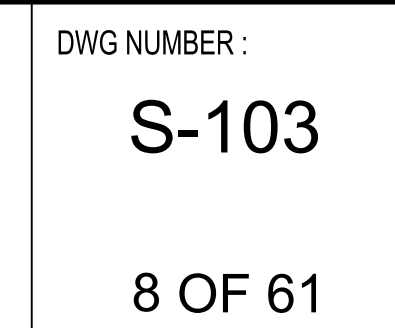
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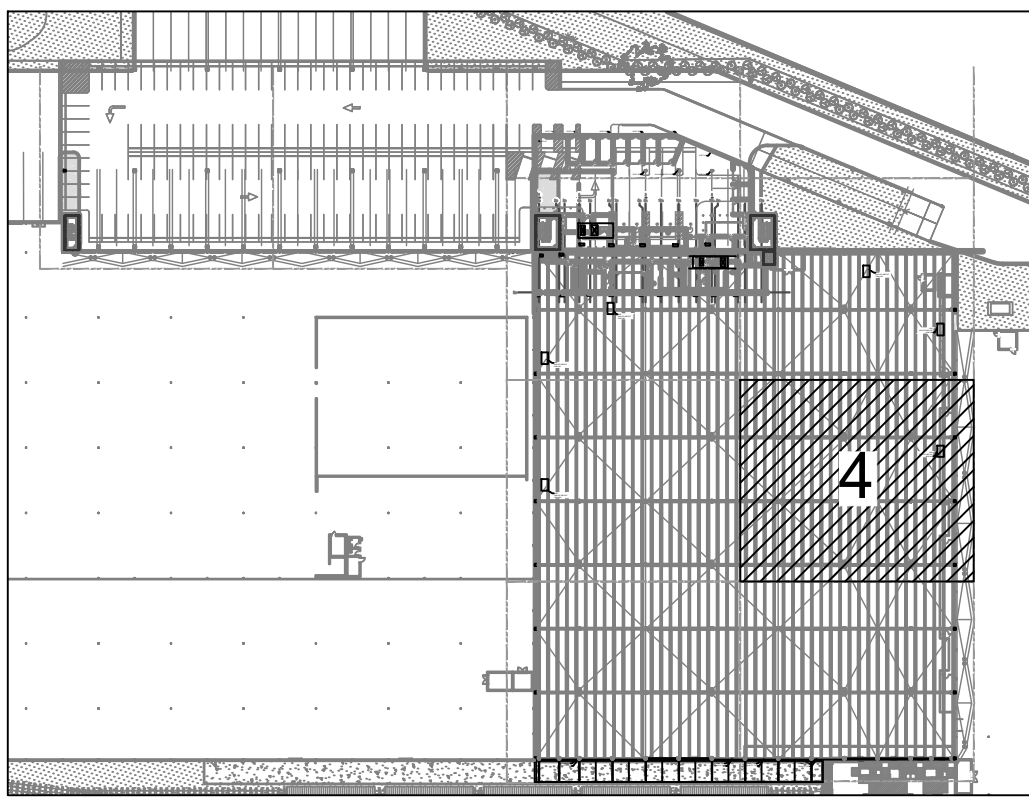
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SCALE :	AS SHOWN

AS/RS FRAMING PART PLAN





KEY PLAN
SCALE: N.T.S.

- LEGEND
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #

XX.X

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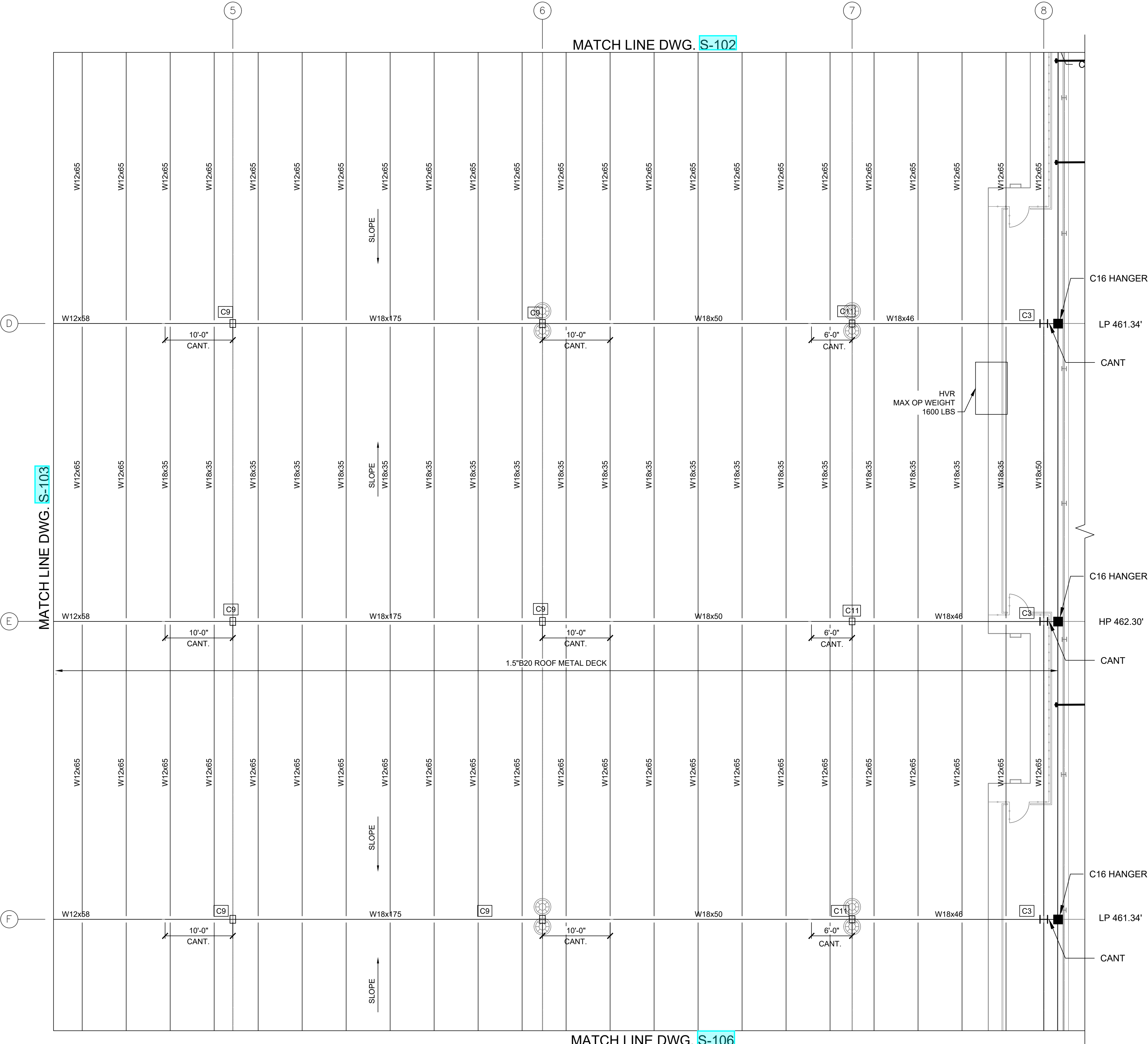


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DWG NUMBER :

S-104

9 OF 61



4 PART PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN

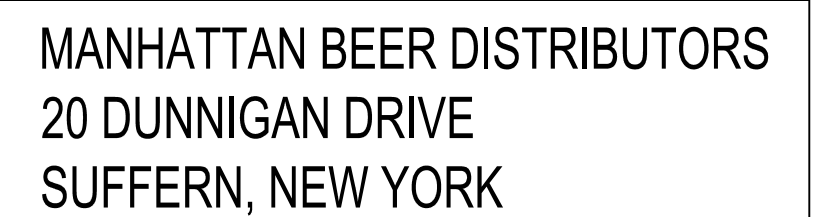


TOS - DENOTES TOP OF STEEL ELEVATION

HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION

LP - DENOTES TOP OF STEEL LOW POINT ELEVATION

C1 - COLUMN #



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AS/RS FRAMING PART PLAN



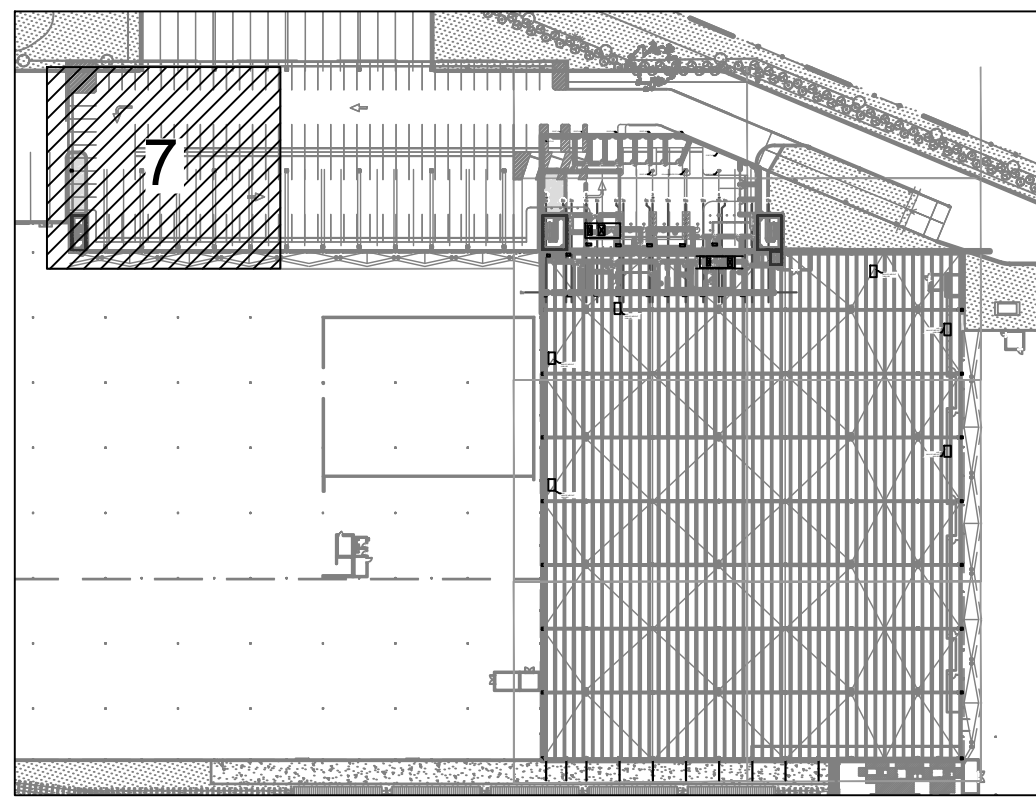
10 OF 61



MATCH LINE DWG. S-106

5 PART PLAN
SCALE: 1/8" = 1'-0"

TOS REFER TO PLAN



KEY PLAN
SCALE: N.T.S.

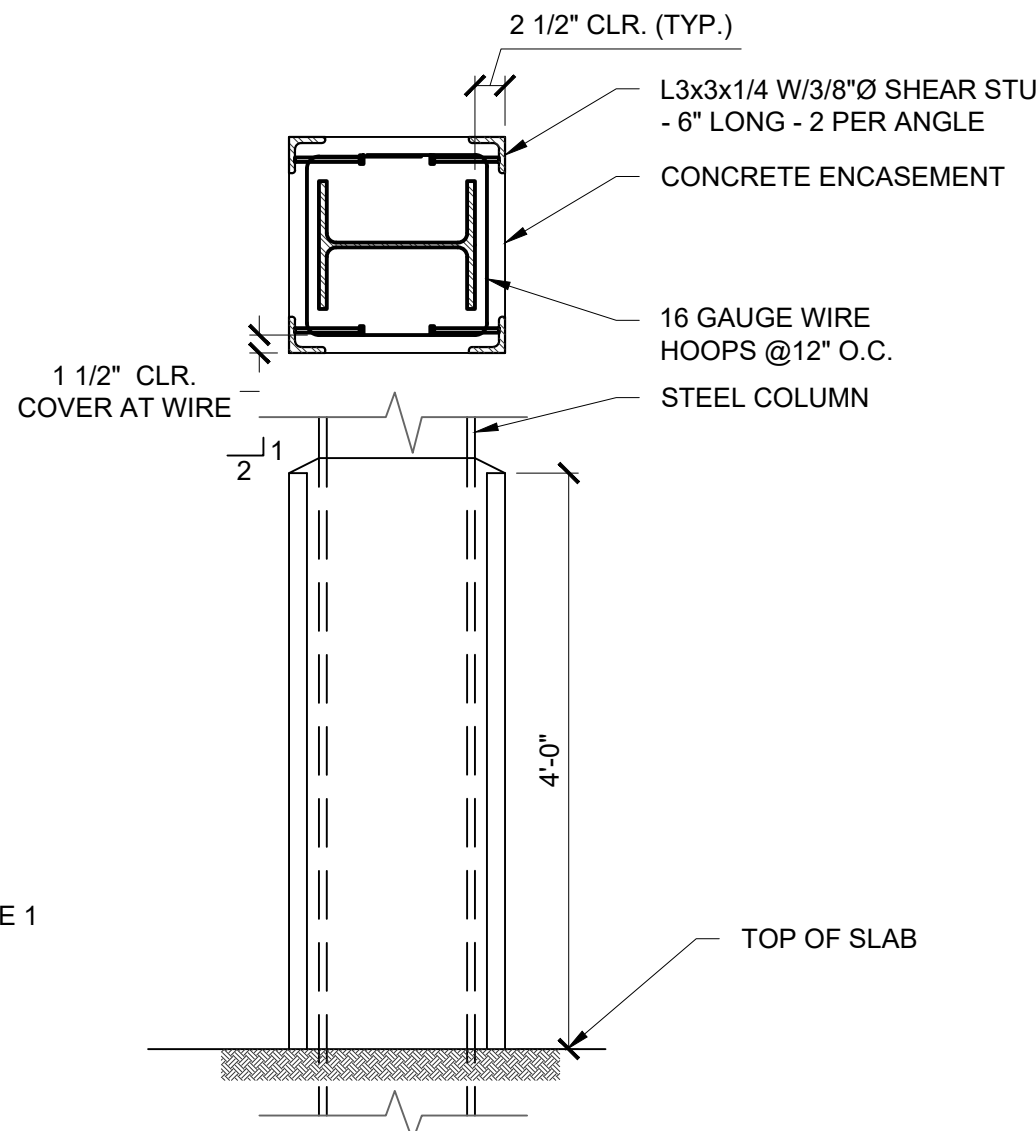
LEGEND

- TOS - DENOTES TOP OF STEEL ELEVATION
HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
C1 - COLUMN #
◀ - DENOTES MOMENT CONNECTION TO DEVELOP 80% MP

NOTE 1:
INSTALL CONCRETE COLUMN PROTECTION AT THIS COLUMNS. REFER TO SECTION ON DRAWING S107.

NOTE
SHEAR STUDS SHALL BE 3/4"-5" LONG (TYP) FOR 6" SLAB. INSTALL SHEAR STUD AT 1' O.C. MAXIMUM AT EVERY BEAM MINIMUM (UNO)

PARKING SLAB SHALL BE REINFORCED WITH 6X6-W2.9XW2.9 WWF



1 CONCRETE ENCASEMENT DETAIL
SCALE: 3/4" = 1'-0"

7 PART PLAN
SCALE: 1/8" = 1'-0"
TOS 429.58

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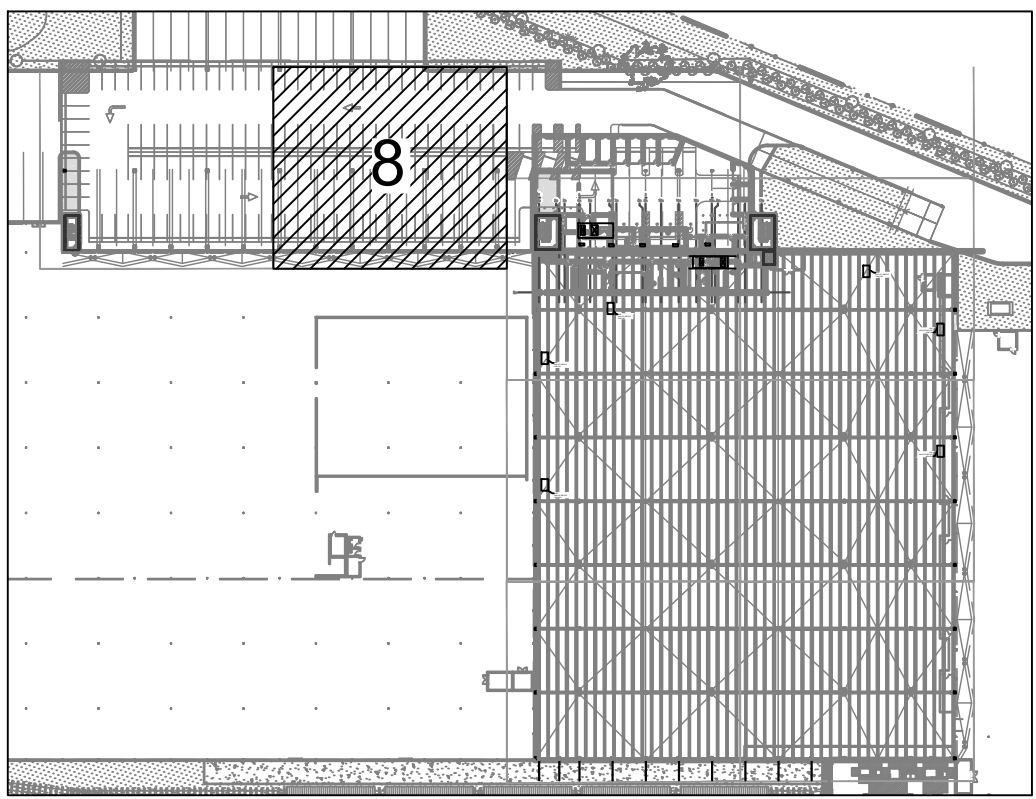
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SCALE : AS SHOWN

DRAWING TITLE :

PARKING FRAMING
PART PLAN



DWG NUMBER :
S-107
12 OF 61
4/4/22



LEGEND

TOS - DENOTES TOP OF STEEL ELEVATION

HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION

LP - DENOTES TOP OF STEEL LOW POINT ELEVATION

C1 - COLUMN #

— NOTE 1

— NOTE 1



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PARKING FRAMING PART PLAN



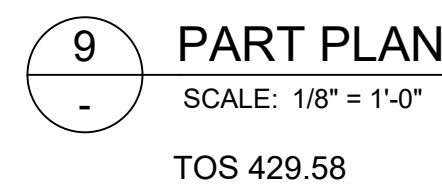
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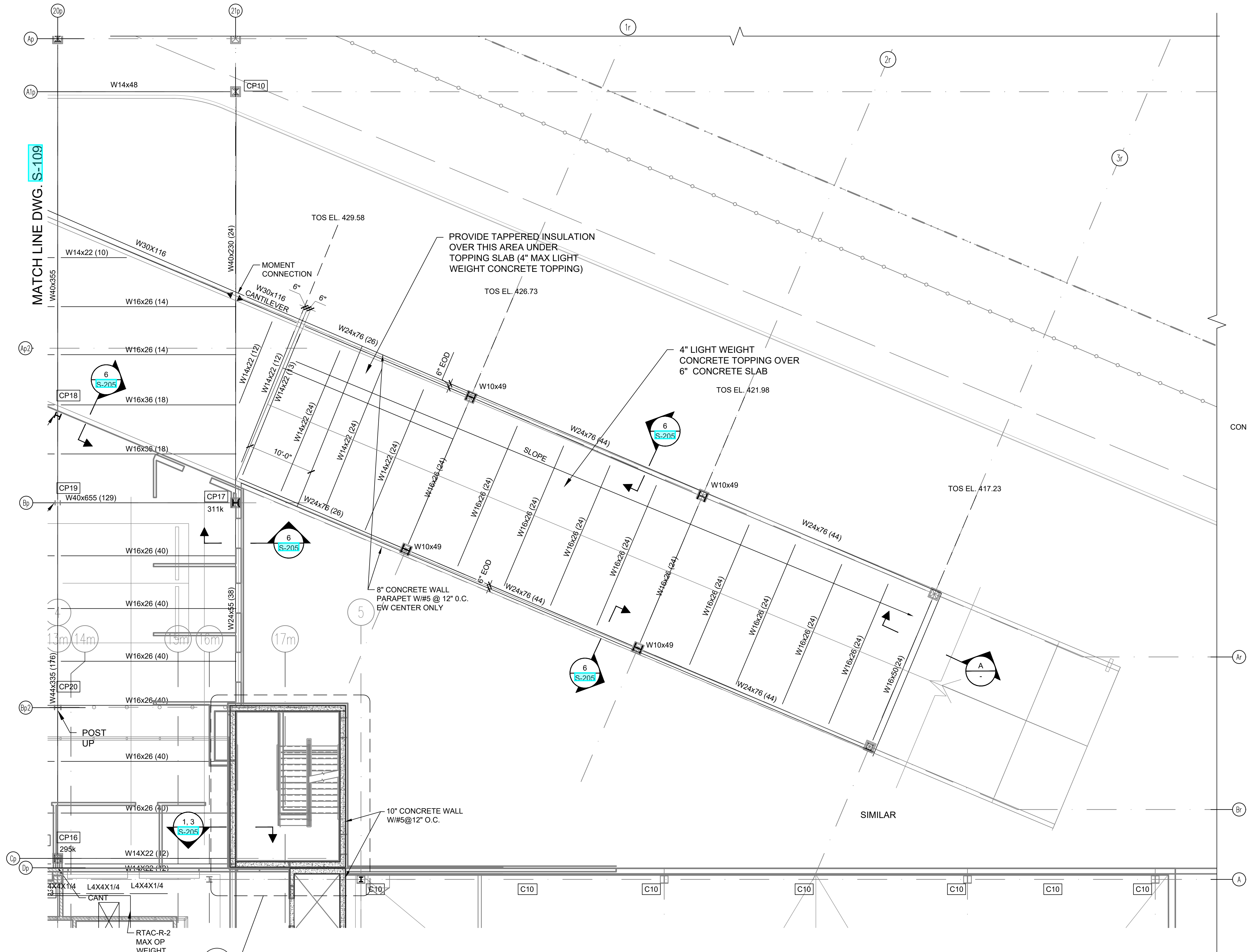
S-108

13 OF 61



NOTE 1:
INSTALL CONCRETE COLUMN
PROTECTION AT THIS COLUMNS.
REFER TO SECTION ON DRAWING S107.

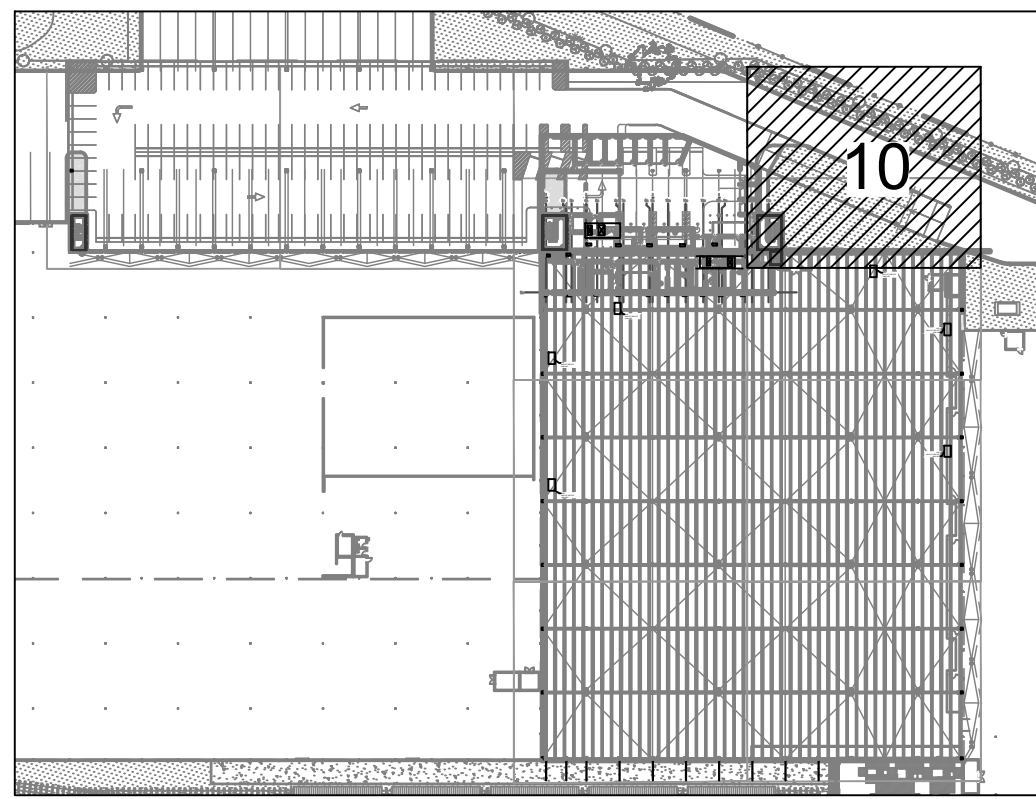




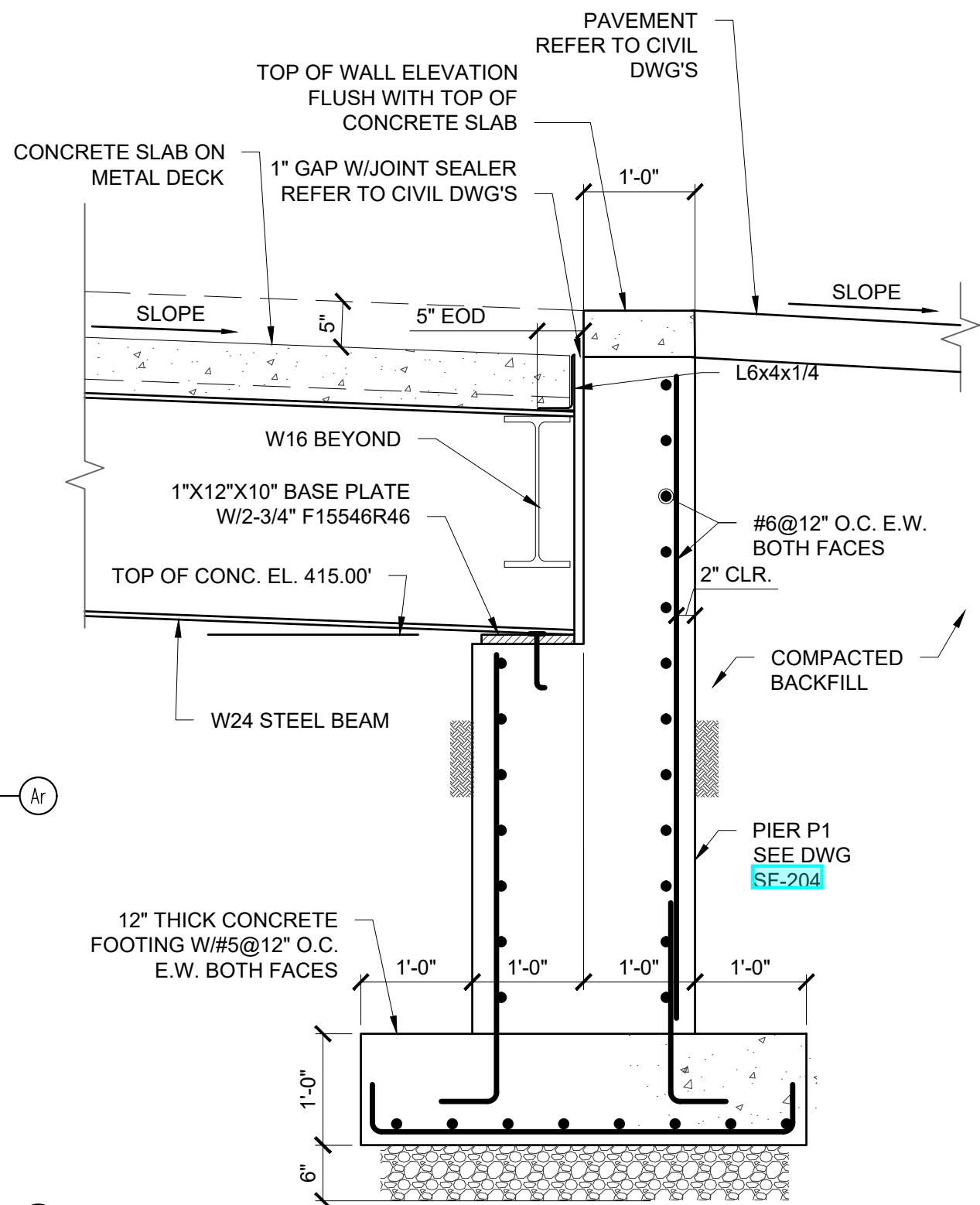
MATCH LINE DWG. S-102

10 PART PLAN
SCALE: 1/8" = 1'-0"

TOS 429.58 UNO.
REFER TO PLAN FOR
RAMP ELEVATIONS



KEY PLAN
SCALE: N.T.S.



A BUTTRESS SECTION
SCALE: 3/4" = 1'-0"

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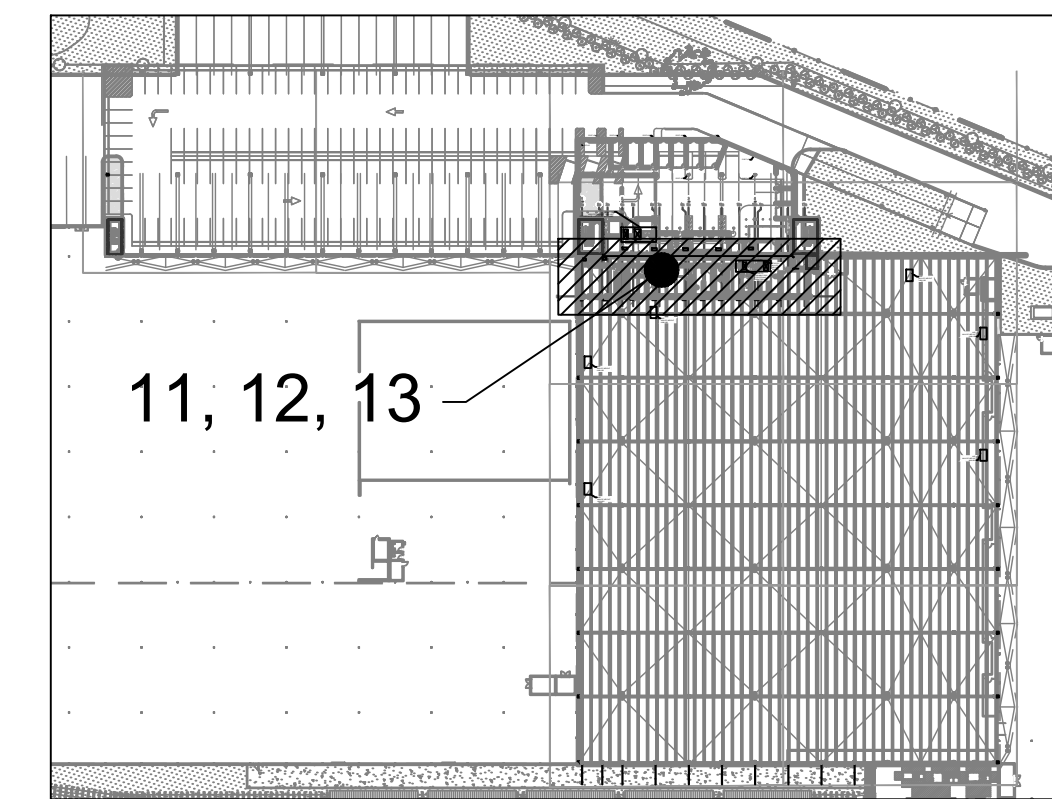
PARKING FRAMING
PART PLAN



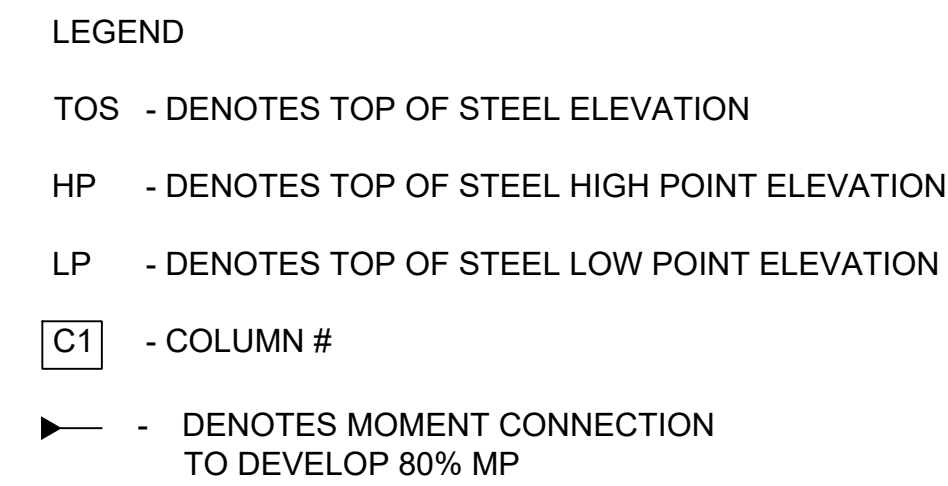
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S-110

15 OF 61

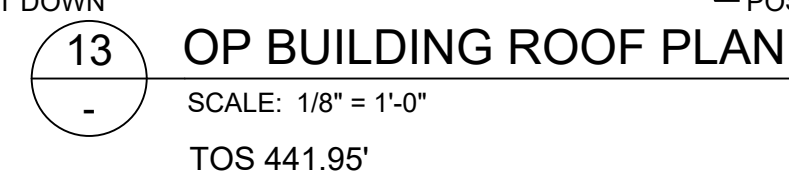
4/4/22



11 MEZZANINE FRAMING PLAN
SCALE: 1/8" = 1'-0"
TOS 421.00'



12 UPPER MEZZANINE FRAMING PLAN
SCALE: 1/8" = 1'-0"
TOS 430.75'



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SCALE :	AS SHOWN

DRAWING TITLE :

MEZZANINE
FRAMING PLAN



DWG NUMBER :

S-111

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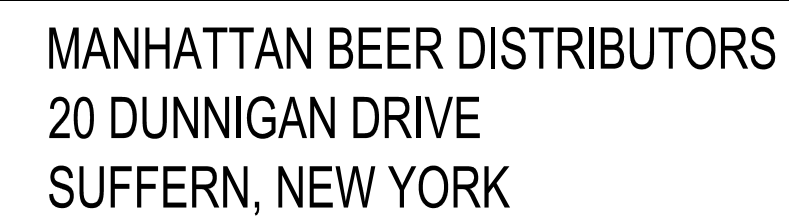
LEGEND

TOS - DENOTES TOP OF STEEL ELEVATION

HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION

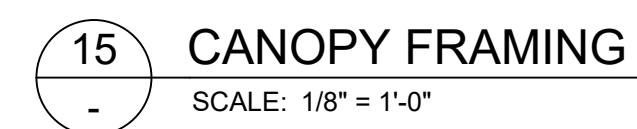
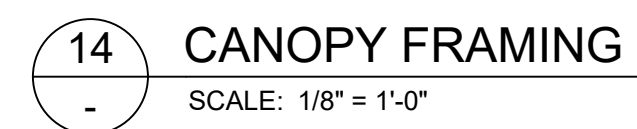
LP - DENOTES TOP OF STEEL LOW POINT ELEVATION

C1 - COLUMN #

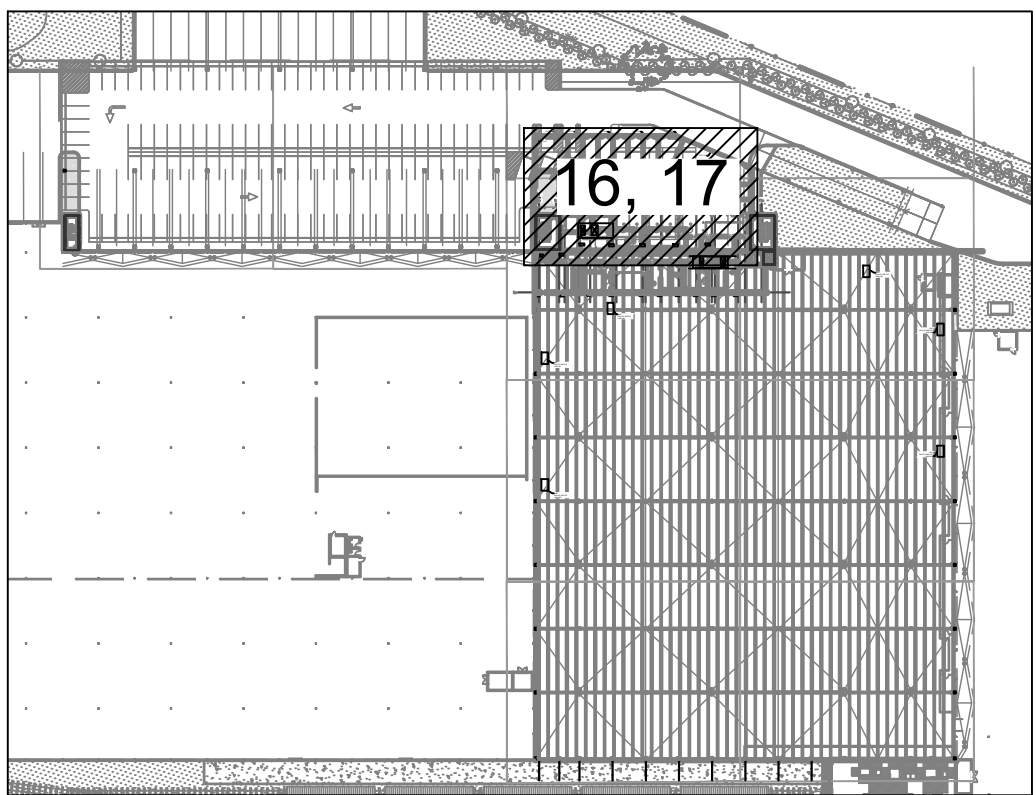


DRAWING TITLE :

17 OF 61



TOS REFER TO PLAN



INSTALL CONCRETE COLUMN
PROTECTION AT THIS COLUMNS.
REFER TO SECTION ON
DRAWING S107.

C1 - C

TOS 444.14'



TOS REFER TO PLAN



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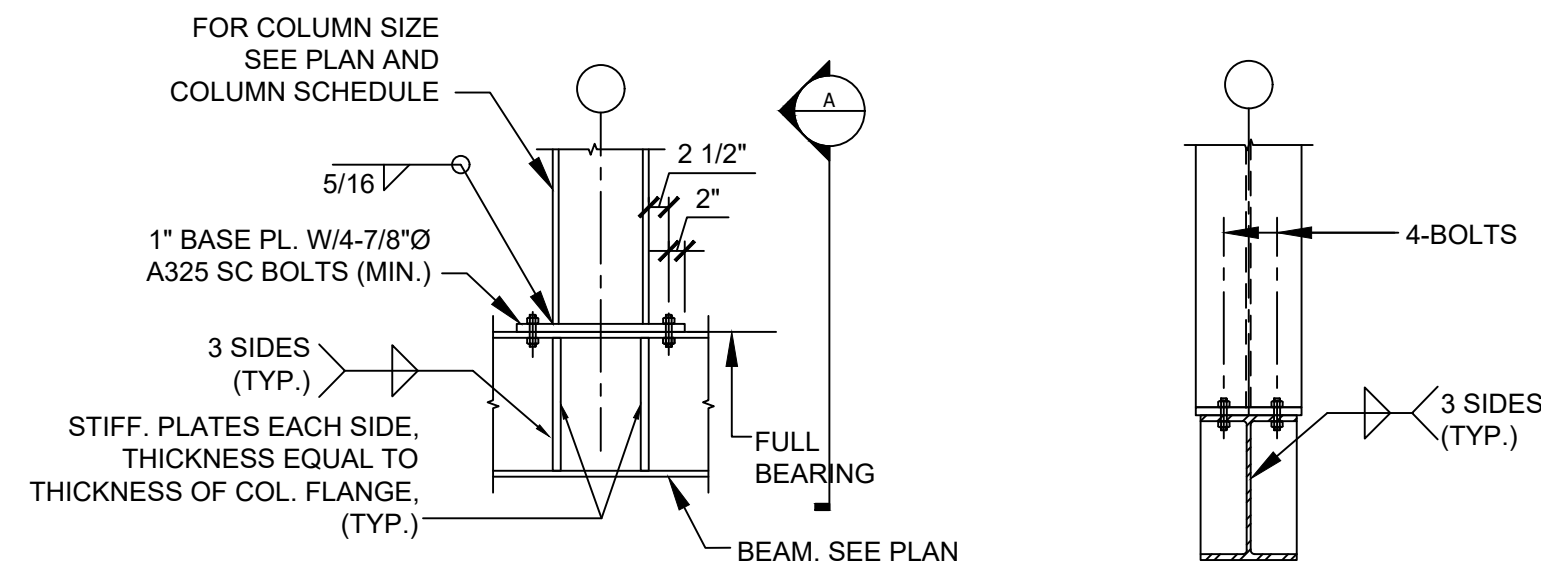
SCALE: AS SHOWN

DRAWING TITLE:

ADMIN. BUILDING 3RD FLOOR AND ROOF FRAMING PLAN

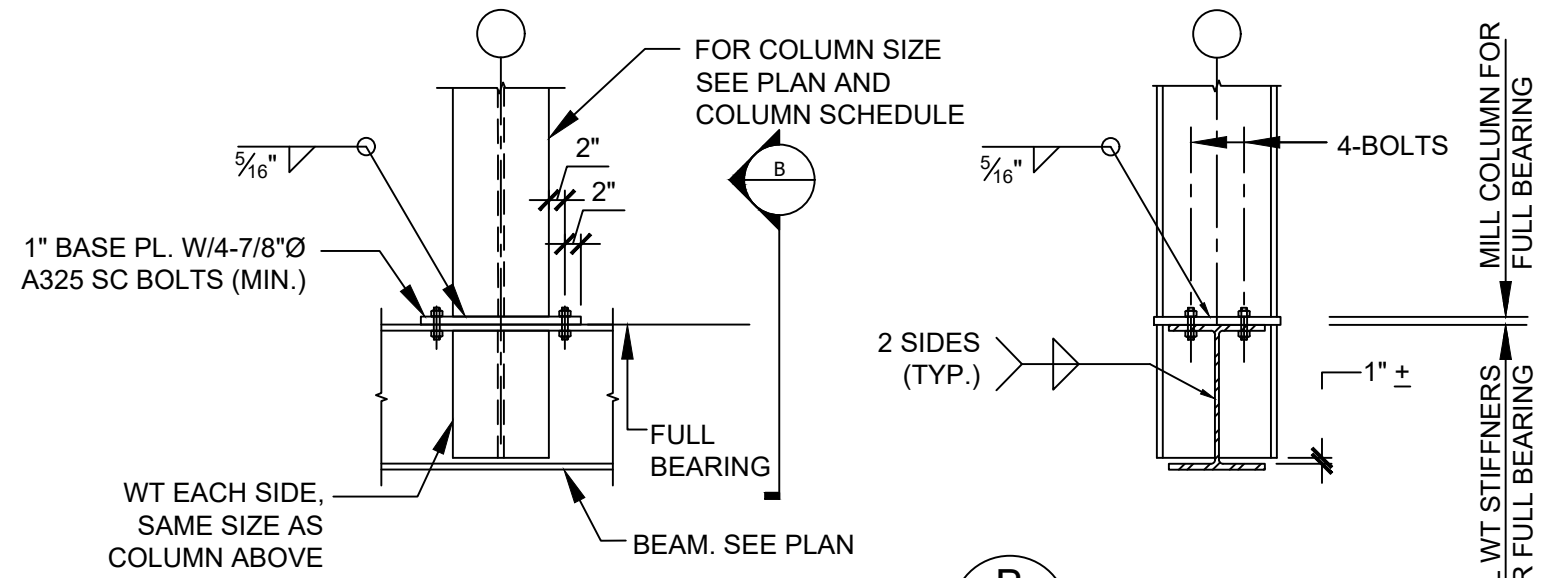


S-130



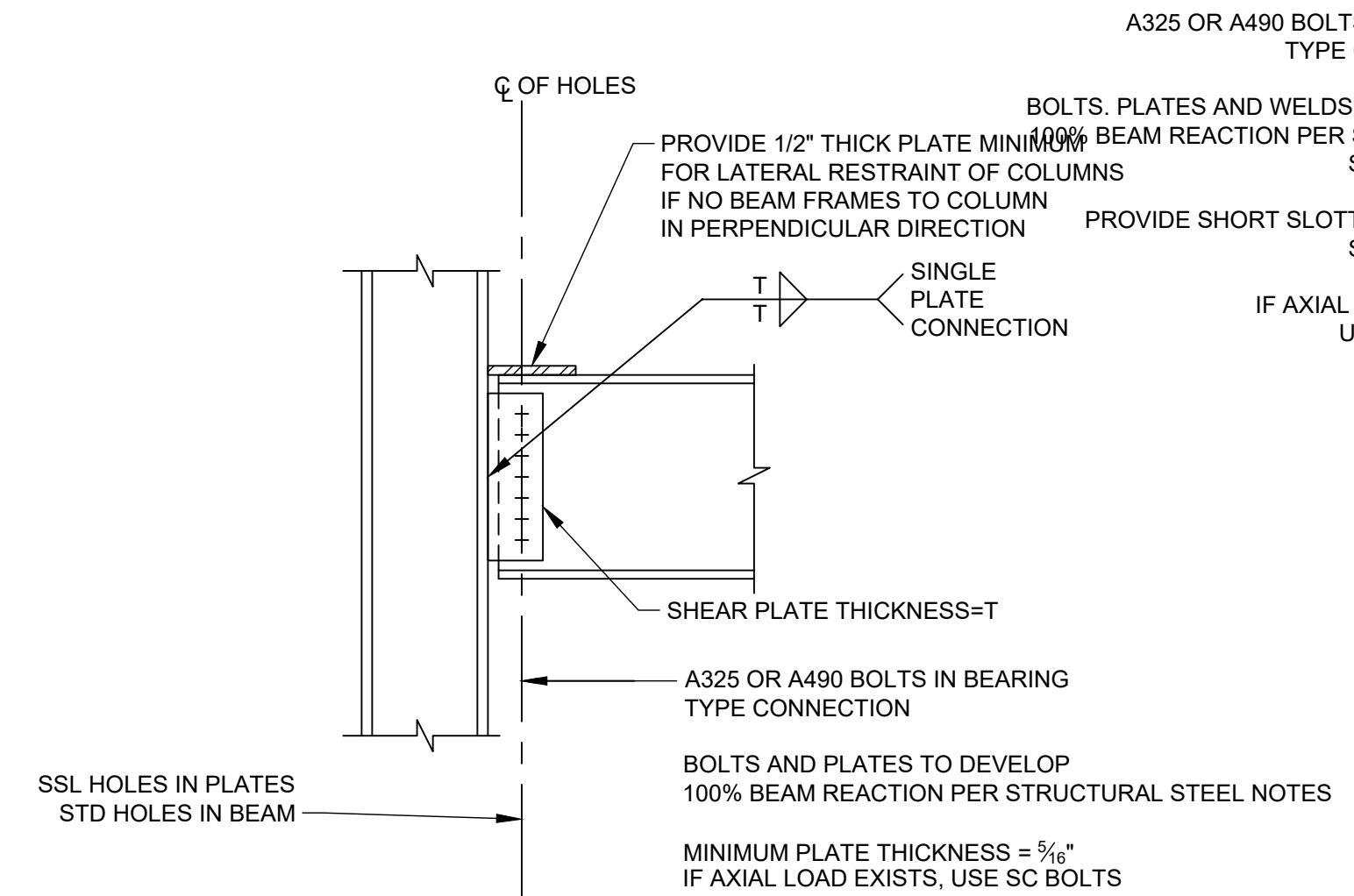
SECTION AT W SECTION POST/ COLUMN

SCALE: 12"=1'-0"



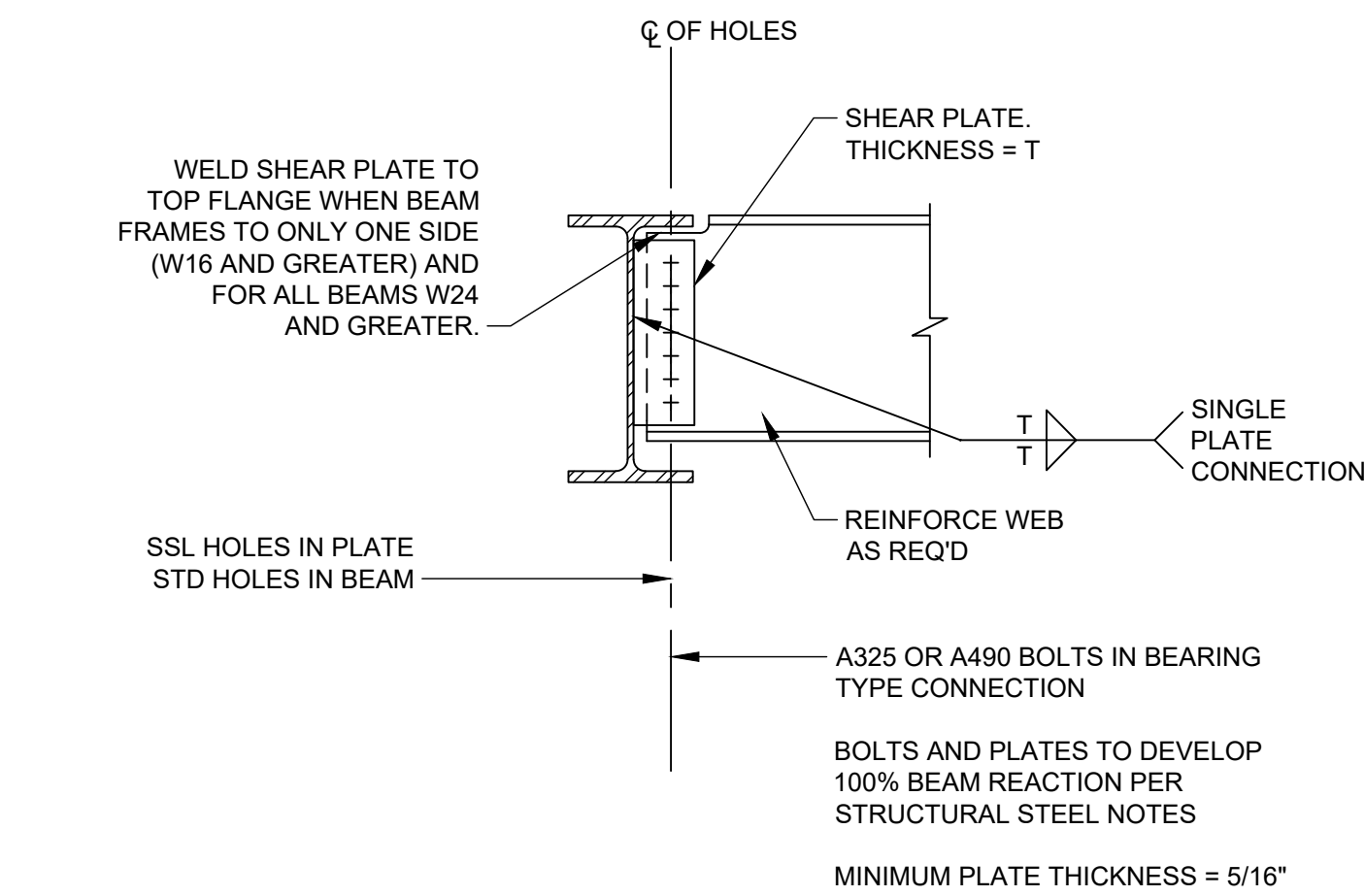
TYP. COLUMN OR POST SUPPORTED ON BEAM

SCALE: 1 1/2" = 1'-0"



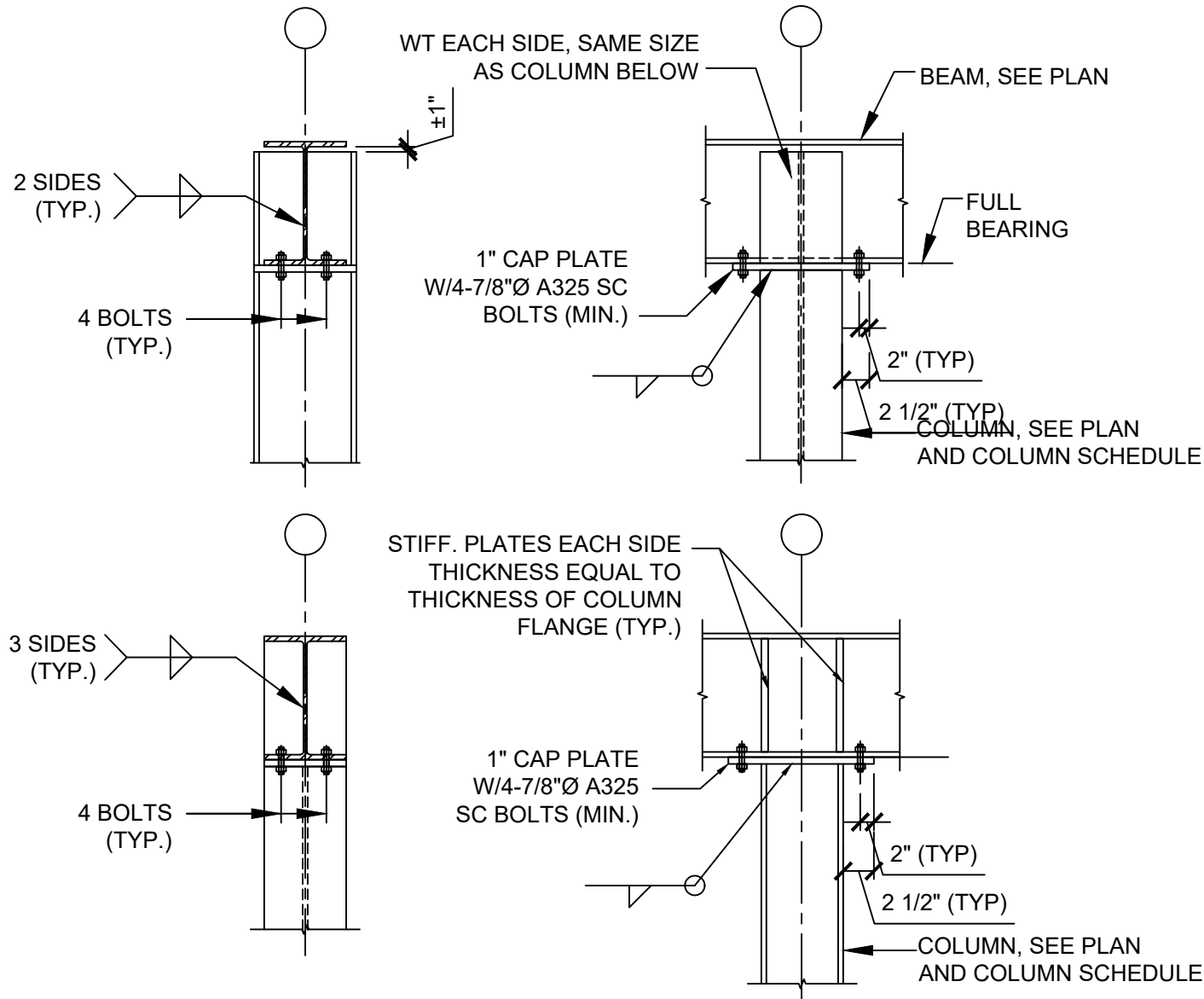
TYP. BEAM TO COLUMN FLANGE SHEAR CONNECTION

SCALE: 3/4" = 1'-0"



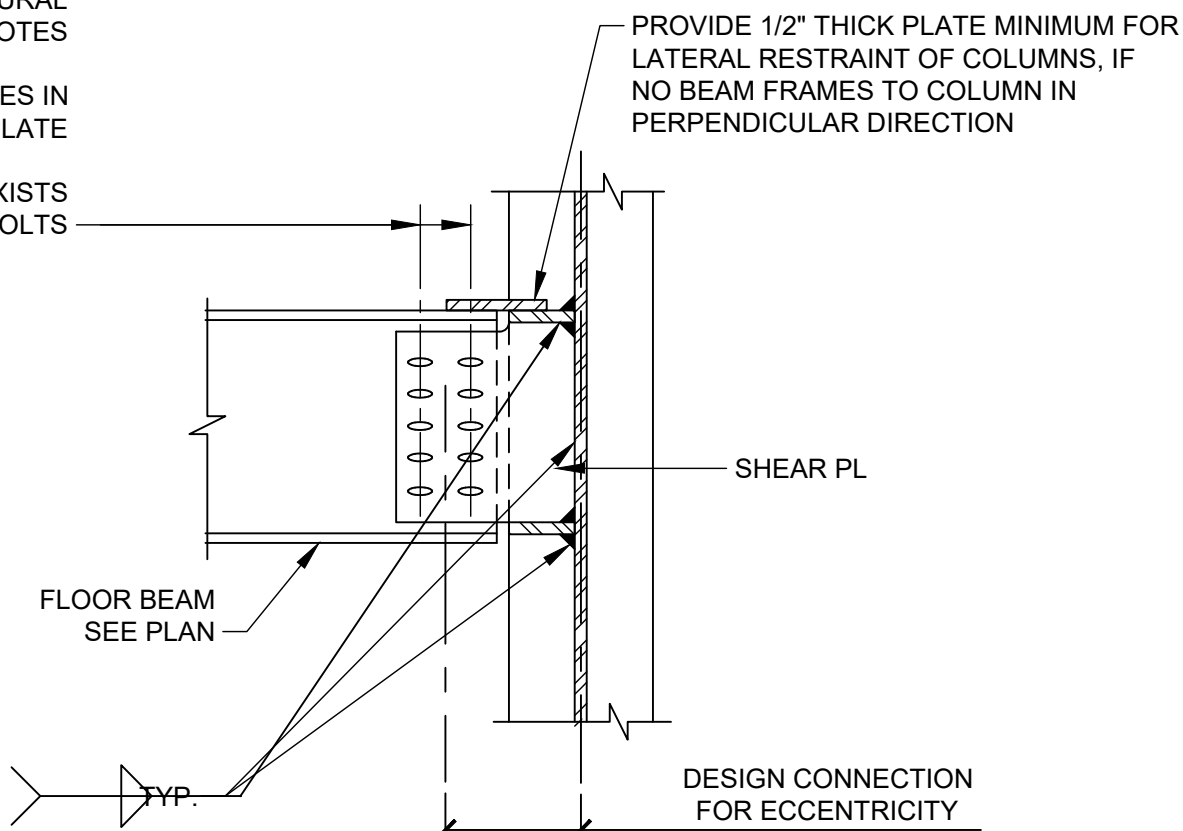
TYP. BEAM TO GIRDER CONNECTION

SCALE: 3/4" = 1'-0"



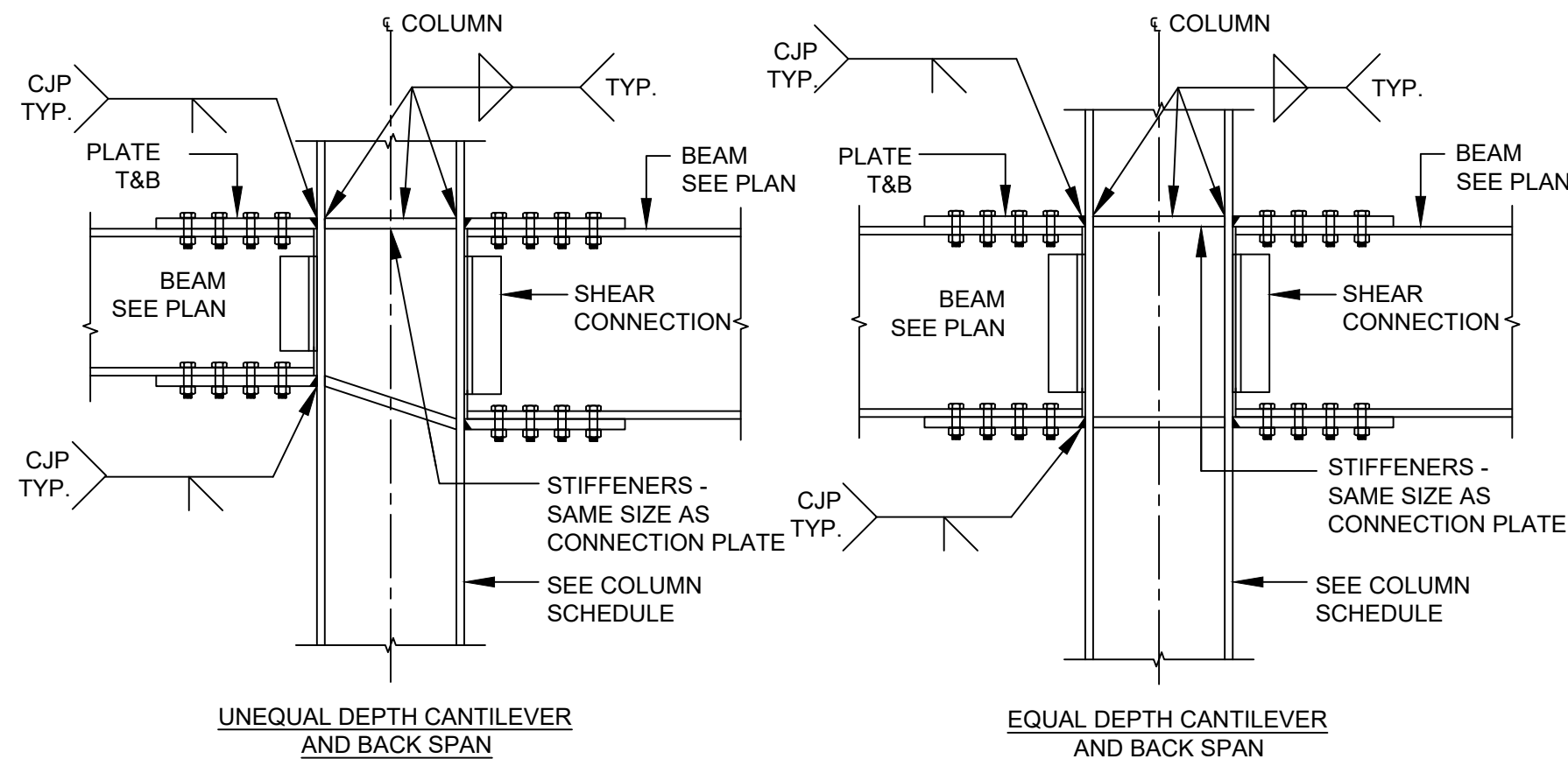
TYP. BEAM SUPPORTED OVER COLUMN

SCALE: 1 1/2" = 1'-0"



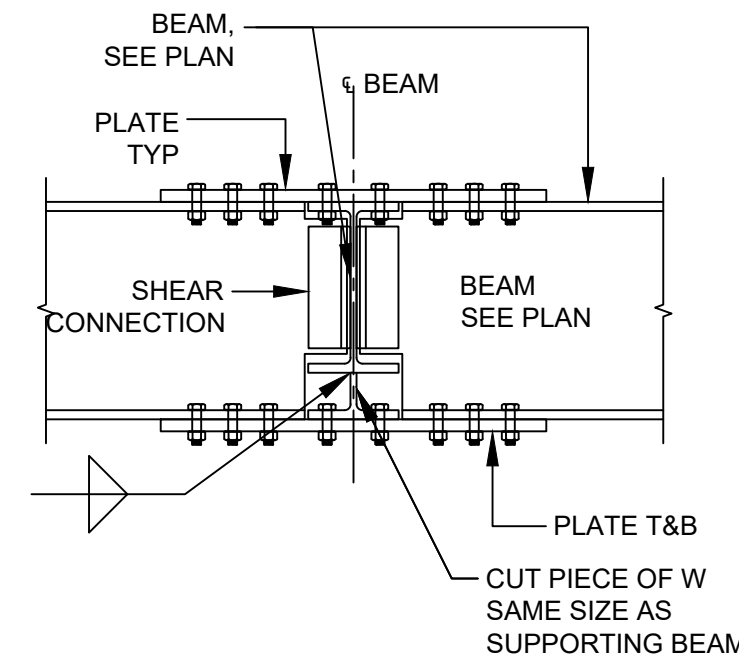
TYP. BEAM TO COLUMN WEB SHEAR CONNECTION

SCALE: 3/4" = 1'-0"

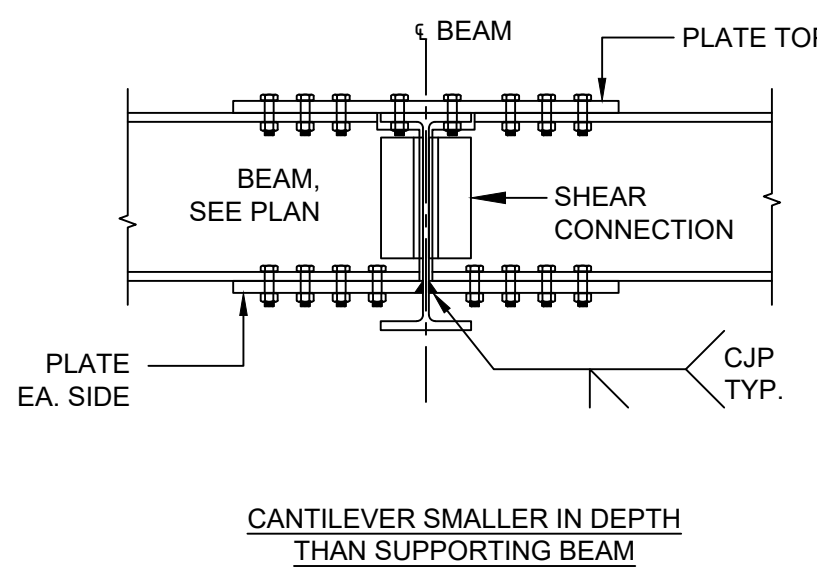


UNEQUAL DEPTH CANTILEVER AND BACK SPAN

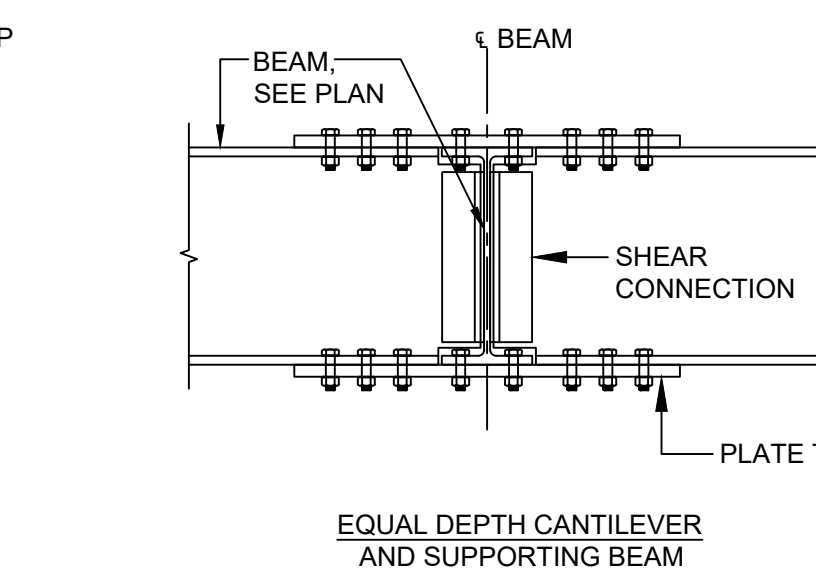
EQUAL DEPTH CANTILEVER AND BACK SPAN



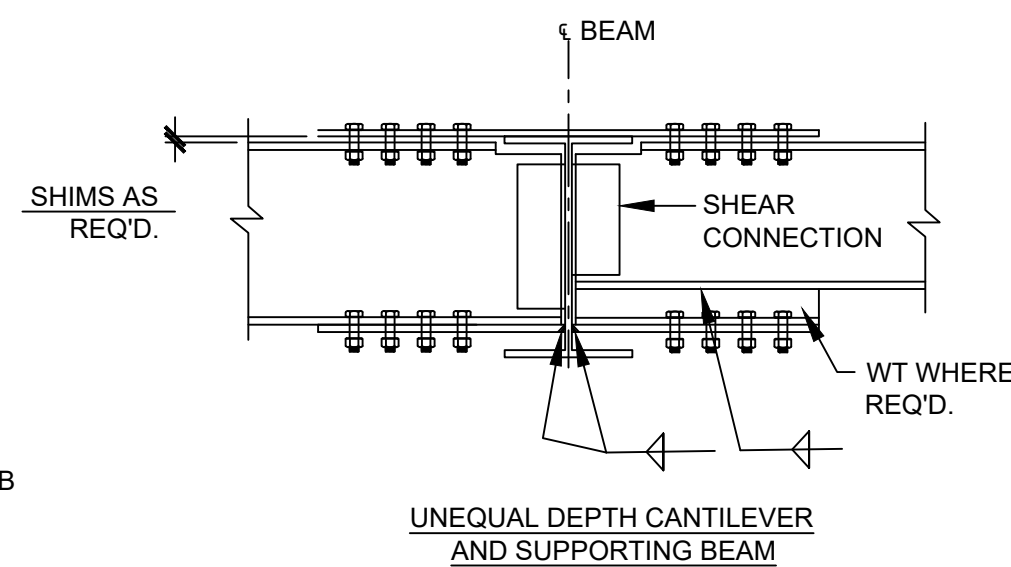
CANTILEVE LARGER IN DEPTH THAN SUPPORTING BEAM



CANTILEVER SMALLER IN DEPTH THAN SUPPORTING BEAM



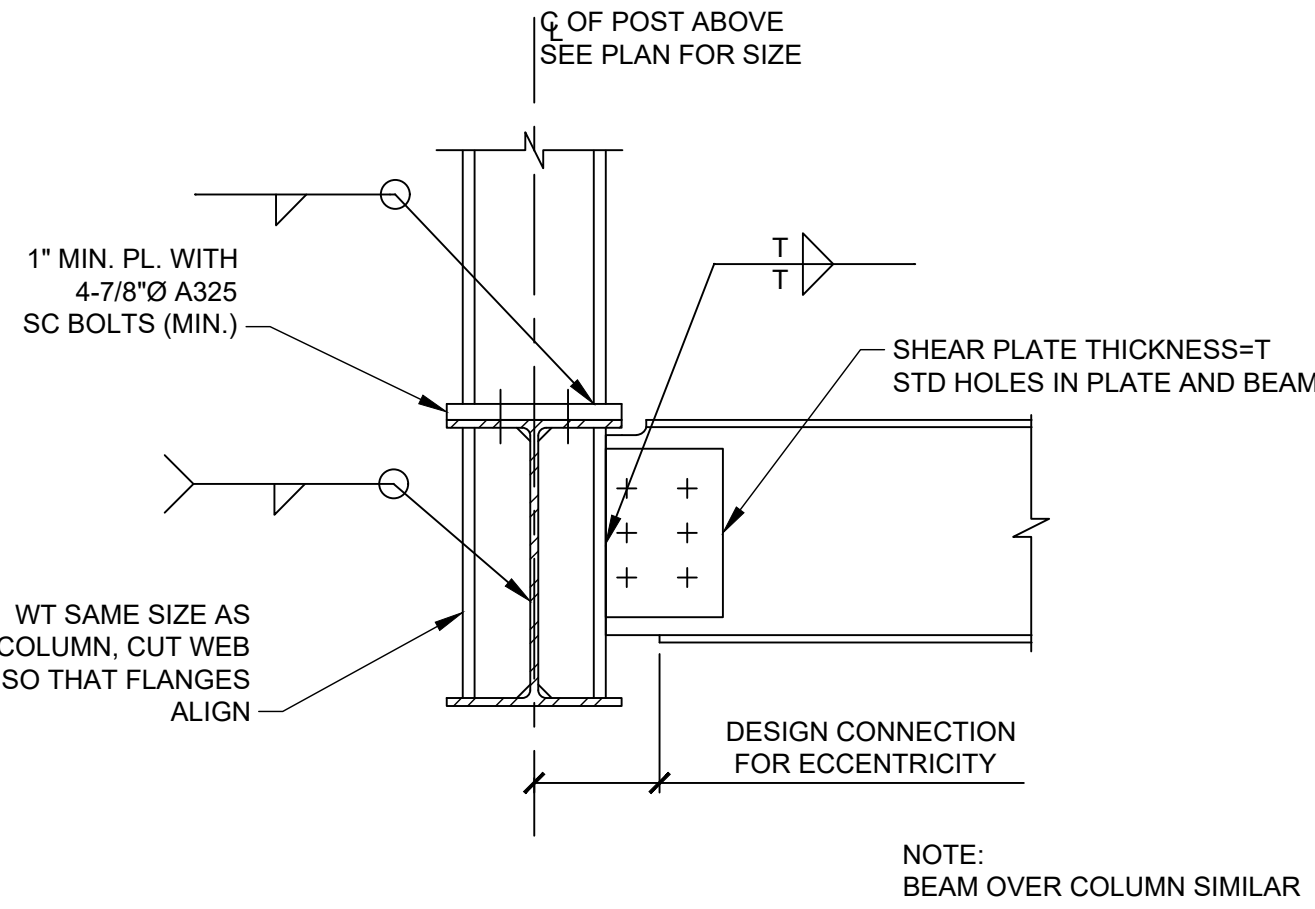
EQUAL DEPTH CANTILEVER AND SUPPORTING BEAM



UNEQUAL DEPTH CANTILEVER AND SUPPORTING BEAM

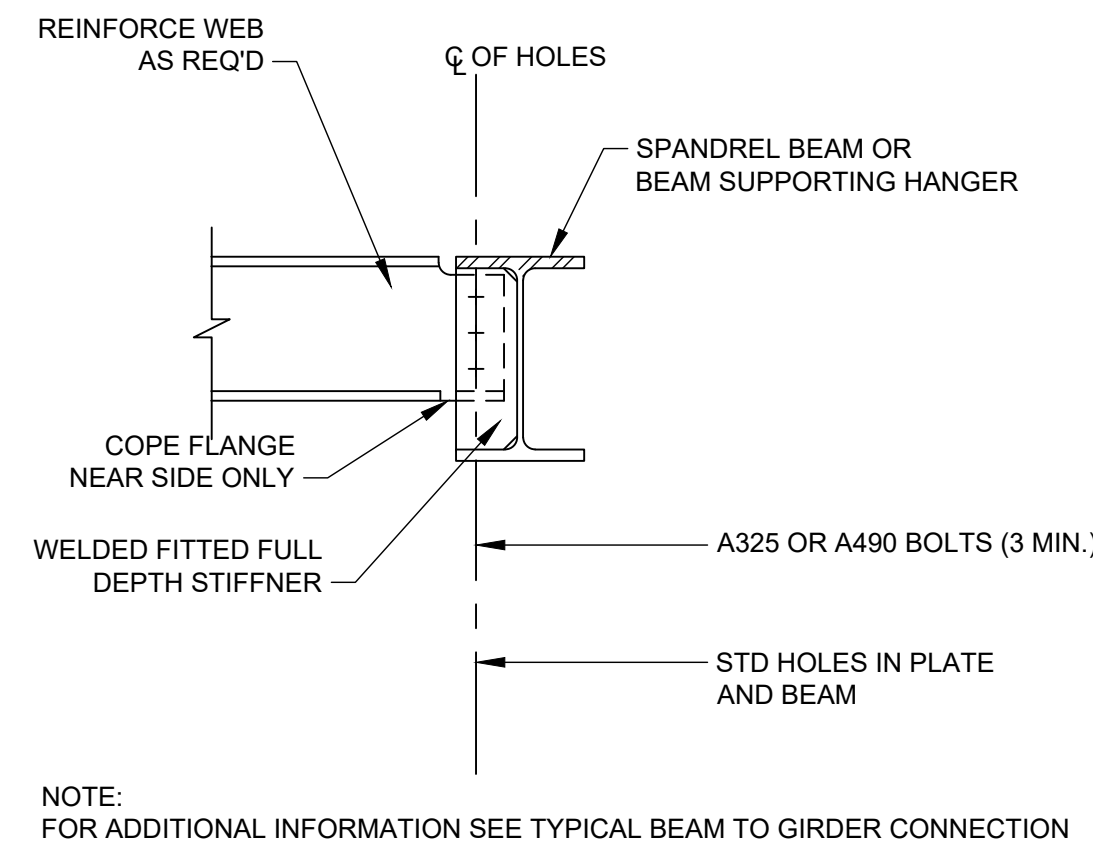
NOTES:

1. MOMENT CONNECTIONS ARE TO DEVELOP FULL MOMENT CAPACITY OF CANTILEVER BEAMS UNLESS SHOWN OTHERWISE ON PLAN.
2. WELD, BOLT AND CONNECTION PLATE CAPACITIES SHALL BE DESIGNED AS PER AISC SPECIFICATIONS.
3. CONNECTION PLATES AND BOLTS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN NY STATE. CALCULATIONS SHALL BE SUBMITTED FOR THE ENGINEER OF RECORD'S REVIEW.
4. FIELD WELDED CONNECTIONS MAY BE USED IF REQUESTED BY CONTRACTOR AND APPROVED BY ENGINEER OF RECORD.
5. CJP INDICATES COMPLETE JOINT PENETRATION GROOVE WELDING.
6. FOR CLARITY, FIREPROOFING IS NOT INDICATED.
7. REFER TO PLANS FOR LOCATION OF CANTILEVERED BEAMS.
8. PROVIDE DOUBLE PLATE @ COLUMN B-1 & C-1 AT SECOND FLOOR AND THIRD FLOOR LEVEL.



TYP. COLUMN SUPPORT WITH WEB PERPENDICULAR TO SUPPORT BEAM

SCALE: 3/4" = 1'-0"



TYP. END CONNECTION AT BEAMS SUPPORTING STAIR HANGERS AT SPANDRELS

SCALE: 3/4" = 1'-0"

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CHECKED BY : P. BRUNO

APPROVED BY : P. BRUNO

DATE : 9/10/2021

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STEEL BEAM AND COLUMN
DETAILS

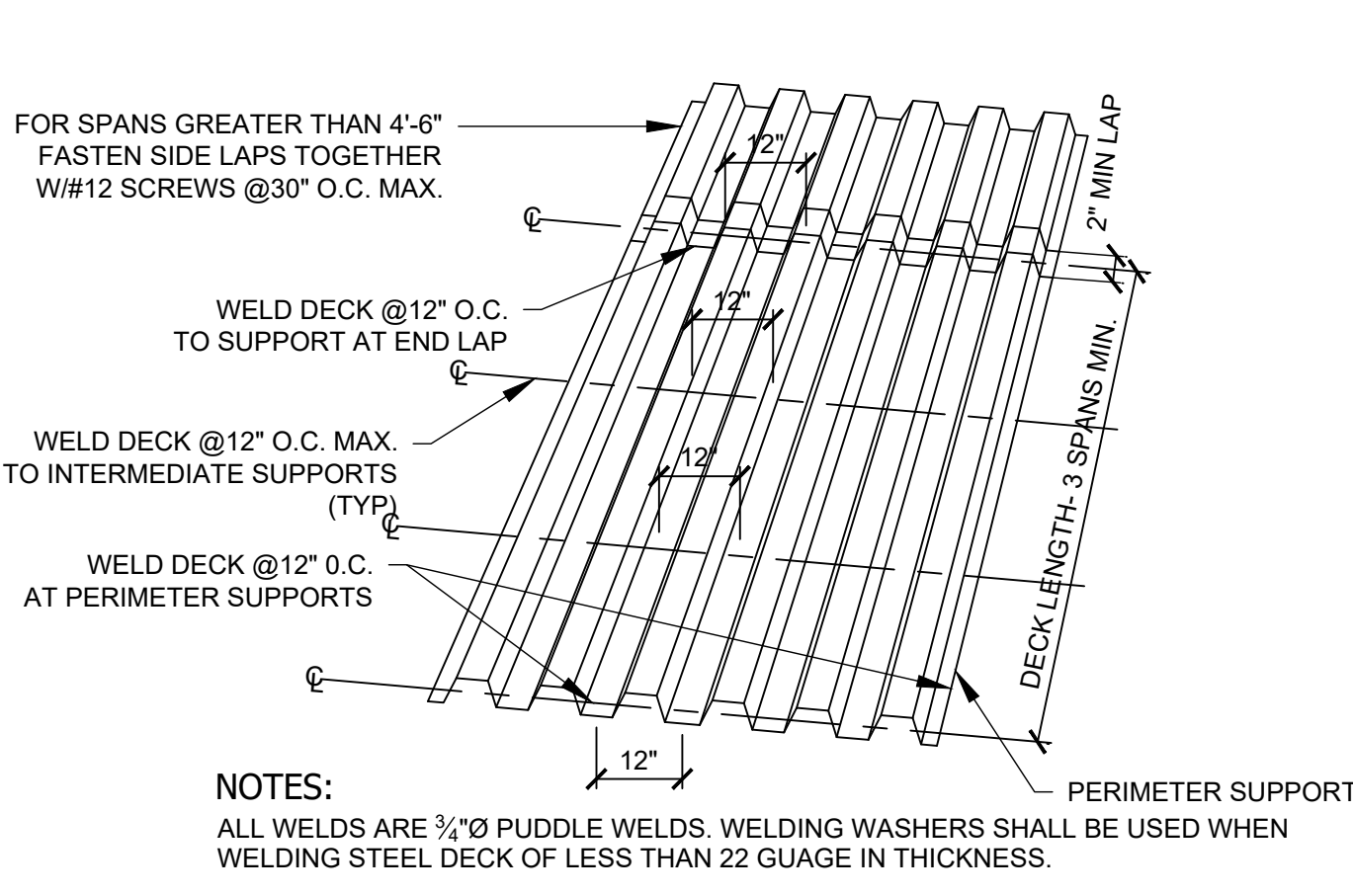


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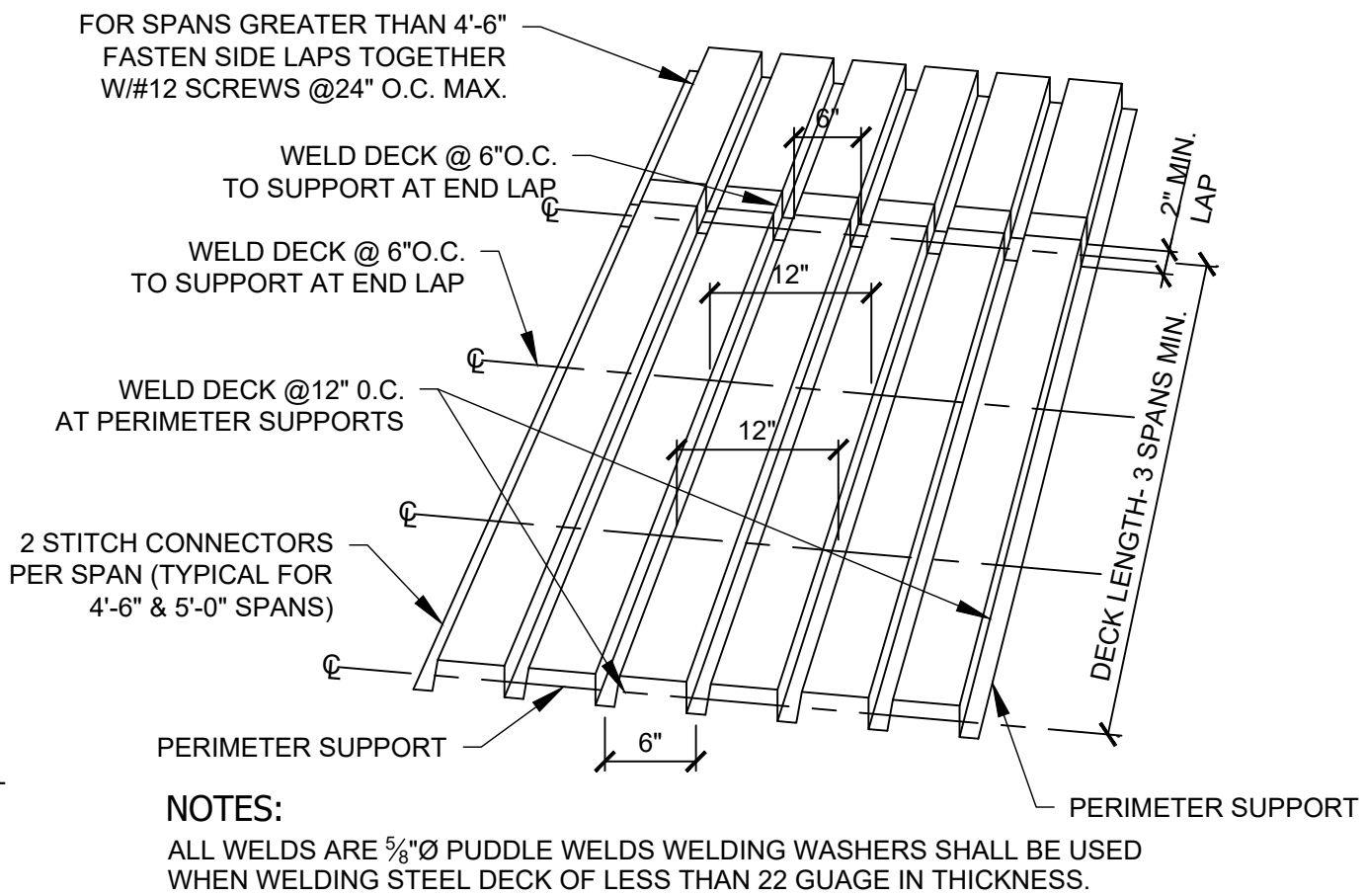
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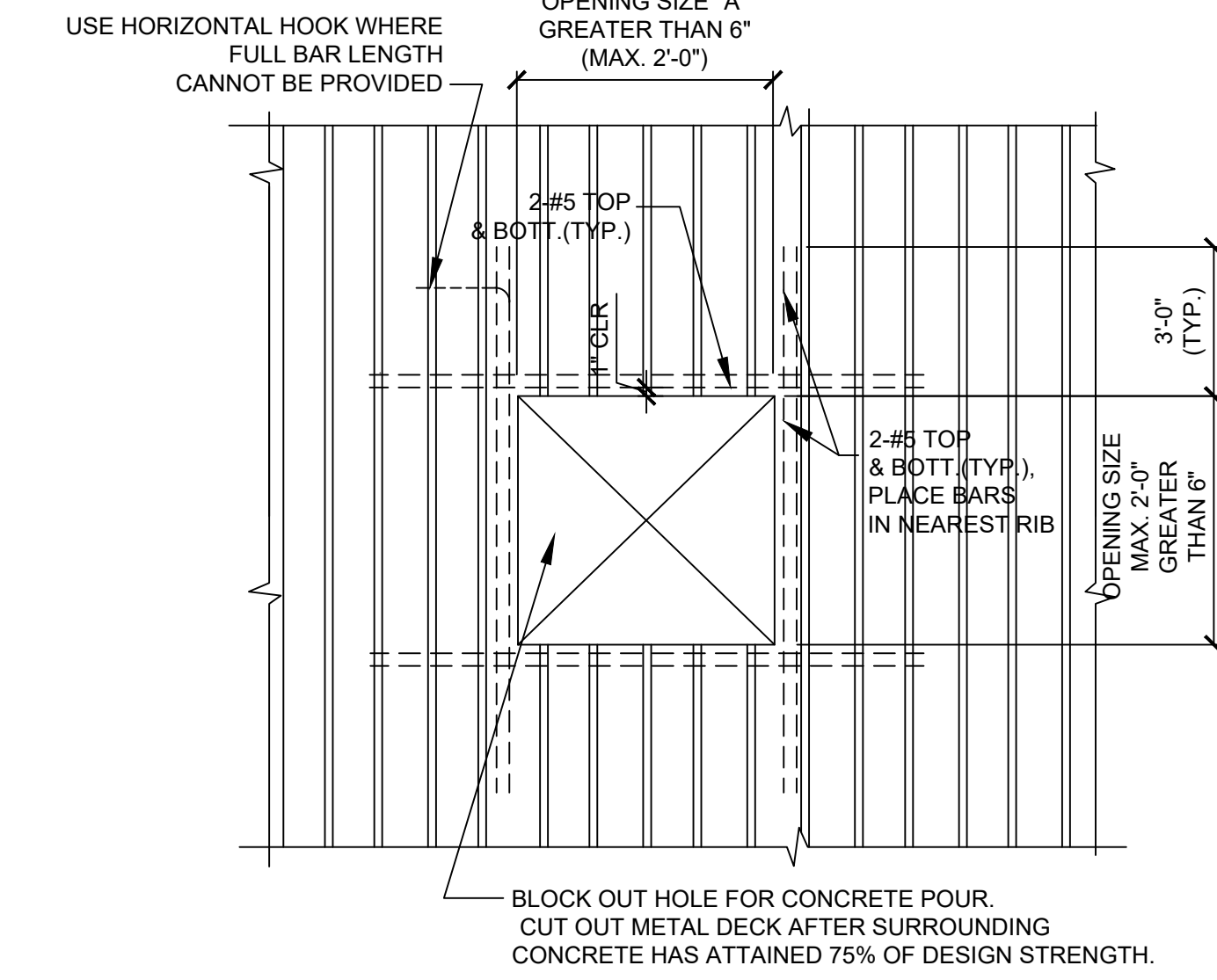
20 OF 61



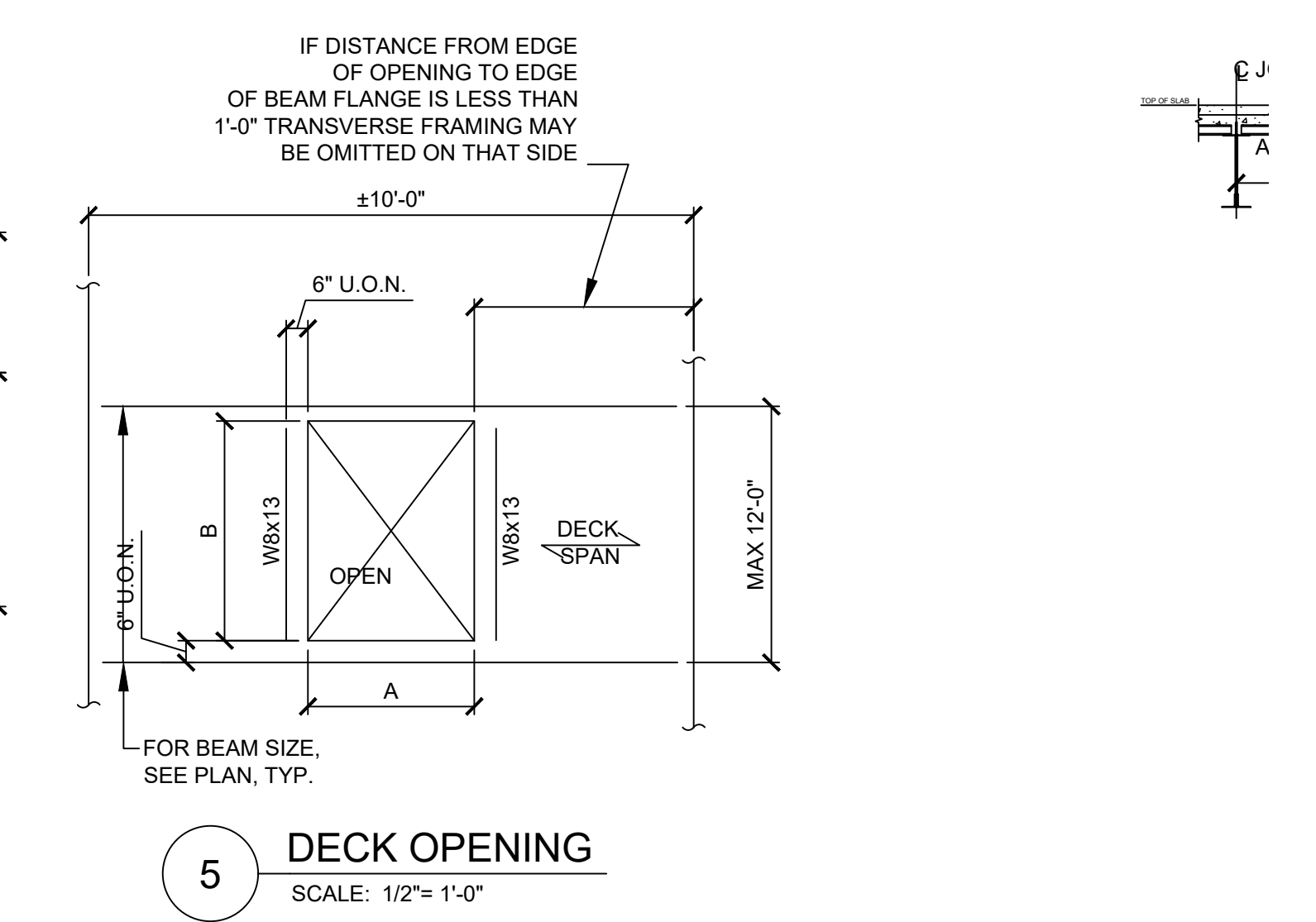
1 TYP. SLAB DECK WELDING
SCALE: 3/4"= 1'-0"



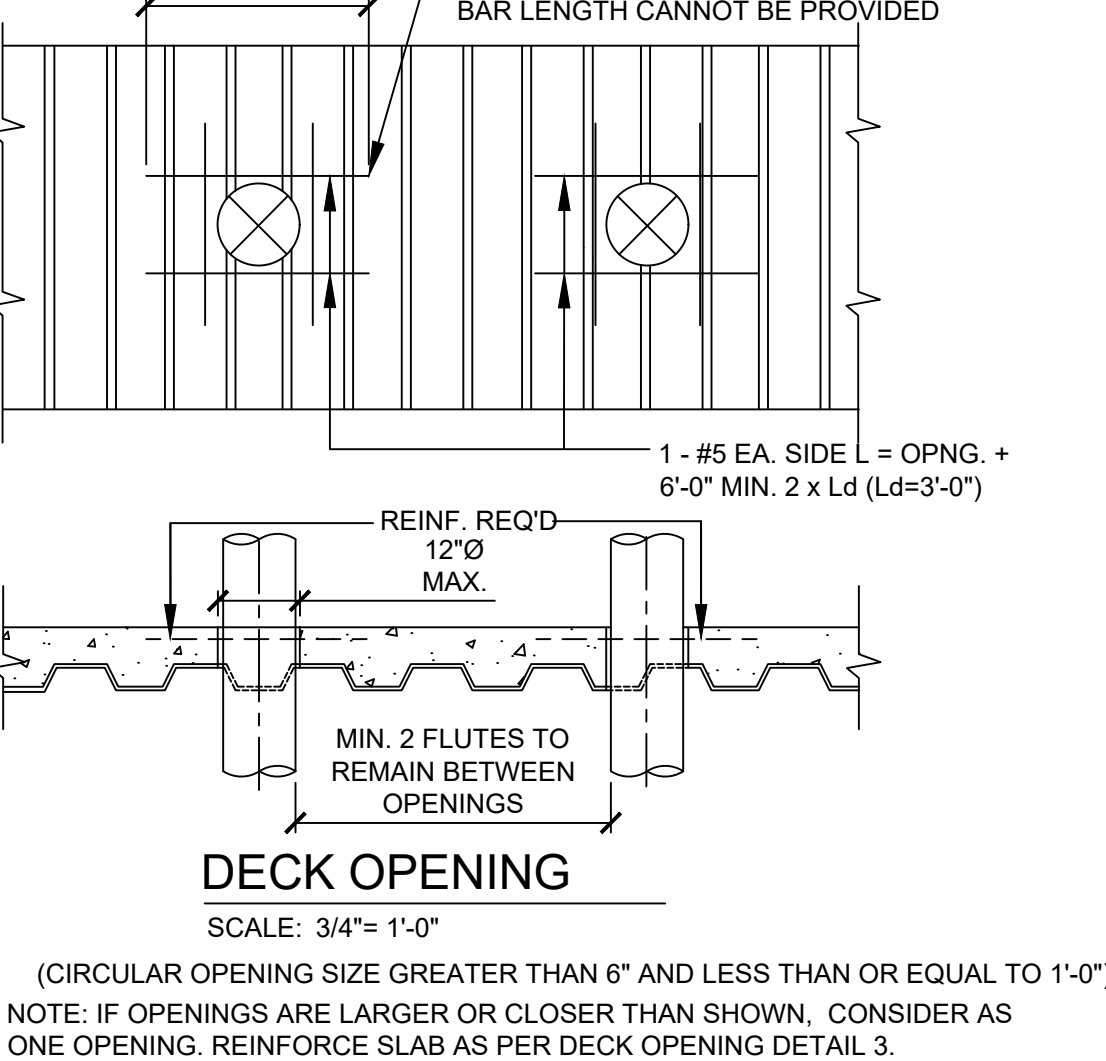
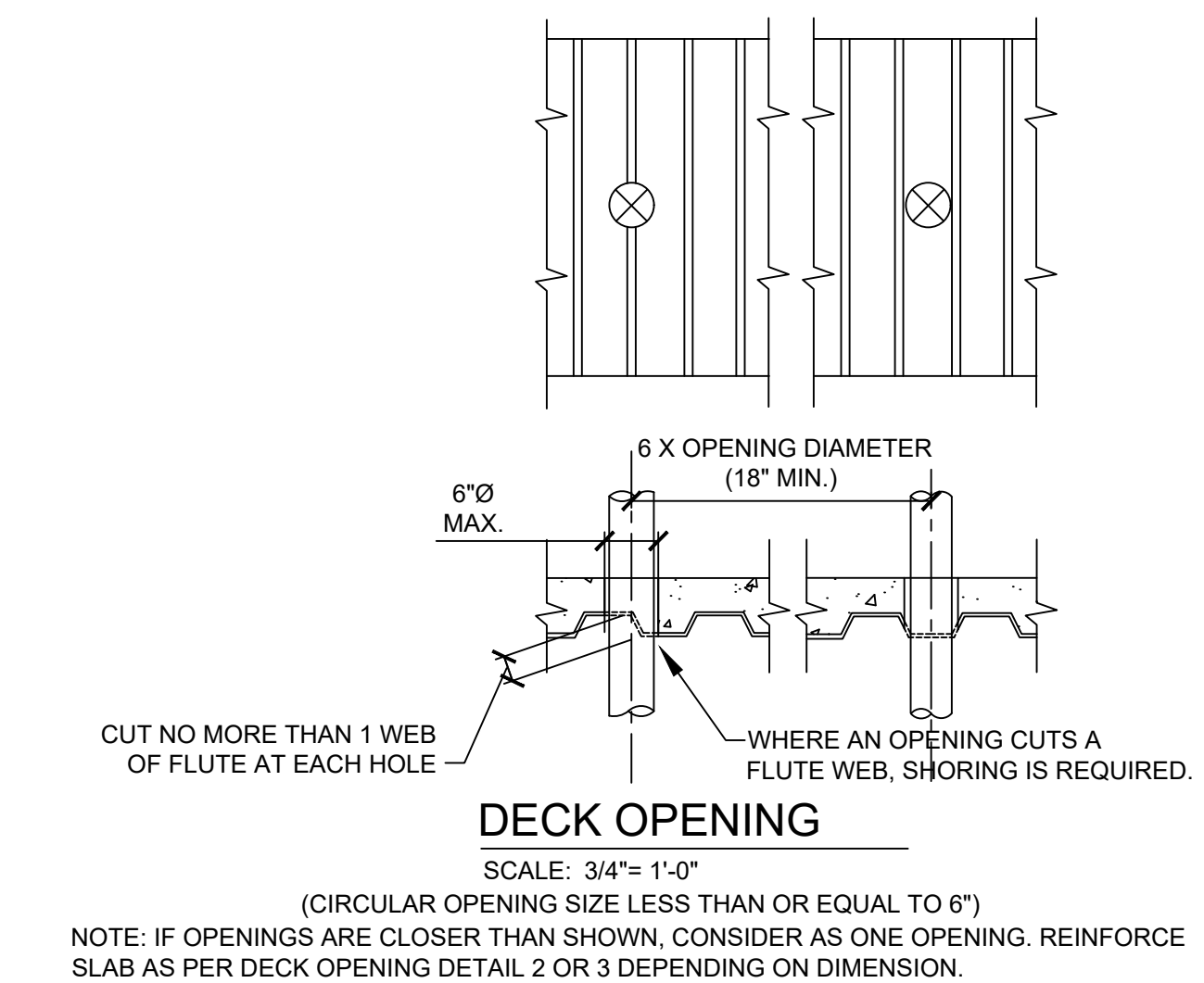
2 TYP. SLAB ROOF WELDING
SCALE: 3/4"= 1'-0"



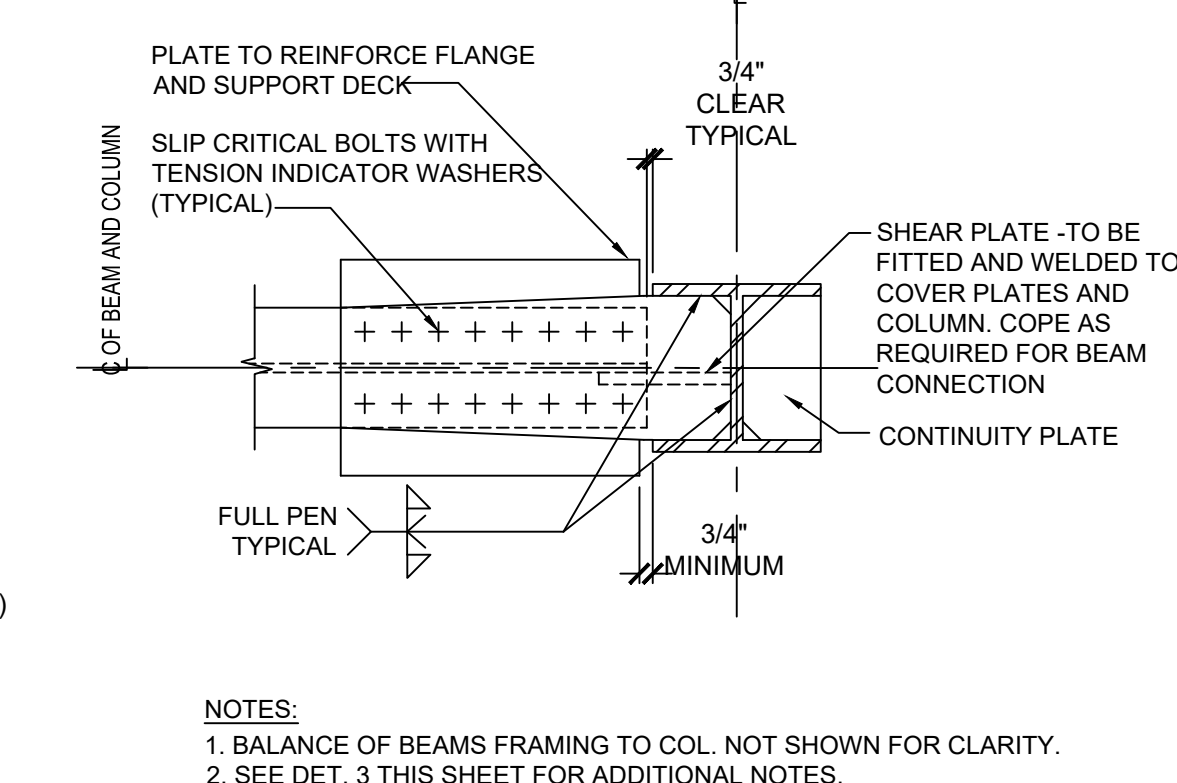
4 DECK OPENING
SCALE: 1/2"= 1'-0"



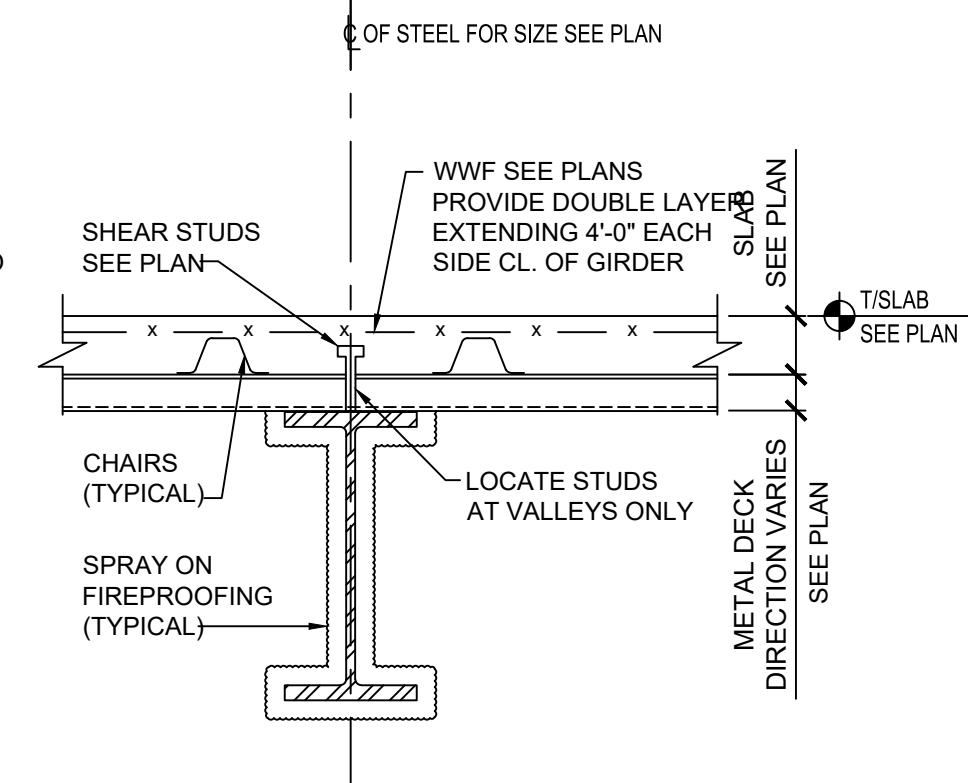
5 DECK OPENING
SCALE: 1/2"= 1'-0"



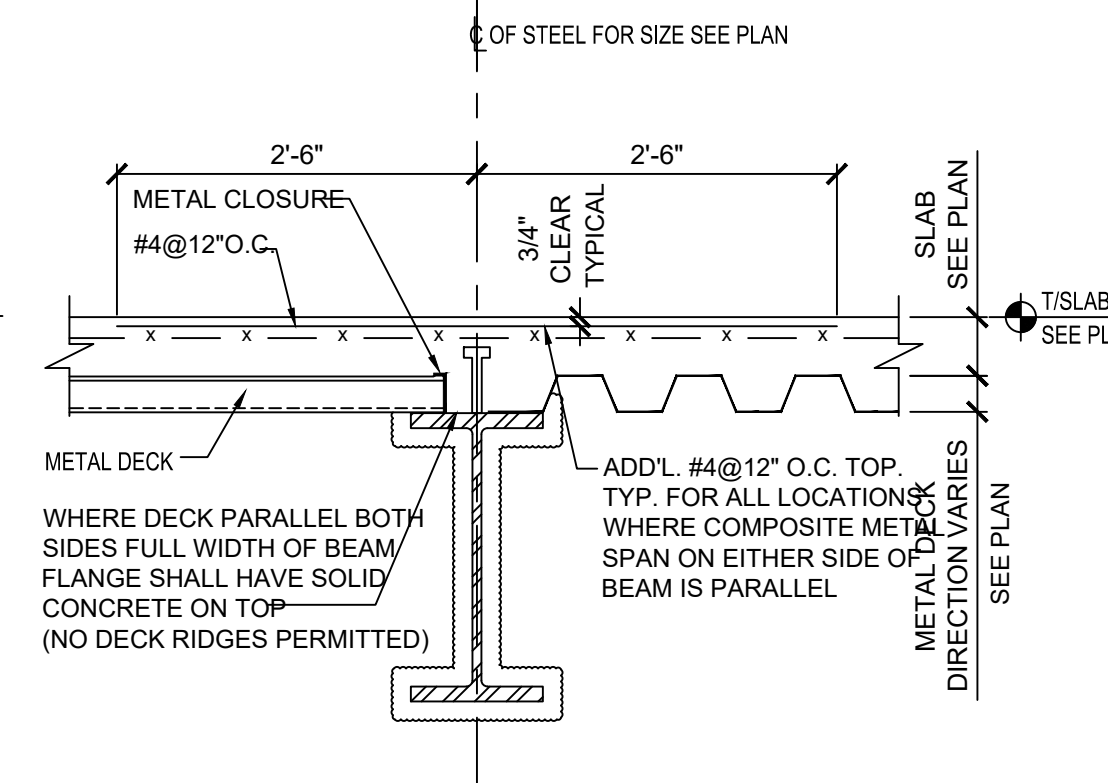
3 TYP. METAL DECK OPENING
SCALE: 3/4"= 1'-0"



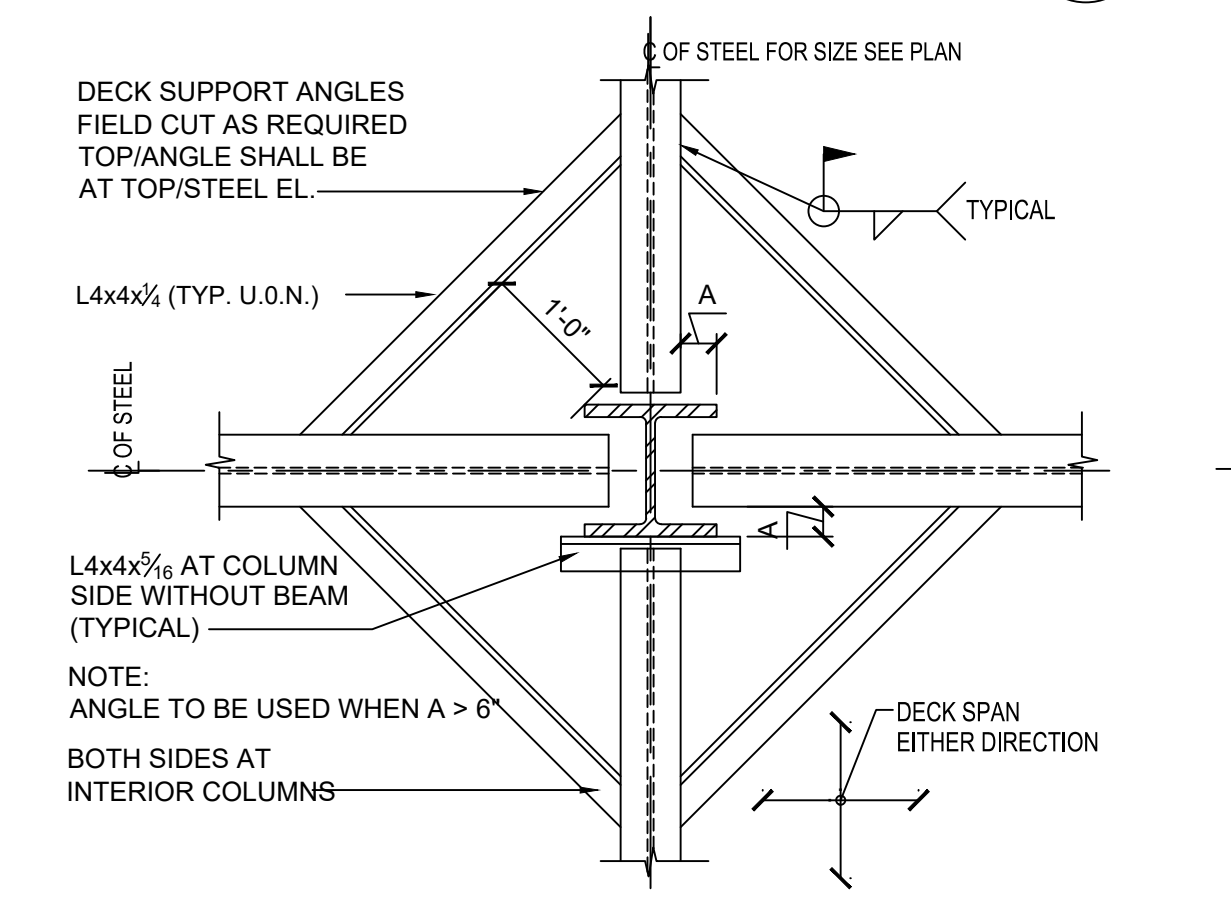
5 TYPICAL DECK SUPPORT AT BEAM MOMENT CONNECTION TO COLUMN WEB
SCALE: 3/4"=1'-0"



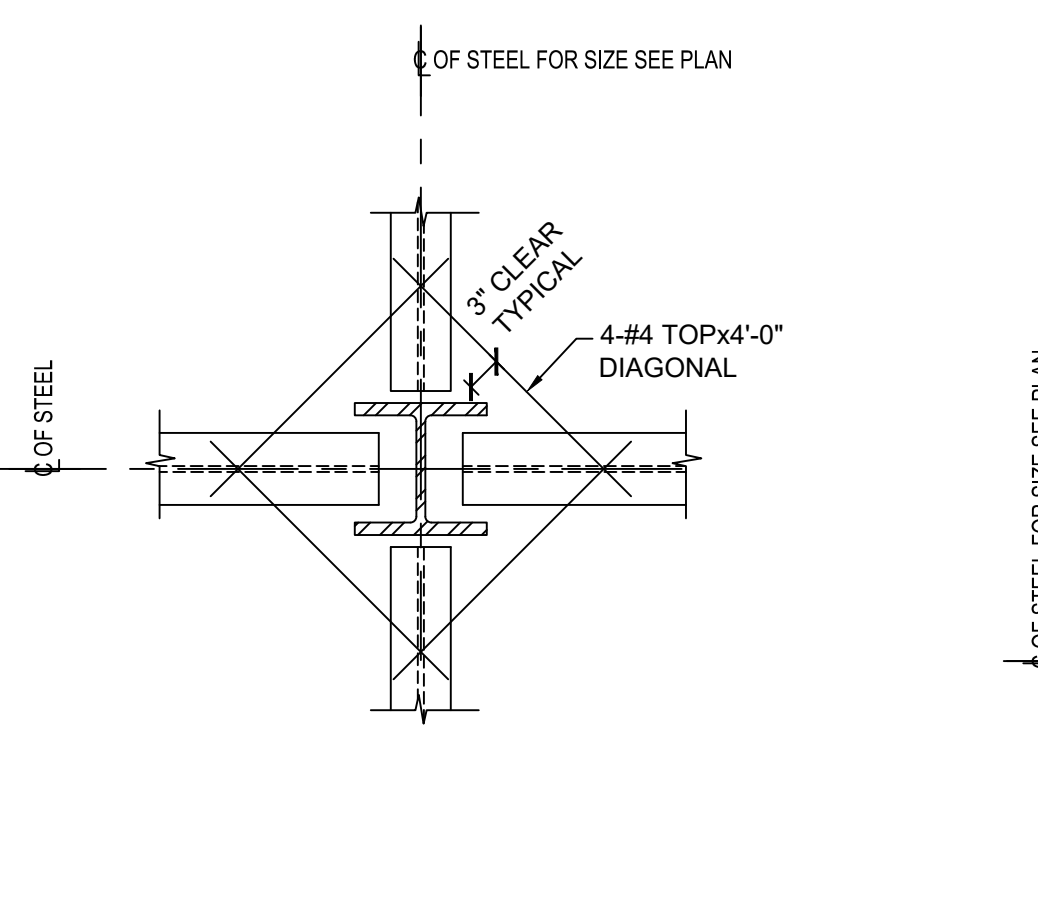
10 TYPICAL FLOOR FRAMING
SCALE: 3/4"=1'-0"



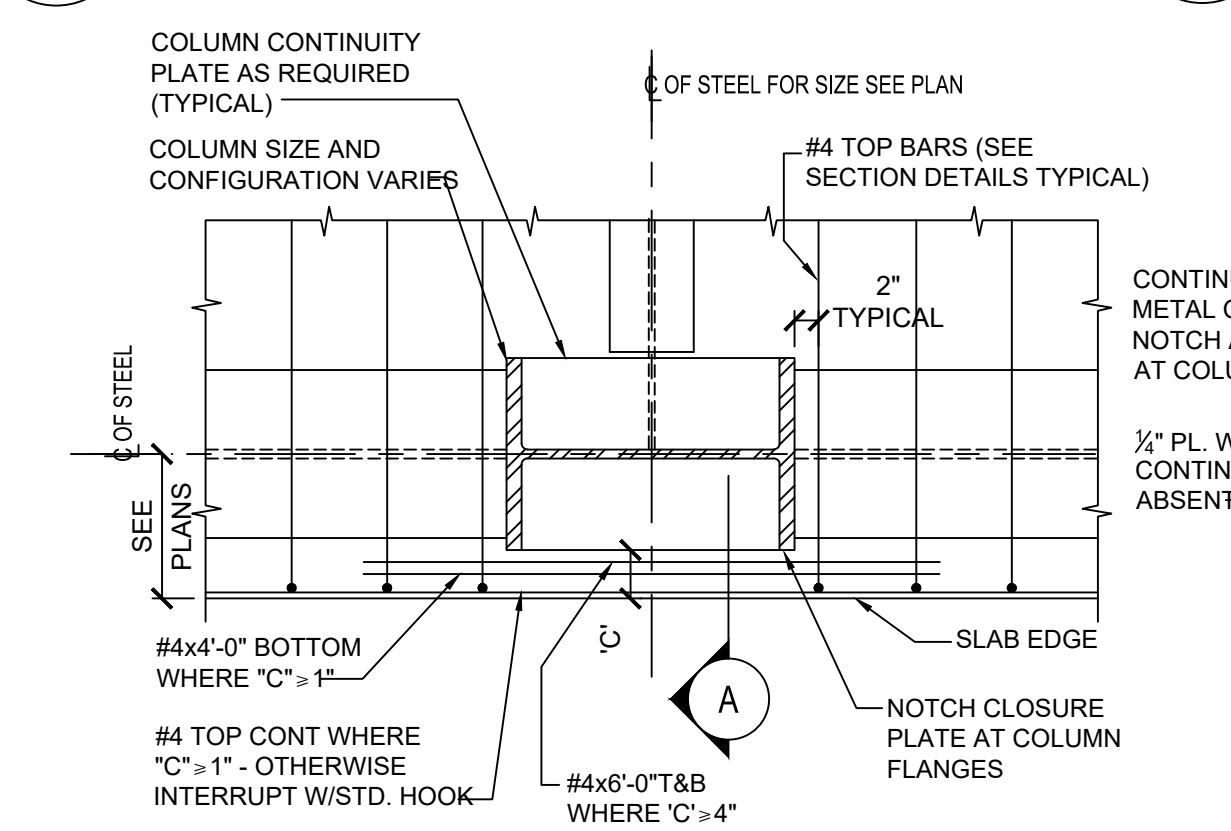
11 TYPICAL REINFORCING OVER GIRDERS OR DIRECTION CHANGE OF METAL DECK
SCALE: 3/4"=1'-0"



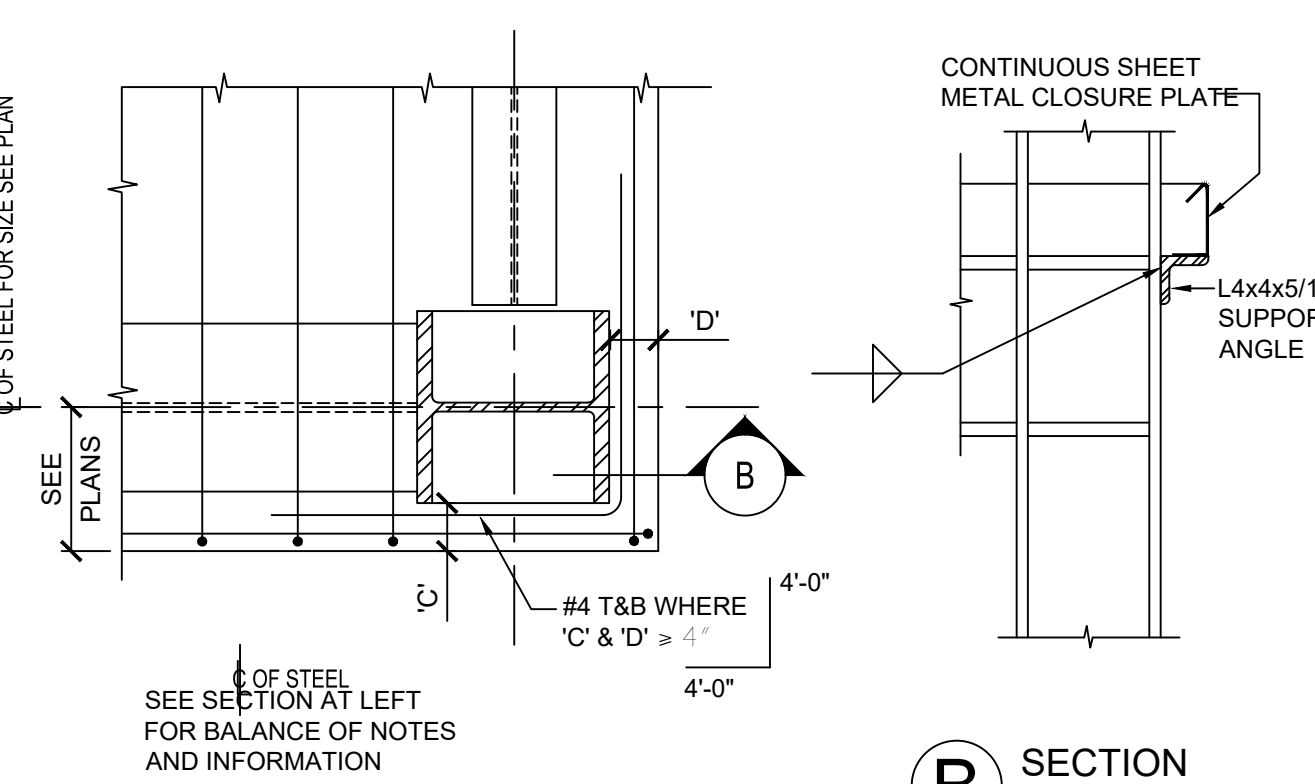
3 TYPICAL DECK SUPPORT AT INTERIOR OR EXTERIOR COLUMNS
SCALE: 3/4"=1'-0"



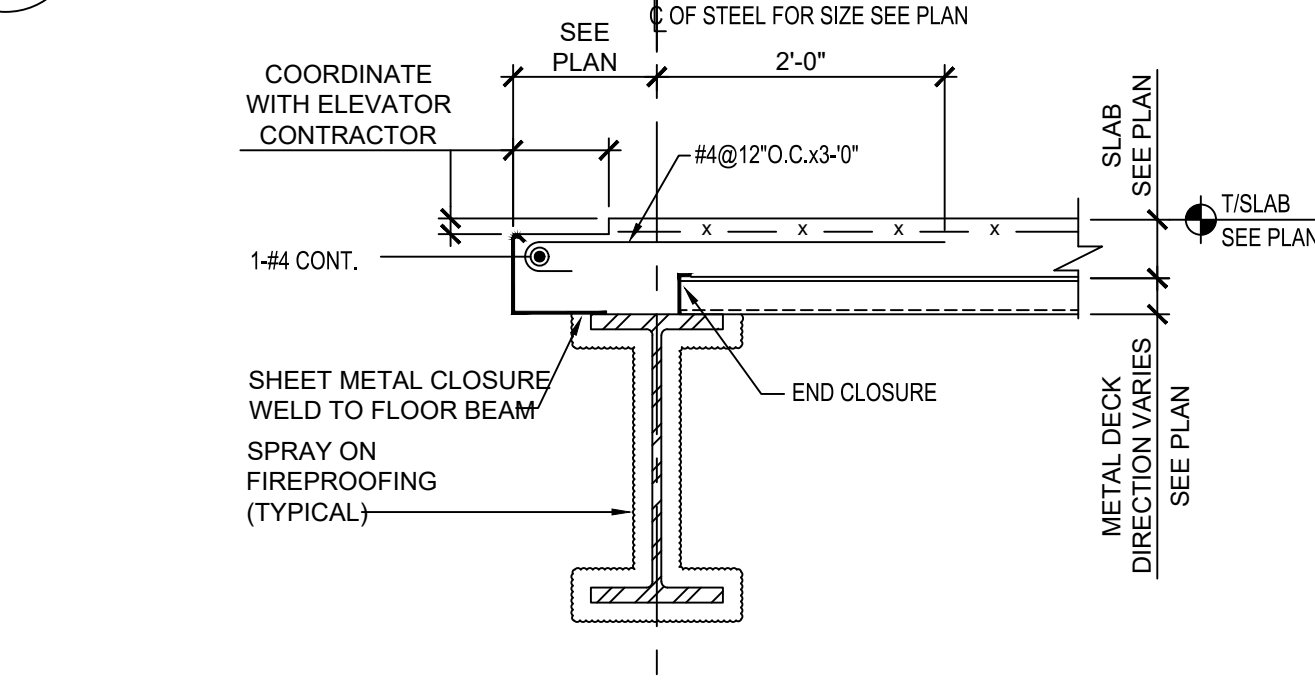
4 TYPICAL SLAB REINFORCING AT INTERIOR COLUMNS
SCALE: 3/4"=1'-0"



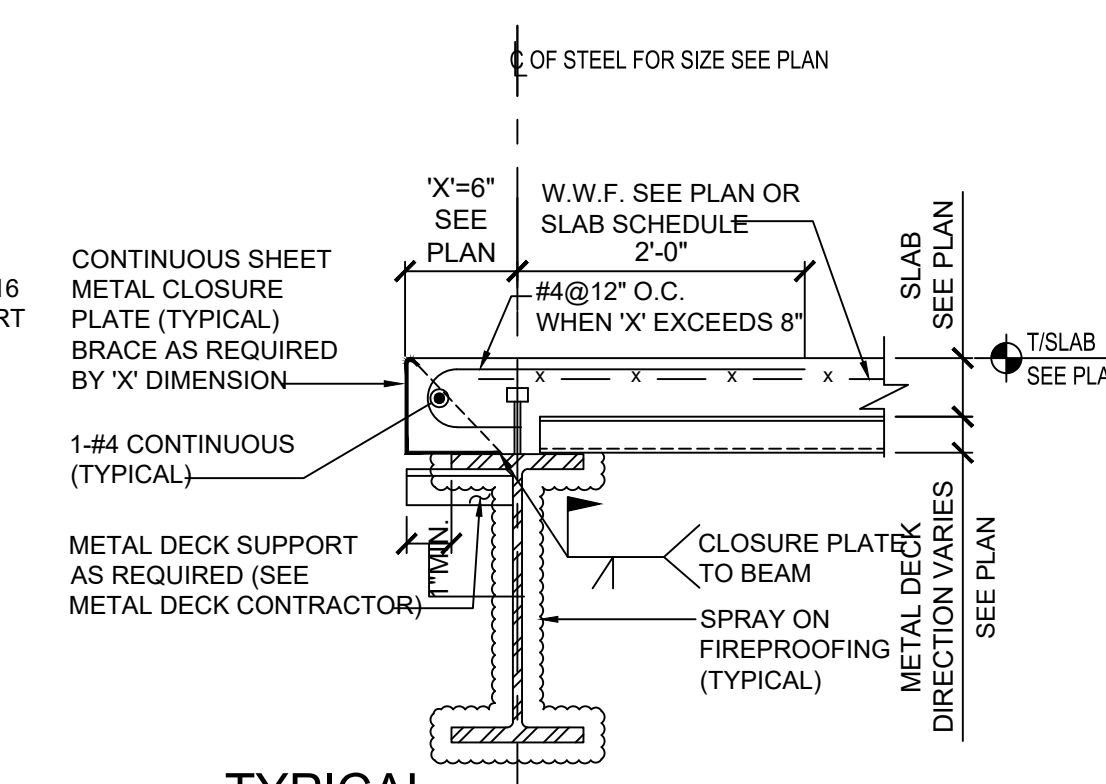
8 TYPICAL SLAB REINFORCING AT EXTERIOR COLUMNS
SCALE: 3/4"=1'-0"



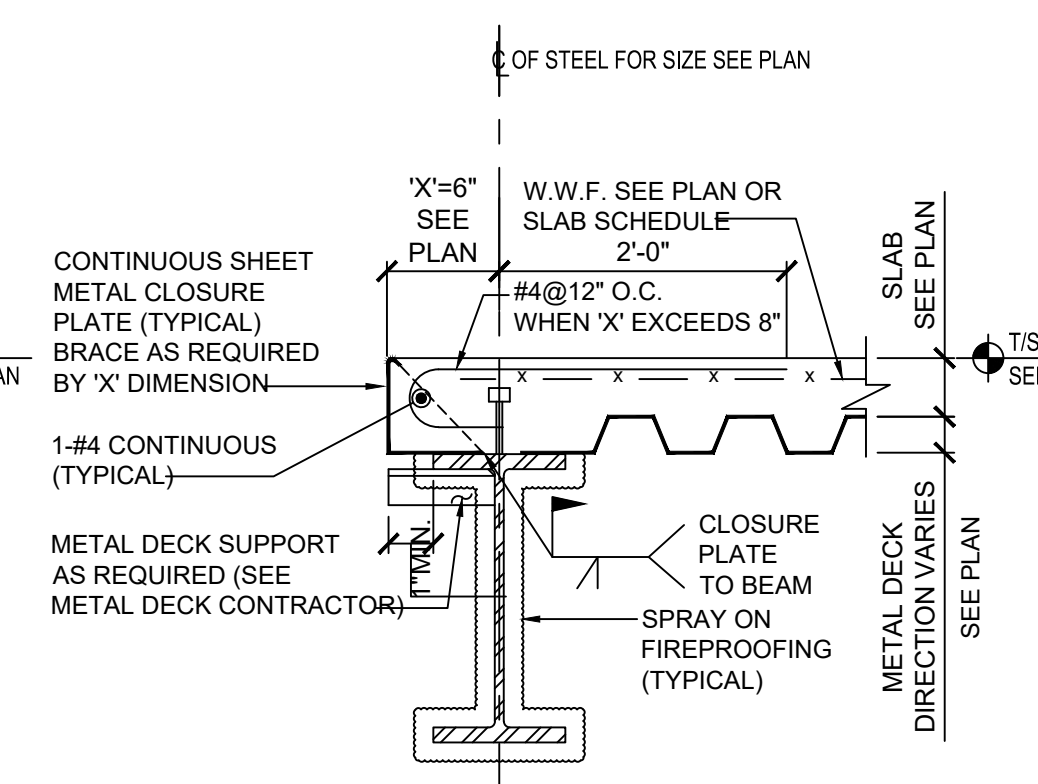
8 TYPICAL SLAB REINFORCING AT CORNER COLUMNS
SCALE: 3/4"=1'-0"



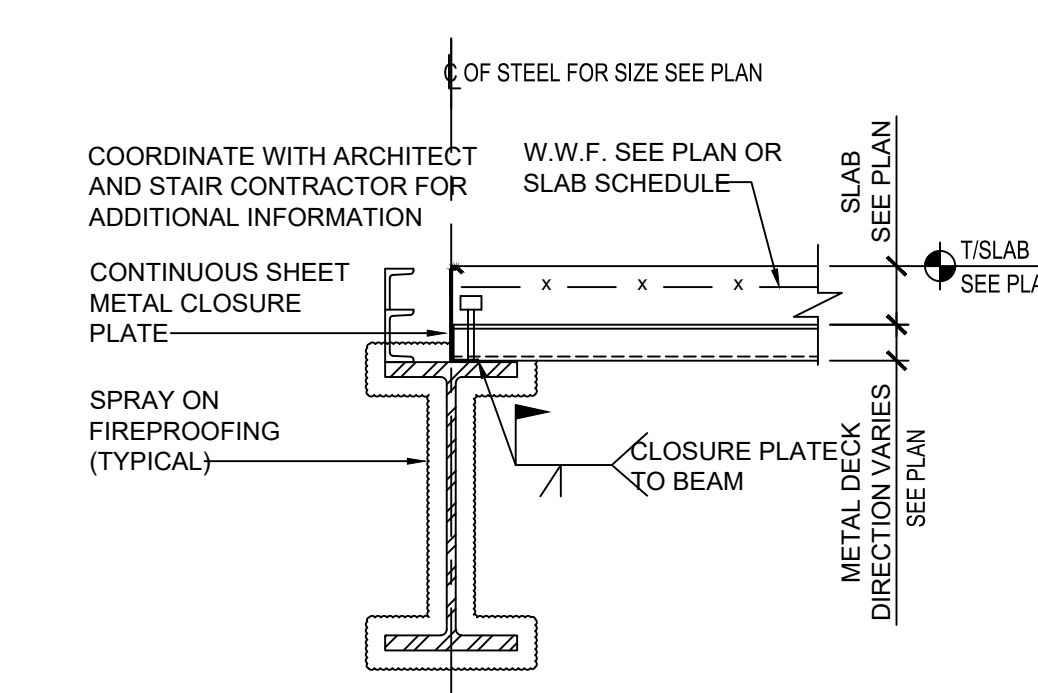
12 TYPICAL SLAB SLAB EDGE AT ELEVATOR SILL
SCALE: 3/4"=1'-0"



15 TYPICAL SLAB EDGE AT INTERIOR OPENINGS
SCALE: 3/4"=1'-0"



16 TYPICAL SLAB EDGE
SCALE: 3/4"=1'-0"



17 TYPICAL SLAB SLAB EDGE AT STAIR LANDING
SCALE: 3/4"=1'-0"

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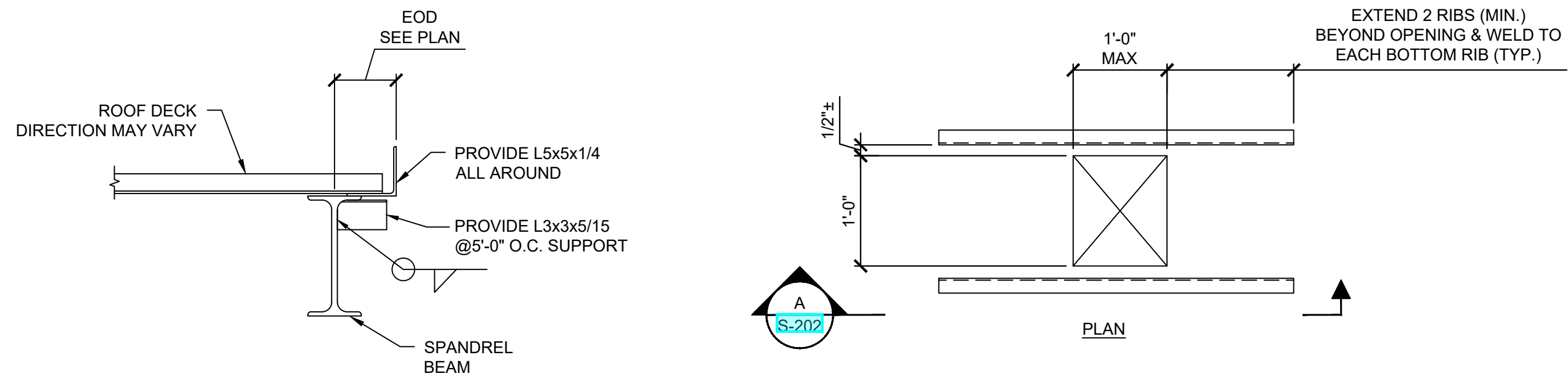
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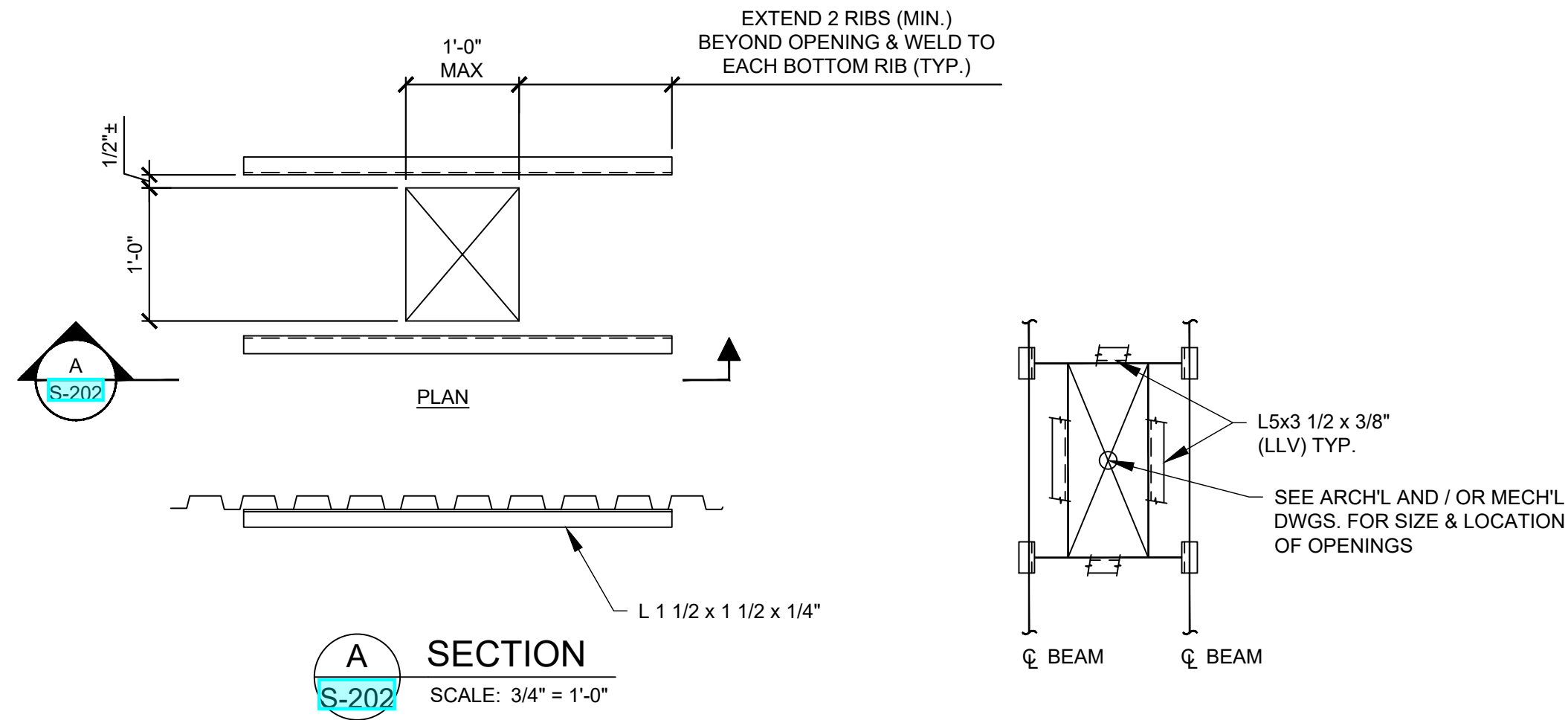
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METAL DECK DETAILS

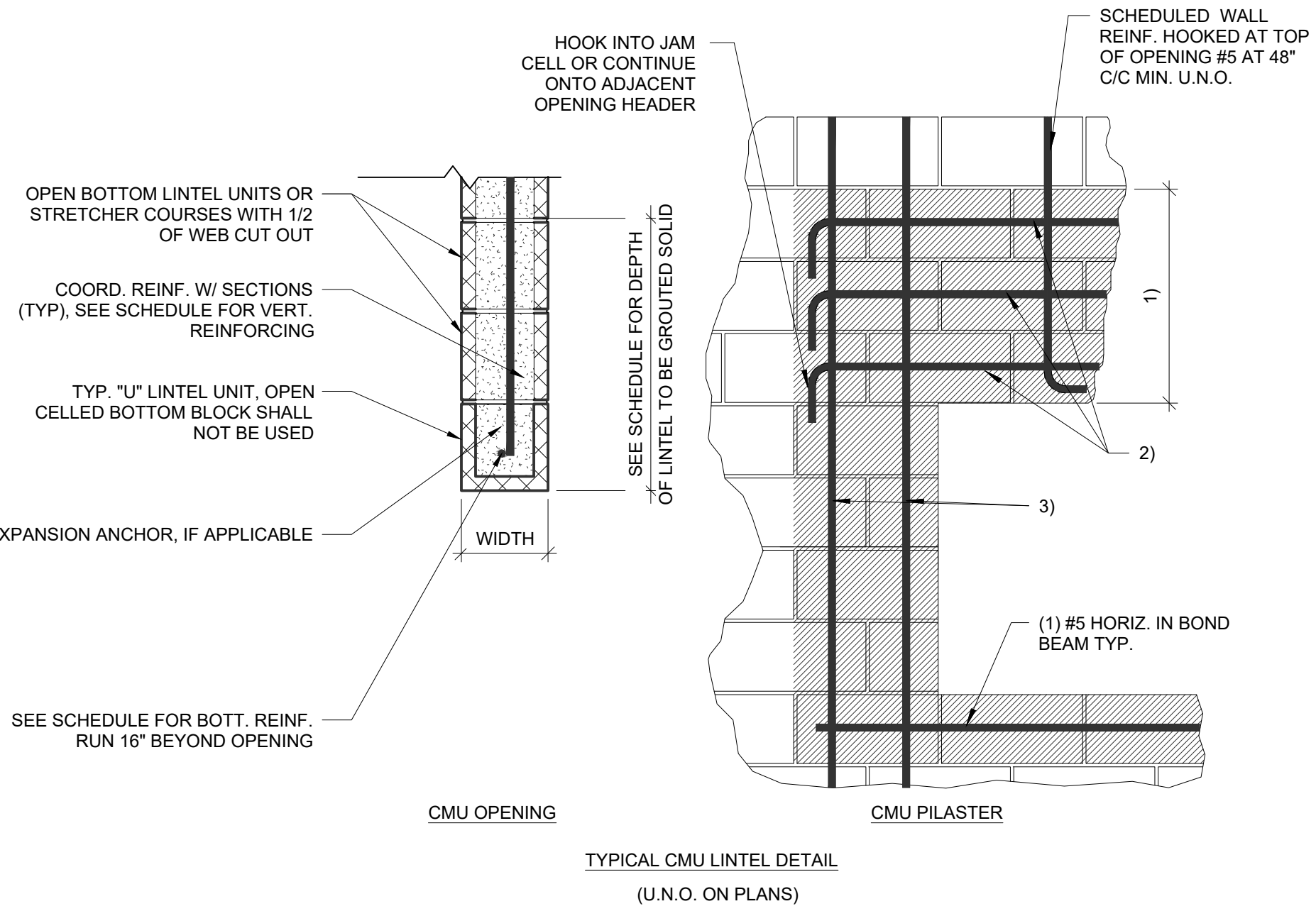
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1A TYP. PERIMETER BEAM DETAIL @ ROOF
SCALE: 1"= 1'-0"

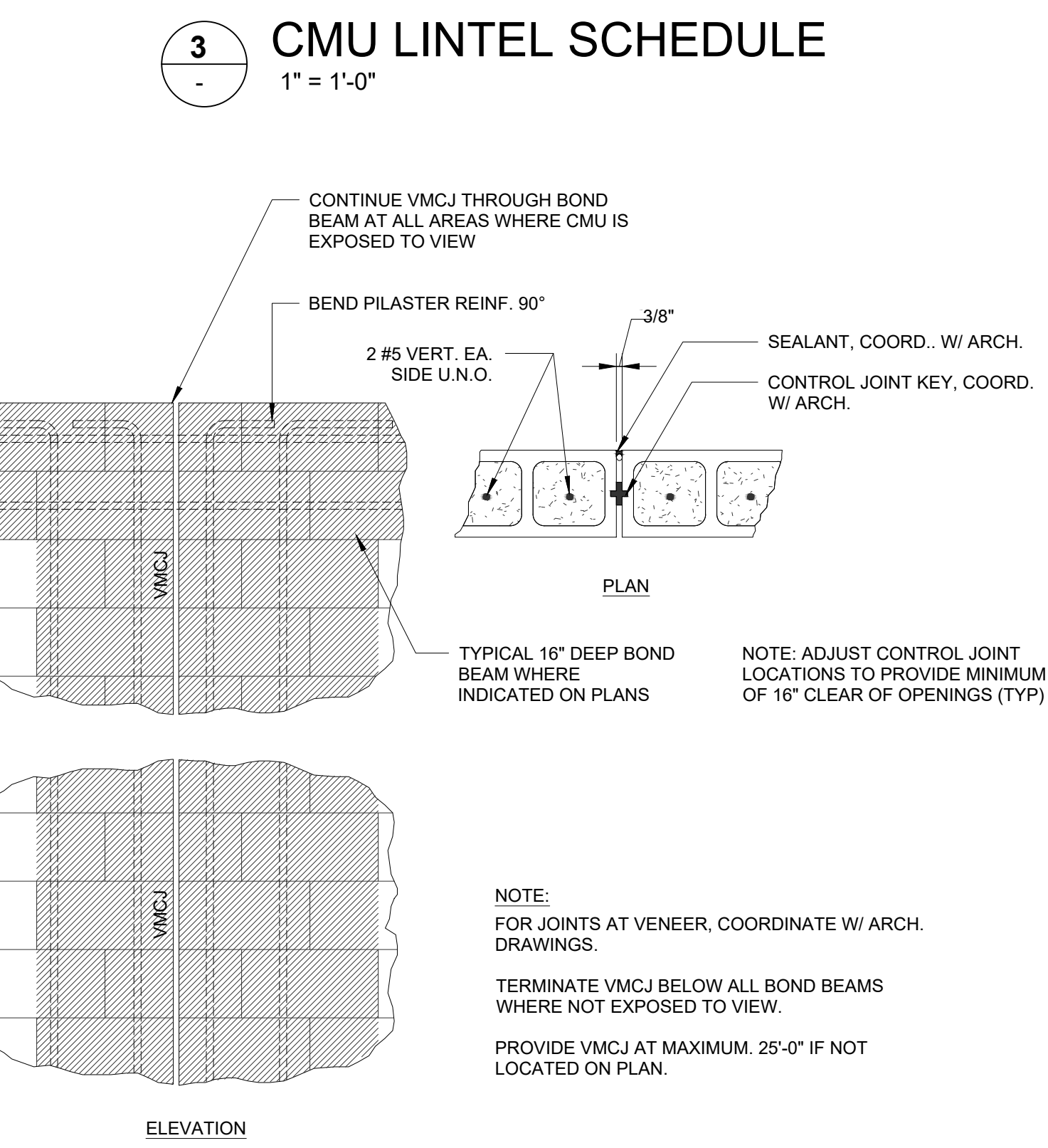


ROOF DECK PENETRATION AT SMALL OPENINGS
SCALE: 3/4" = 1'-0"

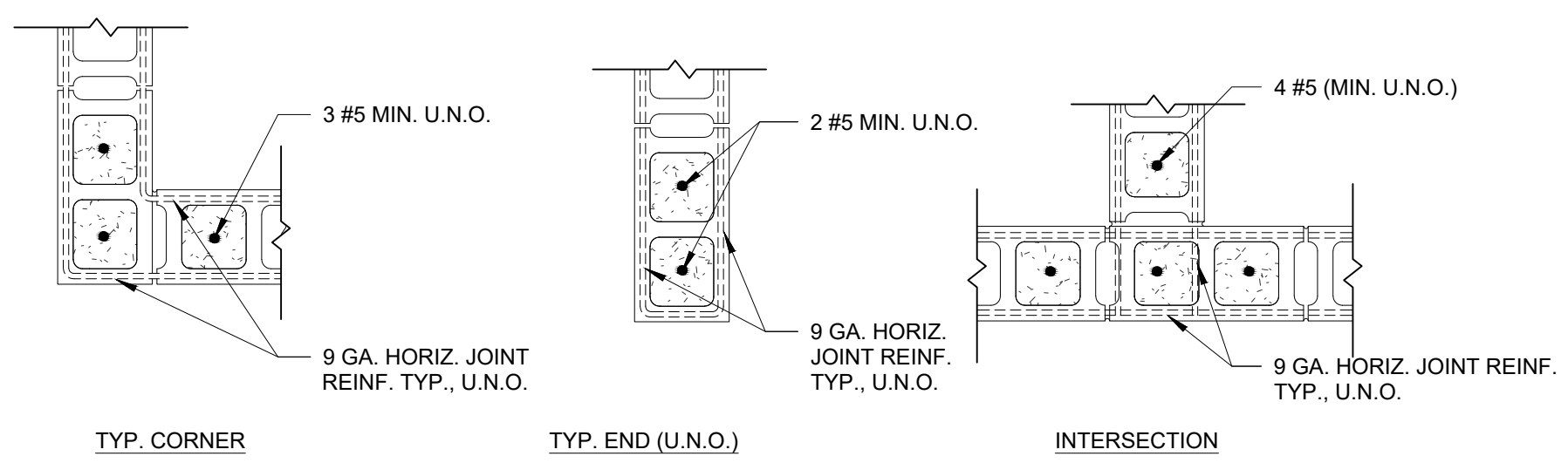


CMU LINTEL/JAMB SCHEDULE					
MAX OPENING SPAN	LOCATION	WIDTH	1) MIN. DEPTH OF FULL GROUTED CELLS	2) HORIZONTAL BOTT. REINF. U.N.O. (1 HORIZ.) PER COURSE	3) MIN. ADJACENT FULLY GROUTED CELLS AND REINF. (1 VERT.) PER CELL U.N.O.
≤ 4'-4"	TYP.	8"	8"	(1) #5 BOTT.	8" #5
4'-4" < OPNG. ≤ 6'-4"	TYP.	8"	16"	(1) #5 BOTT.	16" (2) #5
6'-4" < OPNG. ≤ 8'-4"	TYP.	8"	24"	(1) #5 BOTT.	16" (2) #6
8'-4" < OPNG. ≤ 10'-8"	TYP.	8"	32"	(2) #5 BOTT.	16" (2) #6

- NOTES:
- #6 VERT. SHALL START ABOVE FOOTING WITH A MINIMUM LAP LENGTH OF 30".
 - VERIFY ALL CMU OPENINGS W/ ARCHITECT DWG'S.
 - ALL CMU LINTEL BEAMS ABOVE BOTTOM COURSE SHALL BE CONSTRUCTED OF OPEN BOTTOM LINTEL UNITS, OR STRETCHER COURSES WITH 1/2 WEB CUT OUT.
 - COORDINATE W/ SECTIONS, DETAILS, & GENERAL NOTES.
 - PRE-CAST CONCRETE LINTEL MAY BE USED IN LIEU OF LINTEL BLOCK.
 - REFER TO PLANS FOR FULLY GROUTED AREAS.

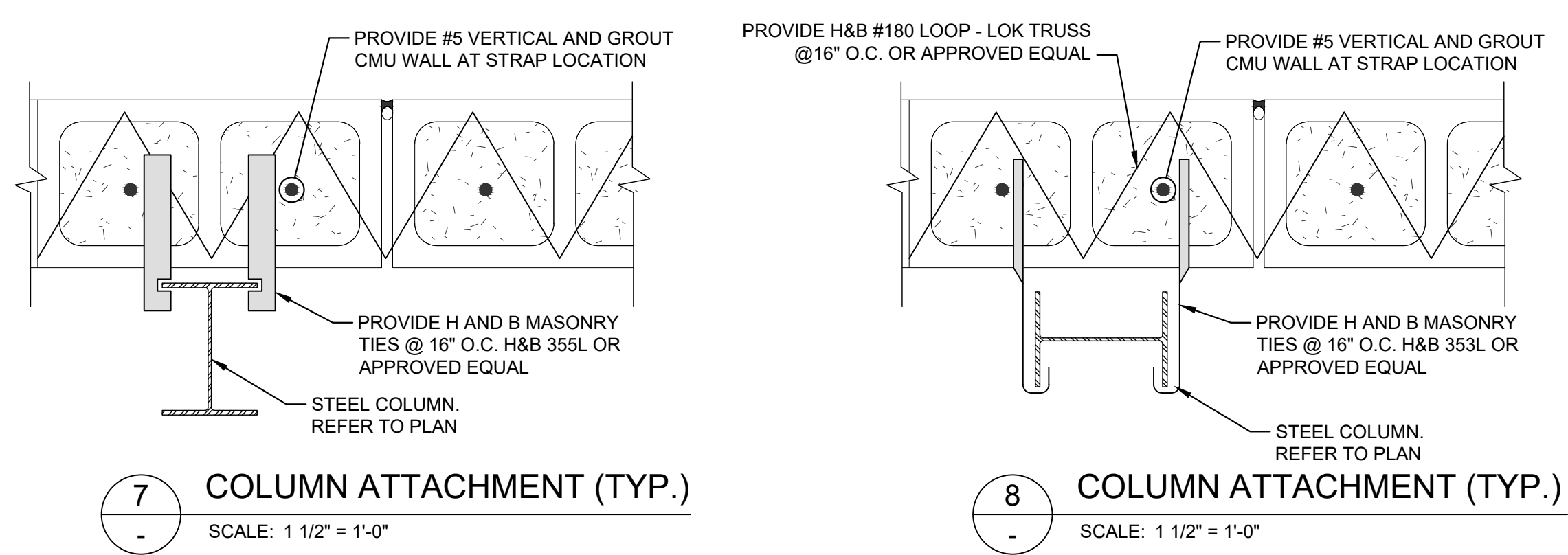


4 TYPICAL VMCJ DETAILS



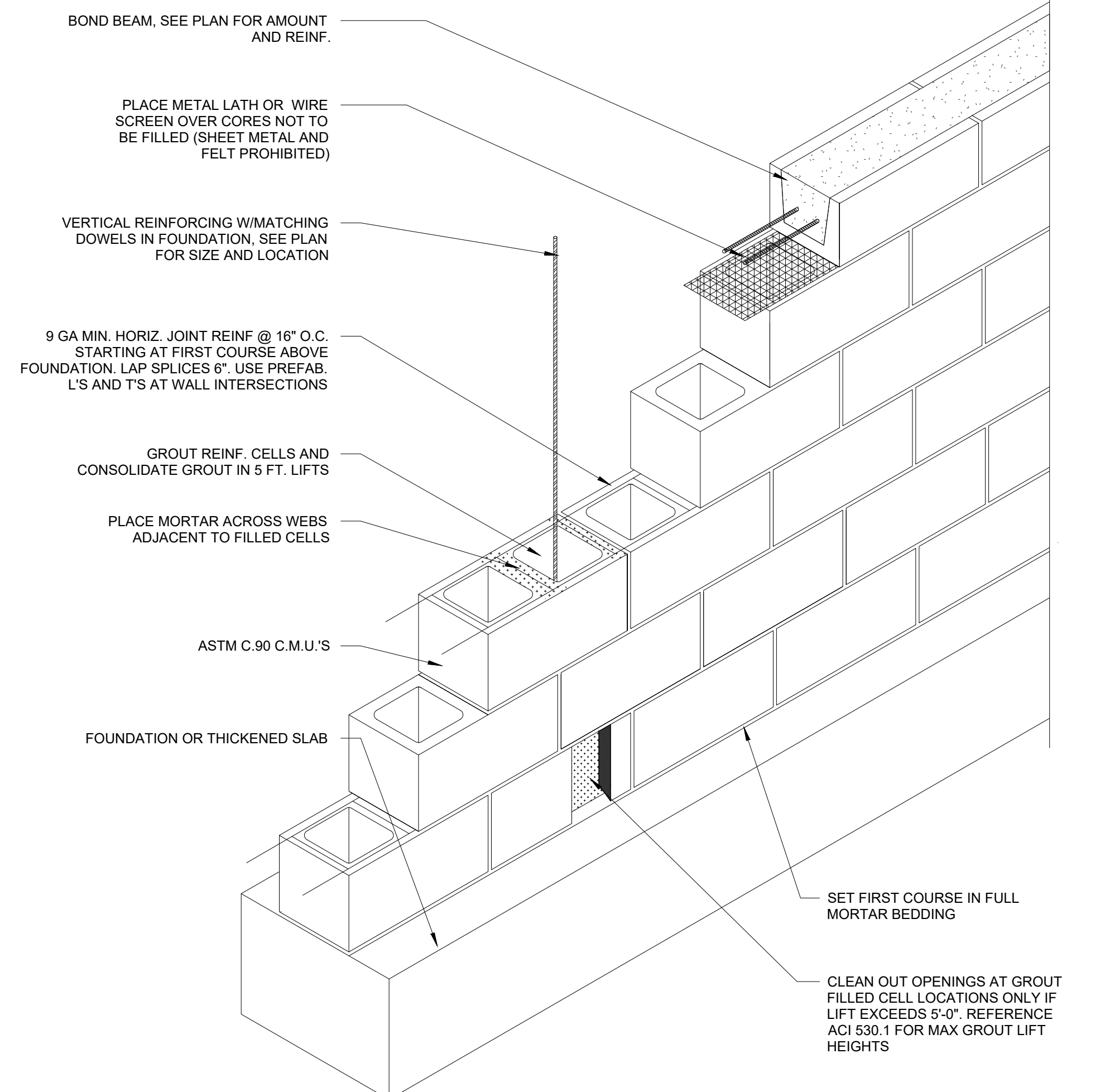
- NOTES:
- MINIMUM REINFORCING SHOWN APPLIES UNLESS SHOWN OTHERWISE ON PLANS OR SECTIONS.
 - USE BAR POSITIONERS
 - DO NOT INTERLOCK LOAD BEARING AND NON-LOAD BEARING WALLS.

5 TYPICAL CMU DETAILS

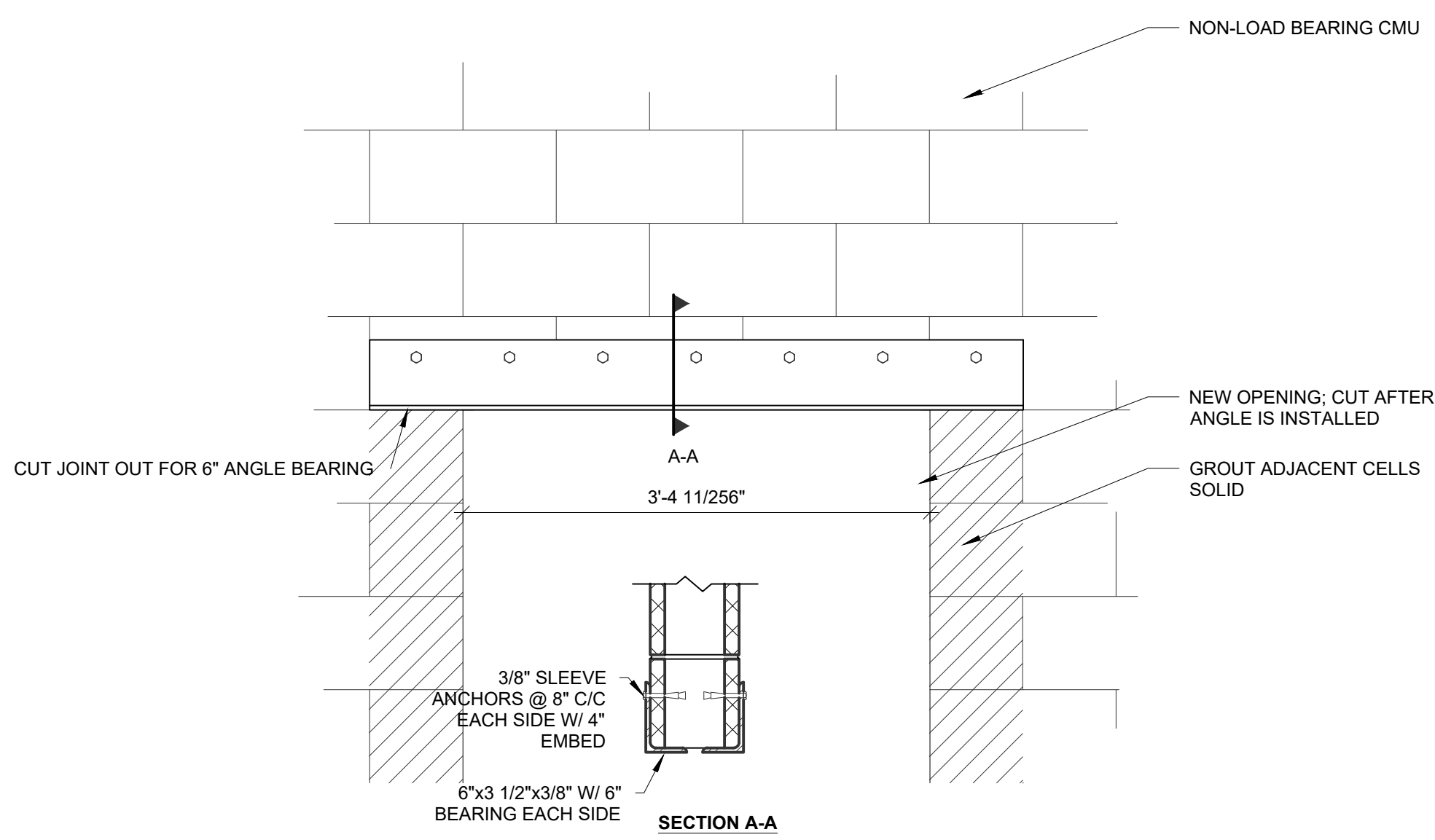


7 COLUMN ATTACHMENT (TYP.)
SCALE: 1 1/2" = 1'-0"

8 COLUMN ATTACHMENT (TYP.)
SCALE: 1 1/2" = 1'-0"



2 TYPICAL MASONRY WALL CONSTRUCTION
3/16" = 1'-0"



6 CMU OPENING

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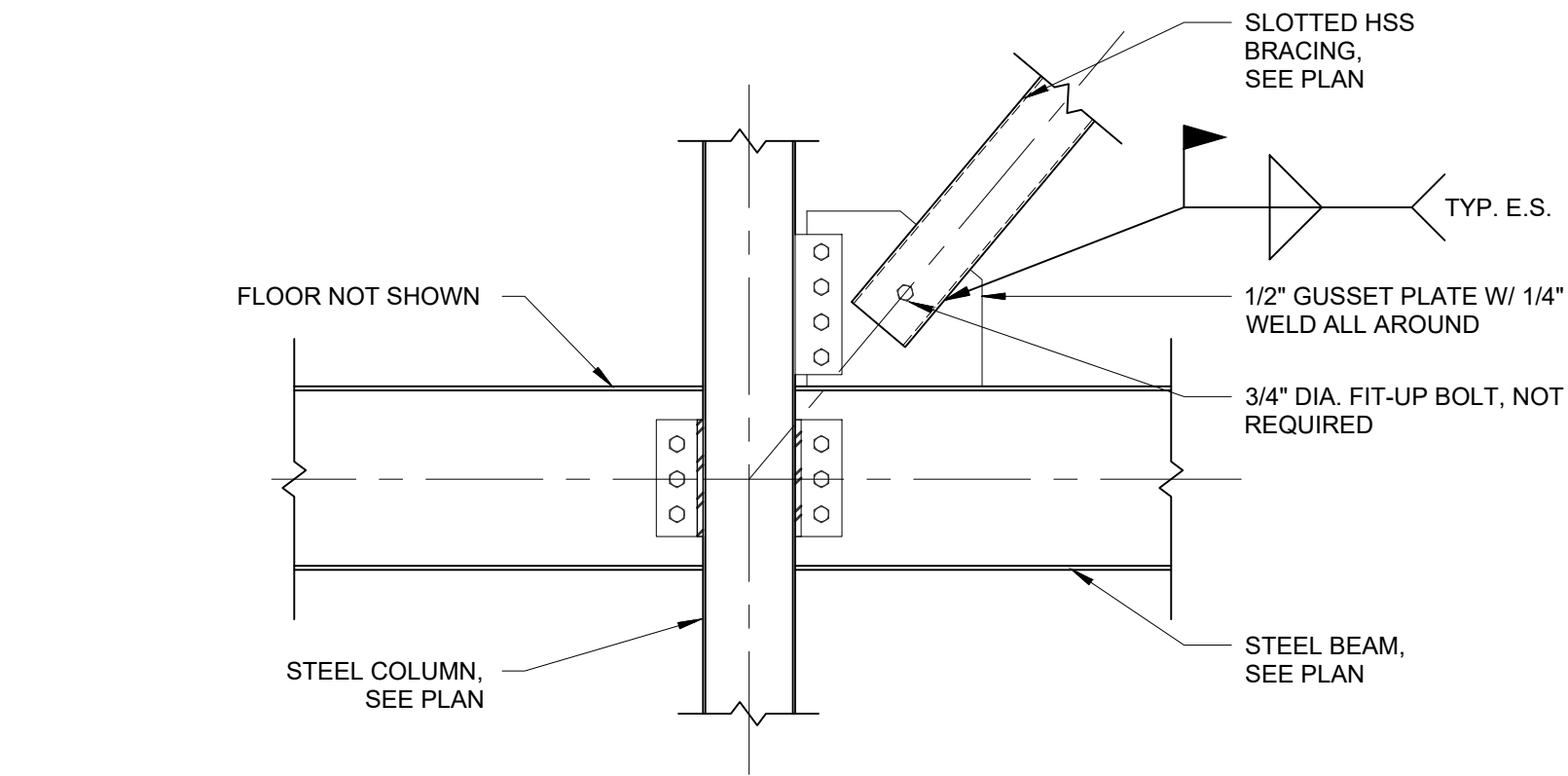
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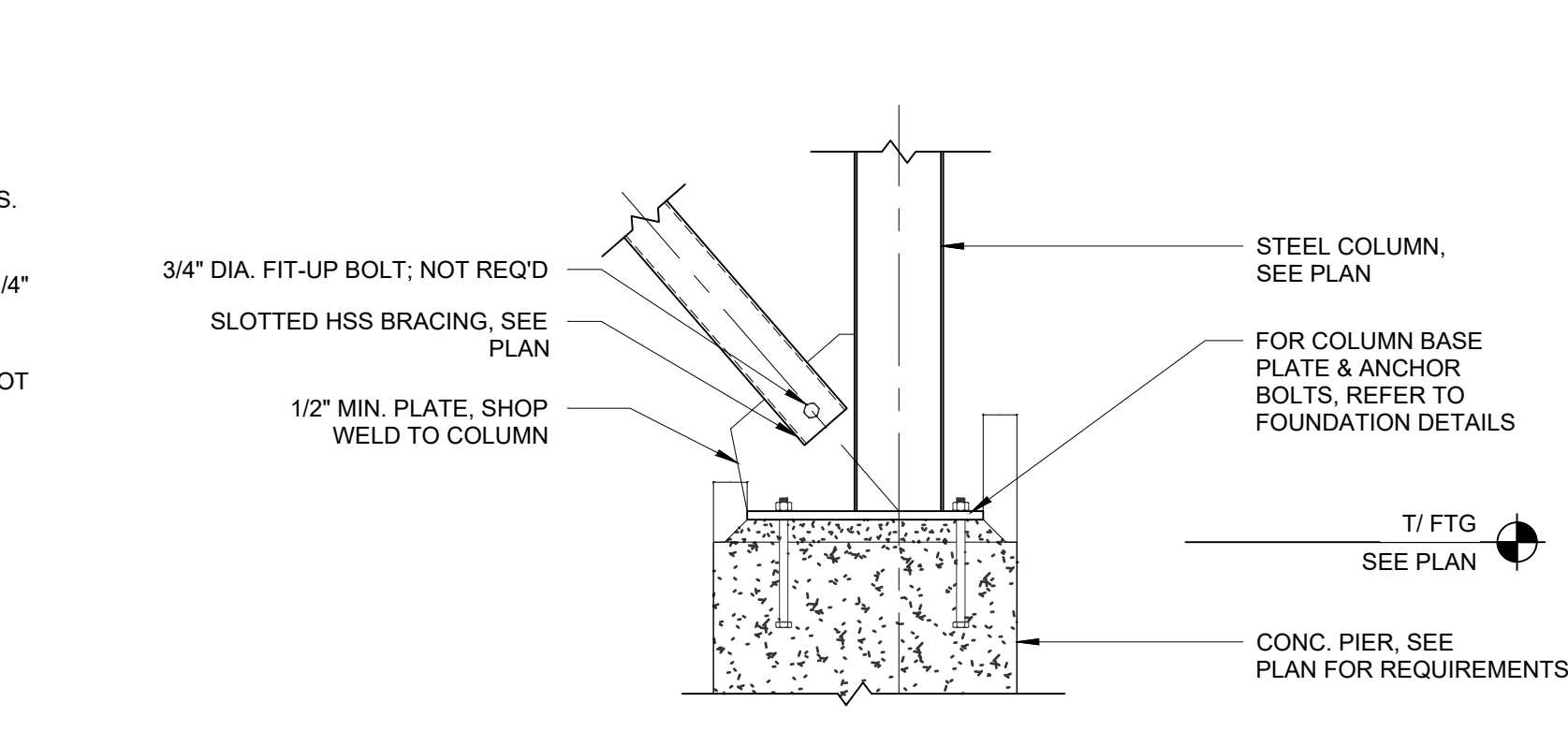
ROOF STRUCTURAL AND
MASONRY DETAILS



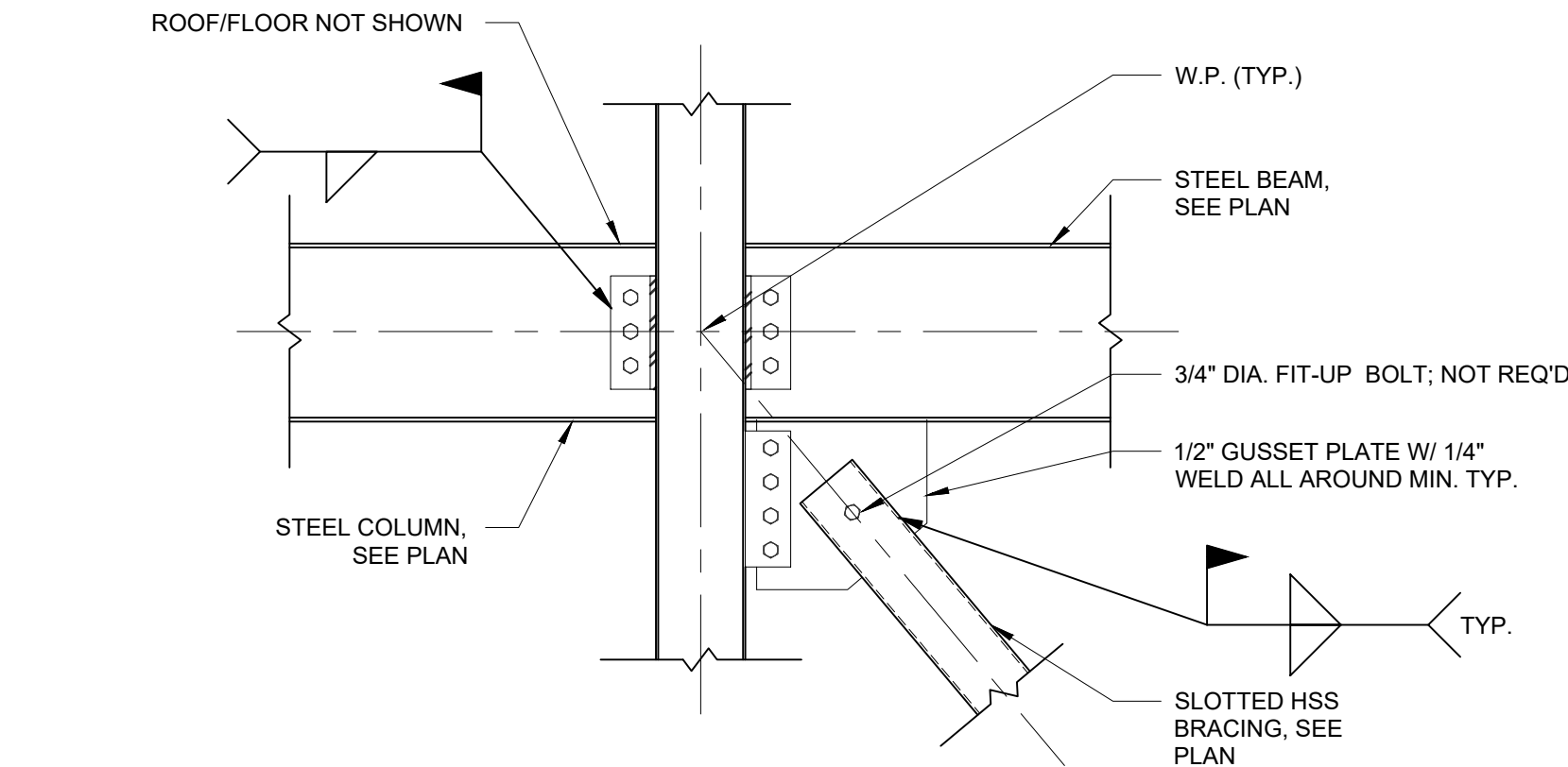
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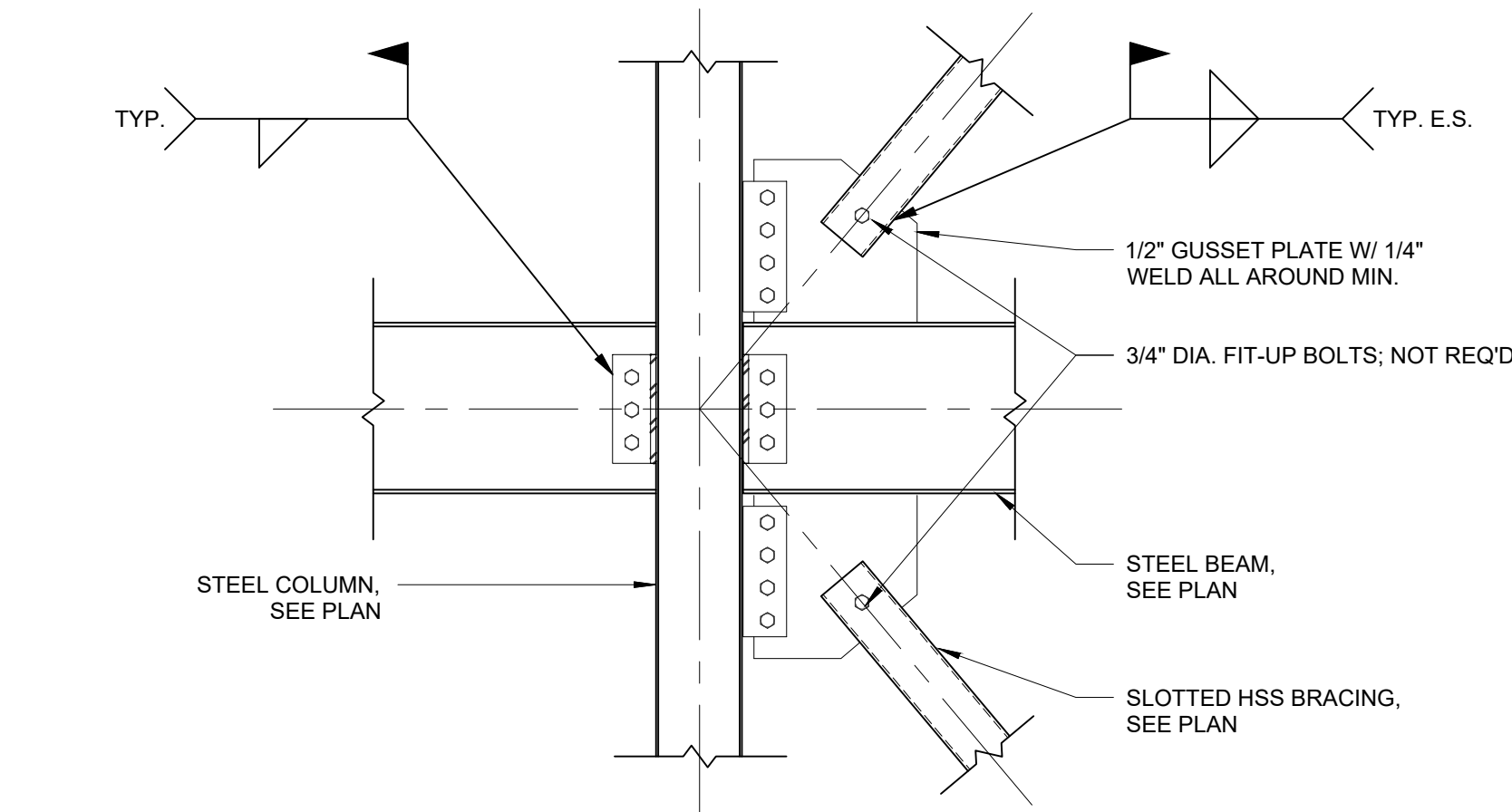
1 HSS BRACE ABOVE BEAM
3/4" = 1'-0"



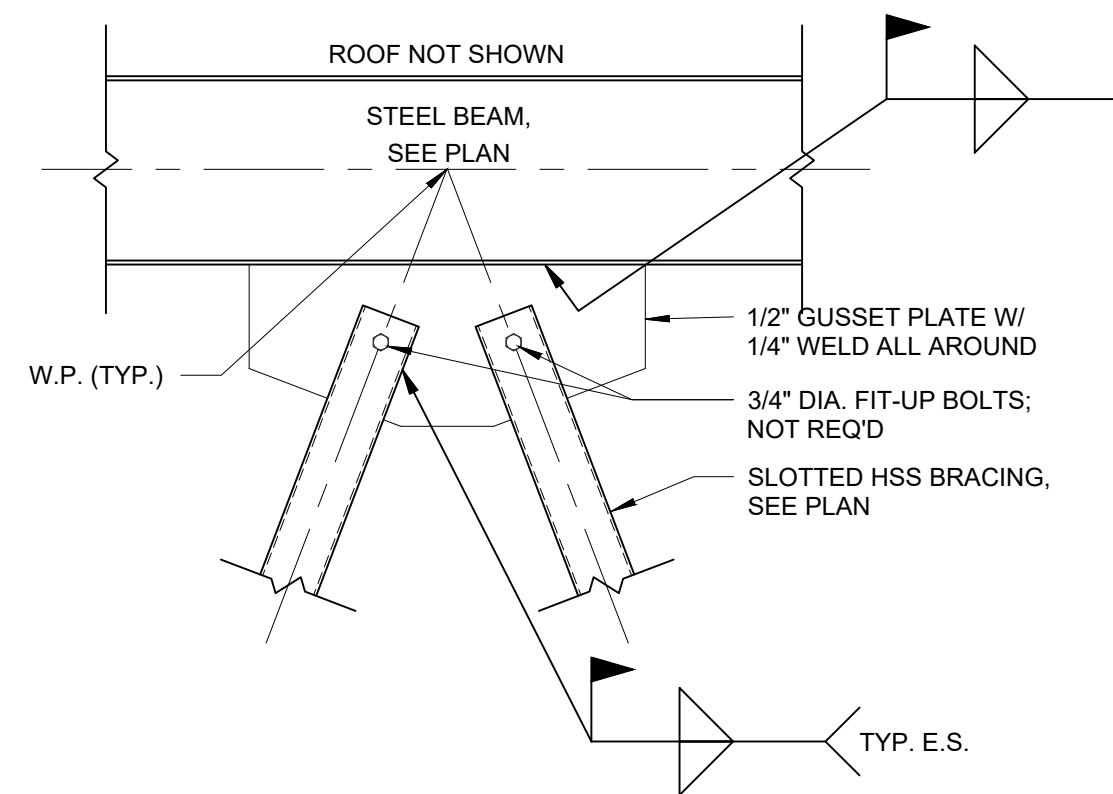
2 HSS BRACE AT BASE
3/4" = 1'-0"



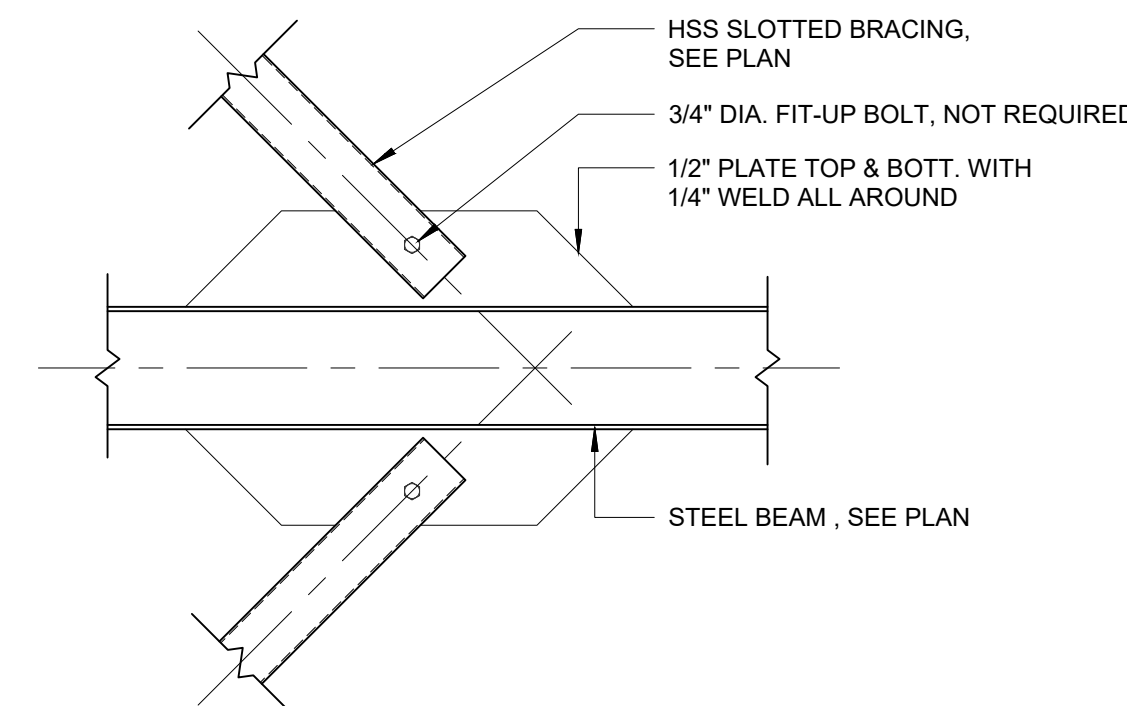
5 HSS BRACE BELOW BEAM
3/4" = 1'-0"



3 HSS BRACE AT BEAM END
3/4" = 1'-0"



4 HSS BRACE AT MID-SPAN
3/4" = 1'-0"



7 HSS BRACED FRAME AT MID-BEAM
3/4" = 1'-0"

ALL DIMENSIONS SHALL BE BE VERIFIED IN FIELD (V.I.F.).
COORDINATE LOCATION OF STEEL BEAMS TO MEET THE
EQUIPMENT SUPPORT REQUIREMENTS.

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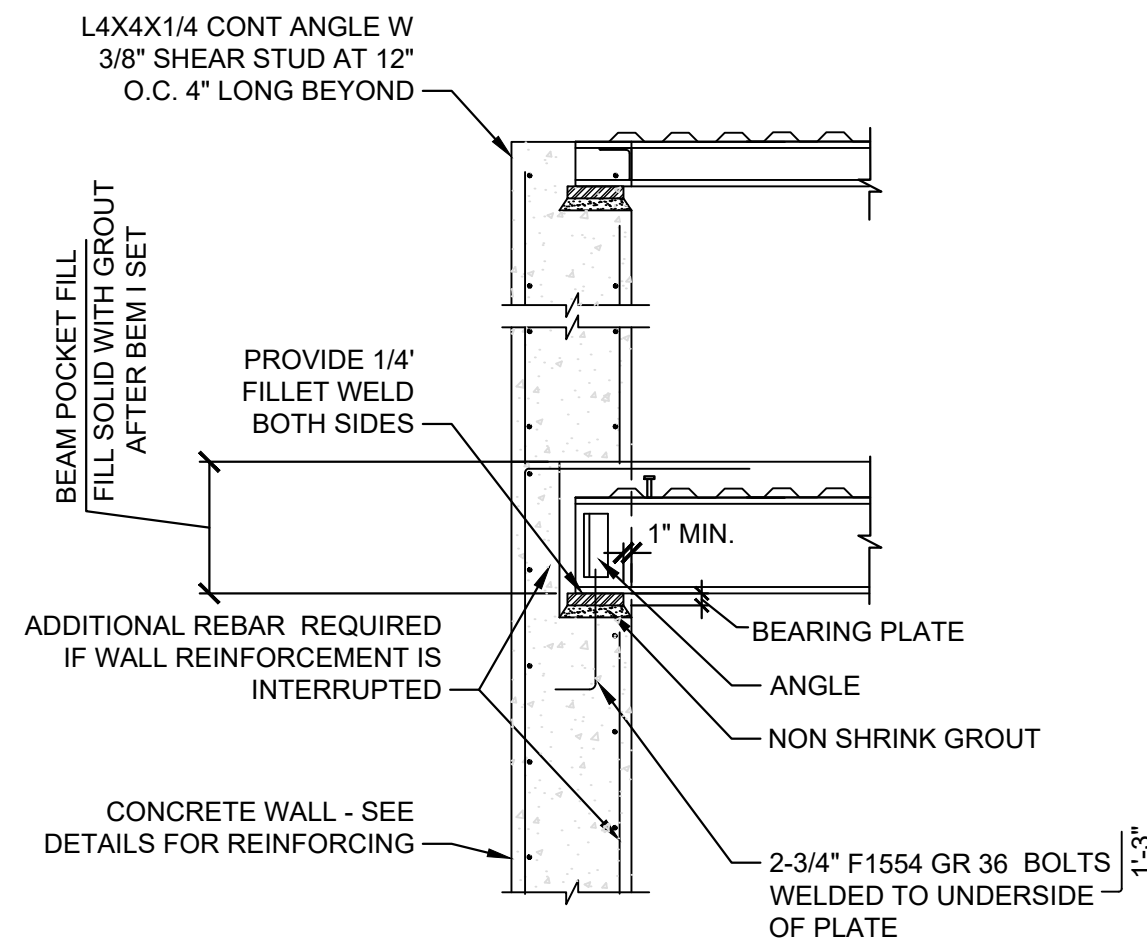
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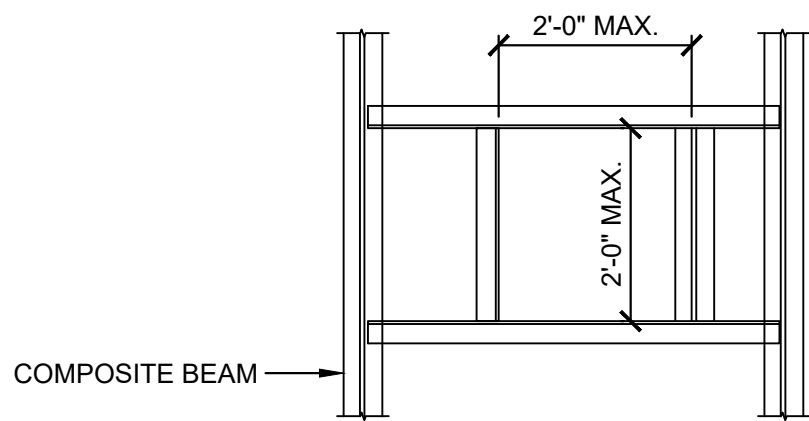
BRACING DETAILS



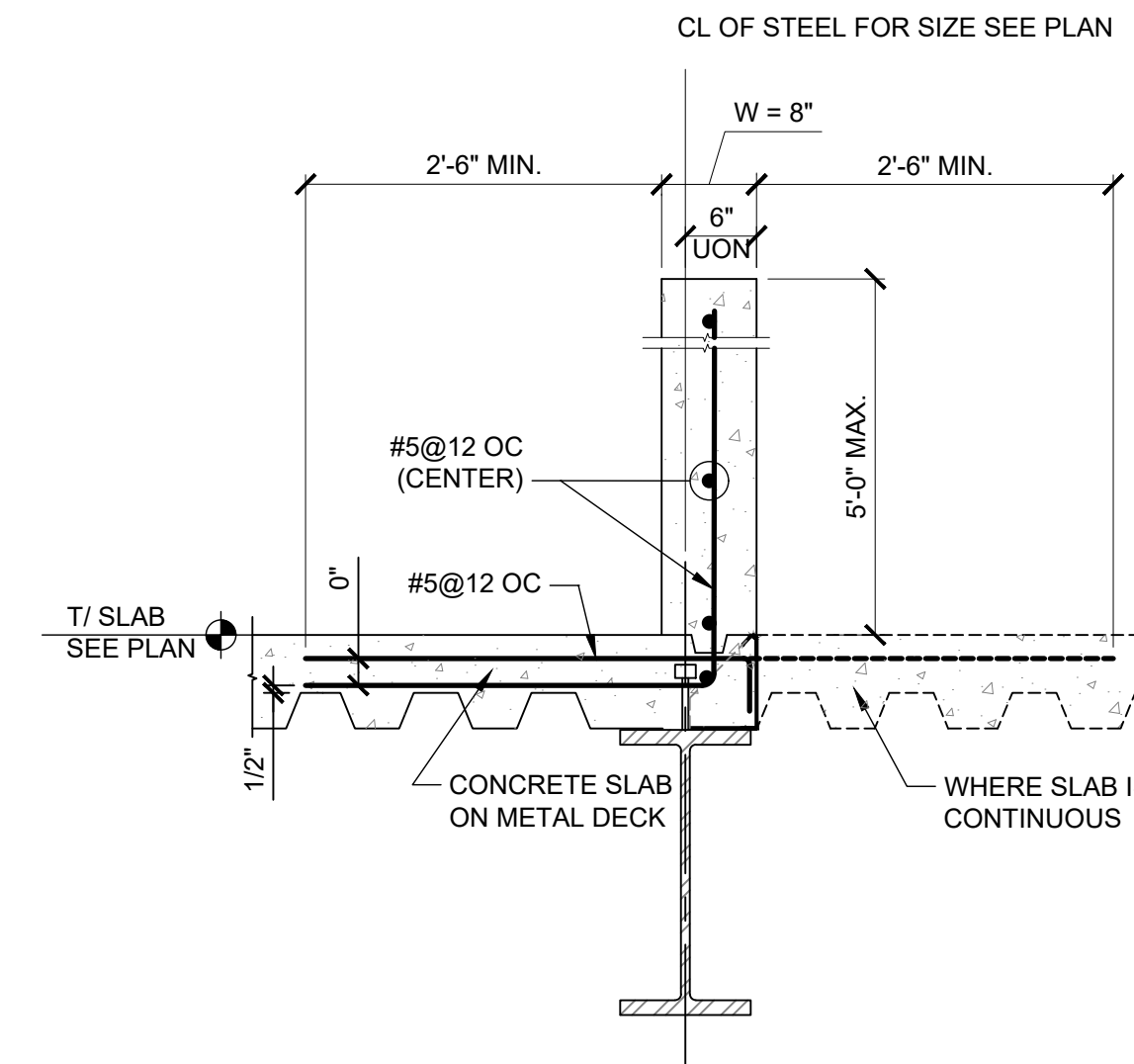
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S-204
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1 WALL POCKET DETAIL
1/2" = 1'-0"

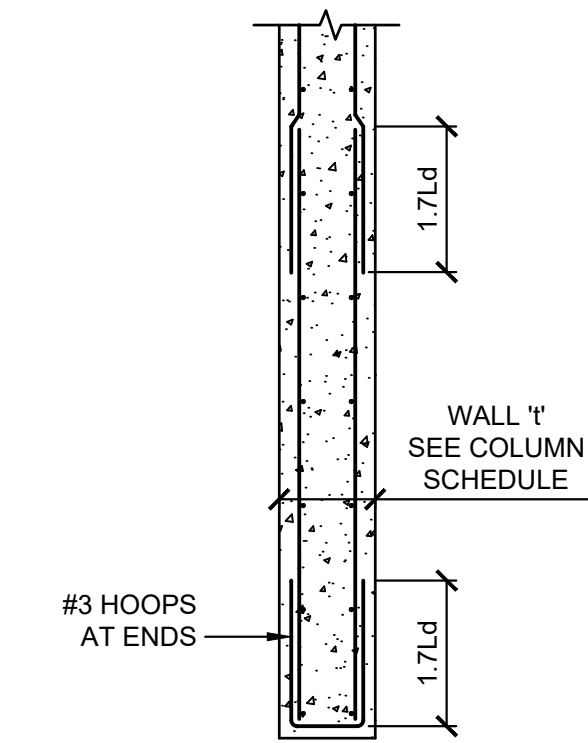


5 TYP. FRAMES AT OPENINGS
1/2" = 1'-0"



NOTES:
FOR CURB SIZE & LOCATION SEE
ARCHITECTURAL DRAWINGS

6 TYPICAL CONCRETE CURB
PARALLEL TO DECK
3/4" = 1'-0"

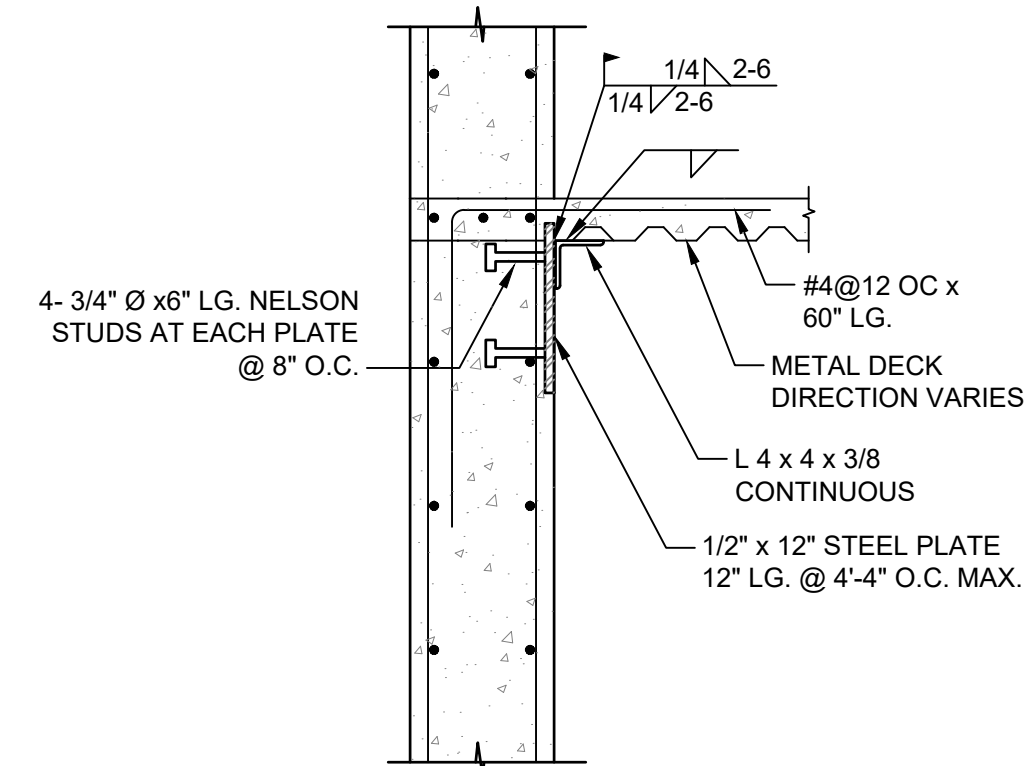


2 BEARING WALL
DETAIL & SCHEDULE
1/2" = 1'-0"

BEARING WALL REINFORCING SCHEDULE		
WALL \"	VERTICAL REINF.	HORIZONTAL REINF.
8"	#5@18	#3@11
9"	#5@18	#3@9
10"	#5@18	#4@16
12"	#5@18	#4@13

SEE PLAN FOR WALL REINFORCING AT ELEVATOR
AND STAIR.

BASE PLATE SCHEDULE		
BEAM	BASE PLATE	STIFFENER
W10	8X8X1"	NO STIFFENER
W12	8X8X1"	NO STIFFENER
W14	8X8X1"	NO STIFFENER
W16	8X8X1"	NO STIFFENER
W18	8X8X1"	NO STIFFENER
W24	10X8X1"	2-1/2" STIFFENER
W30	12X12X1"	2-1/2" STIFFENER NOTCH PLATE AS REQUIRED AT CORNER CONDITION



3 TYPICAL METAL DECK SUPPORT
3/4" = 1'-0"

REINFORCEMENT ANCHORAGE SCHEDULE													
TABULATED VALUES ARE GIVEN IN INCHES & ARE CALCULATED FOR f _c = 60,000 P.S.I.													
STONE CONCRETE													
BAR SIZE	f _c	3000 P.S.I.				4000 P.S.I.				5000 P.S.I.			
		ld	ldt	ldh	ldc	ld	ldt	ldh	ldc	ld	ldt	ldh	ldc
#3	12	13	6/8	8	12	13	6/7						
#4	12	17	8/11	11	12	17	7/9	9	12	17	6/8	9	12
#5	15	21	10/14	14	15	21	8/12	12	15	21	7/11	11	15
#6	19	27	11/16	16	18	25	10/14	14	18	25	9/13	14	18
#7	26	37	13/19	19	23	32	12/17	17	21	29	10/15	16	21
#8	35	48	15/22	22	30	42	13/19	19	27	38	12/17	18	24
#9	44	61	17/25	25	38	53	15/21	21	34	48	13/19	20	31
#10	56	78	19/28	28	48	67	17/24	24	43	60	15/22	23	39
#11	68	96	22/31	31	59	83	19/27	27	53	74	17/24	25	48
NOTES	A & B	C	D	A & B	C	D	A & B	C	D	A & B	C	D	

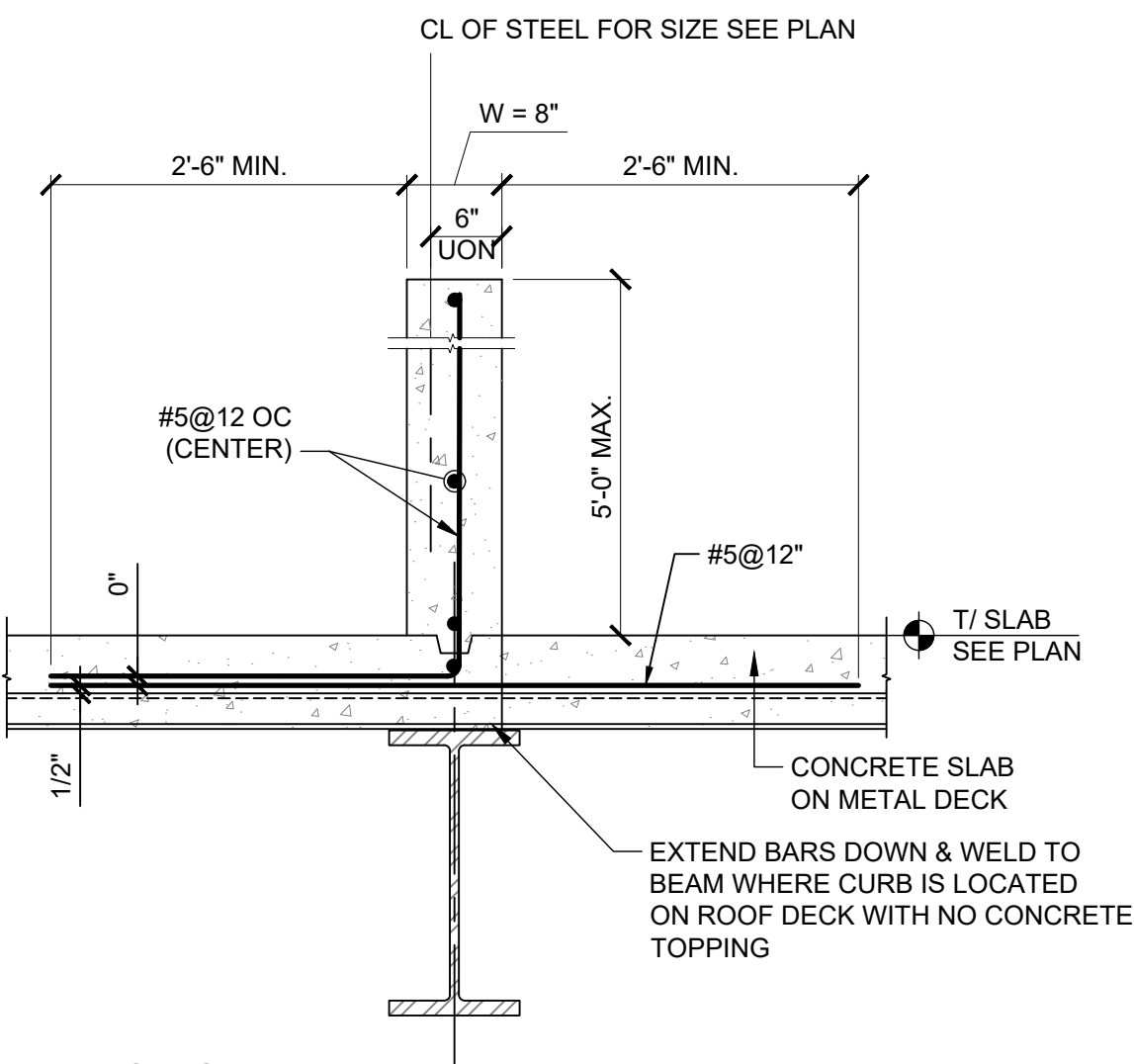
NOTE A : USE ld TABULATED VALUES WHEN BARS HAVE LESS THAN 12" OF CONCRETE
BELOW BOTTOM OF BARS (ie: FOR BOTTOM REINFORCING AND FOR TOP REINFORCING IN
SHALLOW BEAMS AND SLABS). USE ldt WHEN BARS HAVE MORE THAN 12" OF FRESHLY
PLACED CONCRETE BELOW BOTTOM OF BARS.

NOTE B : TABULATED VALUES MAY BE REDUCED BY 20% IF BARS ARE SPACED MORE
THAN 6" AND ARE AT LEAST 3" FROM PARALLEL CONCRETE EDGES.

NOTE C : LOWER VALUES OF DEVELOPMENT LENGTH OF HOOKED BARS, ldh, MAY BE
USED IF TAIL EXTENSION OF THE HOOK HAS AT LEAST 2" COVER AND CONCRETE COVER TO
SIDE OF HOOK IS AT LEAST 2 1/2" THICK. OTHERWISE USE HIGHER ldh VALUES TABULATED.

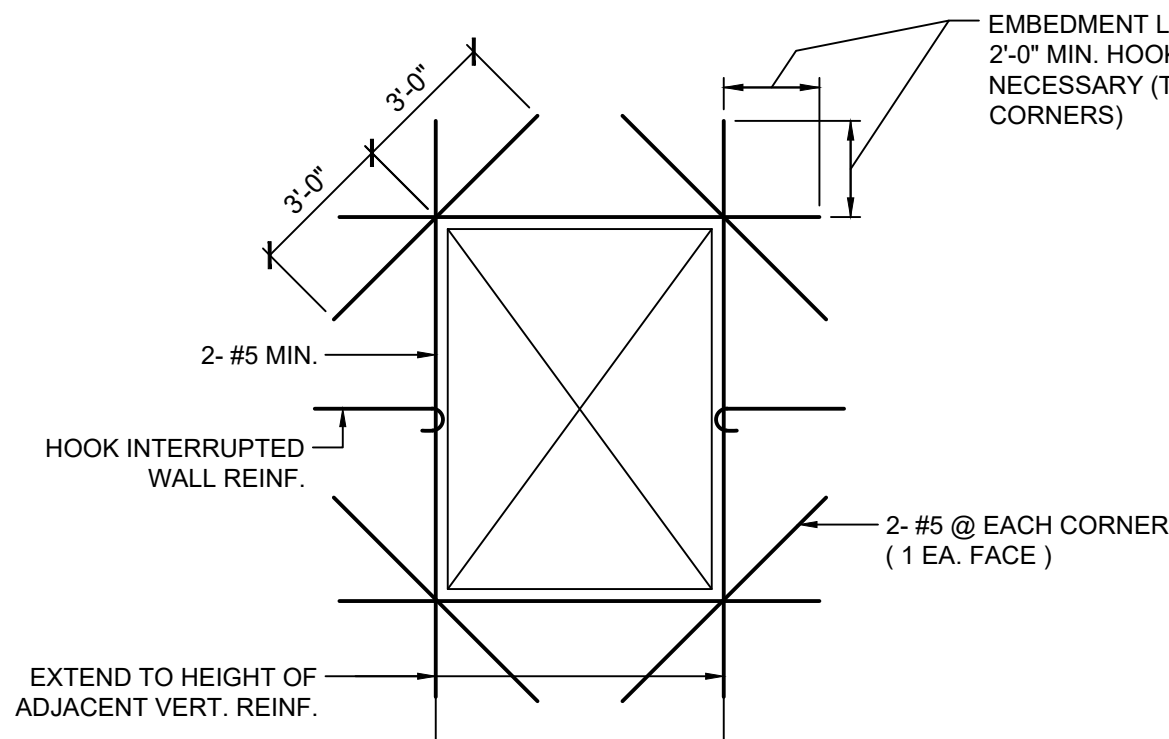
NOTE D : HOOKS SHALL NOT BE CONSIDERED EFFECTIVE IN REDUCED ldc VALUES (COMP-
RESSION DEVELOPMENT LENGTH).

GLOSSARY :
ld = DEVELOPMENT LENGTH (IN.) = LENGTH REQUIRED TO ANCHOR REINFORCING AT
EACH END.
ldt = DEVELOPMENT LENGTH (IN.) = LENGTH REQUIRED TO ANCHOR TOP BARS
HAVING MORE THAN 12" OF FRESH CONCRETE PLACED UNDER THEM.
ldh = DEVELOPMENT LENGTH (IN.) OF HOOKED BARS (TO PERIMETER OF HOOK).
ldc = DEVELOPMENT LENGTH (IN.) FOR BARS IN COMPRESSION.



NOTES:
FOR CURB SIZE & LOCATION SEE
ARCHITECTURAL DRAWINGS

7 TYPICAL CONCRETE CURB
PERPENDICULAR TO DECK
3/4" = 1'-0"



TYPICAL WALL OPENING DETAILS

SCALE : N.T.S.

NOTES :

1. HOOK ALL INTERRUPTED CONTINUOUS BARS.
2. ONE HALF OF REINFORCING BAR AREA INTERRUPTED BY
OPENING SHALL BE PROVIDED EACH SIDE OF OPENINGS
ON THE SAME FACE, MIN. 2- #5 EACH SIDE.

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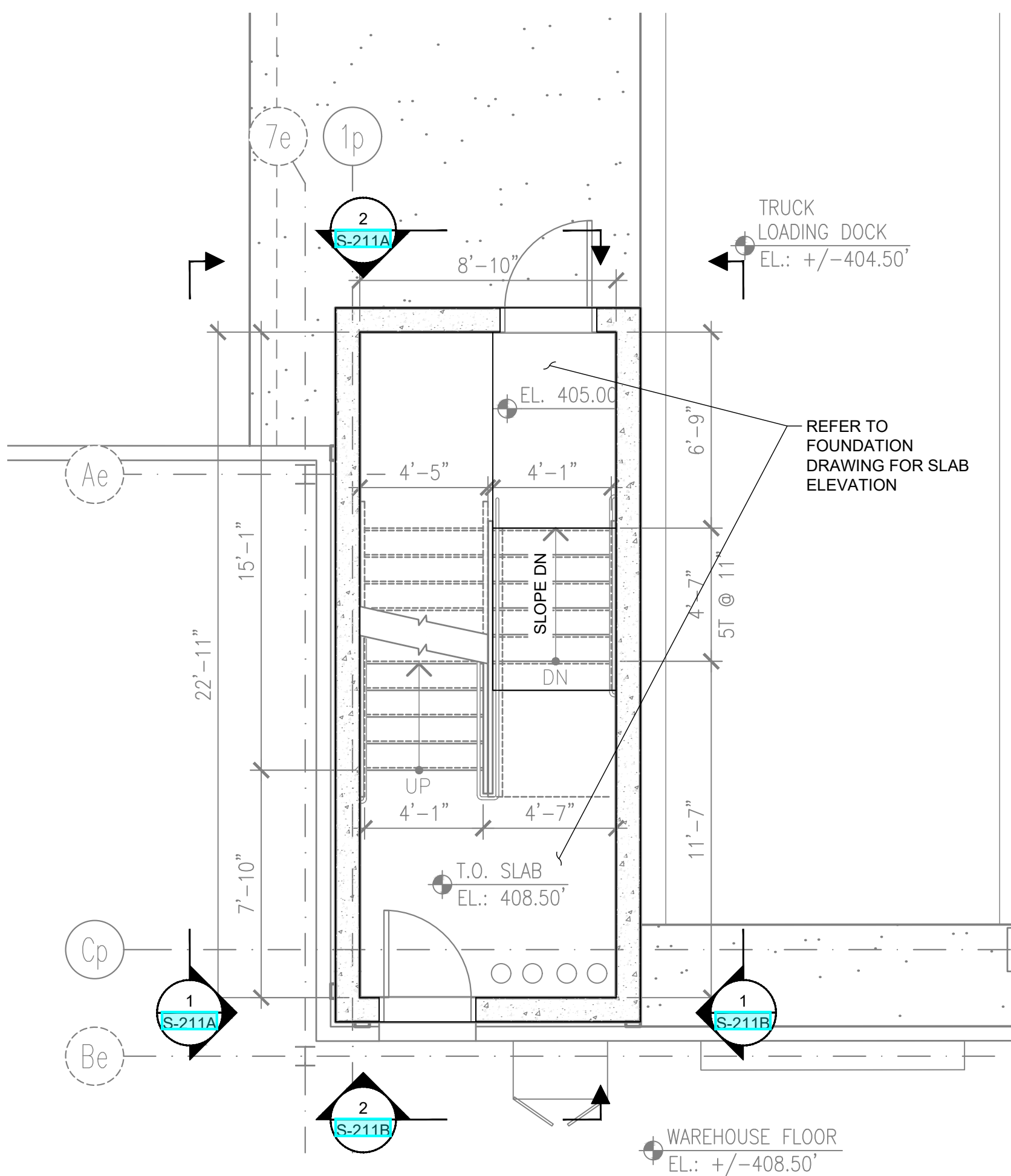
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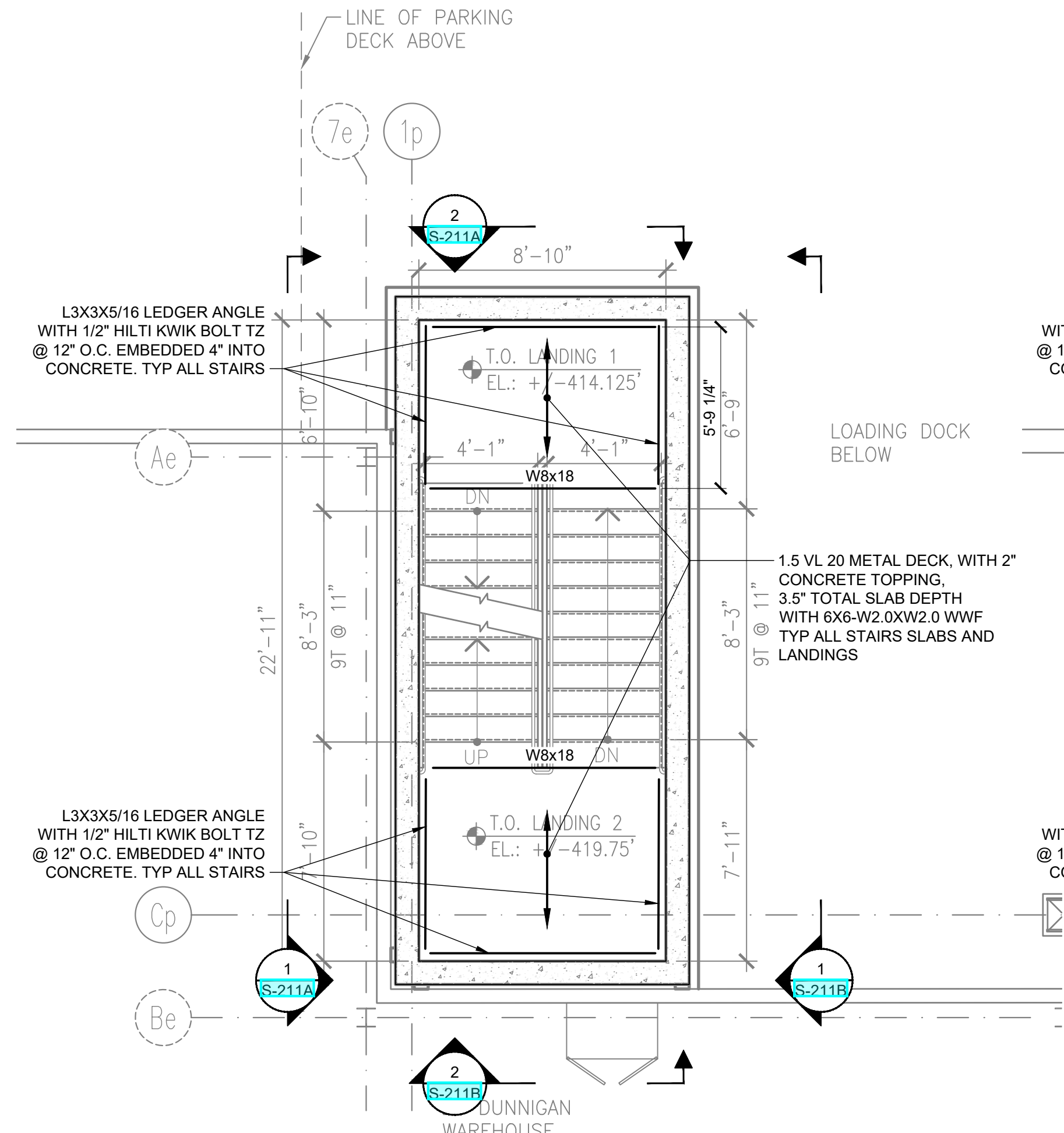
CURB DETAILS



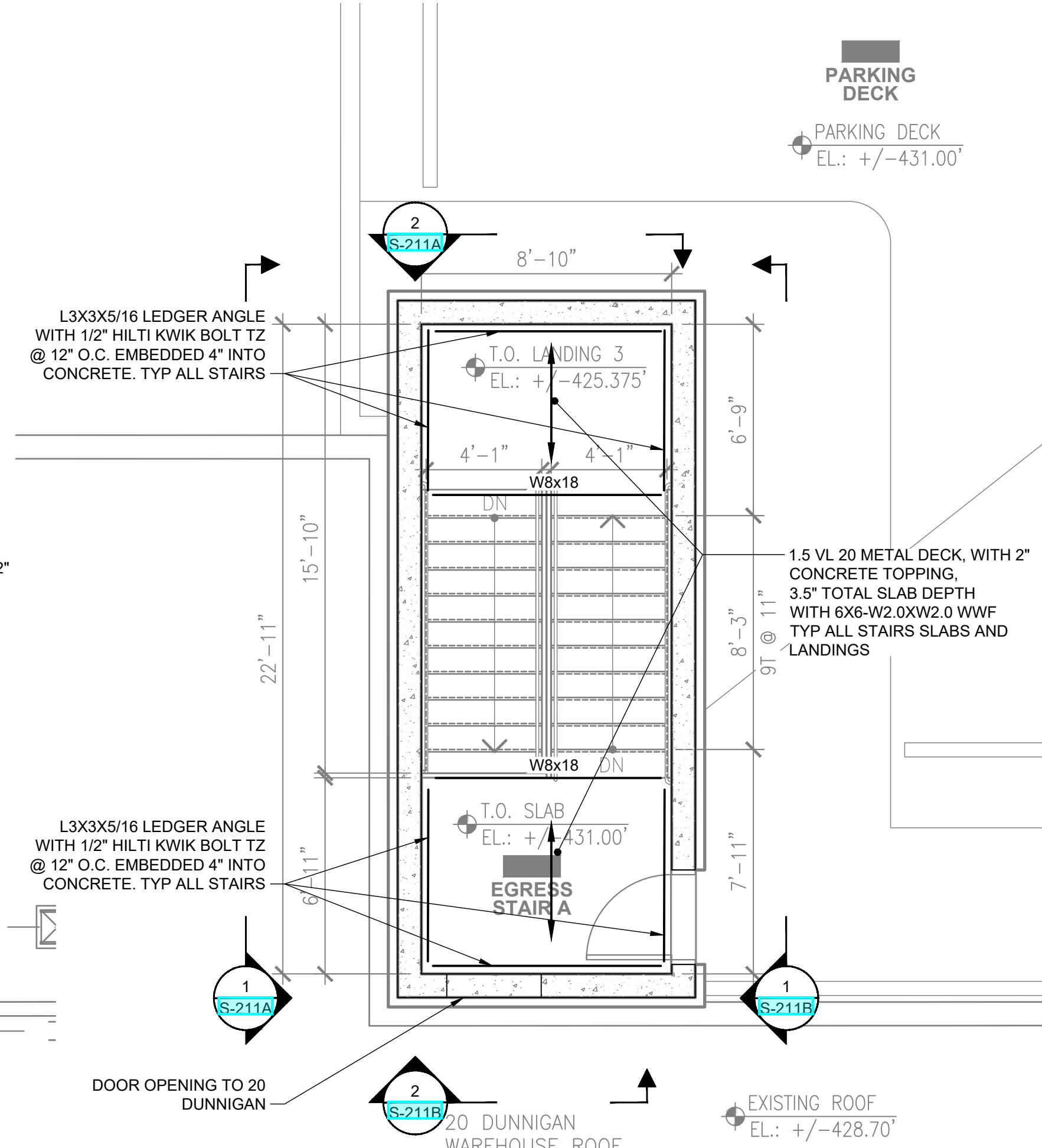
DWG NUMBER :
S-205
24 OF 61



1
-
STAIR A - ENLARGED PLAN
WAREHOUSE LEVEL
SCALE: 1/4" = 1'-0"



2
-
STAIR A - ENLARGED PLAN
INTERMEDIATE LEVEL
SCALE: 1/4" = 1'-0"



3
-
STAIR A - ENLARGED PLAN
PARKING DECK LEVEL
SCALE: 1/4" = 1'-0"

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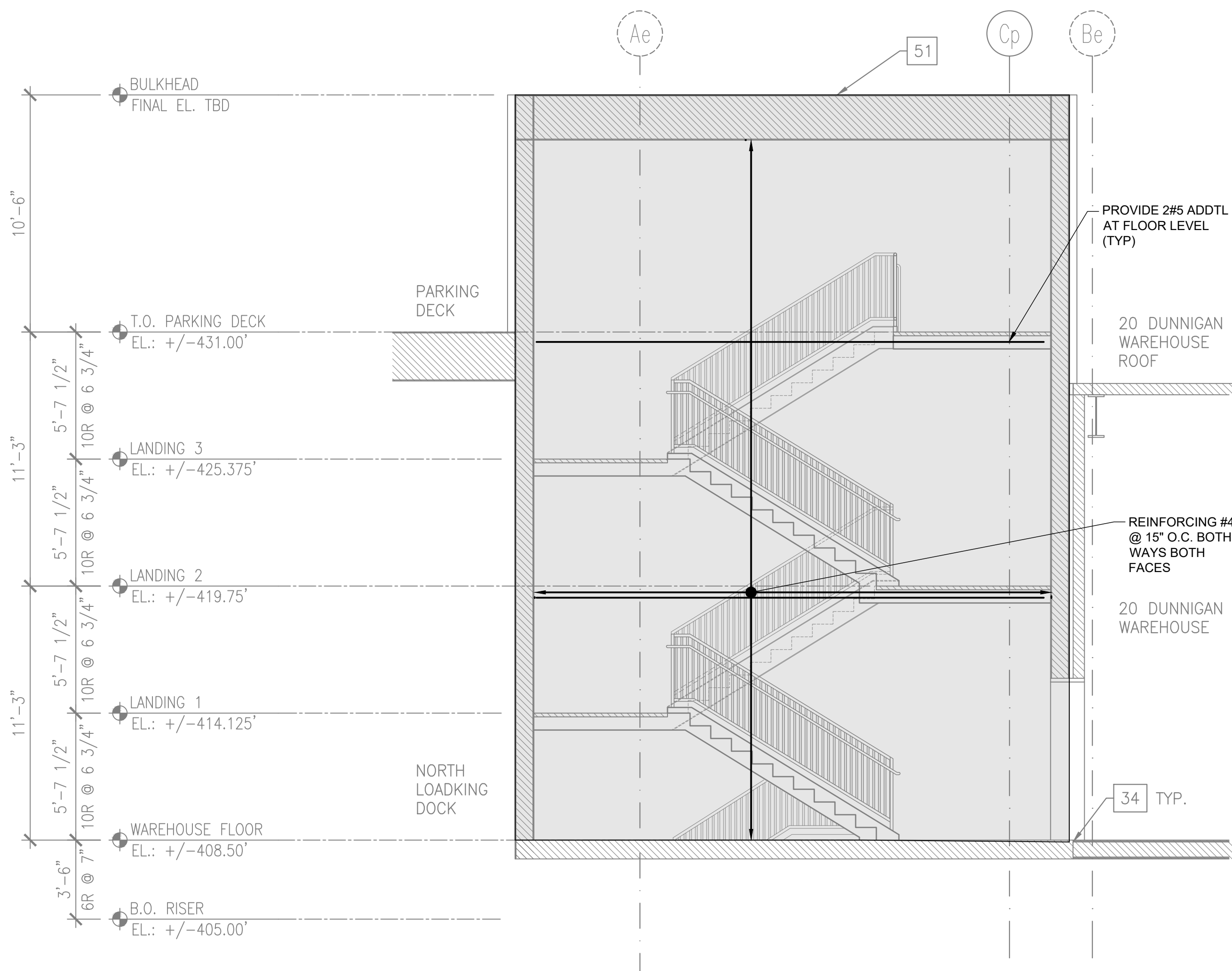
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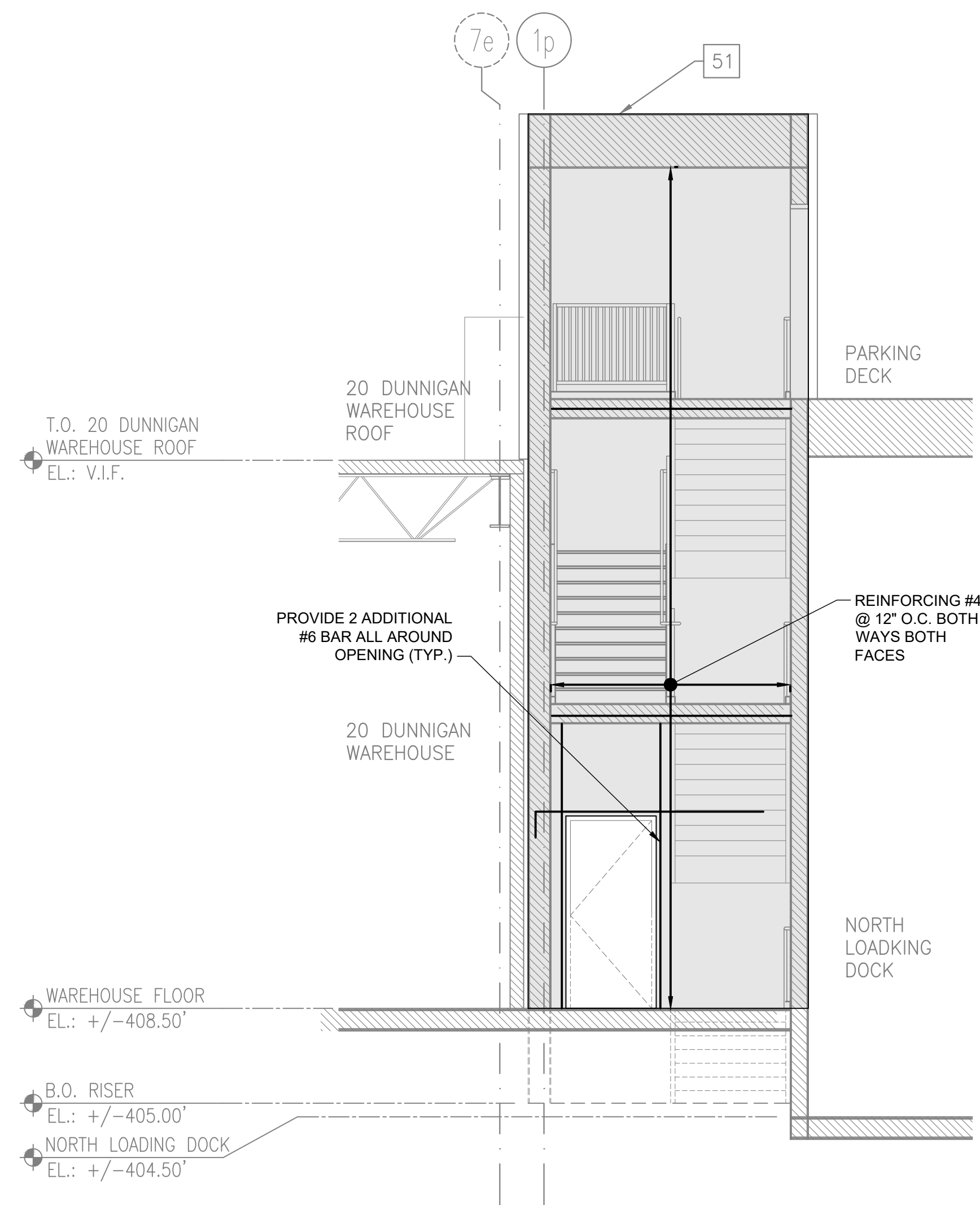


DWG NUMBER :
S-210
25 OF 61

4/4/22



1 STAIR A - ELEVATION (WEST)
210 SCALE: 1/4" = 1'-0"



2 STAIR A - ELEVATION (NORTH)
210 SCALE: 1/4" = 1'-0"

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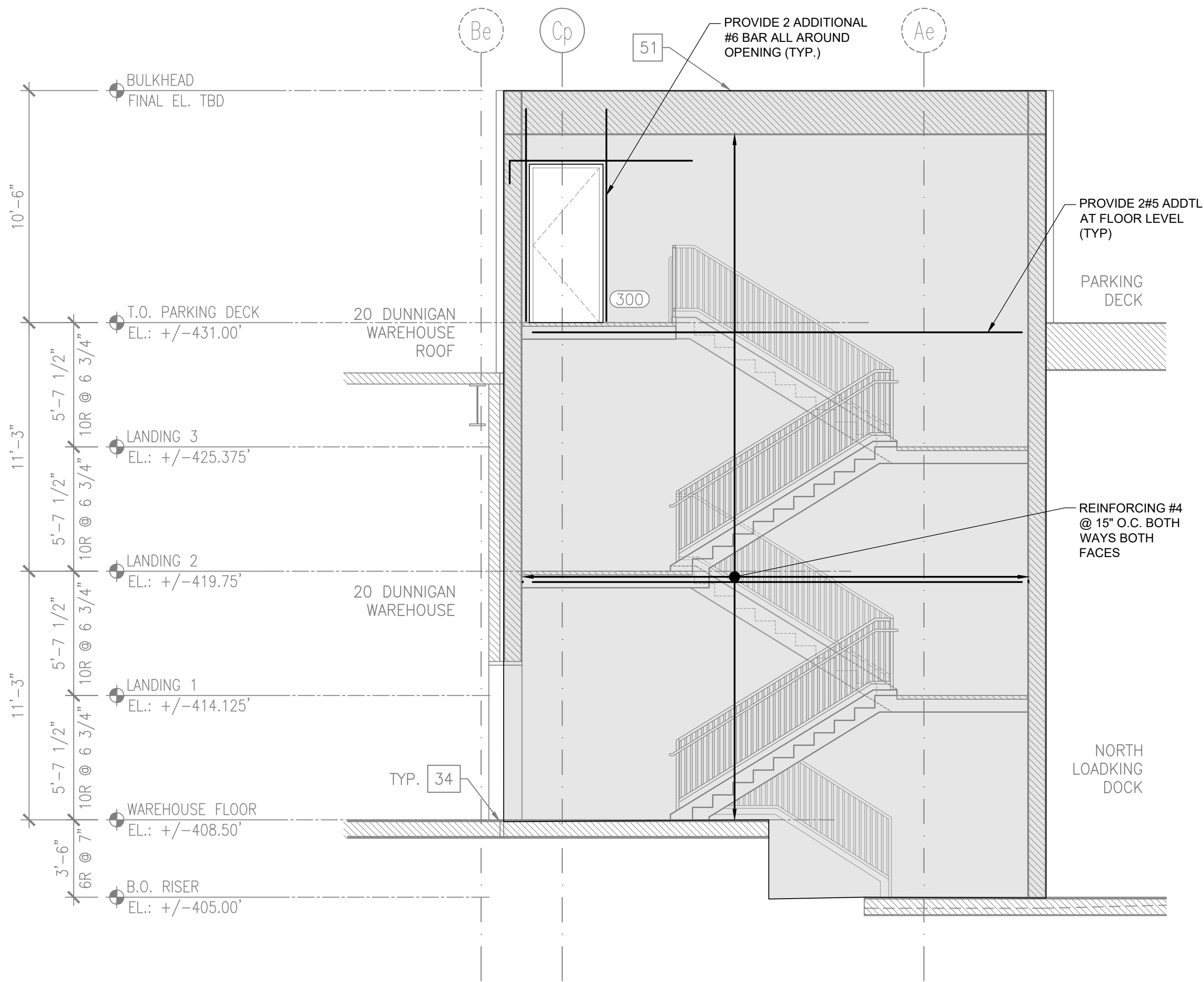
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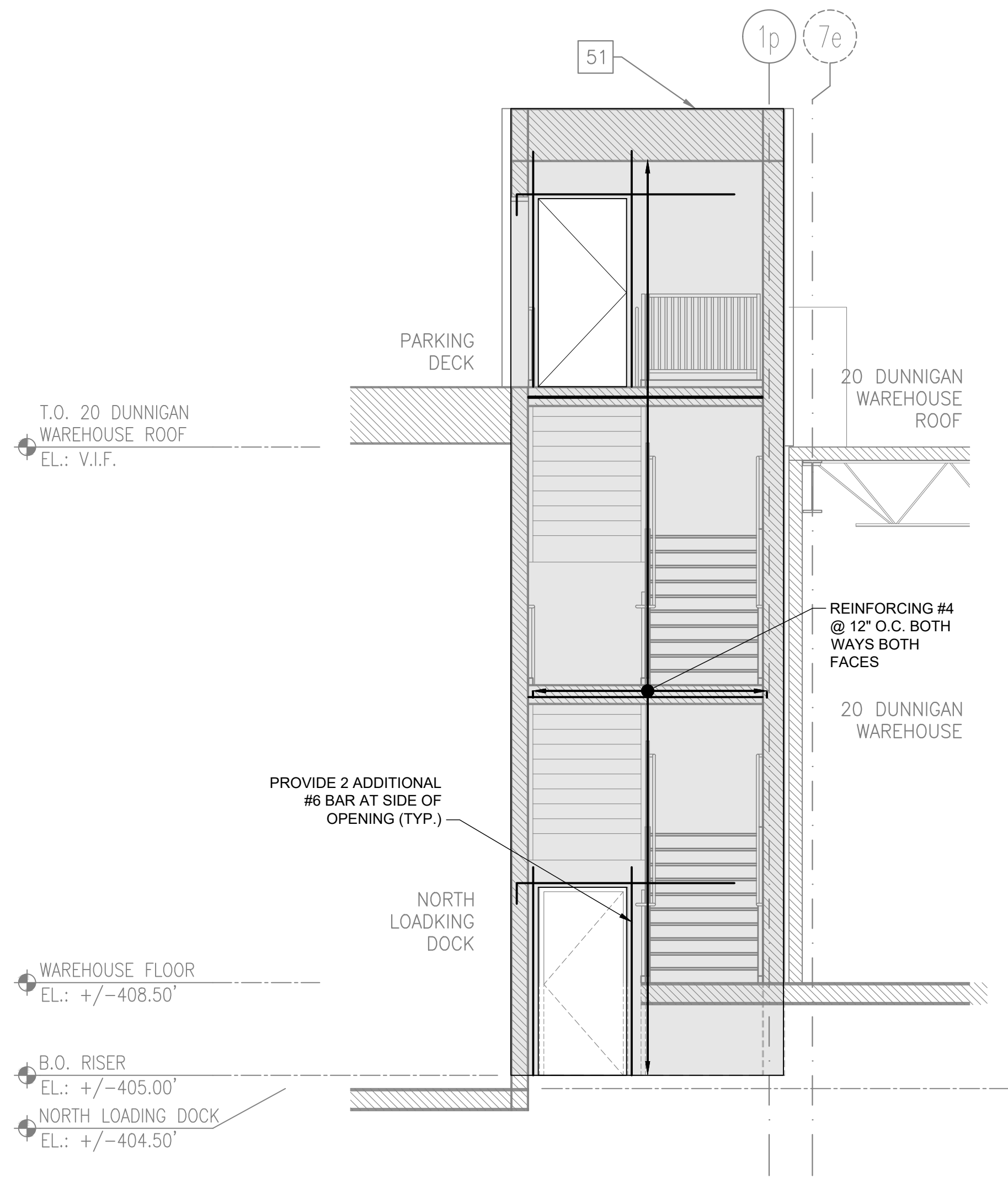
STAIR A - ELEVATIONS



DWG NUMBER :
S-211A
26 OF 61
4/4/22



1 STAIR A - ELEVATION (EAST)
210 SCALE: 1/4" = 1'-0"



2 STAIR A - ELEVATION (SOUTH)
210 SCALE: 1/4" = 1'-0"

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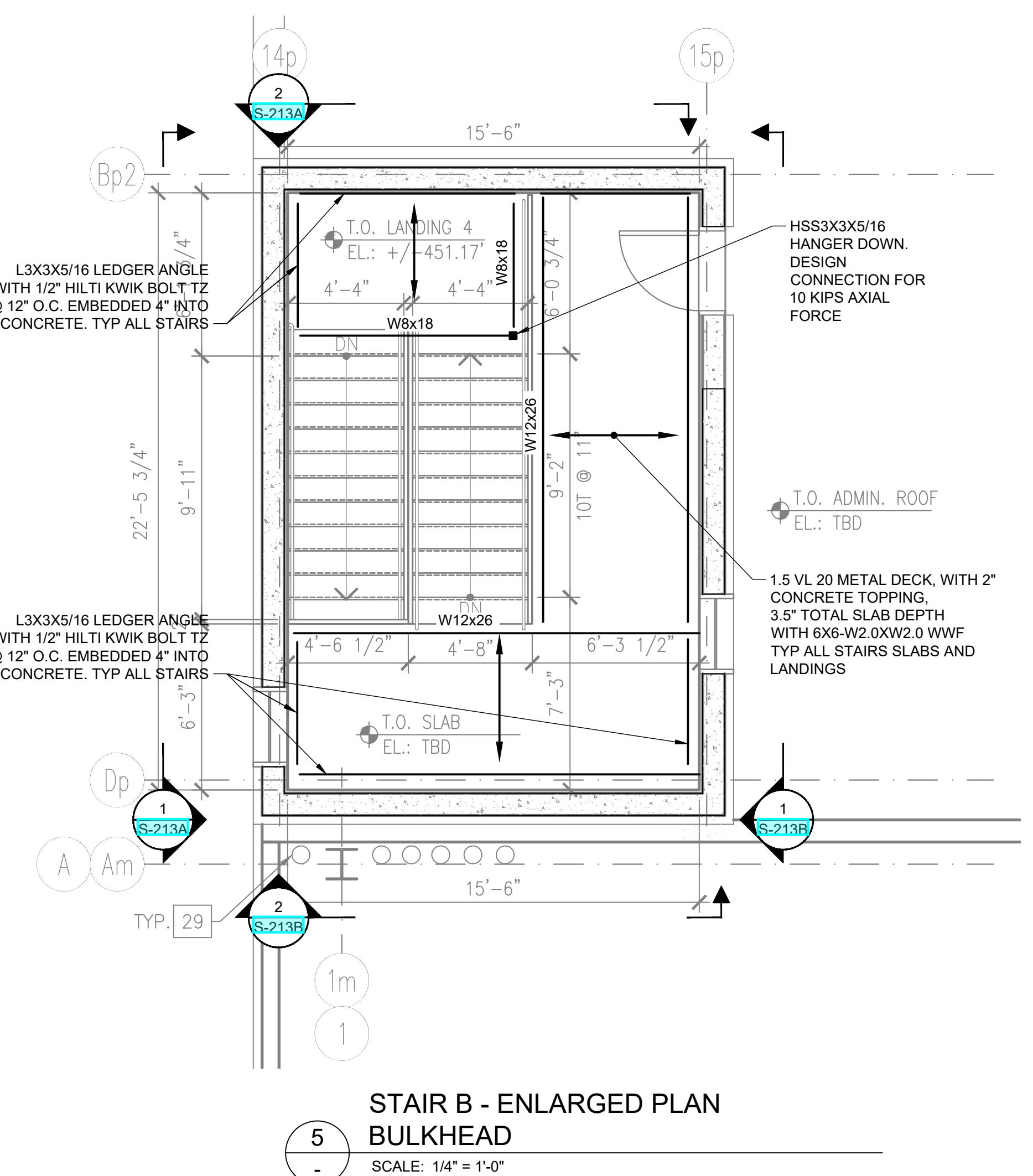
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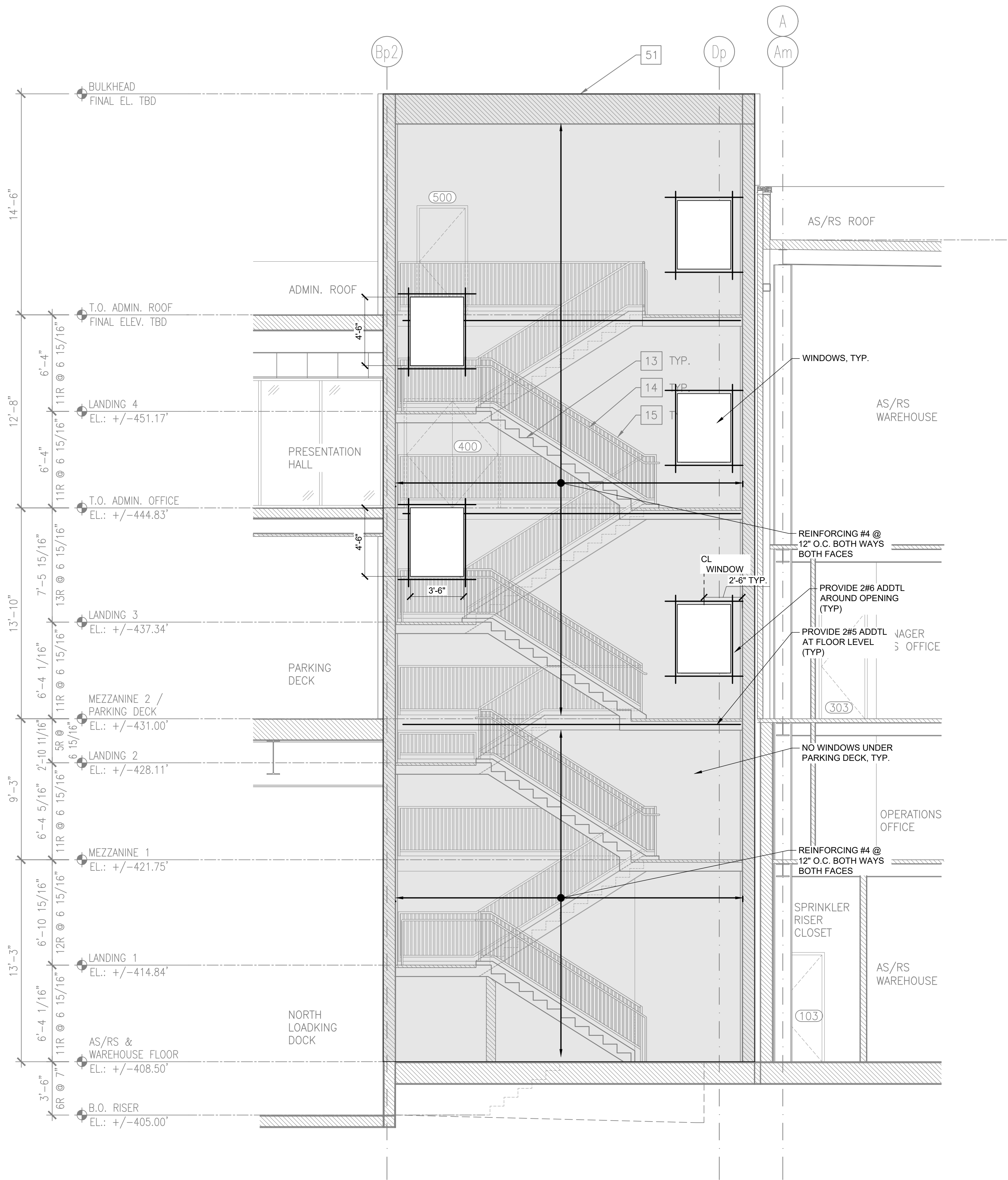
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STAIR A - ELEVATIONS

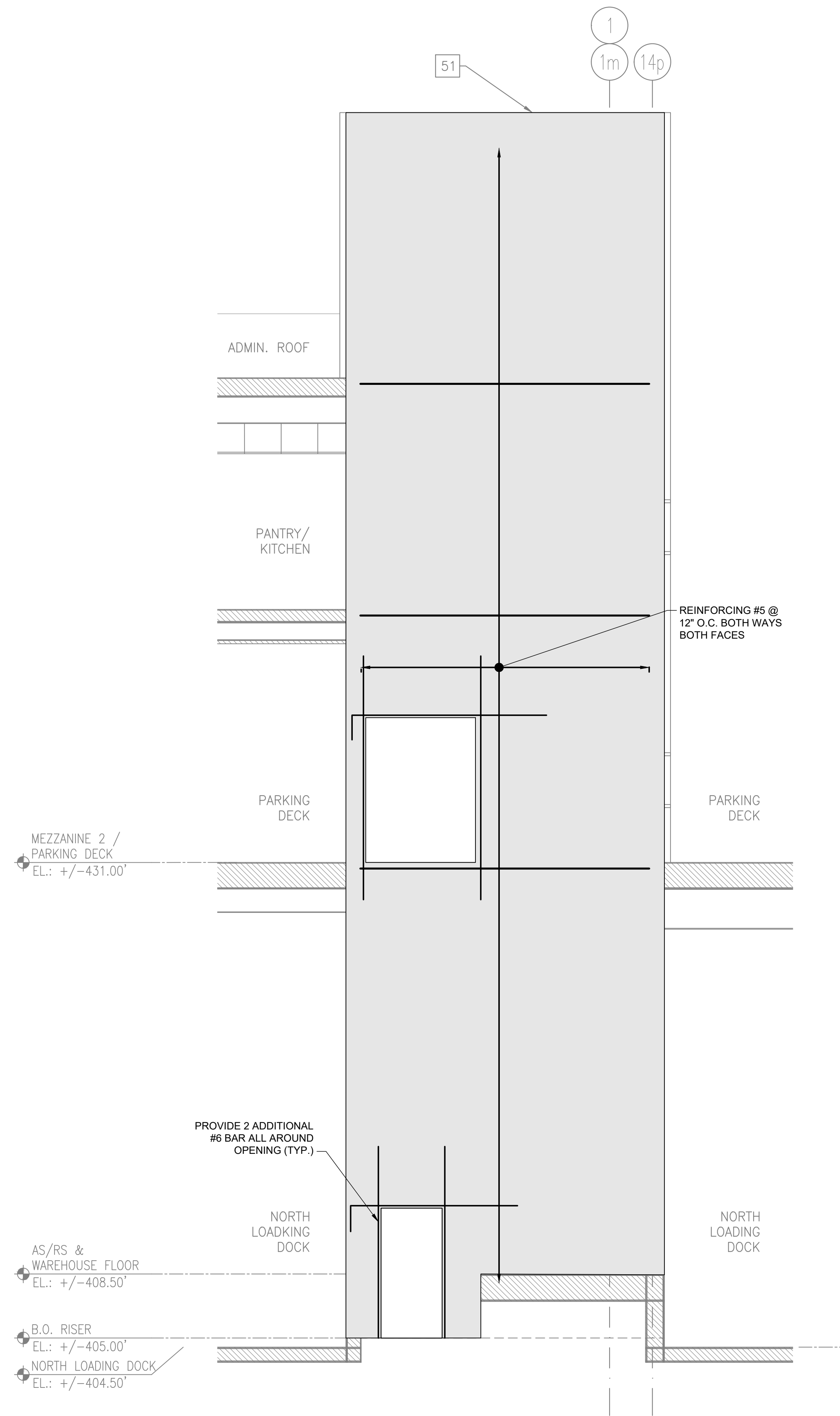


DWG NUMBER :
S-211B
27 OF 61





1 STAIR B - ELEVATION (WEST)
212 SCALE: 1/4" = 1'-0"



2 STAIR B - ELEVATION (NORTH)
212 SCALE: 1/4" = 1'-0"

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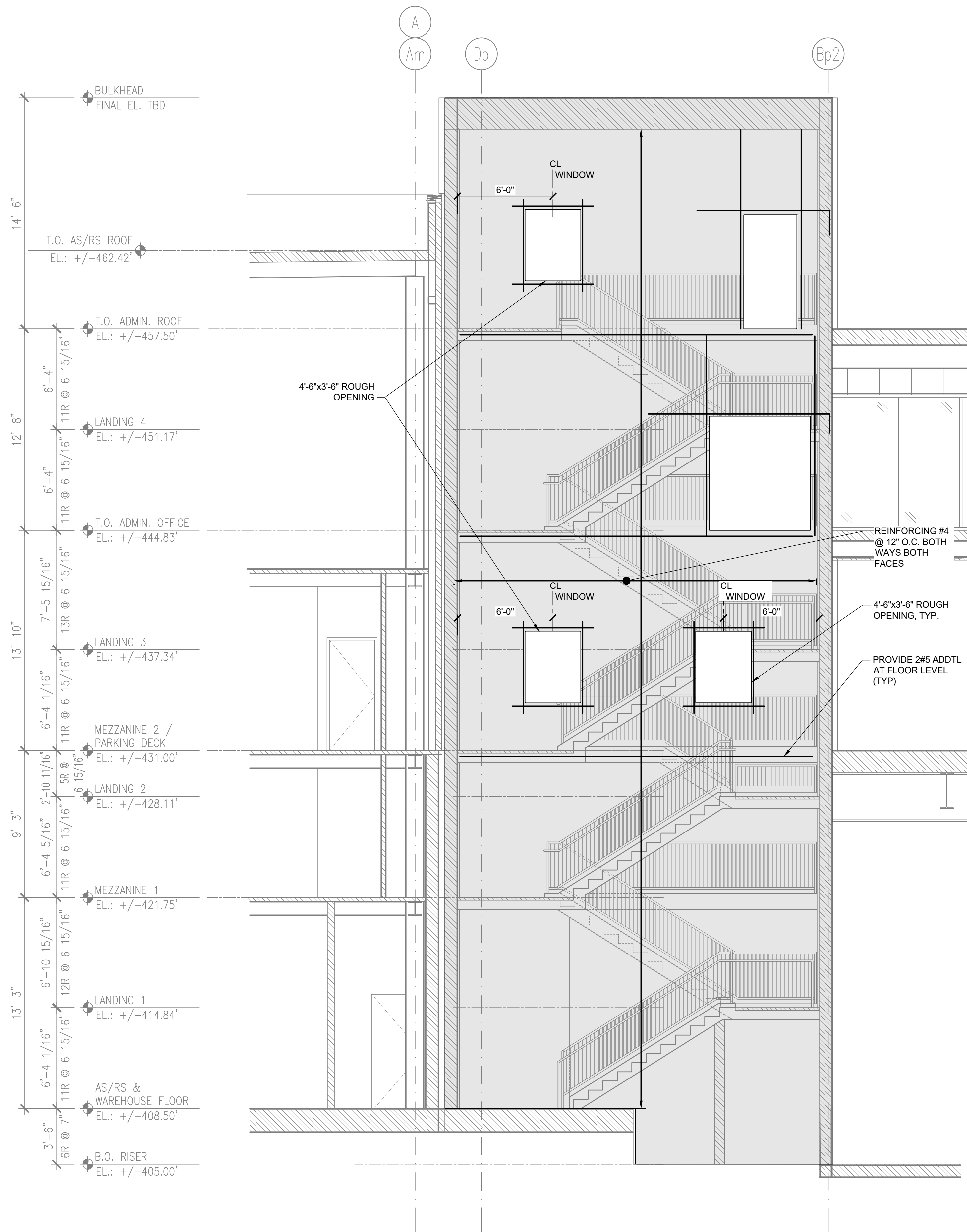
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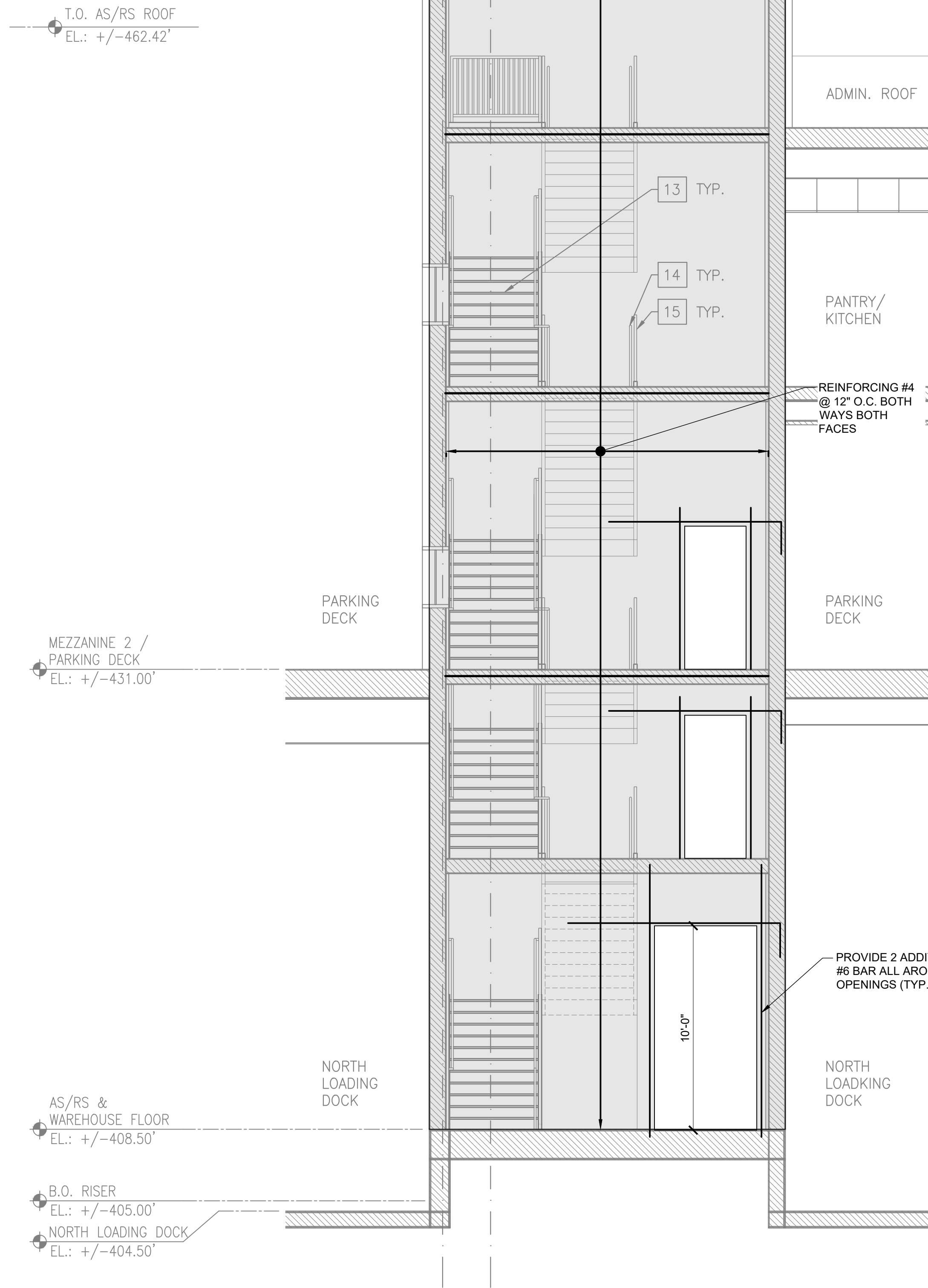
STAIR B - ELEVATIONS



DWG NUMBER :
S-213A
29 OF 61
4/4/22



1 STAIR B - ELEVATION (EAST)
SCALE: 1/4" = 1'-0"



2 STAIR B - ELEVATION (SOUTH)
SCALE: 1/4" = 1'-0"

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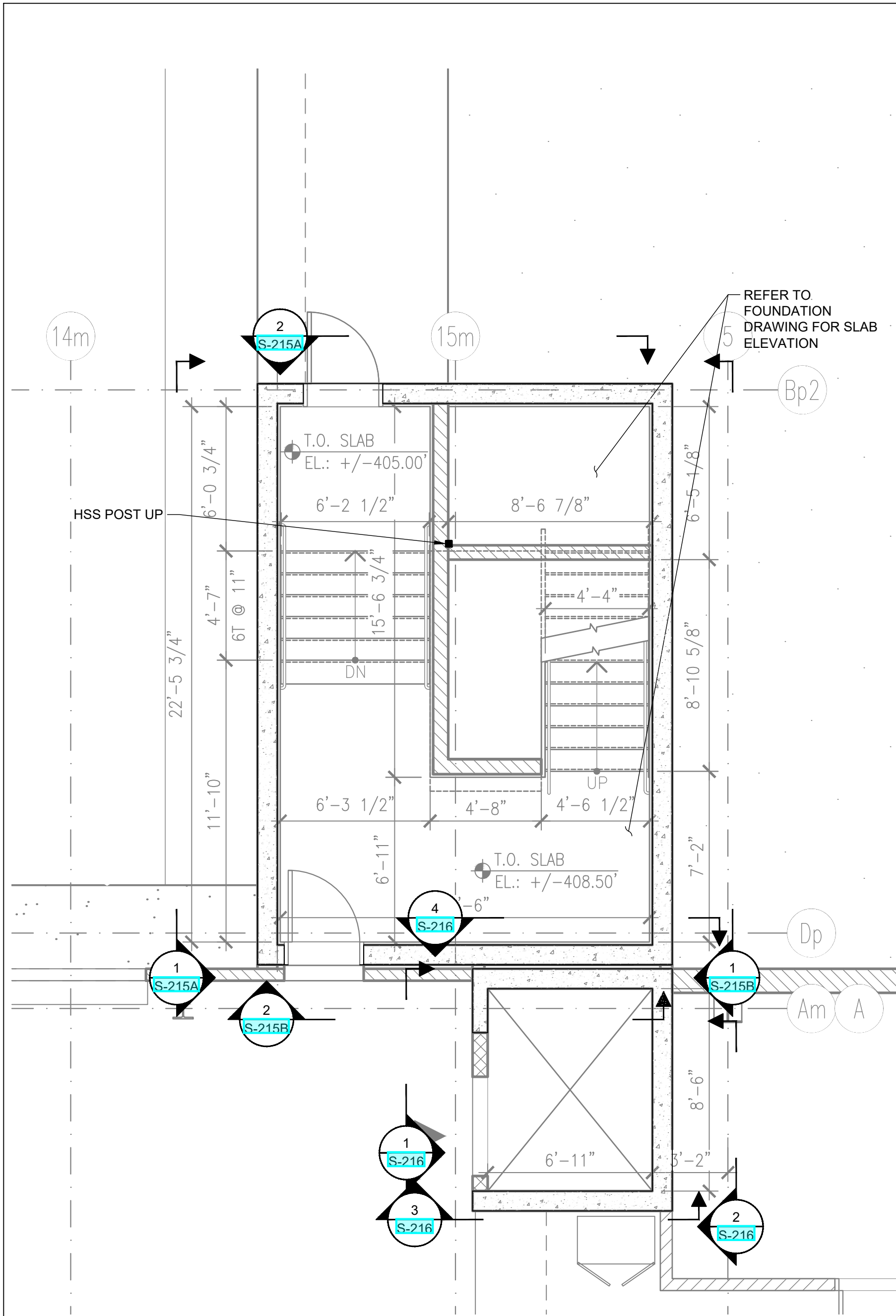
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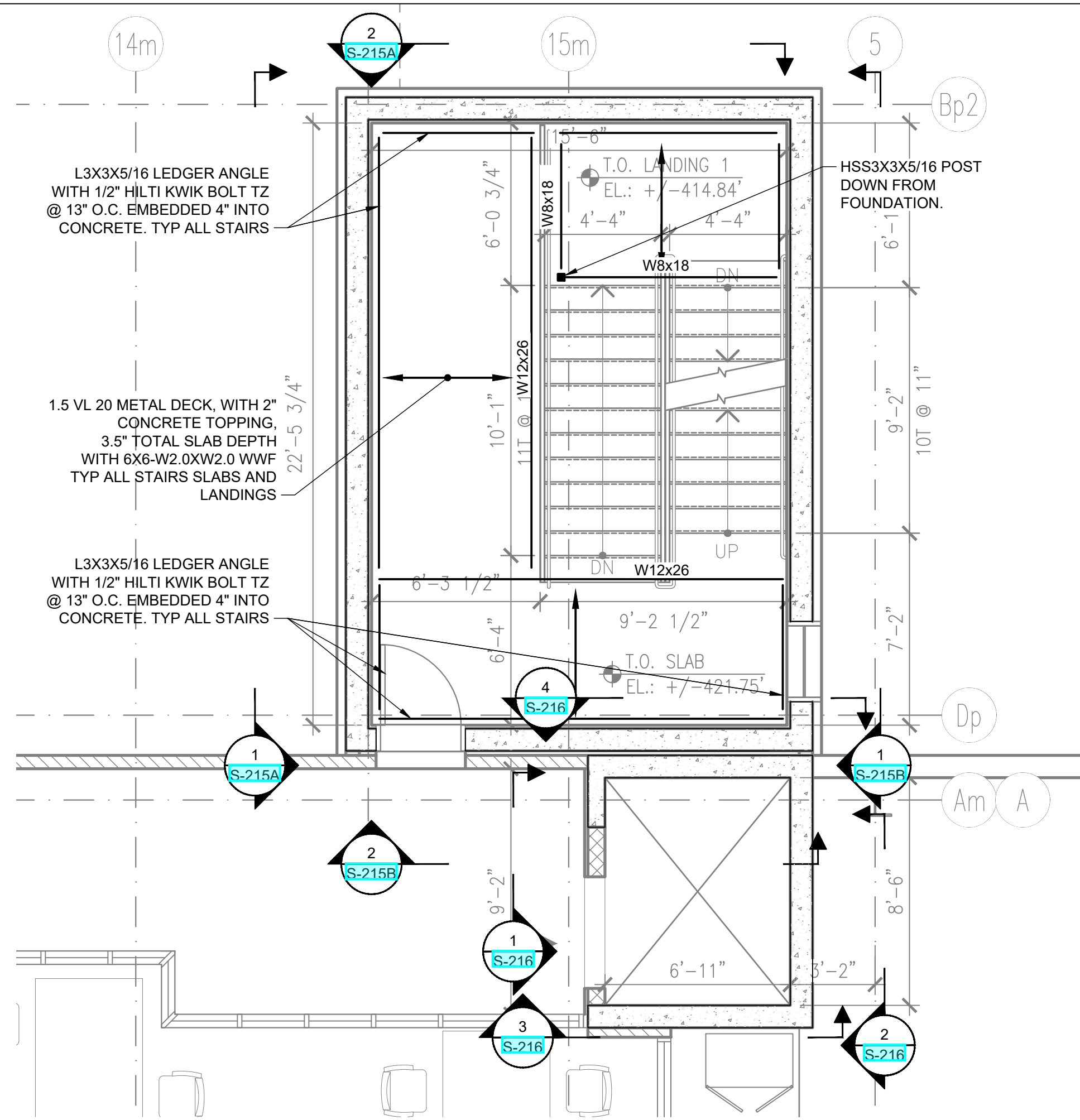
STAIR B - ELEVATIONS



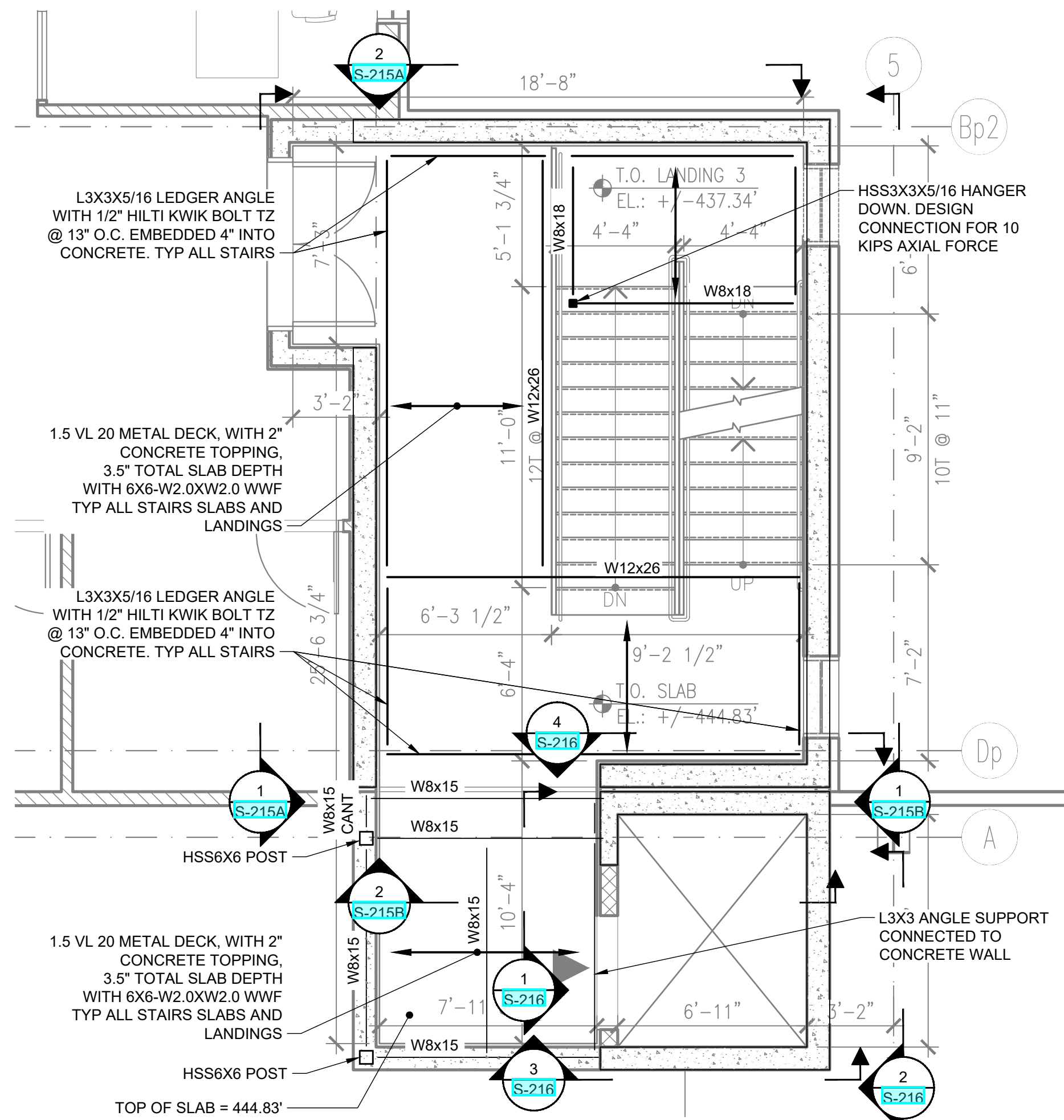
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S-213B
30 OF 61



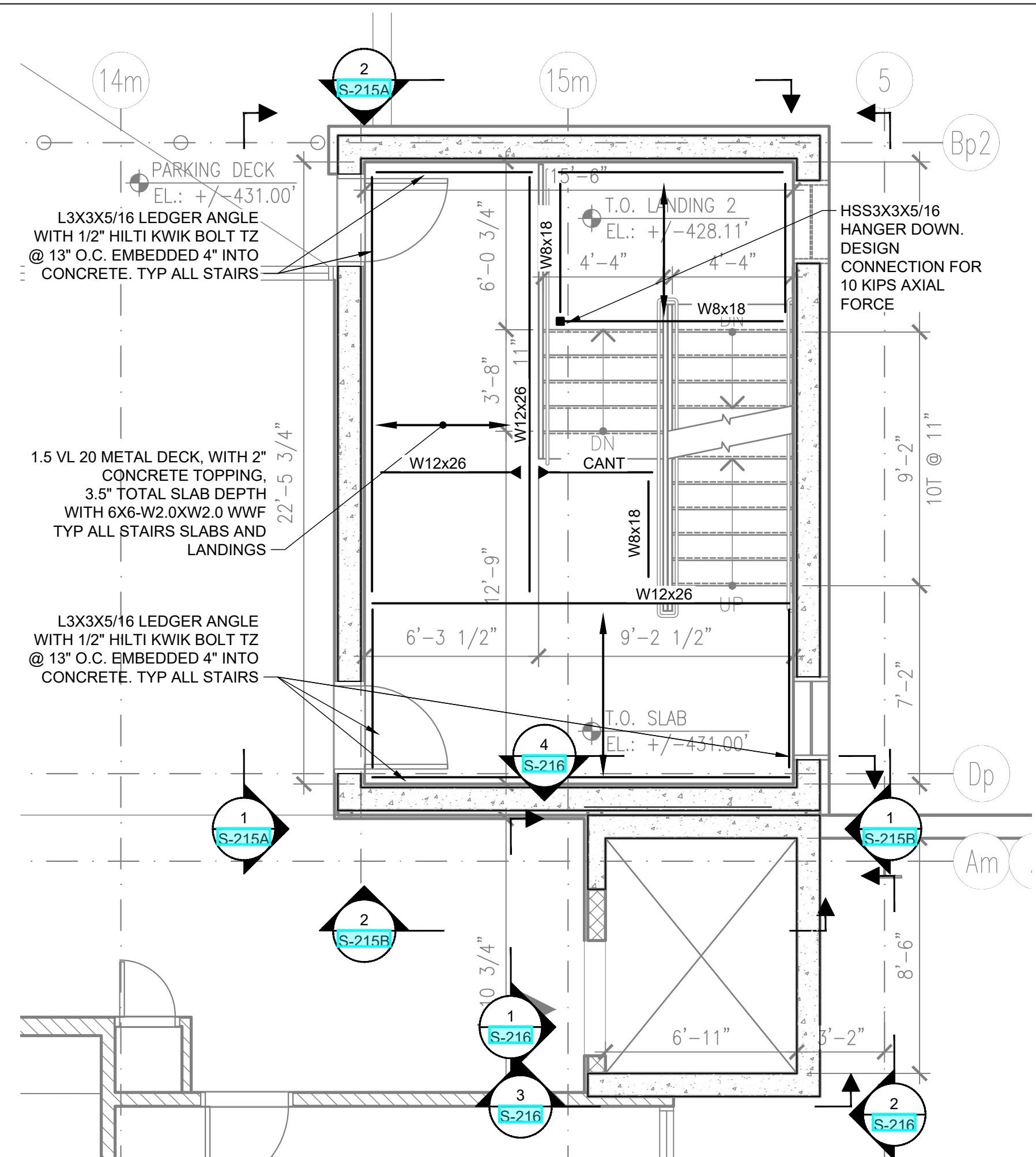
1 STAIR C - ENLARGED PLAN - WAREHOUSE LEVEL
SCALE: 1/4" = 1'-0"



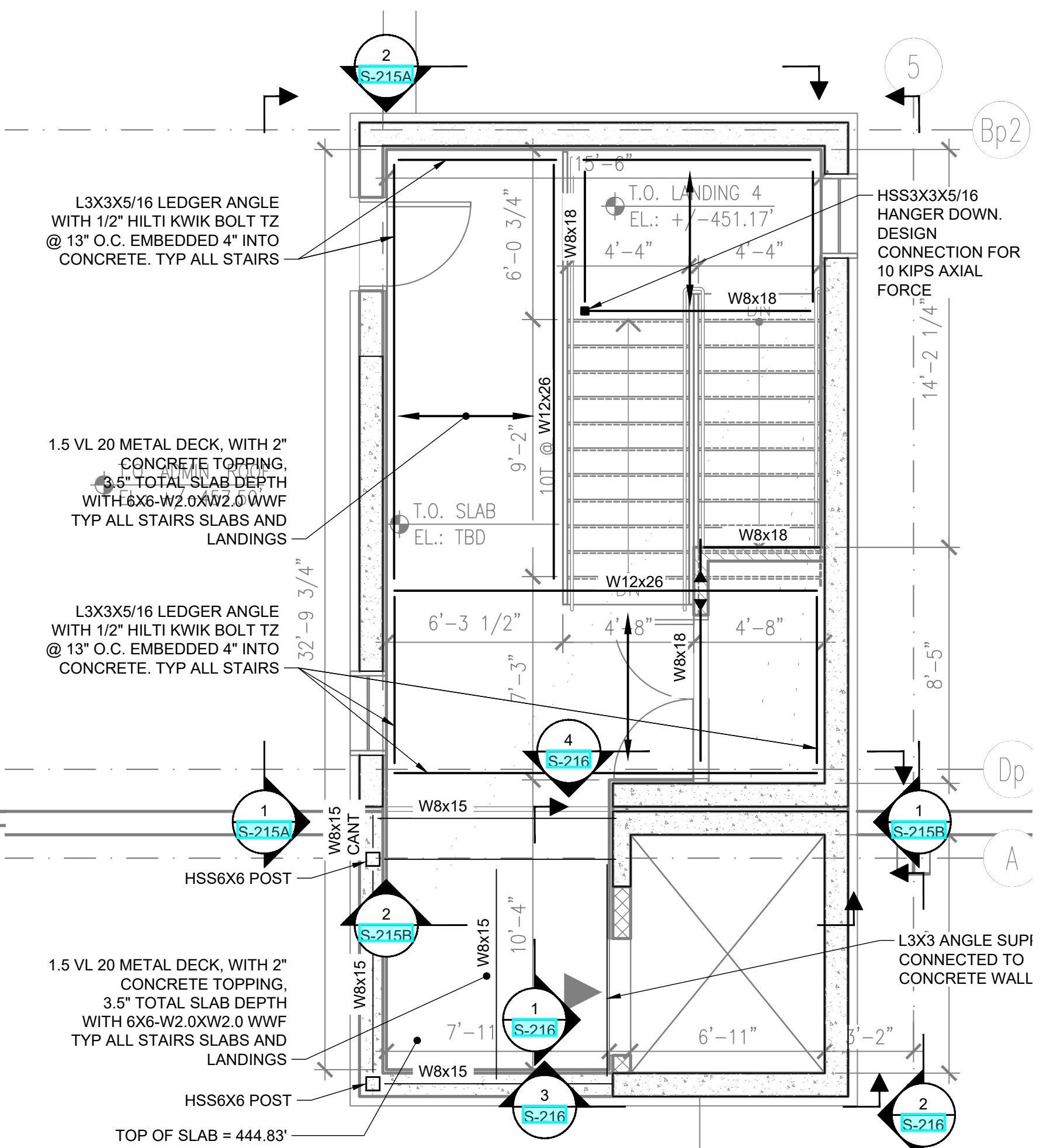
2 STAIR C - ENLARGED PLAN - 1ST MEZZ LEVEL
SCALE: 1/4" = 1'-0"



4 STAIR C - ENLARGED PLAN - ADMIN. OFFICE LEVEL
SCALE: 1/4" = 1'-0"



3 STAIR C - ENLARGED PLAN - 2ND MEZZ. AND PARKING DECK LEVEL
SCALE: 1/4" = 1'-0"



5 STAIR C - ENLARGED PLAN - BULKHEAD
SCALE: 1/4" = 1'-0"

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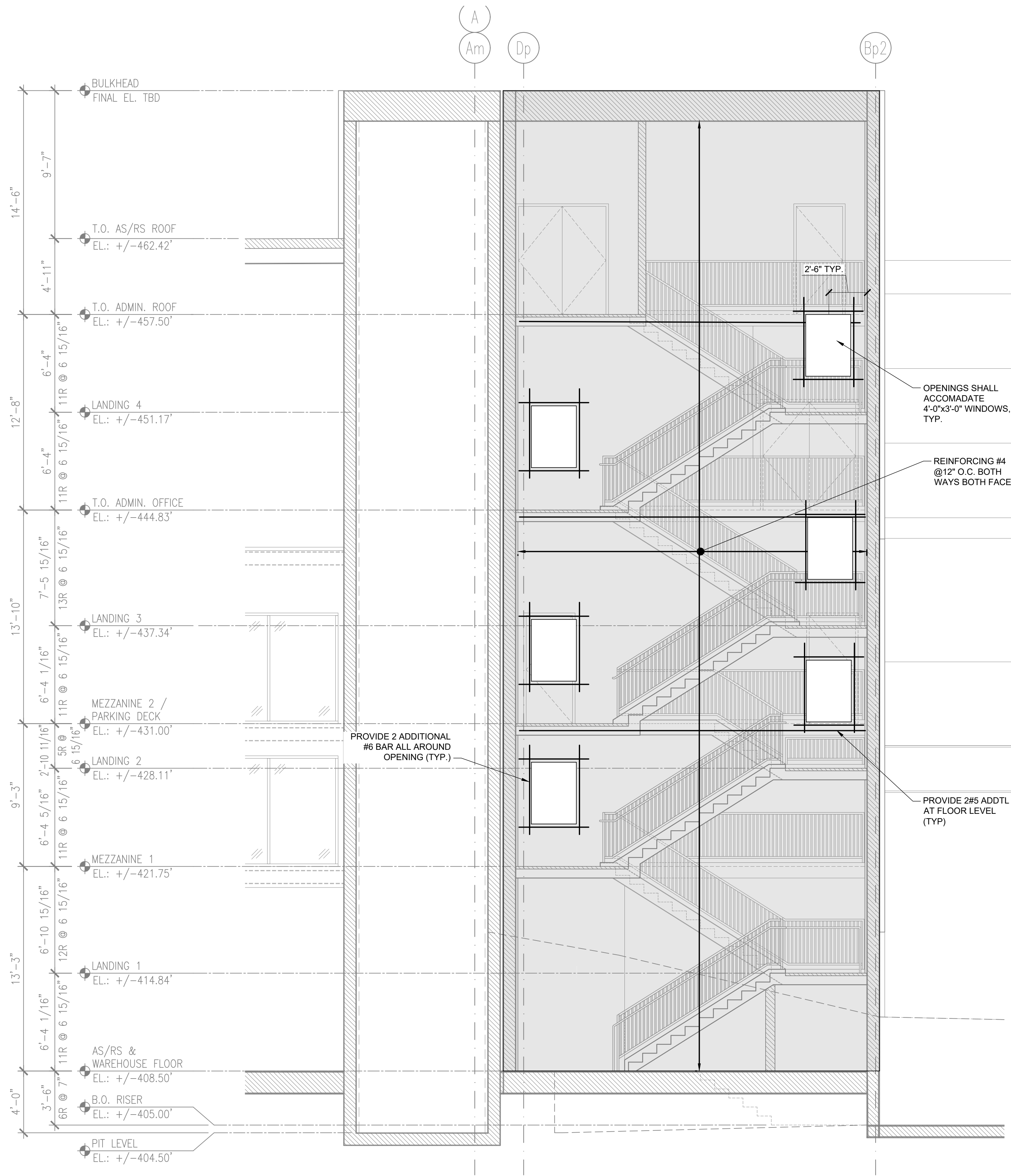
STAIR C - PLANS



DWG NUMBER:
S-214

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4/4/22



1 STAIR C - ELEVATION (EAST)
214 SCALE: 1/4" = 1'-0"



2 STAIR C - ELEVATION (NORTH)
214 SCALE: 1/4" = 1'-0"

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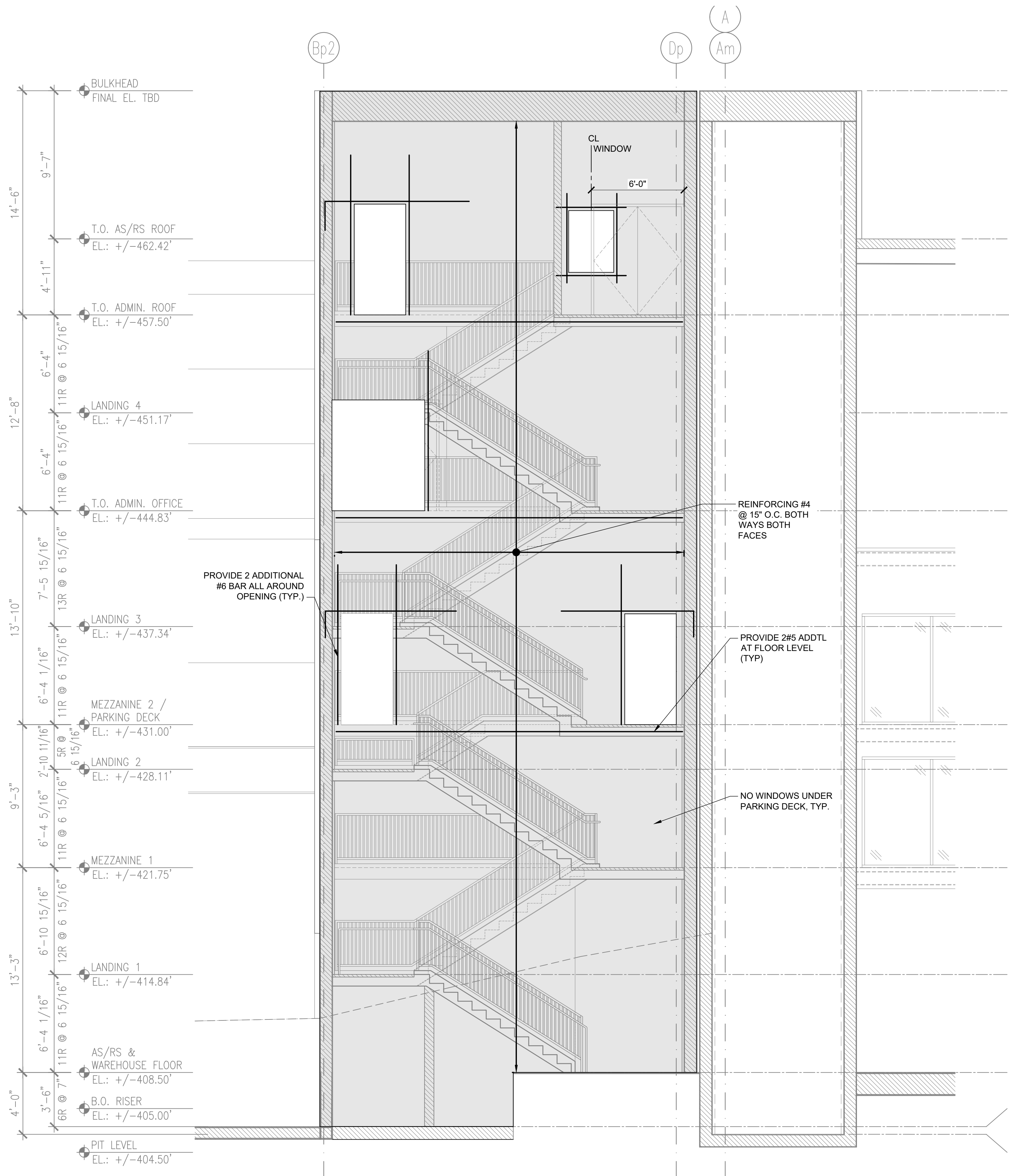
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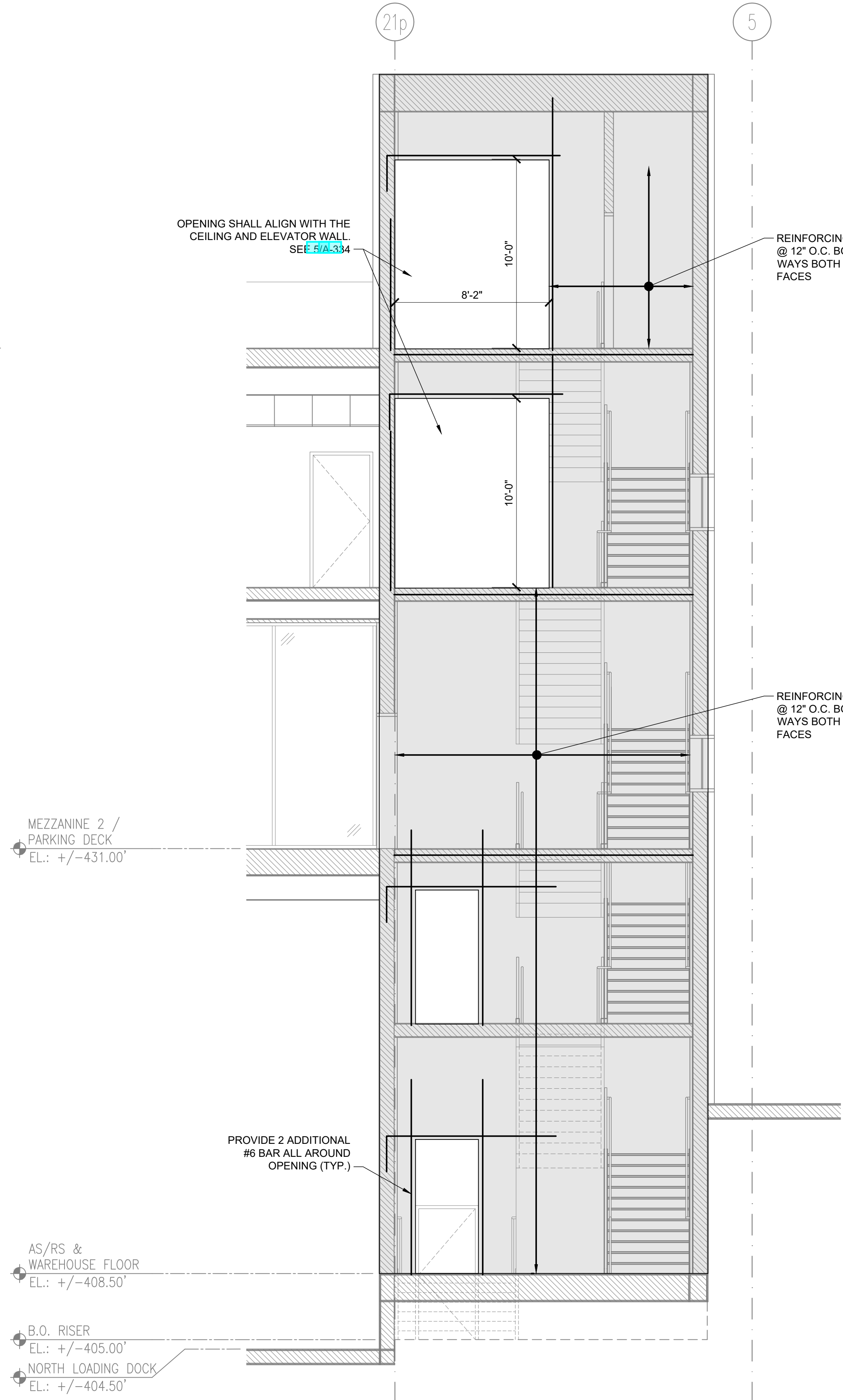
STAIR C - ELEVATIONS



DWG NUMBER :
S-215A
32 OF 61



1 STAIR C - ELEVATION (WEST)
214 SCALE: 1/4" = 1'-0"



2 STAIR C - ELEVATION (SOUTH)
214 SCALE: 1/4" = 1'-0"

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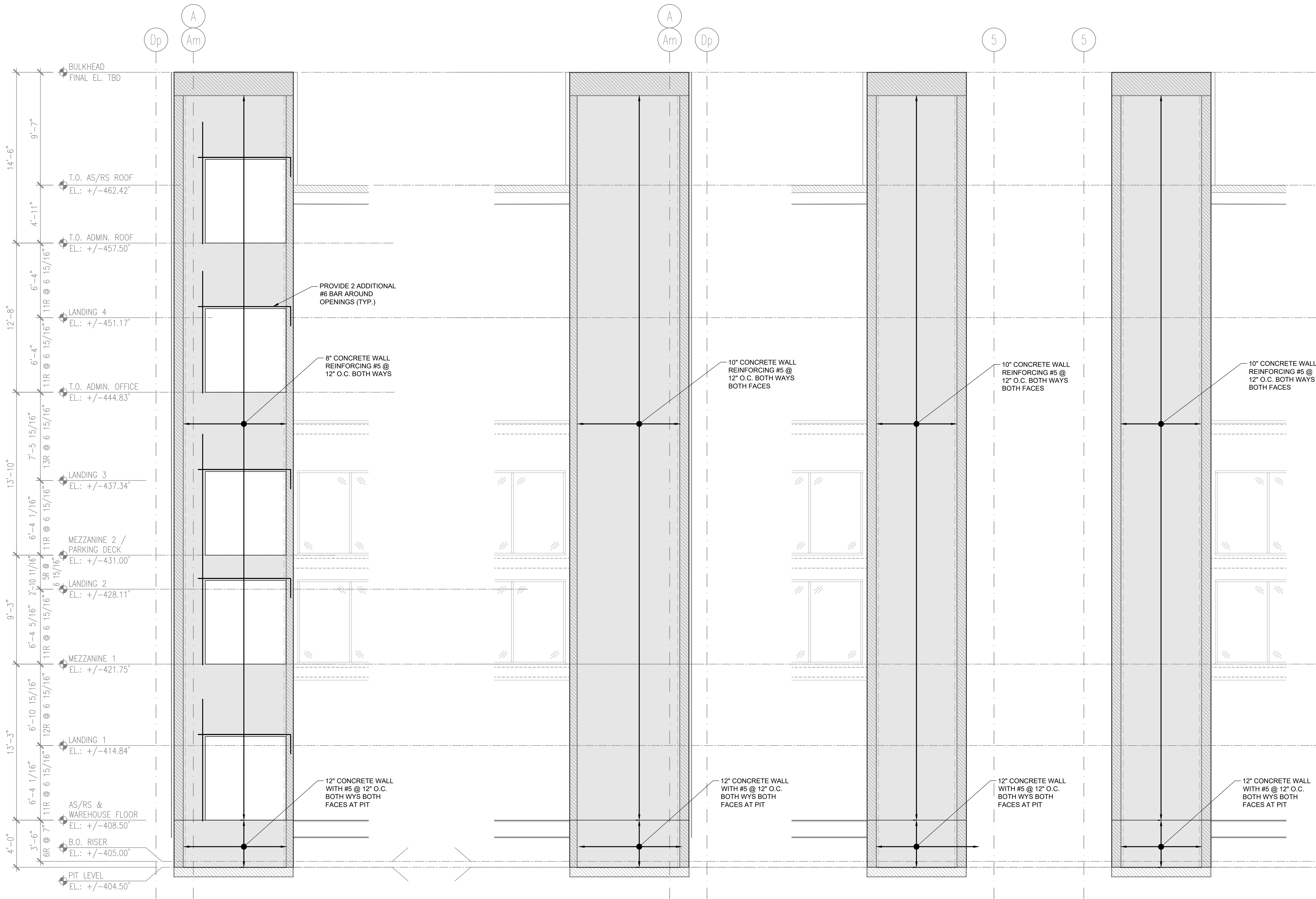
STAIR C - ELEVATIONS



4/4/22

DWG NUMBER :
S-215B

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1 ELEVATOR - ELEVATION (WEST)
214 SCALE: 1/4" = 1'-0"

2 ELEVATOR - ELEVATION (EAST)
214 SCALE: 1/4" = 1'-0"

3 ELEVATOR - ELEVATION (SOUTH)
- SCALE: 1/4" = 1'-0"

4 ELEVATOR - ELEVATION (NORTH)
- SCALE: 1/4" = 1'-0"

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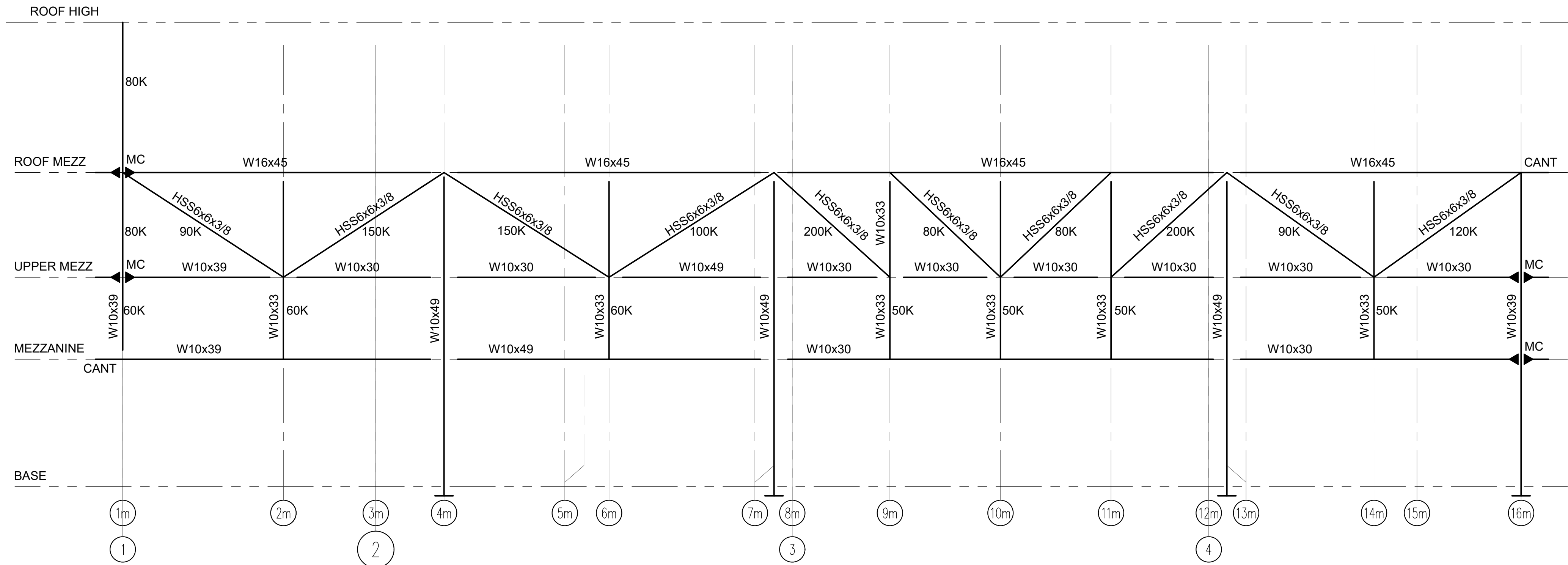
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DRAWING TITLE :

ELEVATOR ELEVATIONS

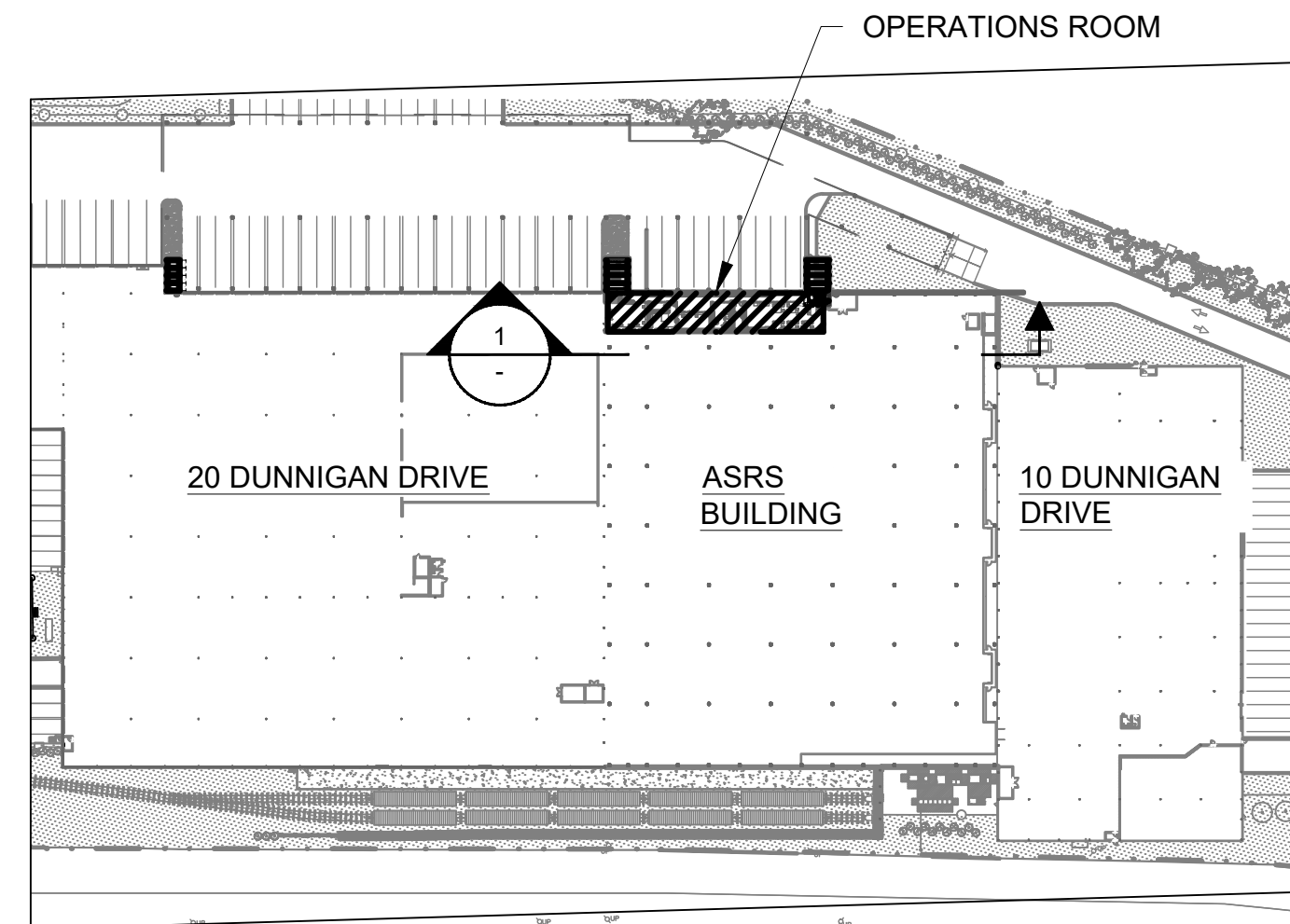


DWG NUMBER :
S-216
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ASRS OPERATIONS BUILDING SOUTH ELEVATION
ELEVATION Bm

SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

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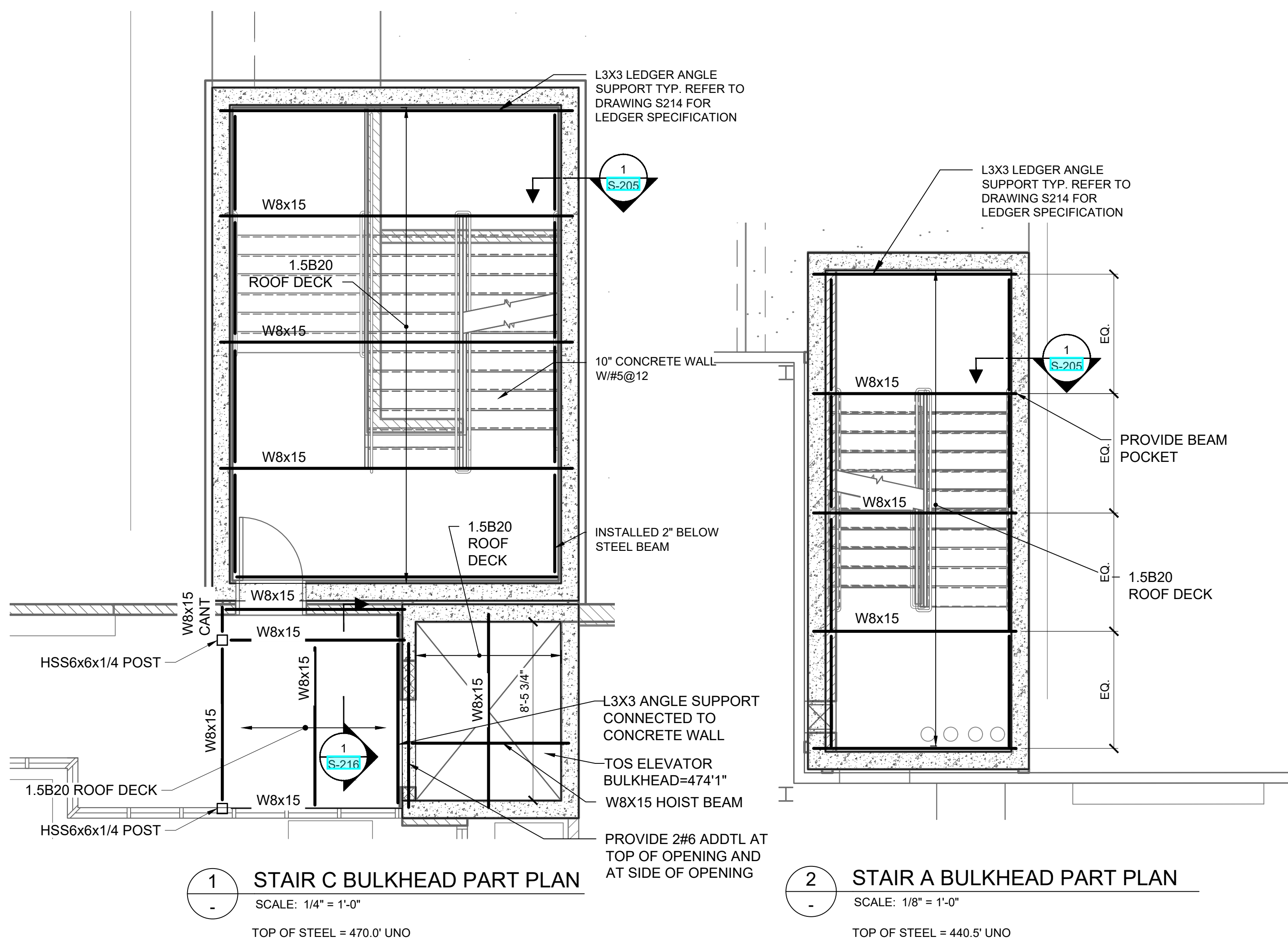
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STAIR C BULKHEAD PART PLAN

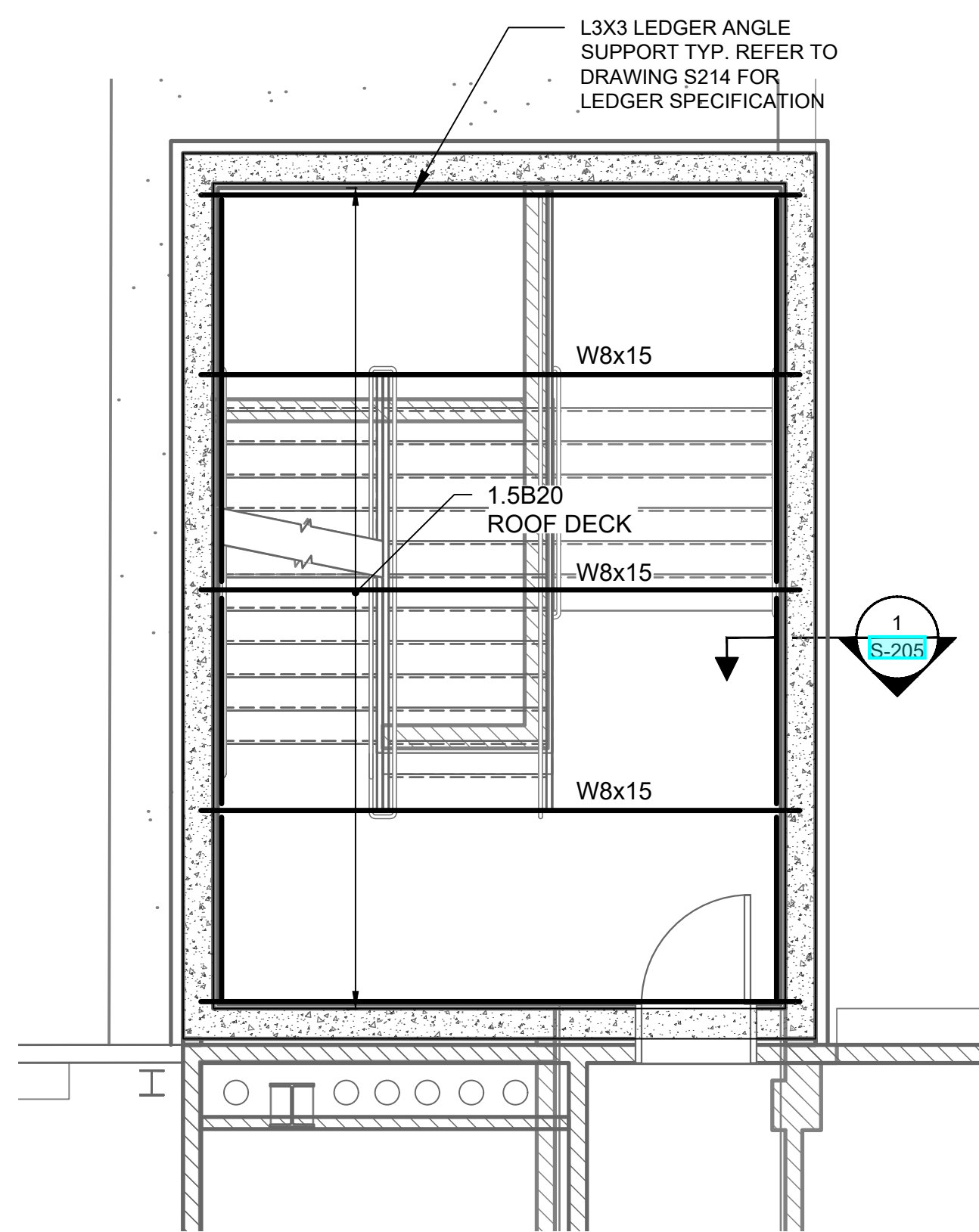
SCALE: 1/4" = 1'-0"

TOP OF STEEL = 470.0' UNO

STAIR A BULKHEAD PART PLAN

SCALE: 1/8" = 1'-0"

TOP OF STEEL = 440.5' UNO



STAIR B BULKHEAD PART PLAN

SCALE: 1/8" = 1'-0"

TOP OF STEEL = 470.0' UNO

- NOTES:
1. DESIGN ALL BRACING CONNECTIONS FOR THE AXIAL FORCES SHOWN IN ELEVATION AND THE SHEAR FORCES NOTED ON DRAWING S-001.
 2. REFER TO DRAWING S-401 FOR CONNECTION DETAILS.
 3. ALL FORCES SHOWN ARE ASD.
 4. MC INDICATES A MOMENT CONNECTION
 5. CANT INDICATES A CANTILEVERED BEAM

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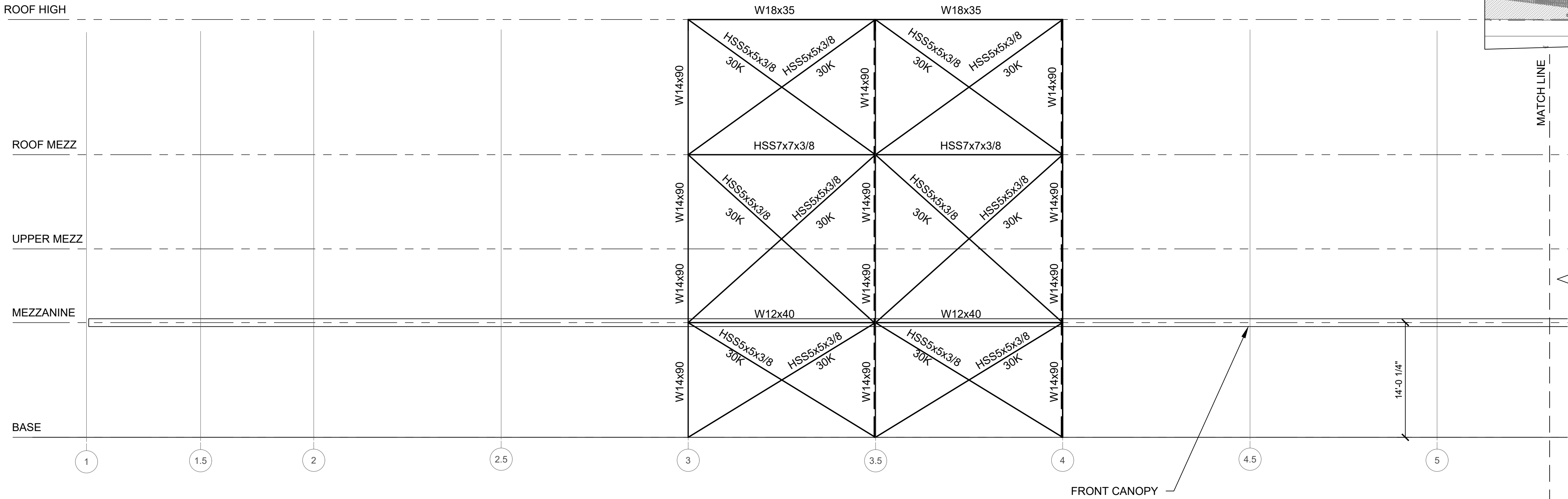
DRAWING TITLE :

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ELEVATIONS

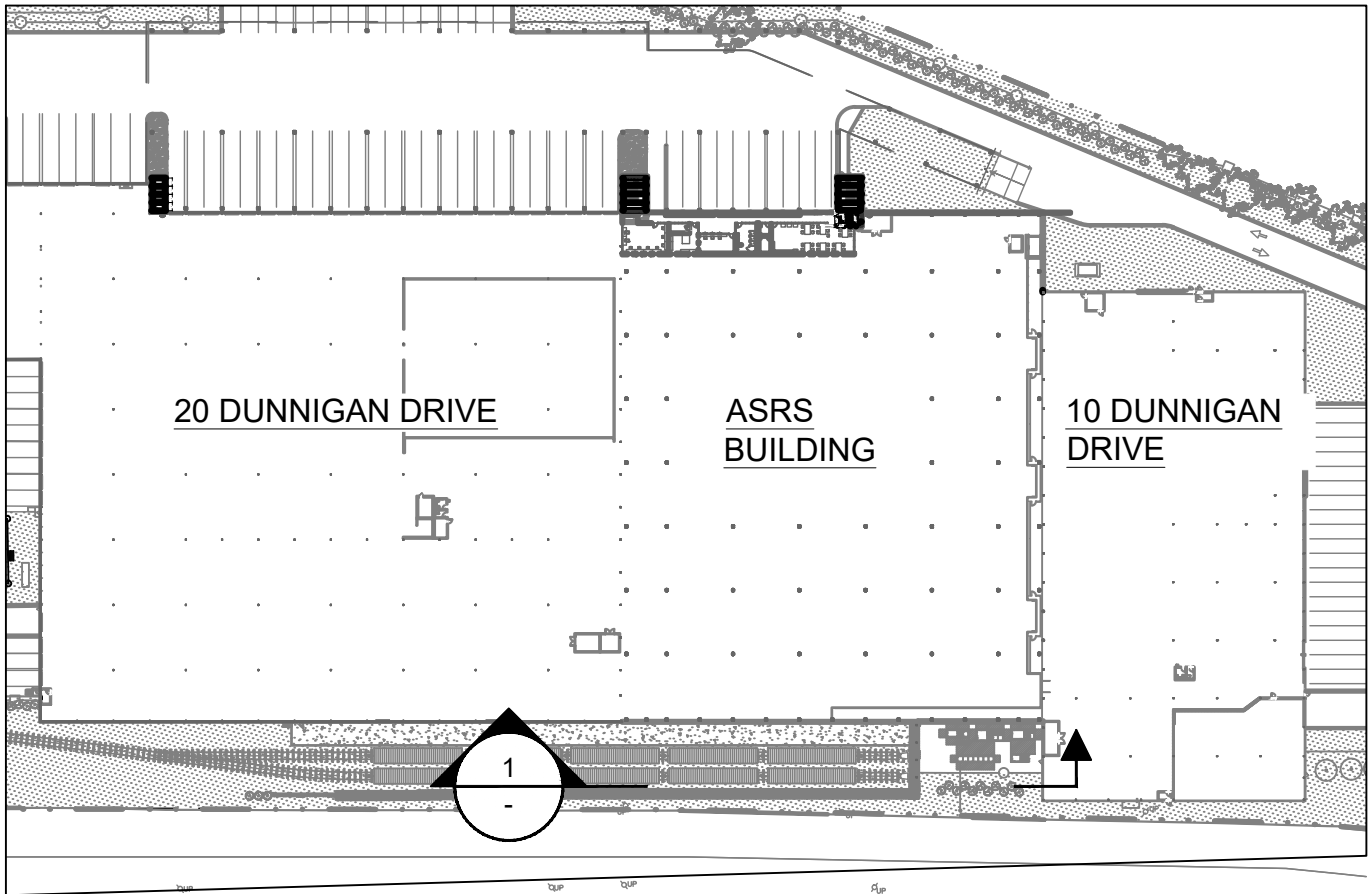


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S-300

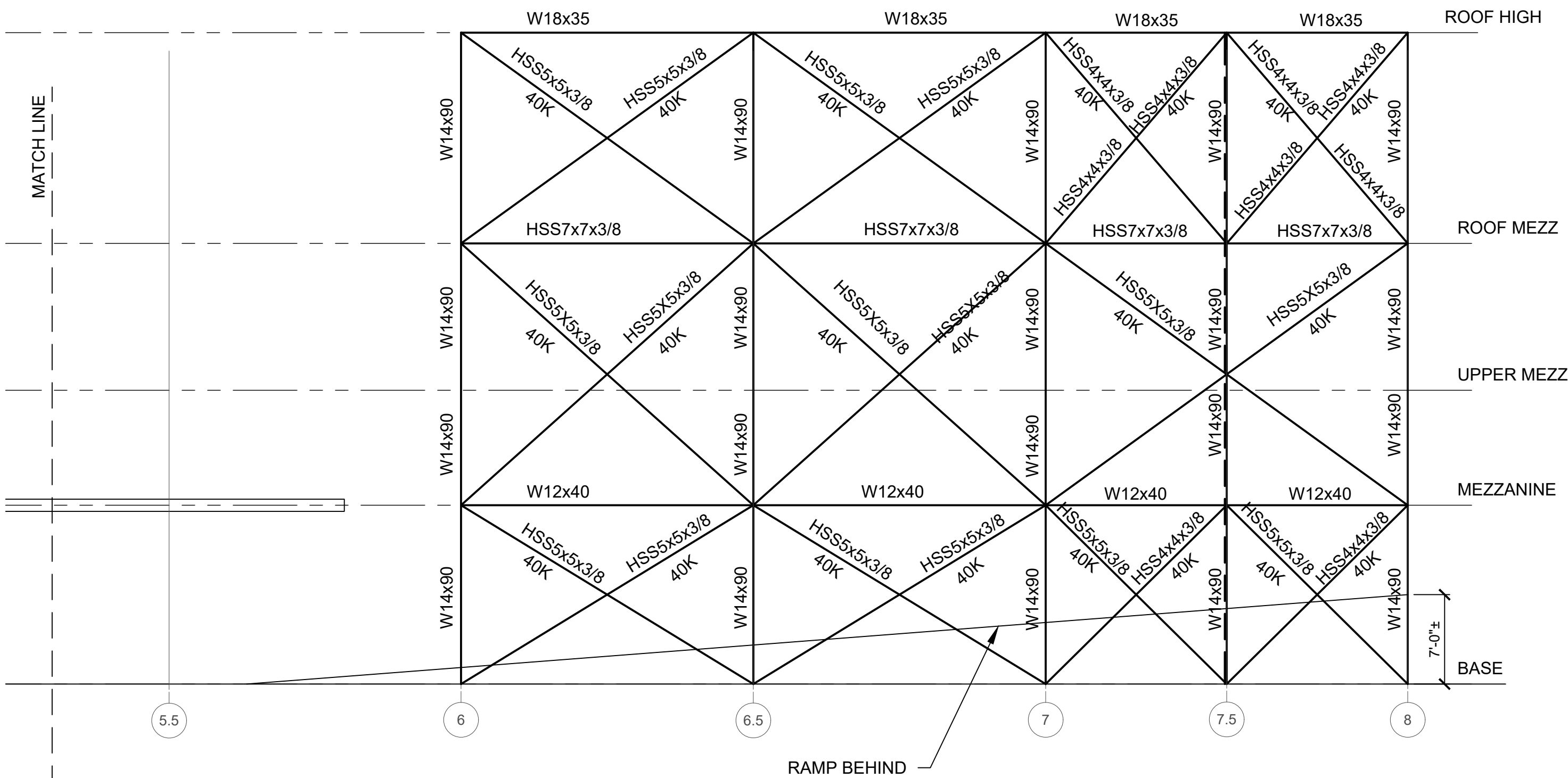
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ASRS BUILDING SOUTH ELEVATION
GRIDLINE I FRAME
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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DRAWING TITLE :

LATERAL FRAMING
ELEVATIONS



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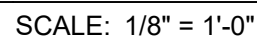
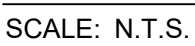
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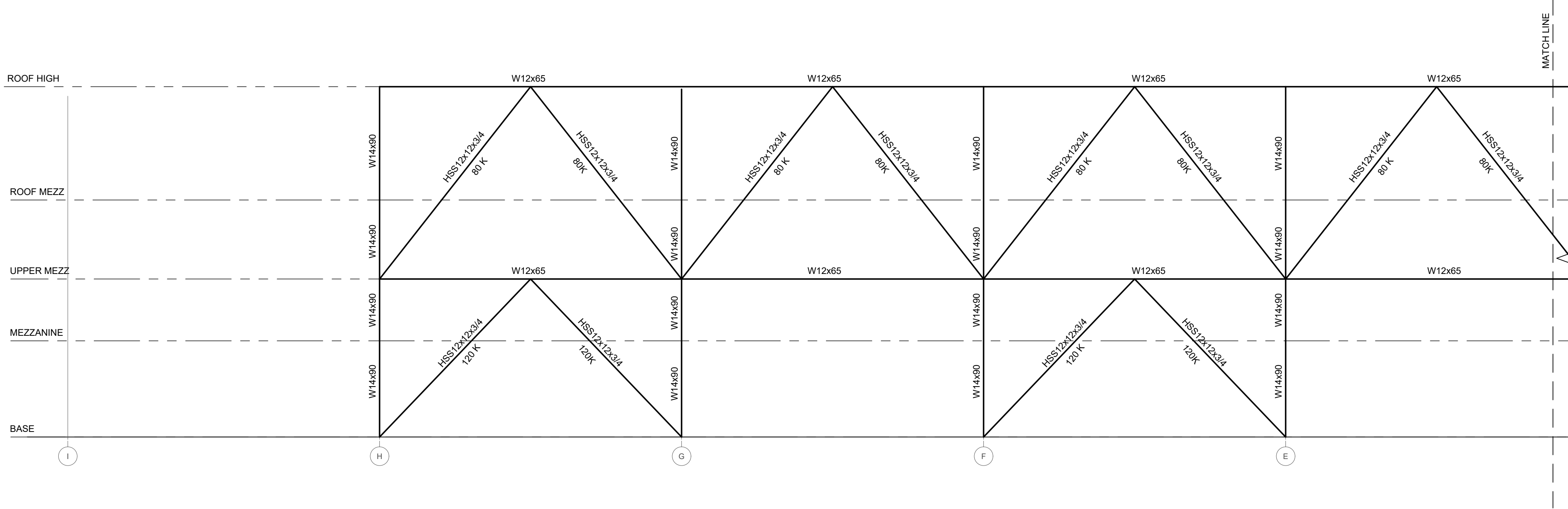
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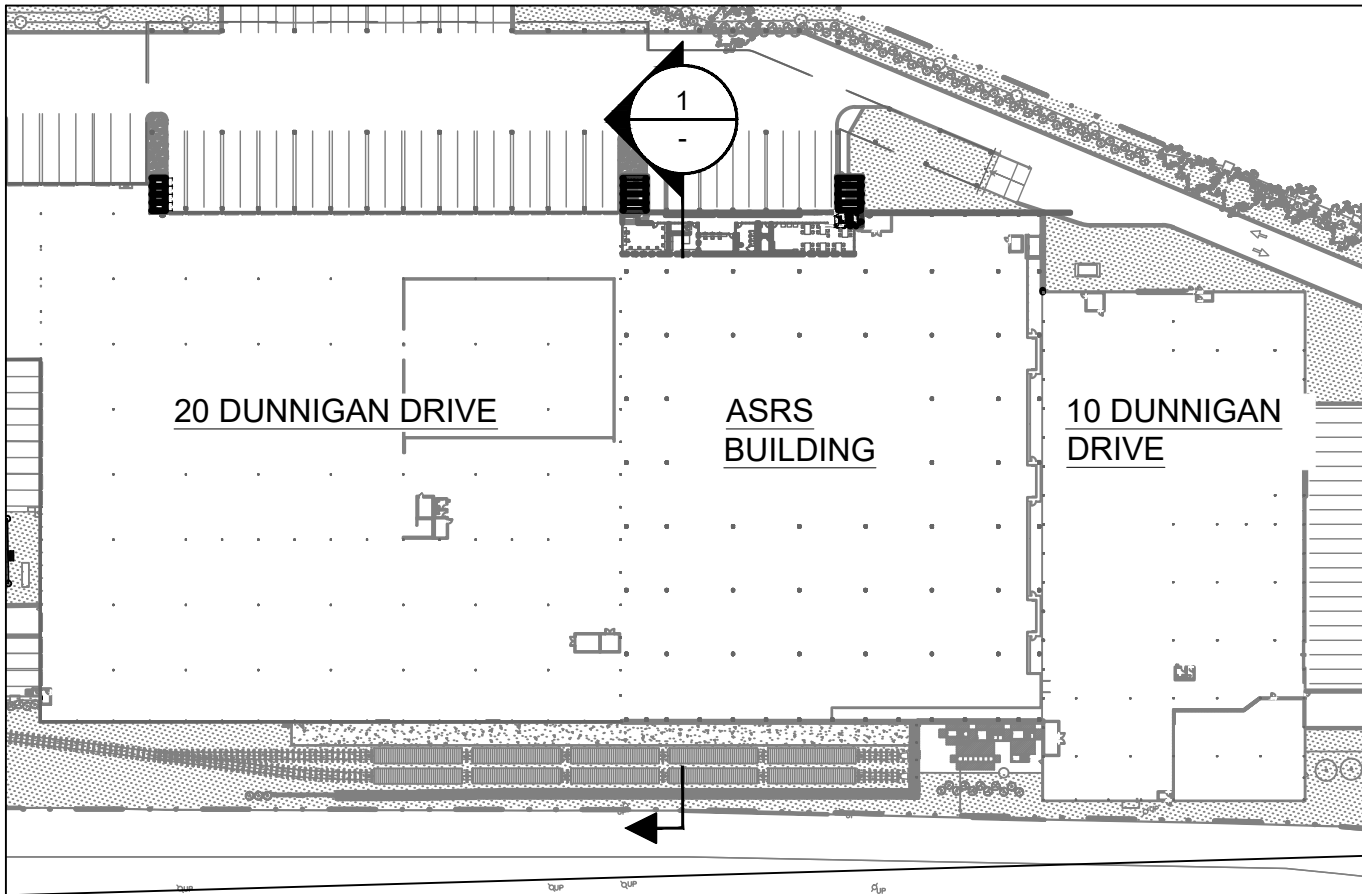
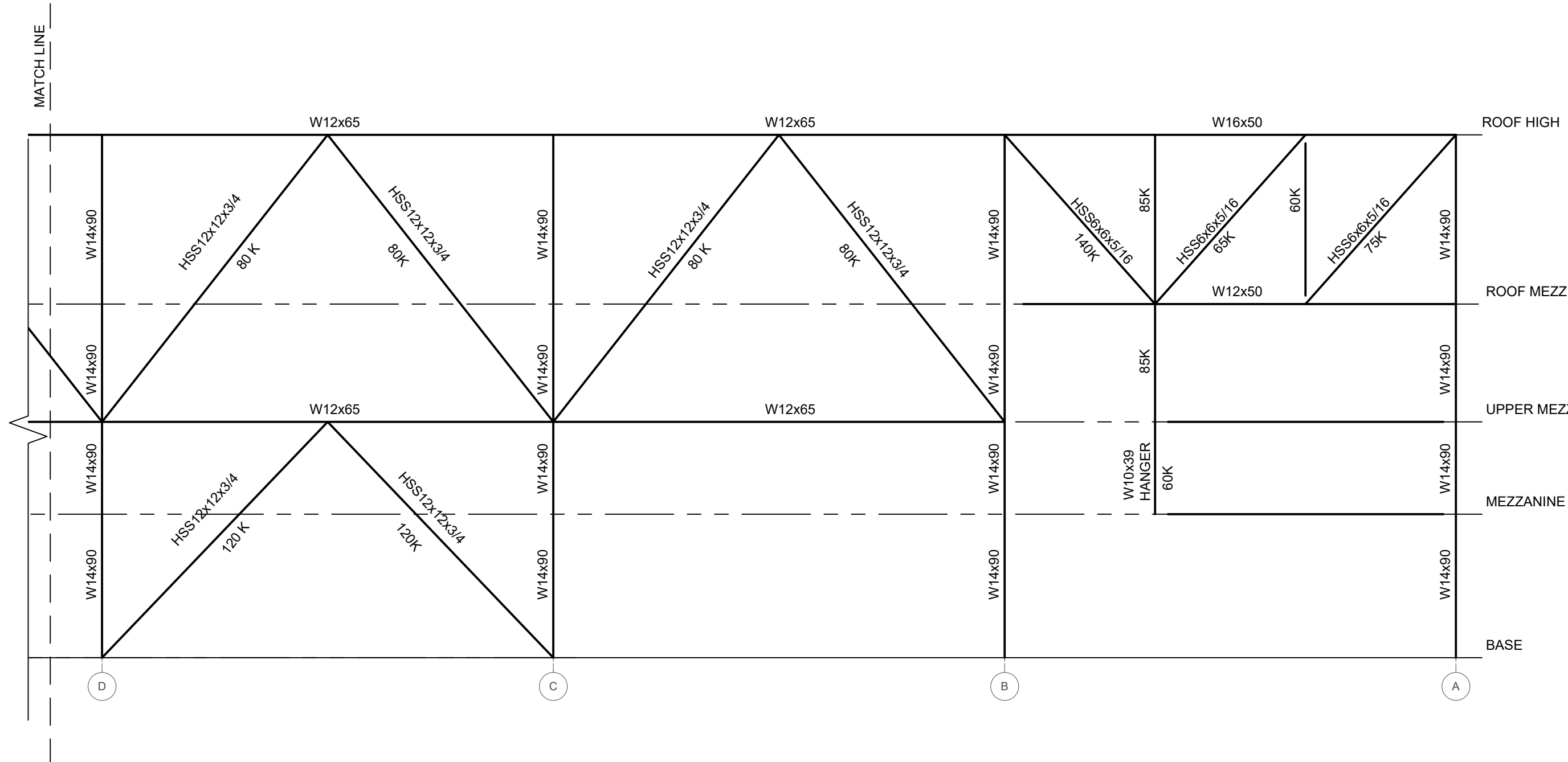
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LATERAL FRAMING ELEVATIONS





ASRS BUILDING WEST ELEVATION
GRIDLINE 1 FRAME
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.

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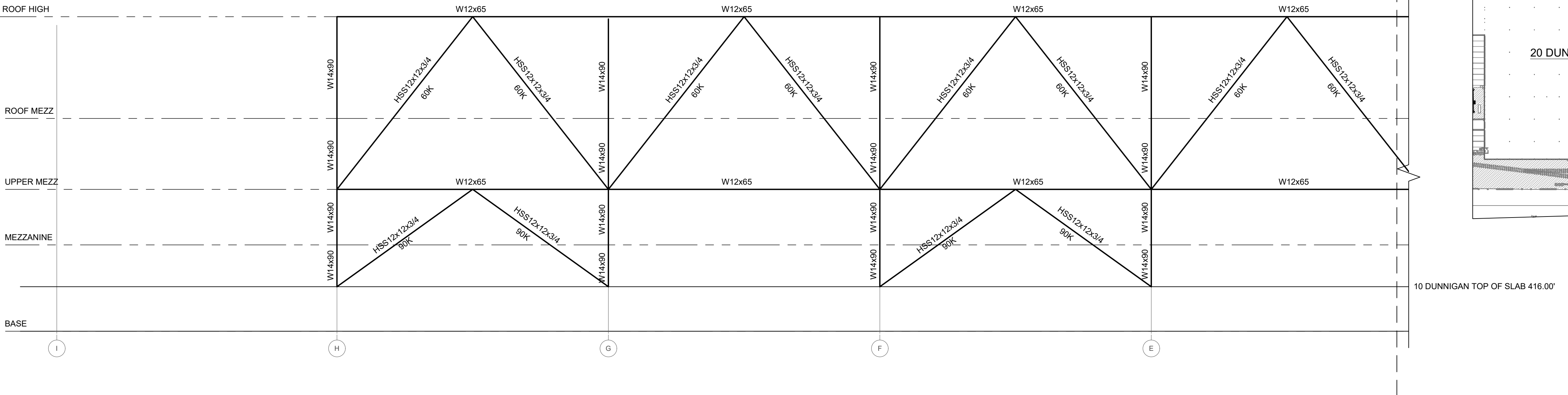
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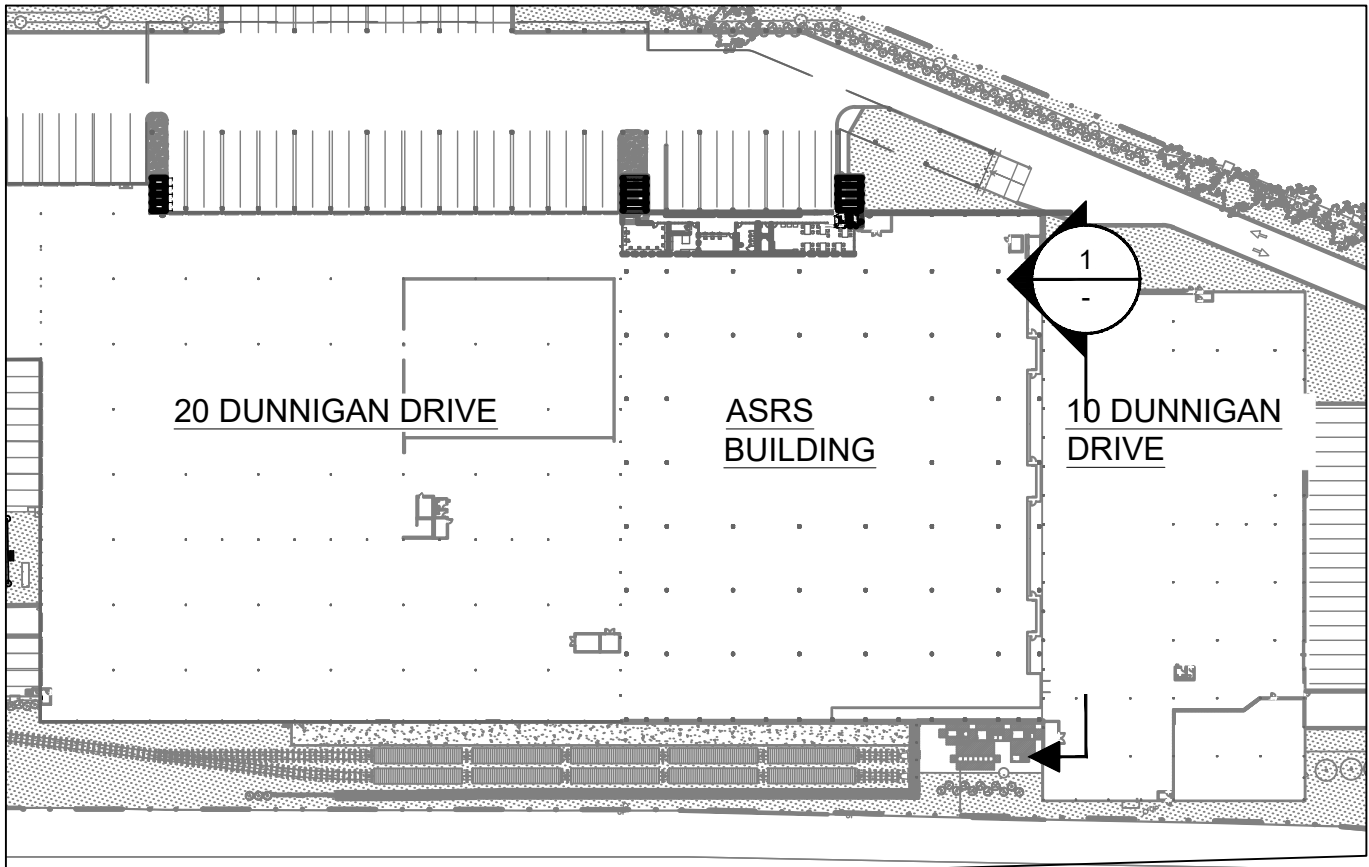
LATERAL FRAMING
ELEVATIONS



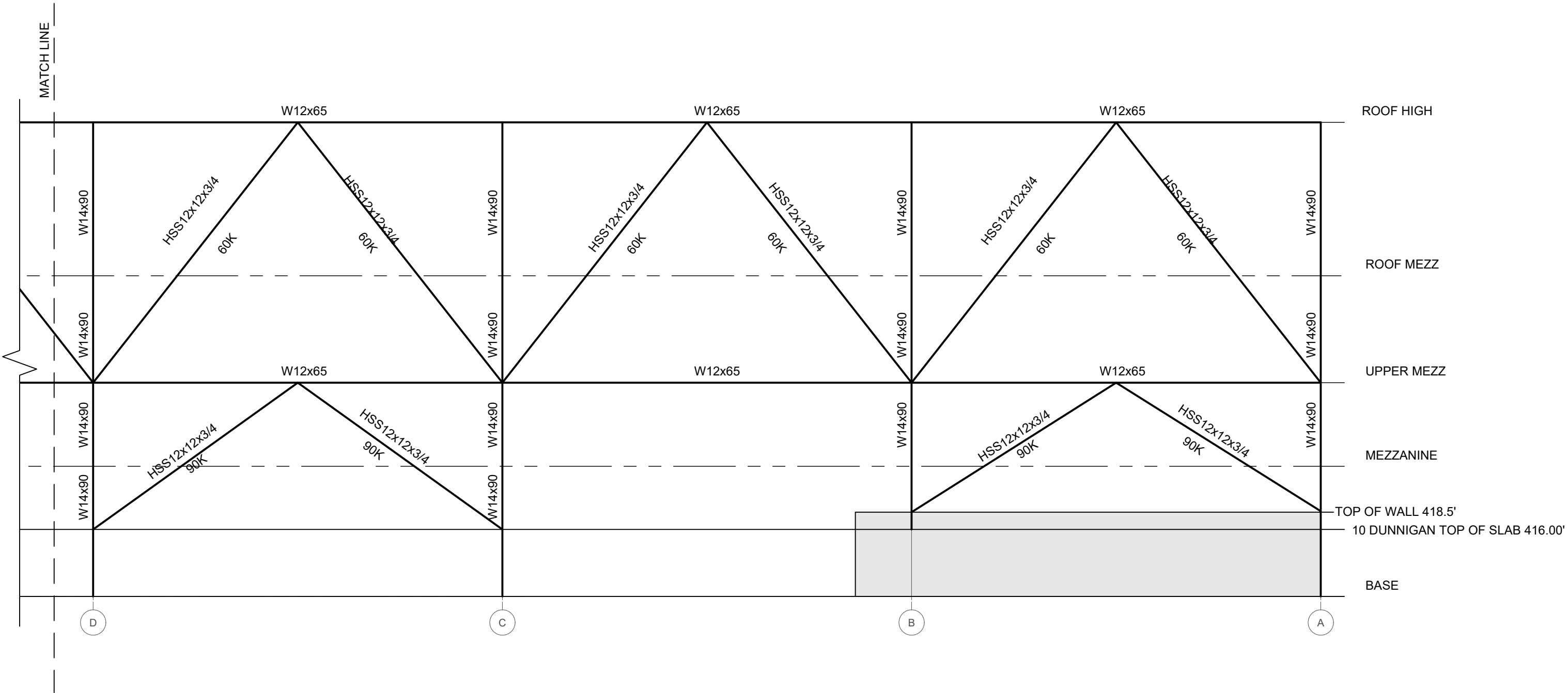
DWG NUMBER :
S-303
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ASRS BUILDING AT GRIDLINE 8
EAST ELEVATION
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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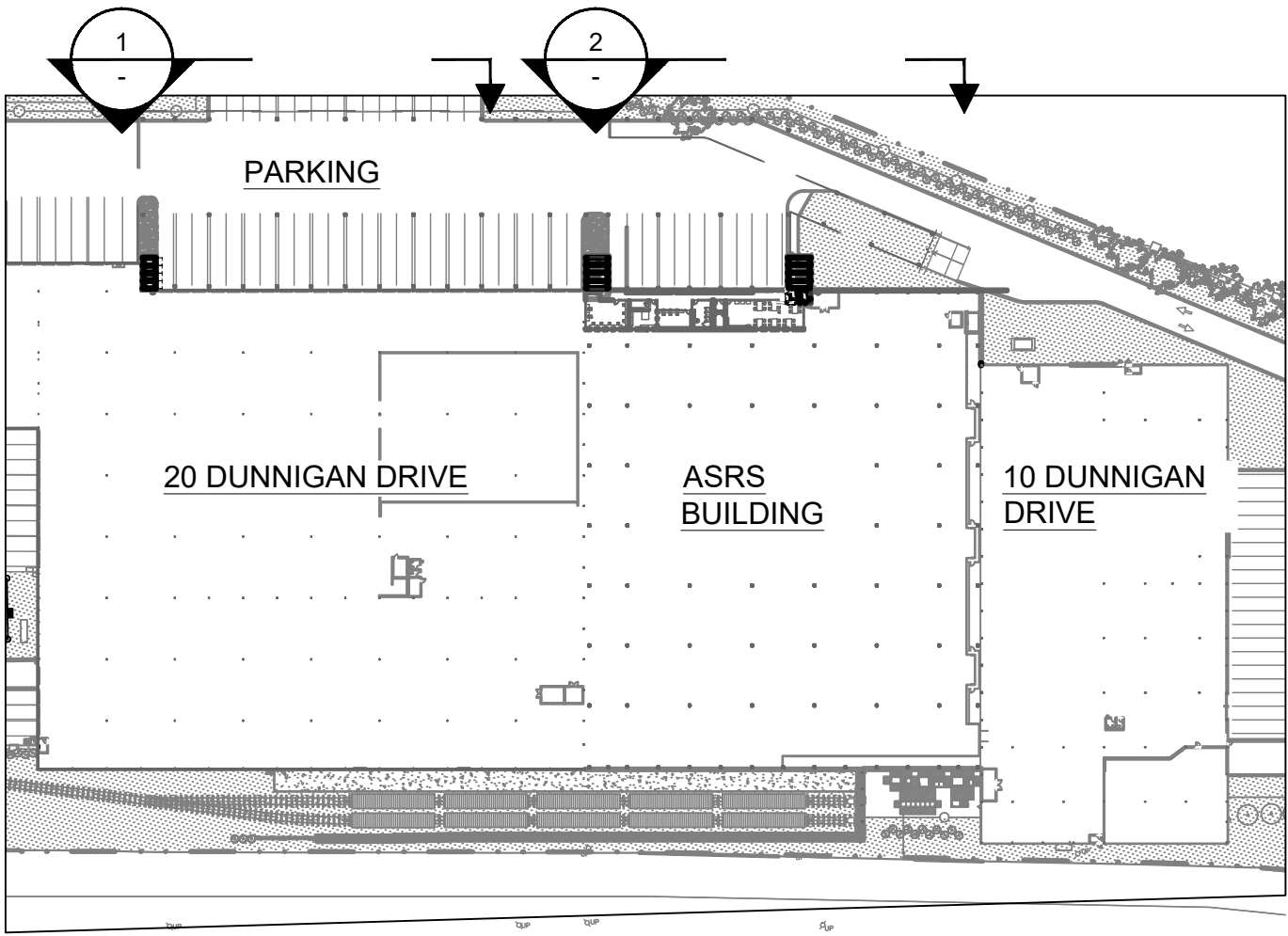
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DRAWING TITLE :

LATERAL FRAMING
ELEVATIONS



DWG NUMBER :
S-304
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KEY PLAN
SCALE: N.T.S.

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SCALE :	AS SHOWN

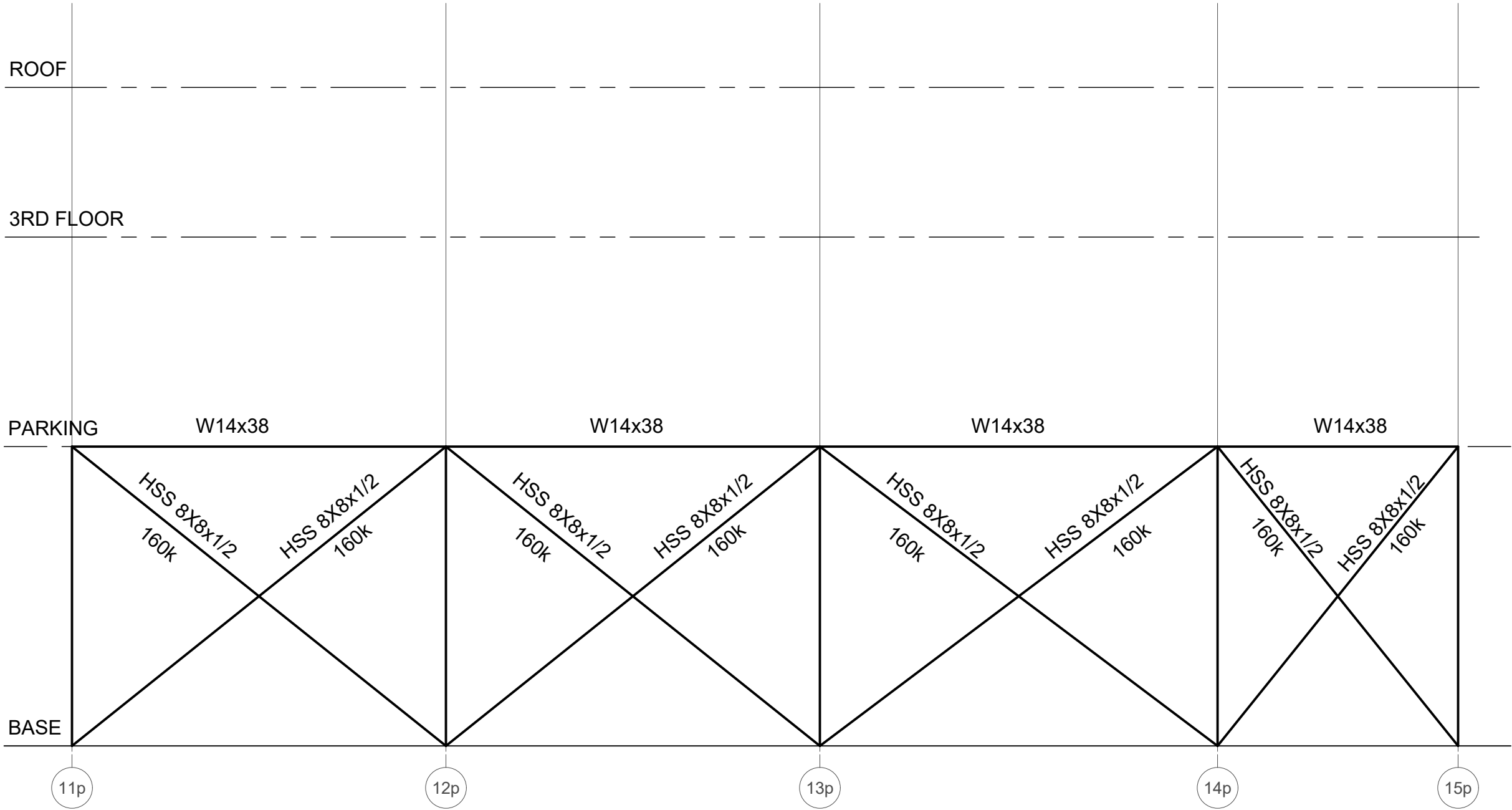
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ELEVATIONS



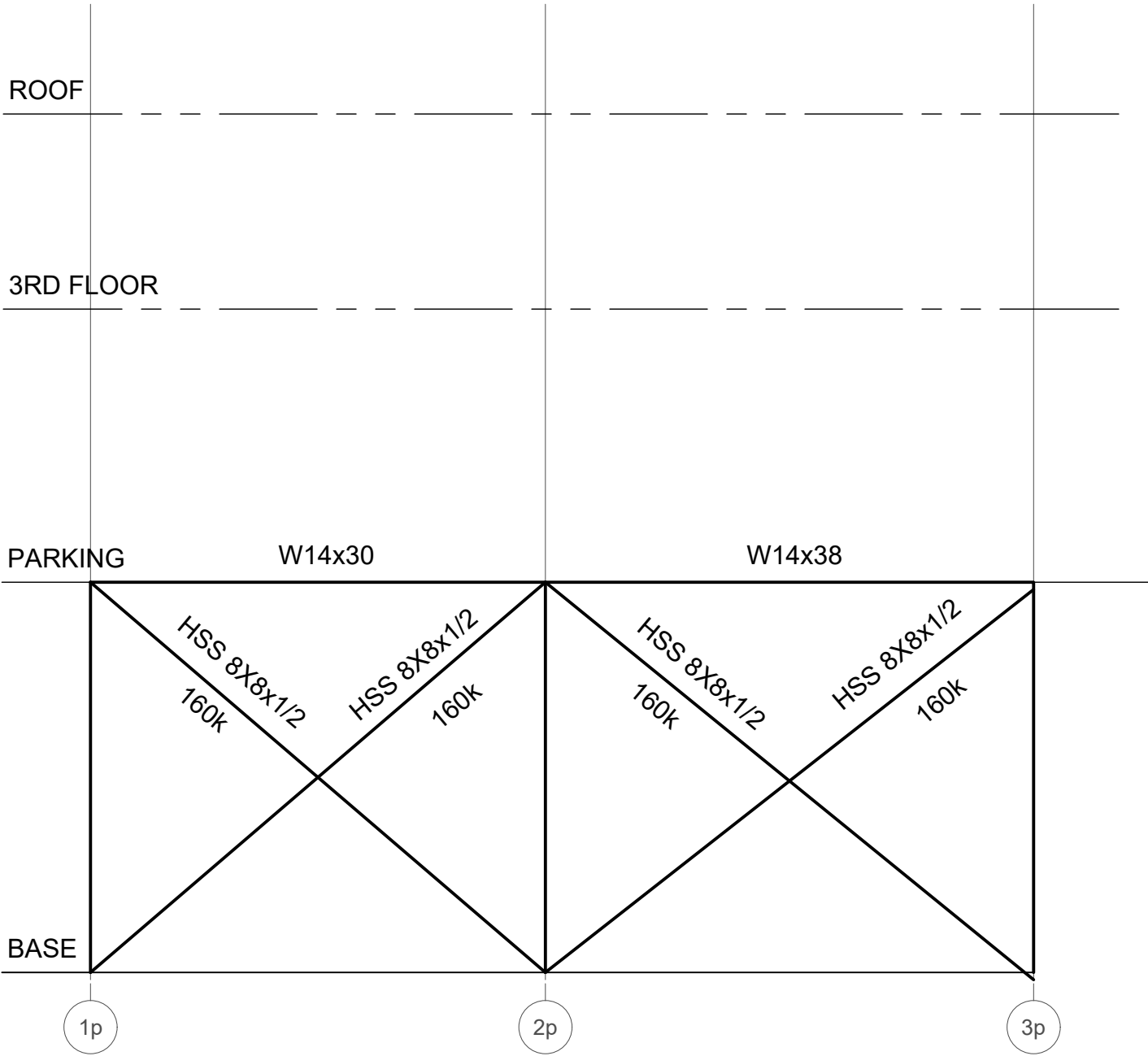
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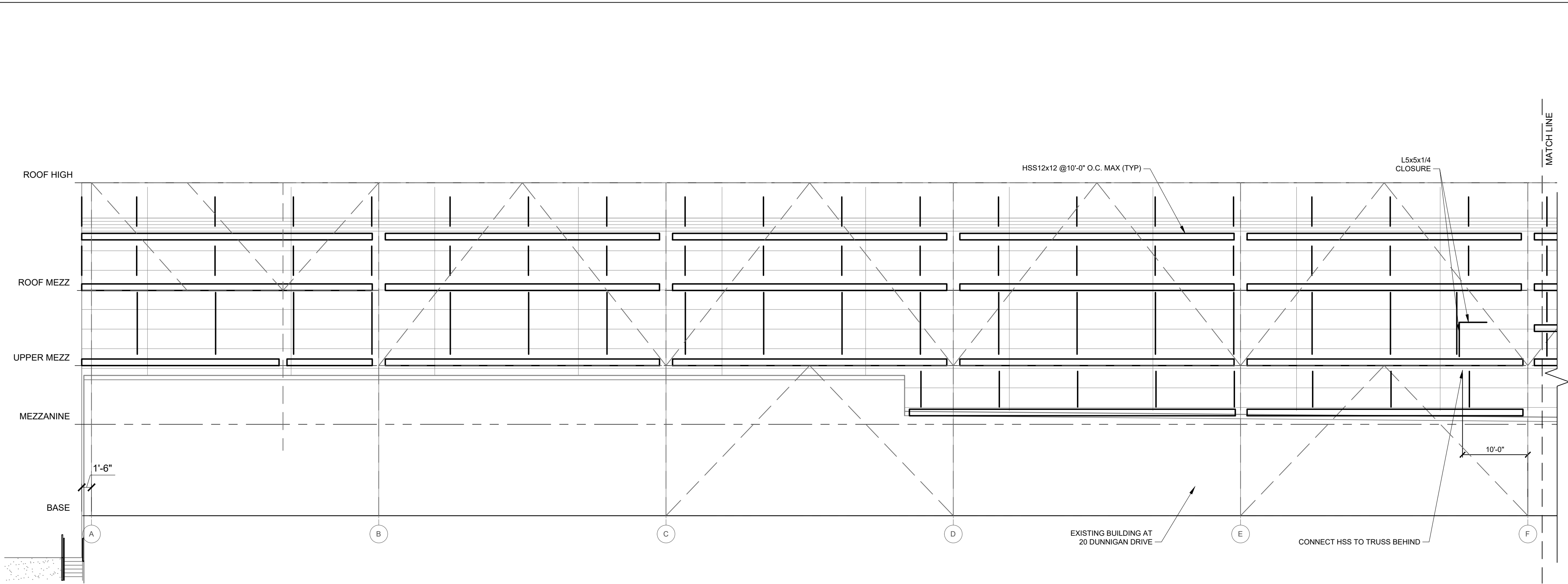
PARKING AREA NORTH ELEVATION
GRIDLINE Ap. EAST SIDE

1
-
SCALE: 1/8" = 1'-0"

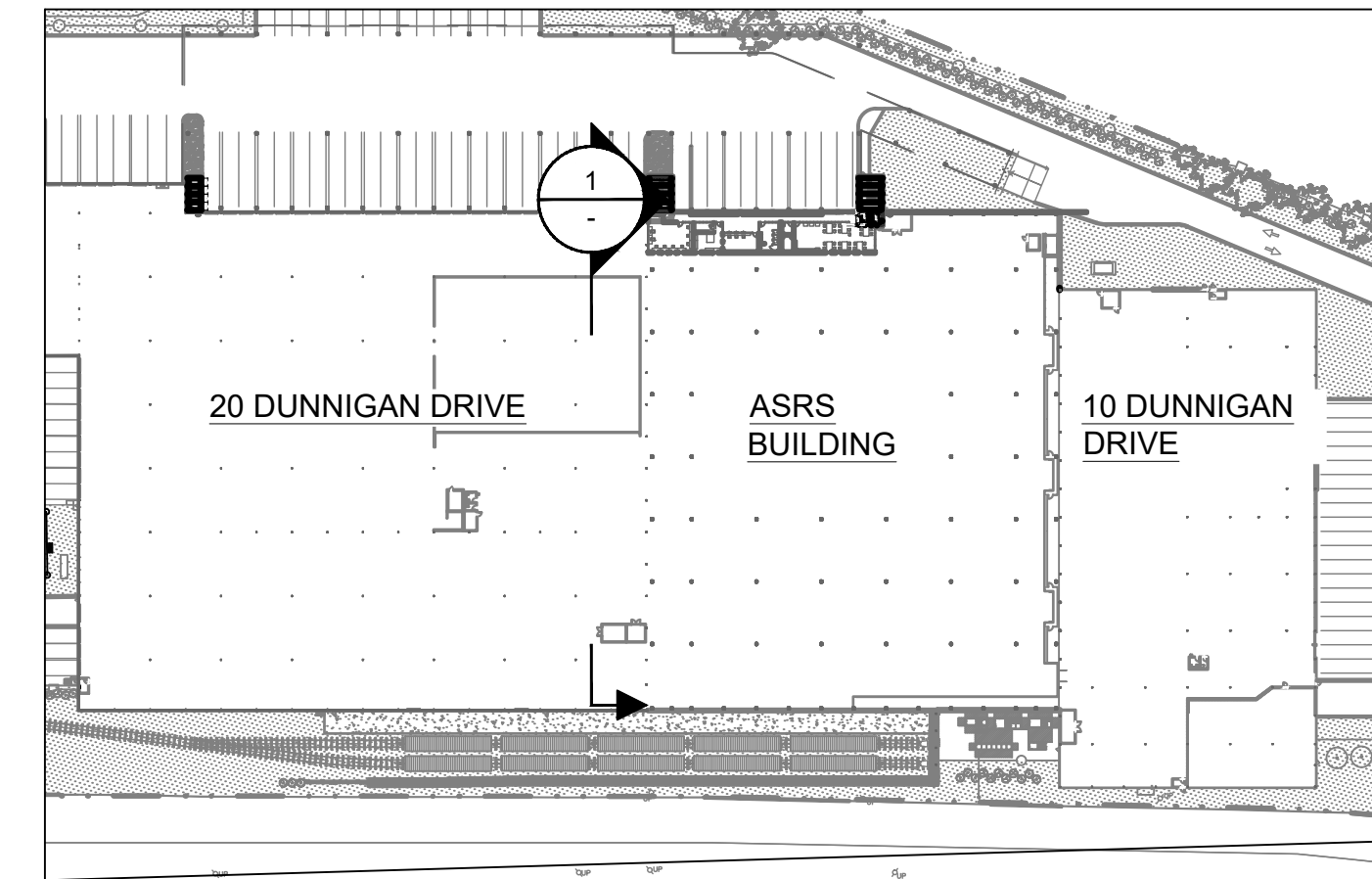


PARKING AREA NORTH ELEVATION
GRIDLINE Ap - WEST SIDE

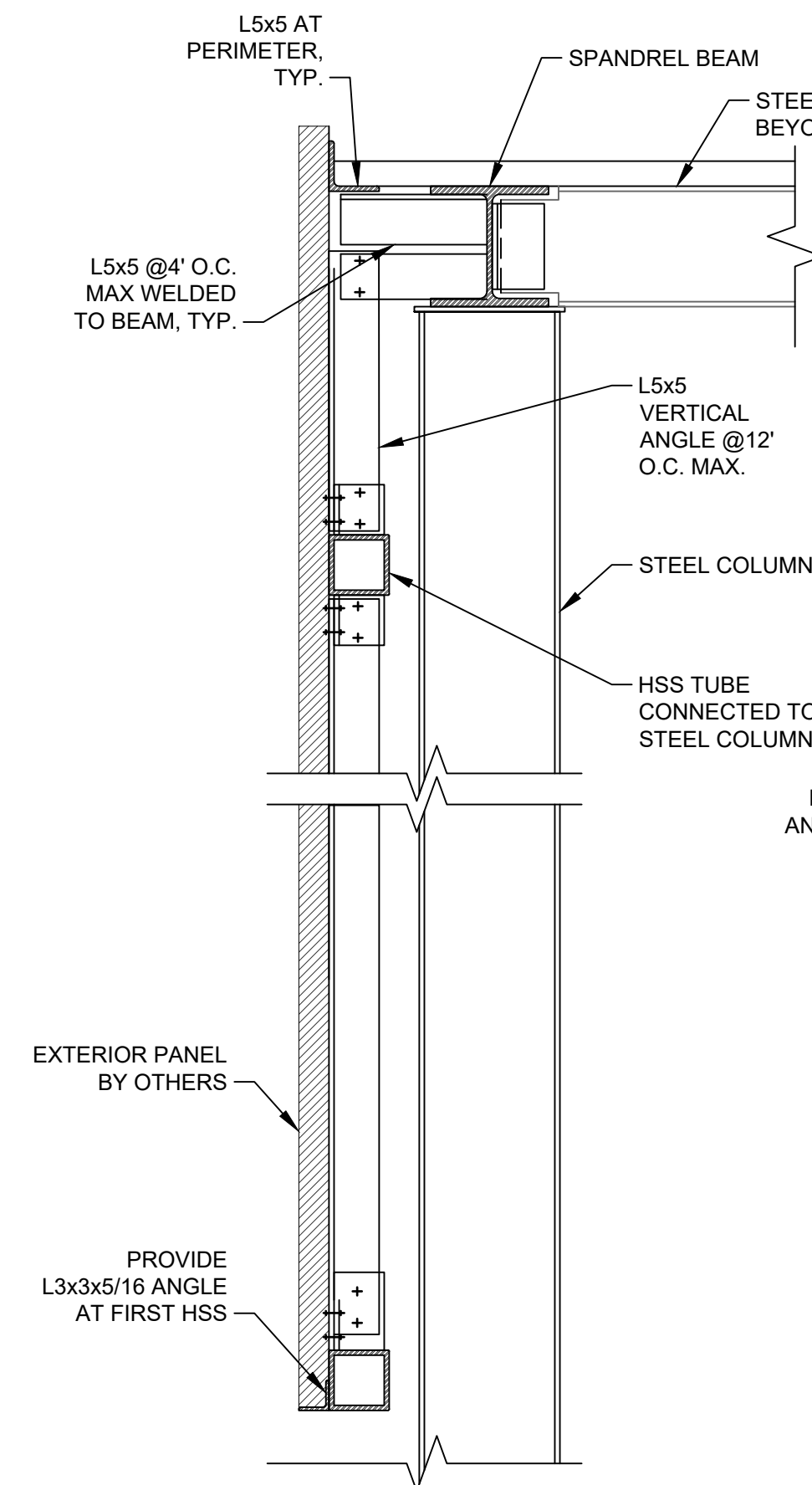
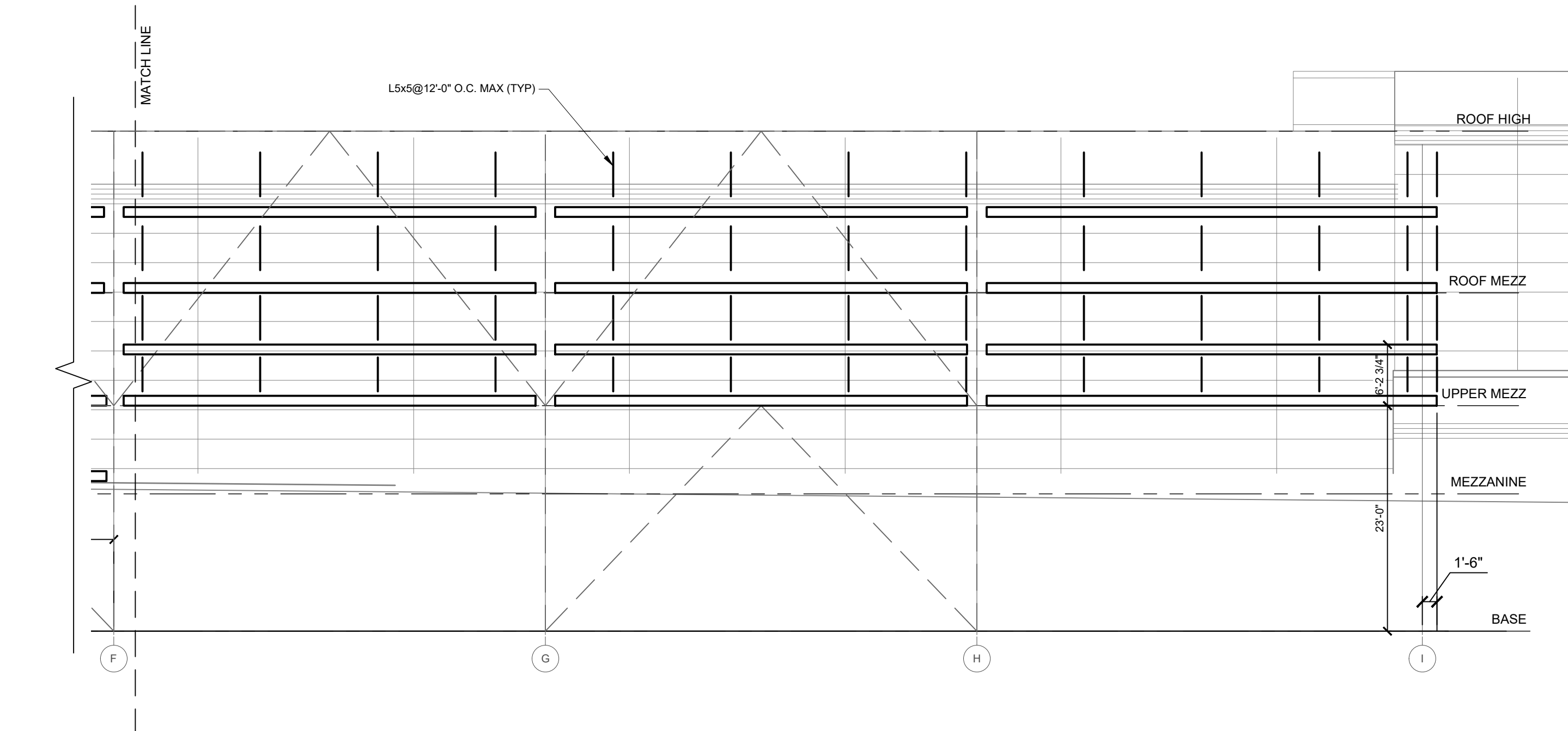
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-
SCALE: 1/8" = 1'-0"



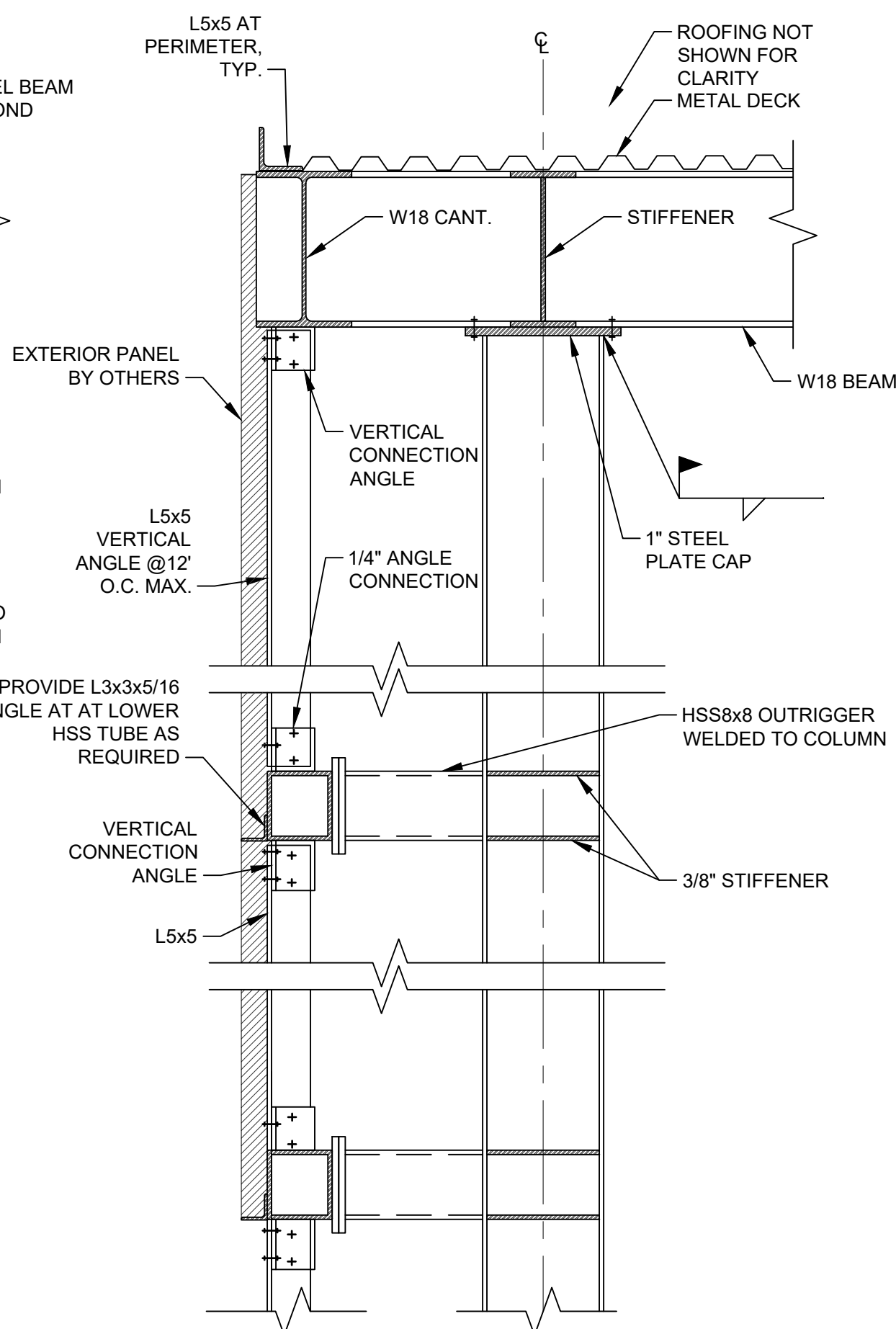
ASRS BUILDING WEST ELEVATION
EXTERIOR PANEL SUPPORTS
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



1 TYP. EXTERIOR SECTION
SCALE: 3/4" = 1'-0"



2 TYP. EXTERIOR SECTION AT EAST AND WEST WALL
SCALE: 3/4" = 1'-0"

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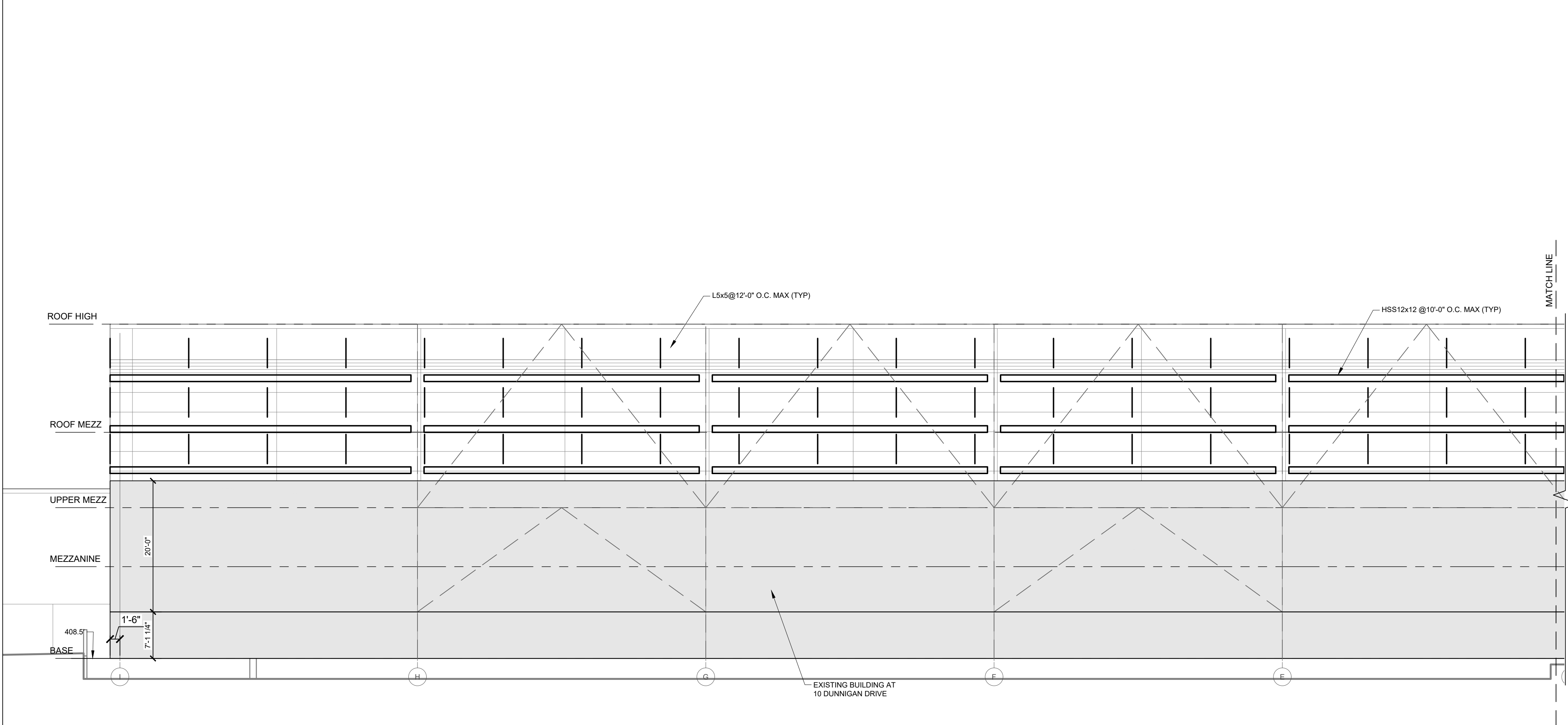
EXTERIOR PANEL
SUPPORTS



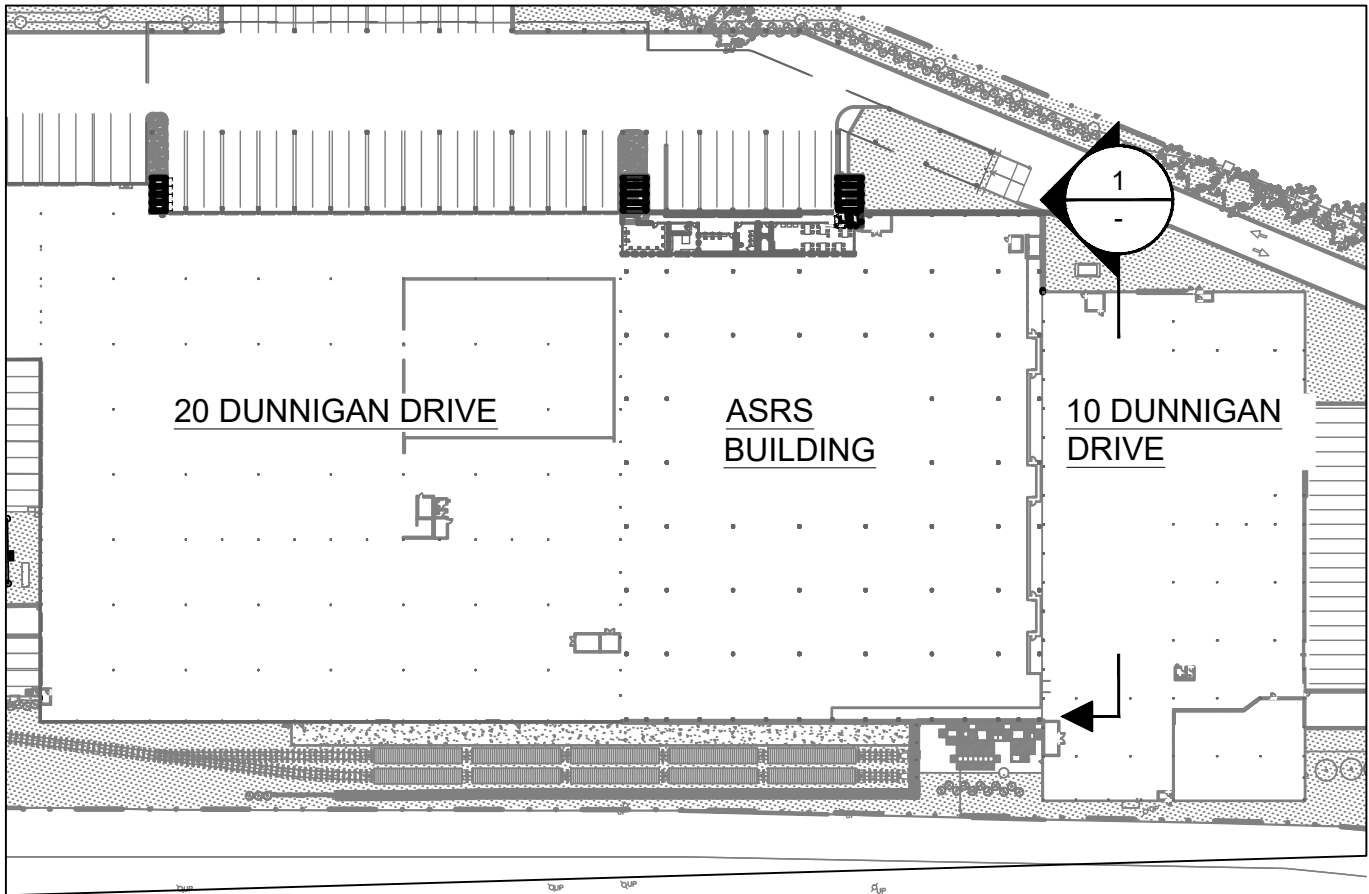
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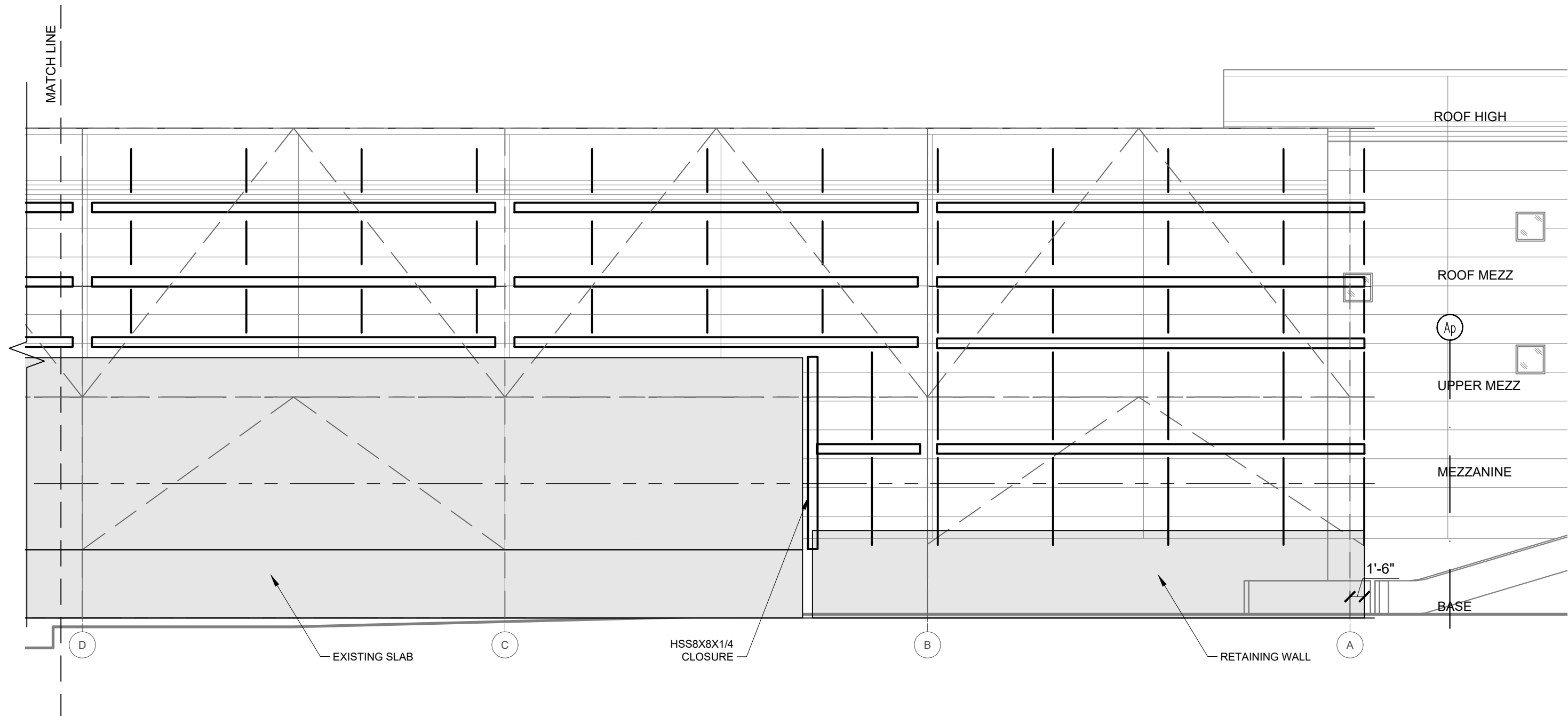
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1
-
ASRS BUILDING EAST ELEVATION
EXTERIOR PANEL SUPPORTS
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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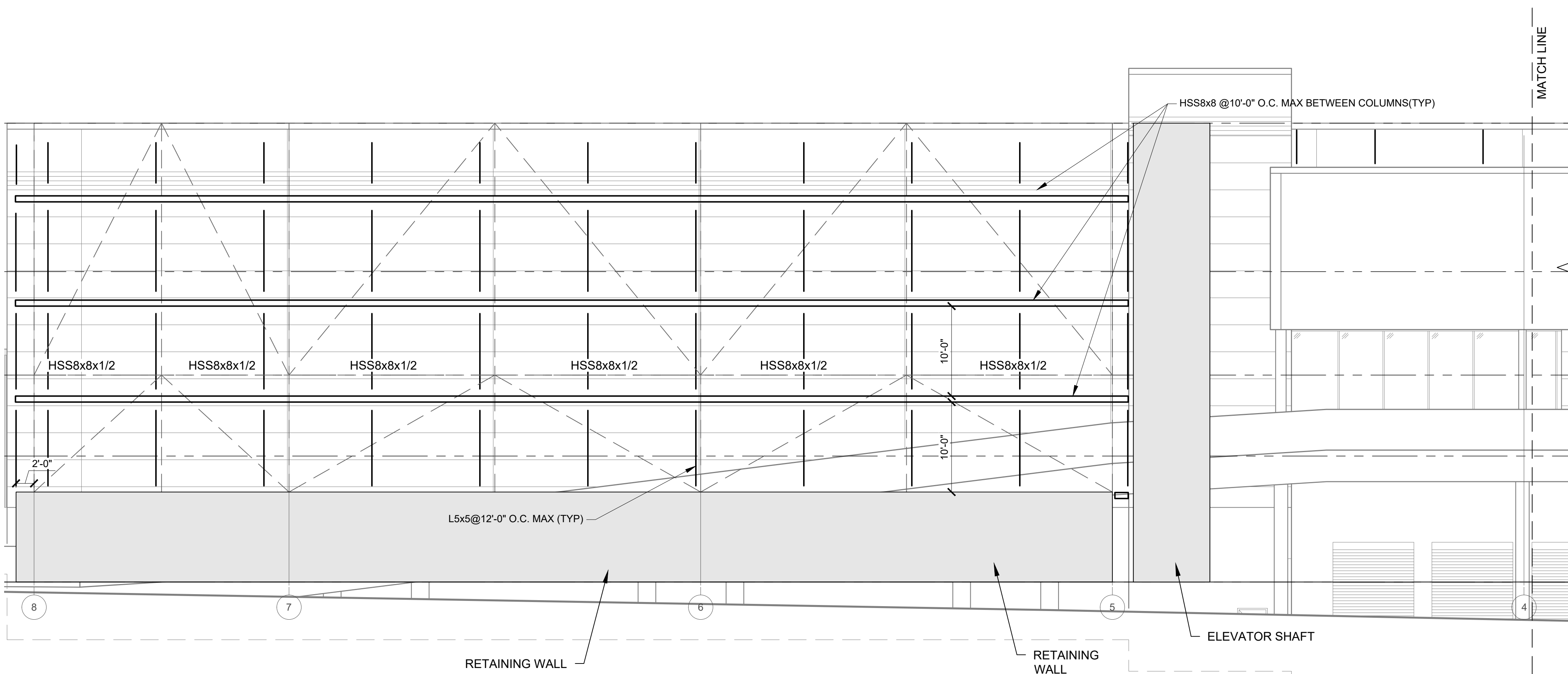
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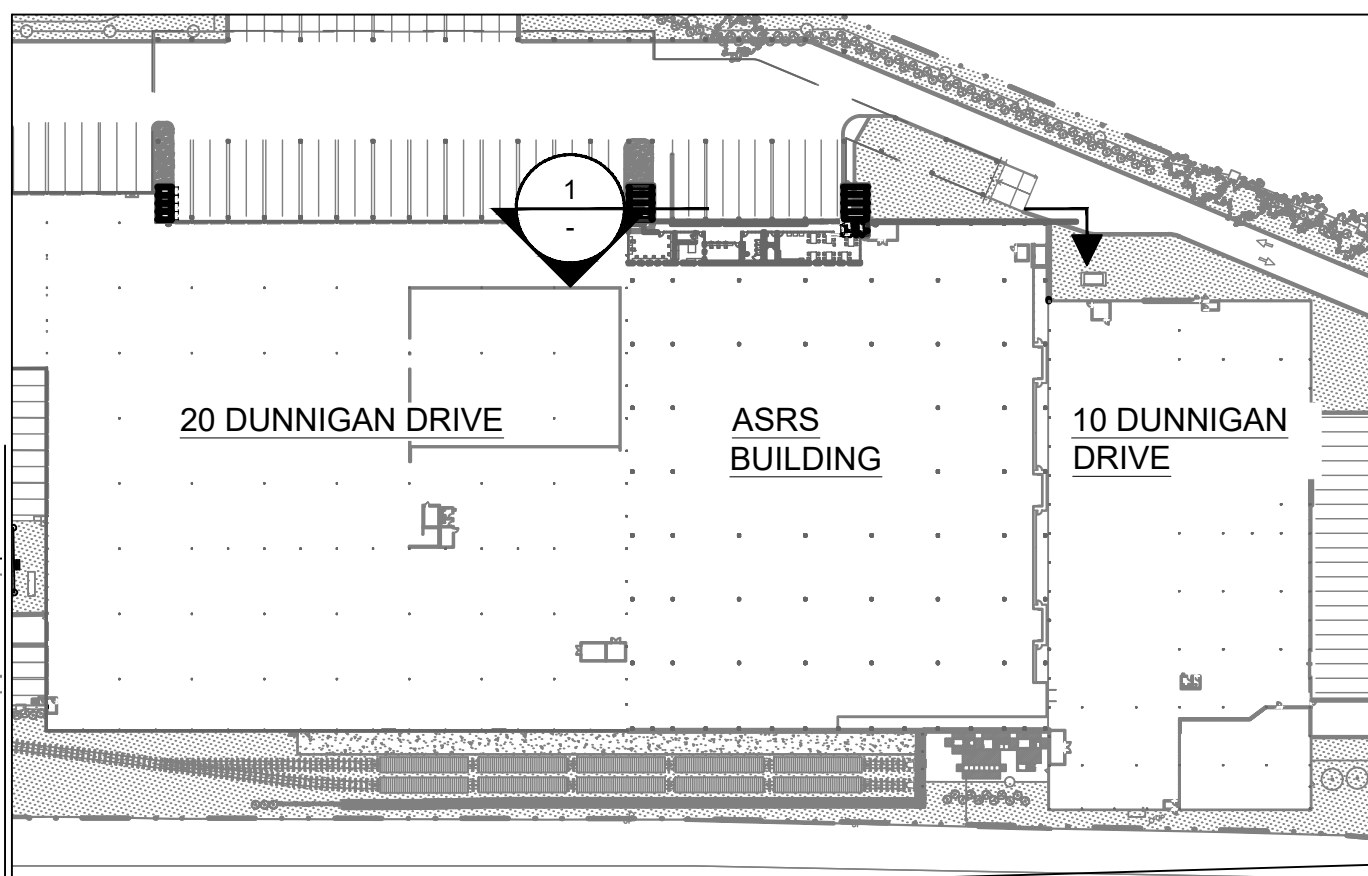
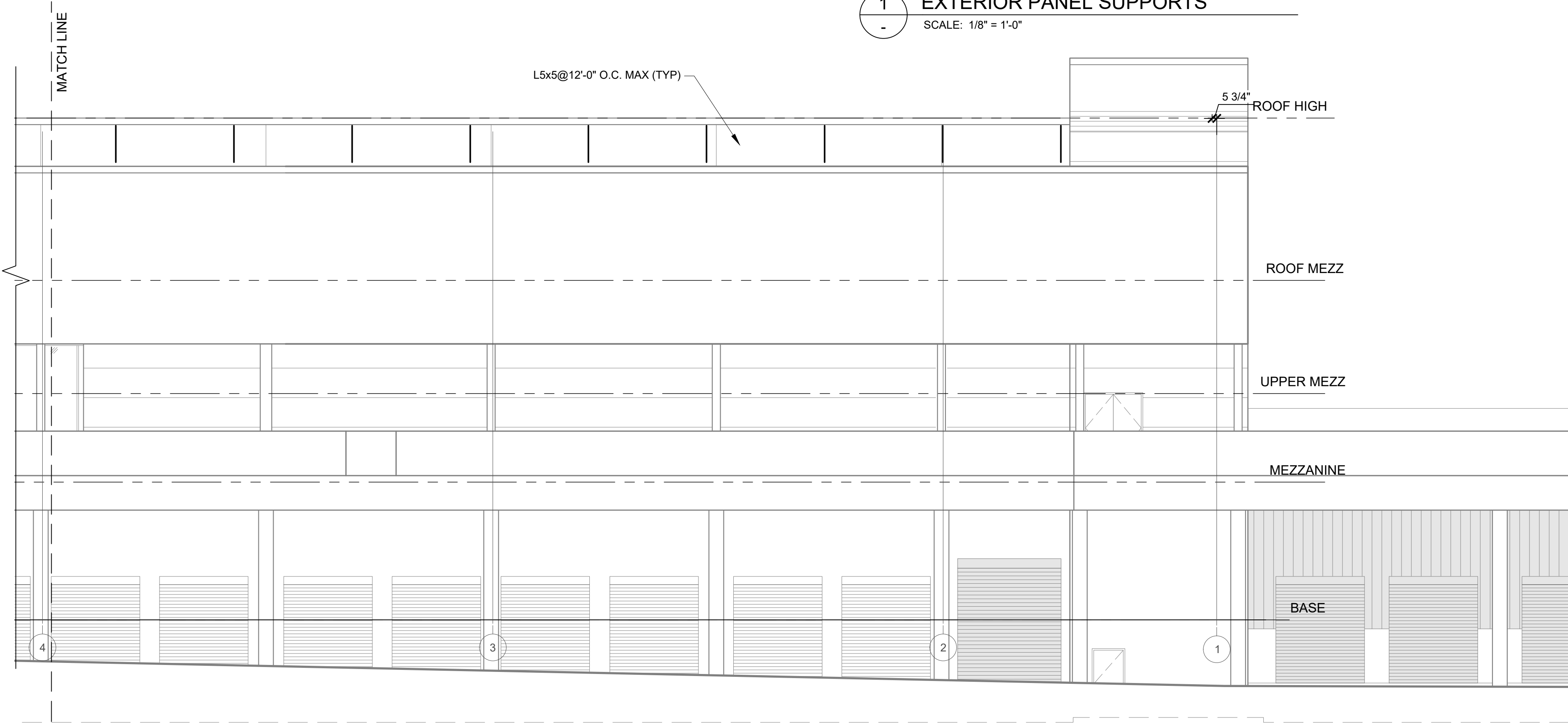
EXTERIOR PANEL
SUPPORTS

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S-311
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4/4/22



ASRS BUILDING NORTH ELEVATION
EXTERIOR PANEL SUPPORTS
1
-
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

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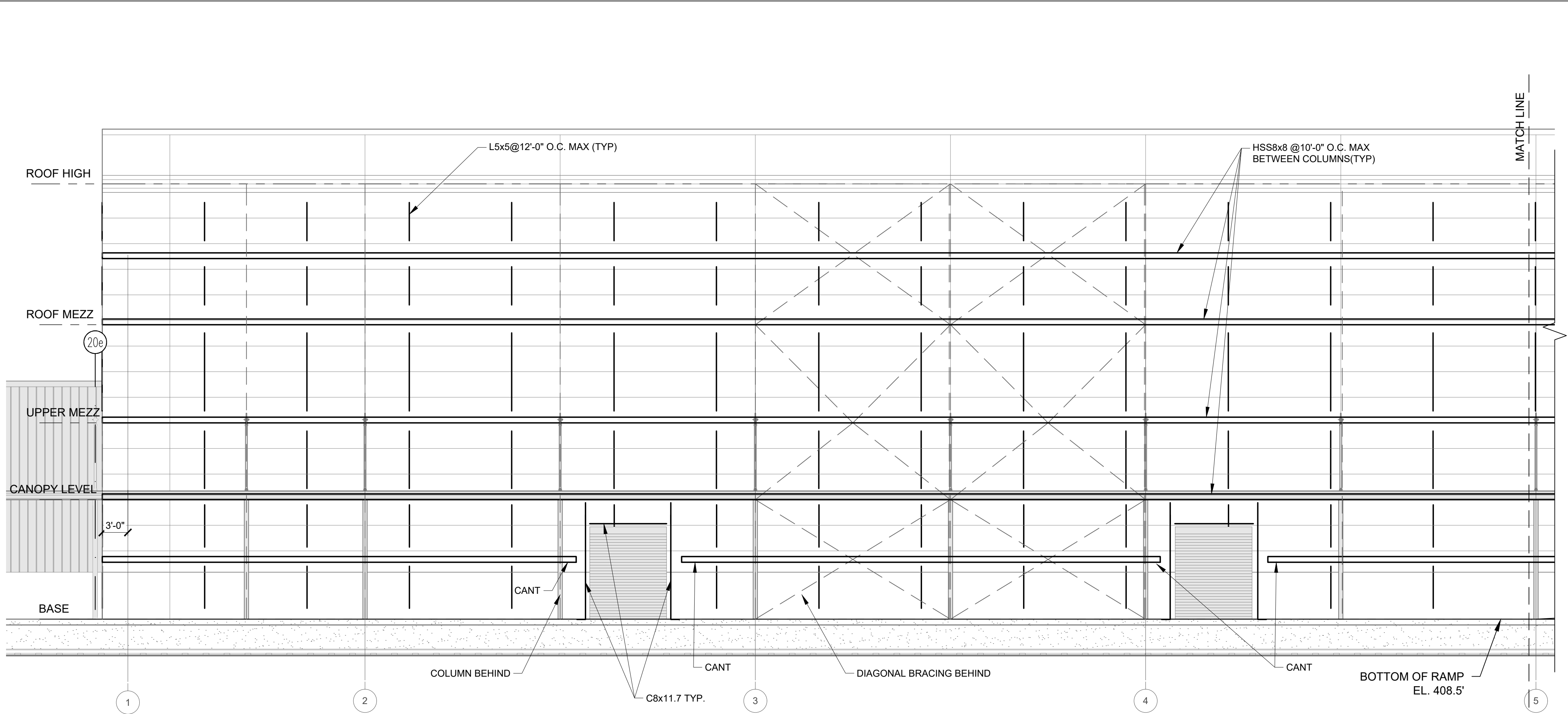
EXTERIOR PANEL
SUPPORTS



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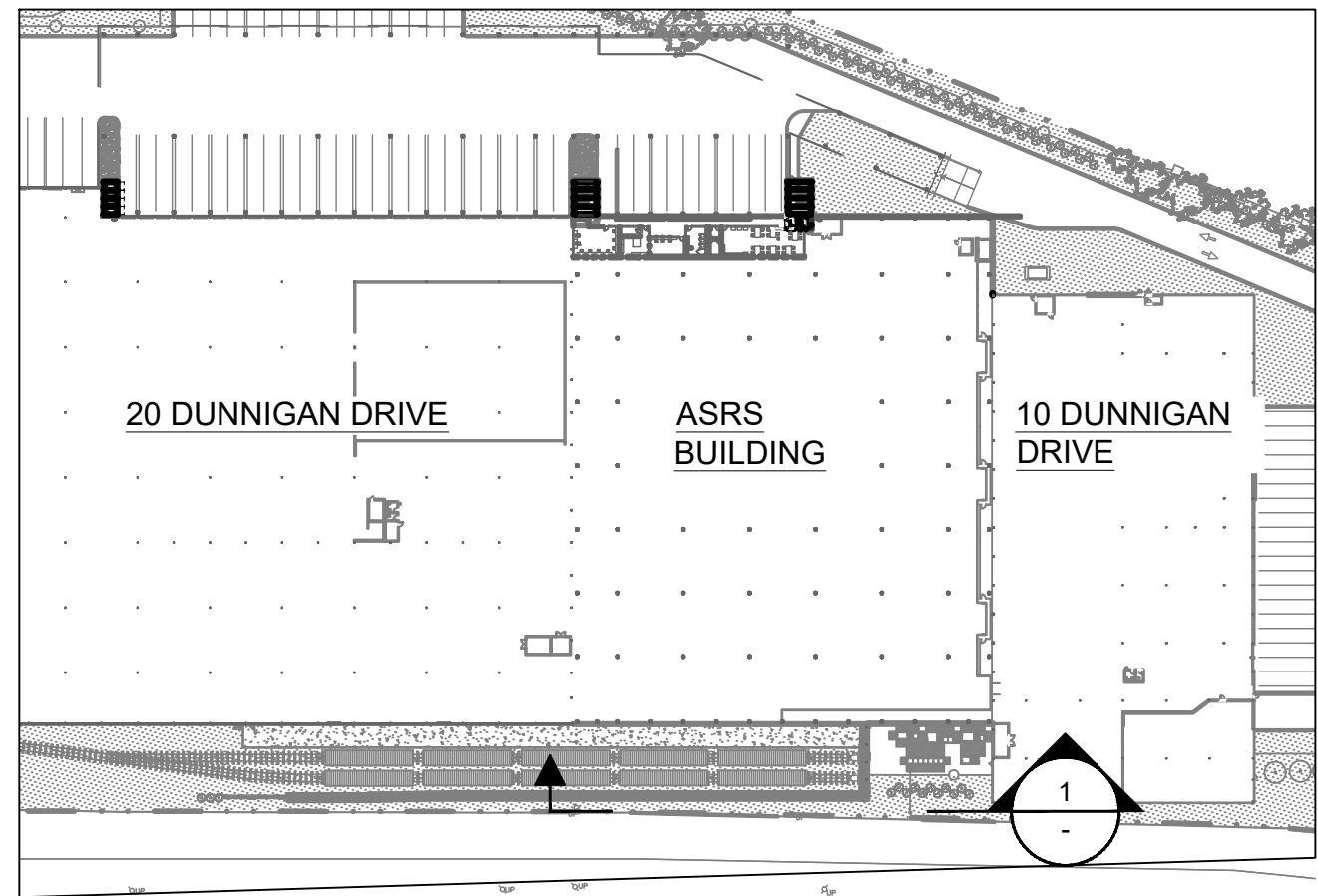
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ASRS BUILDING SOUTH ELEVATION
EXTERIOR PANEL SUPPORTS

SCALE: 1/8" = 1'-0"



KEY PLAN

SCALE: N.T.S.

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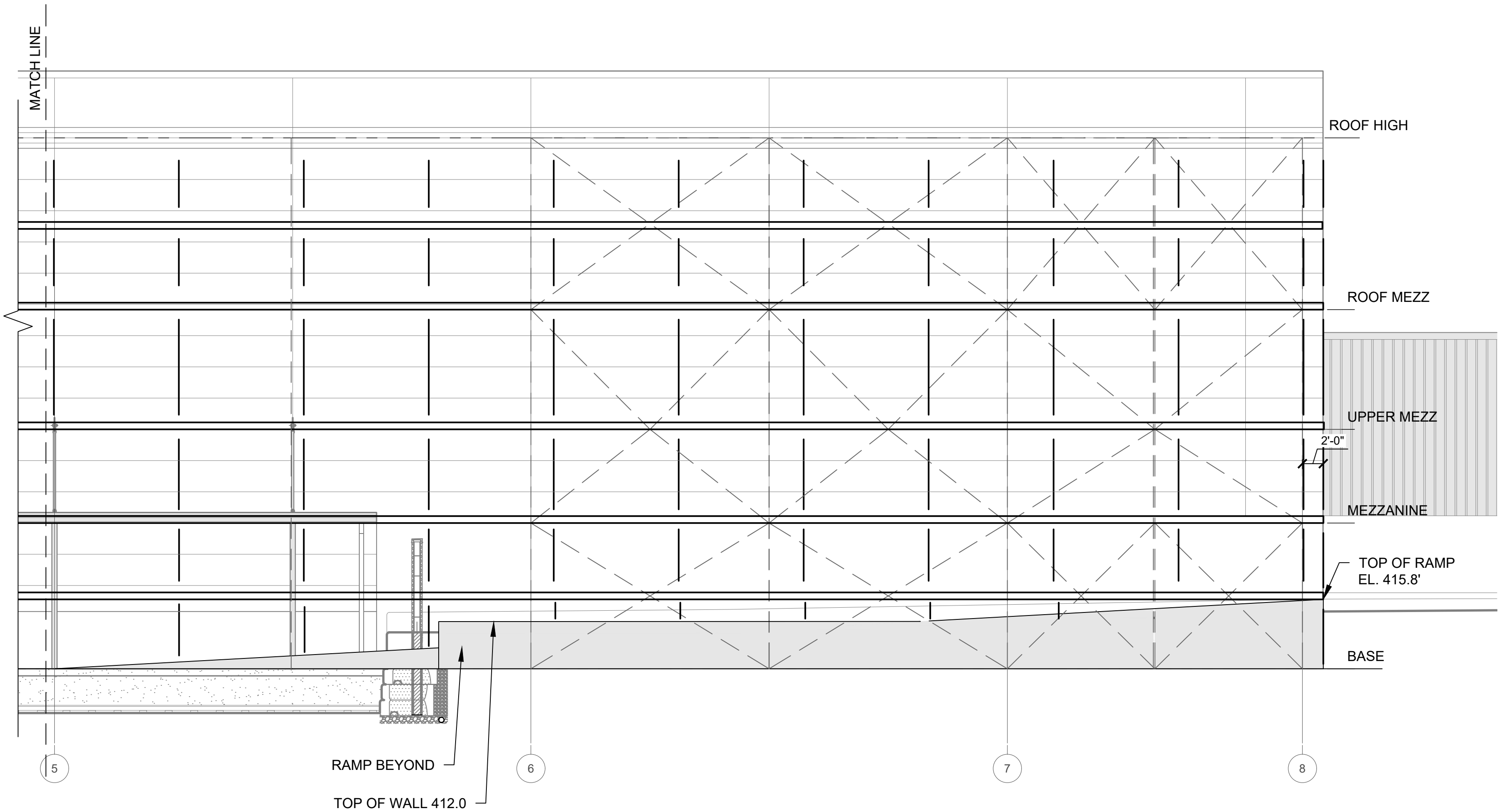
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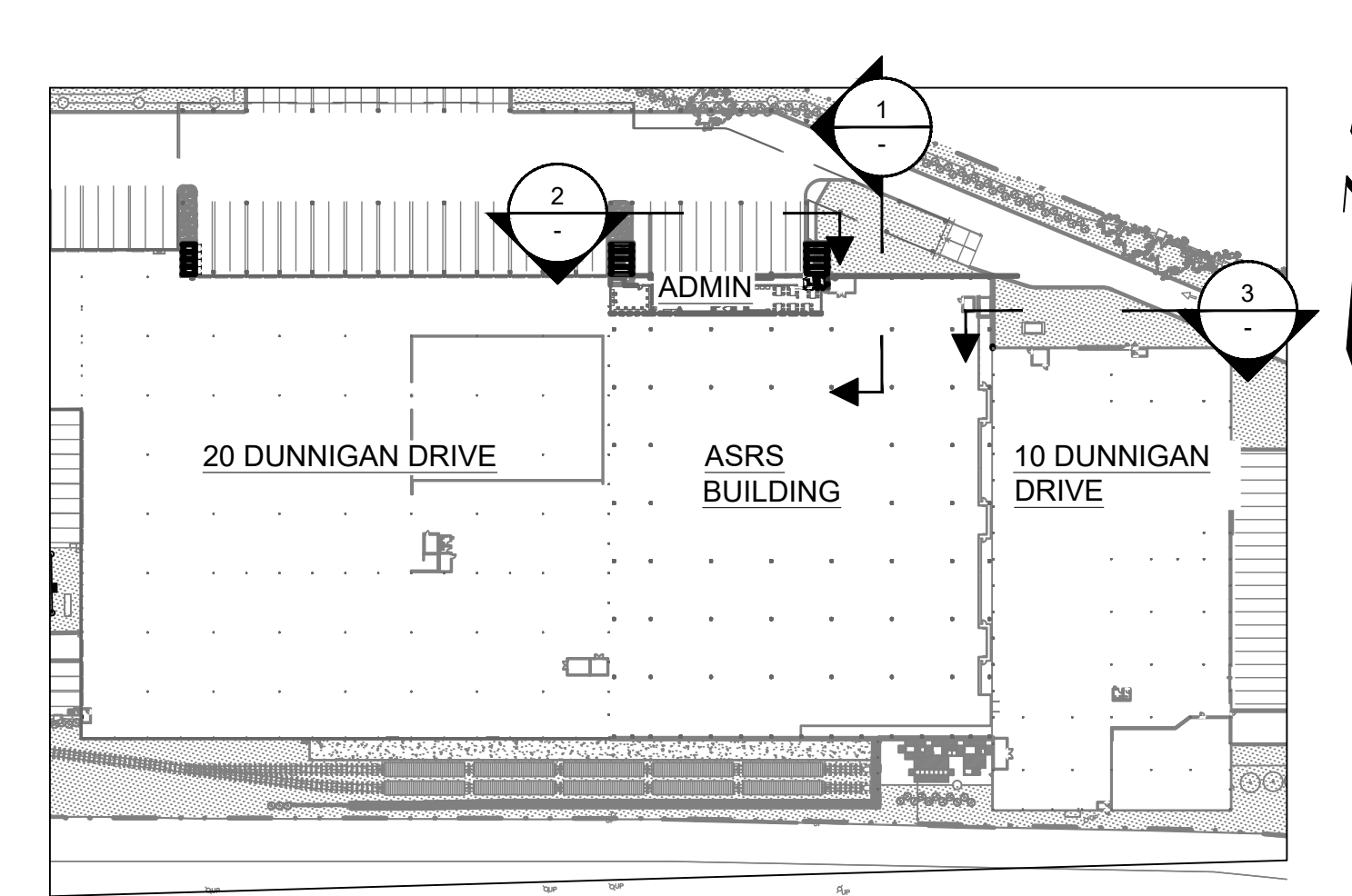
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EXTERIOR PANEL
SUPPORTS



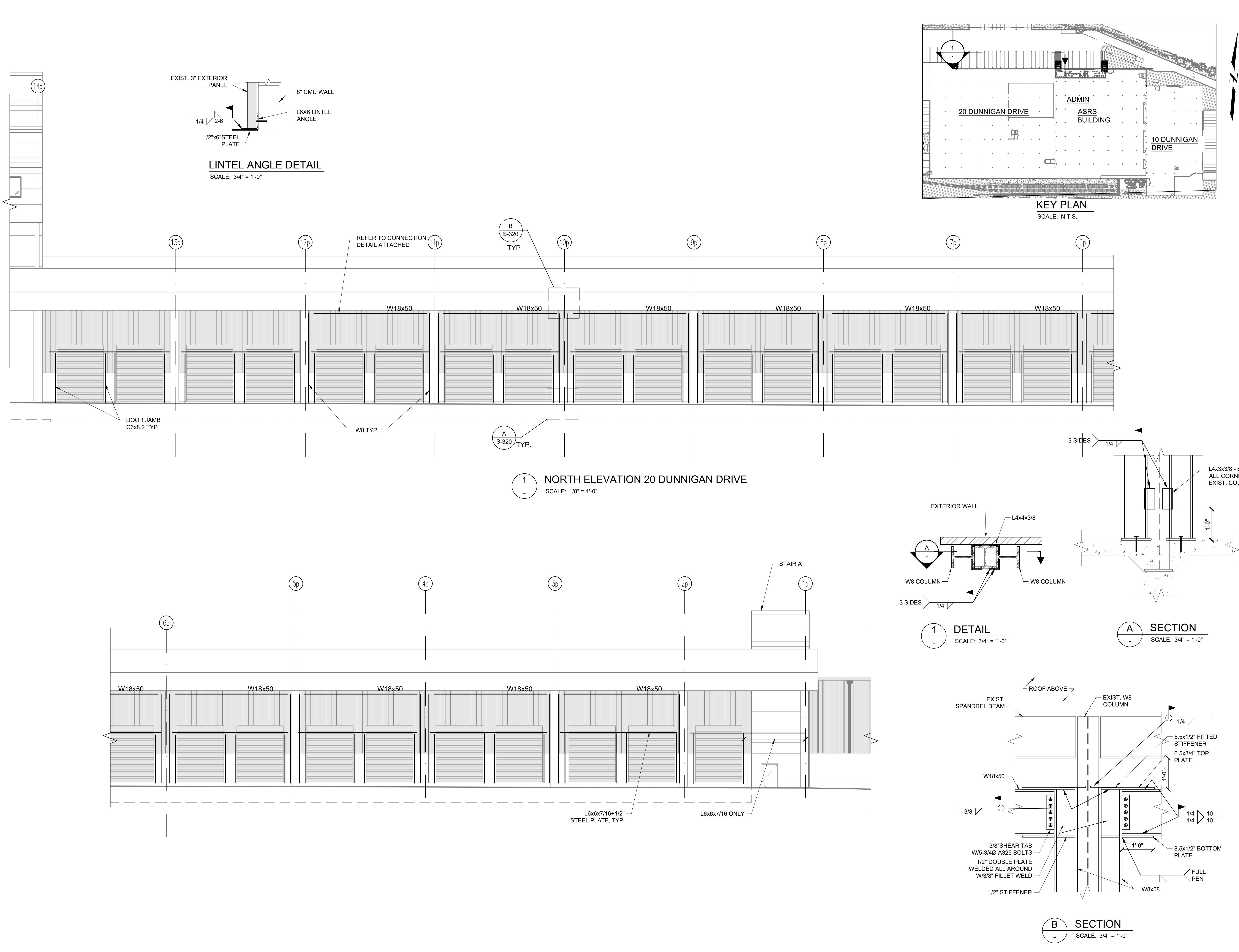
DWG NUMBER :
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2
- EXTERIOR SCALE: 1/8" = 1'-0"



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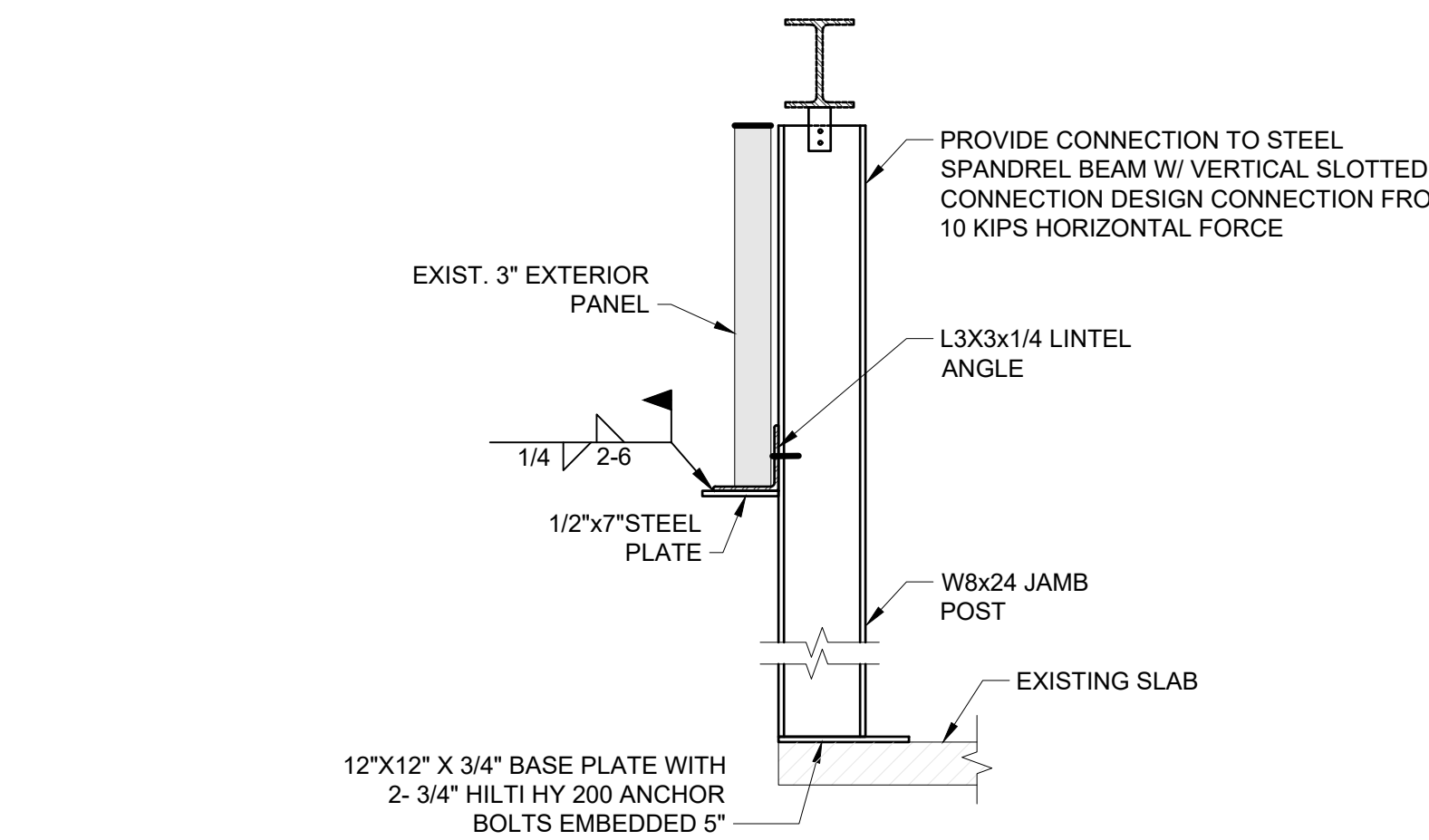
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SCALE :	AS SHOWN

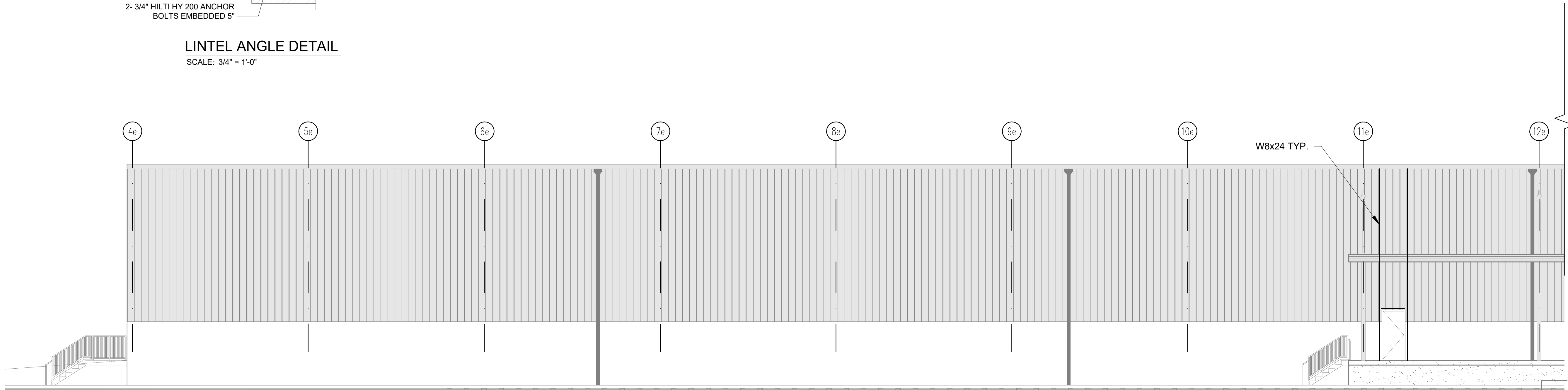
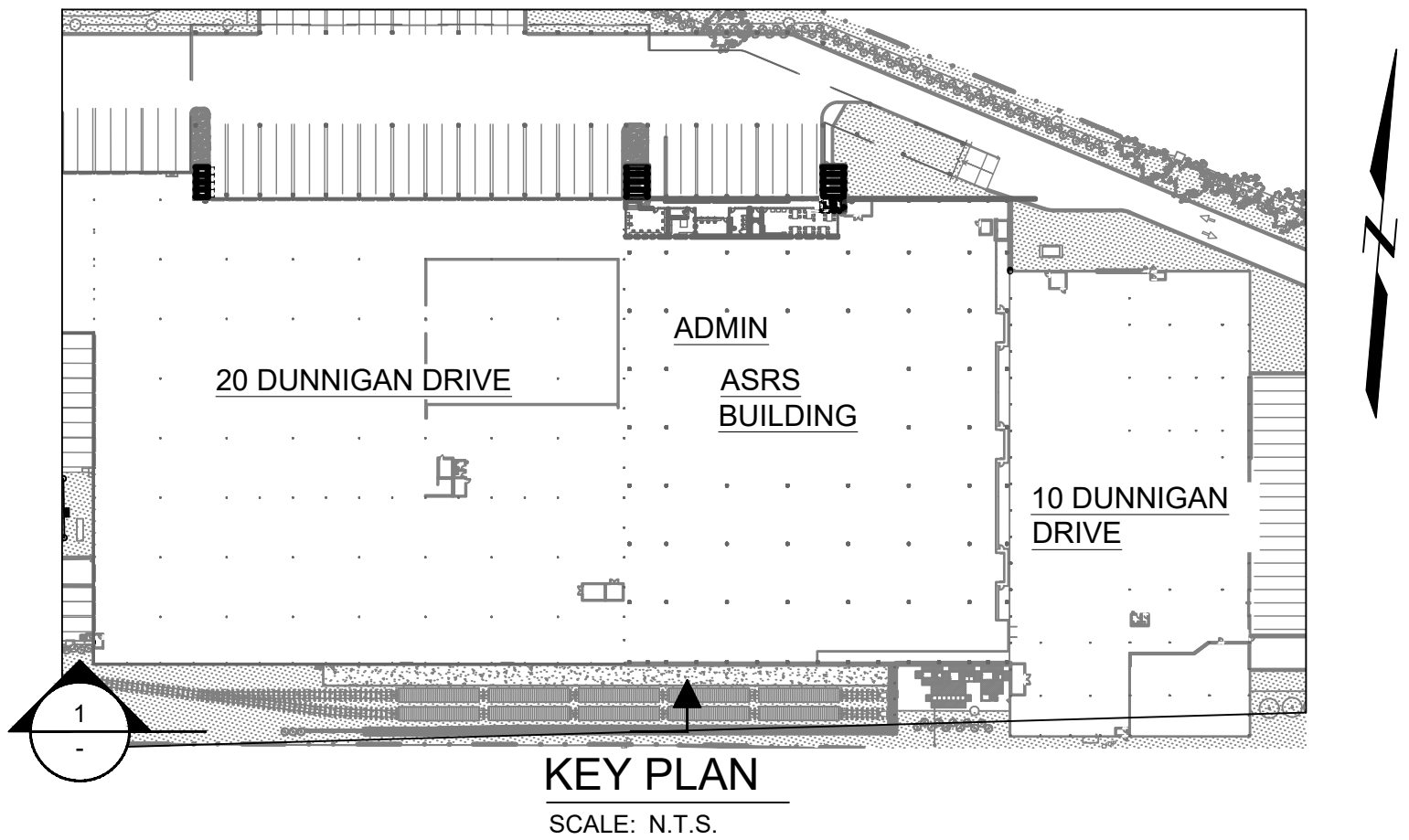
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**NORTH ELEVATION 20
DUNNIGAN DRIVE**

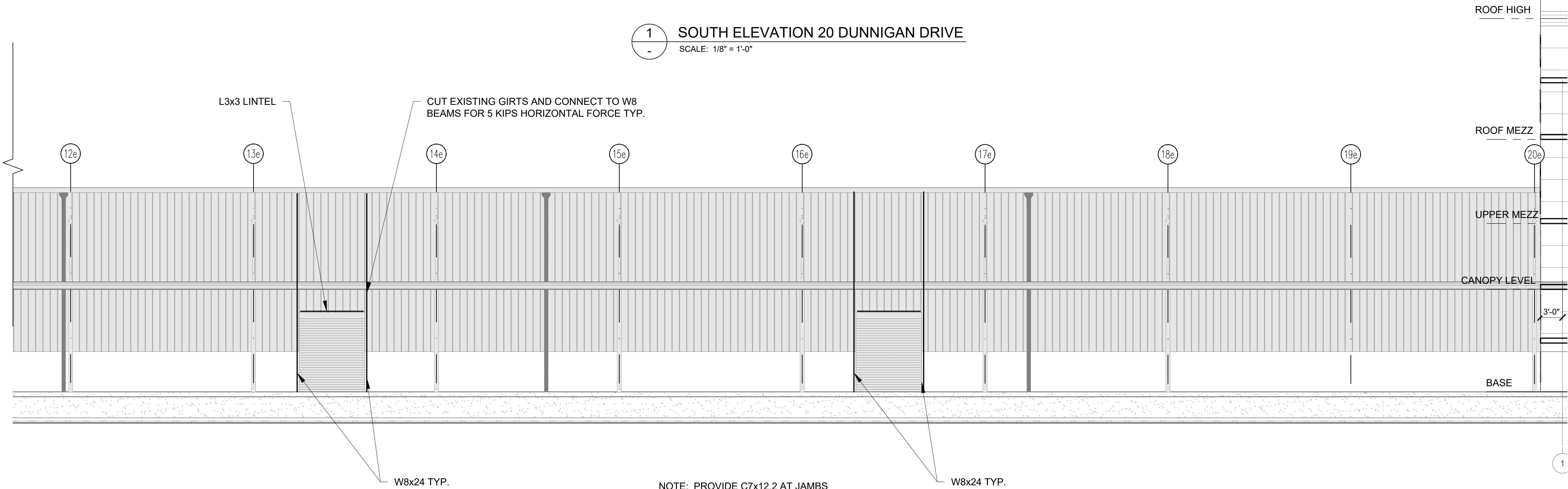

DWG NUMBER :
S-320
4/4/22
46 OF 61



LINTEL ANGLE DETAIL
SCALE: 3/4" = 1'-0"



1 SOUTH ELEVATION 20 DUNNIGAN DRIVE
SCALE: 1/8" = 1'-0"



NOTE: PROVIDE C7x12.2 AT JAMBS
IN FRONT OF W8 UPRIGHT

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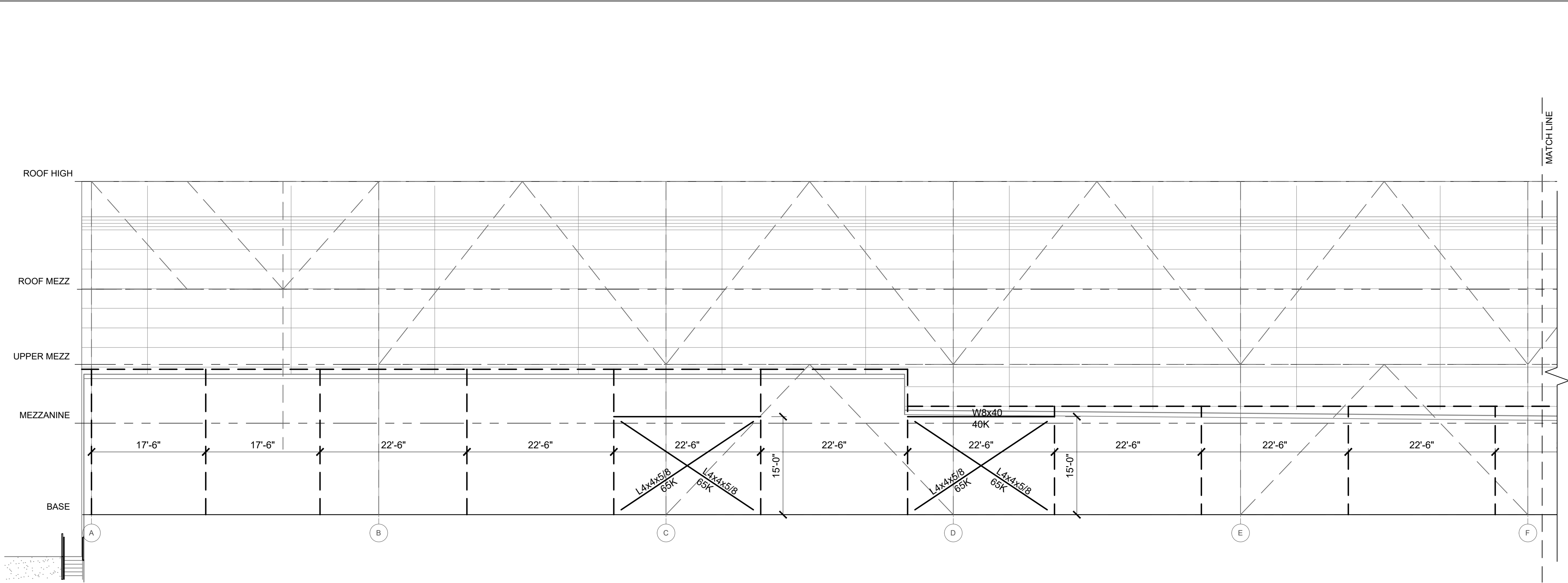
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DUNNIGAN

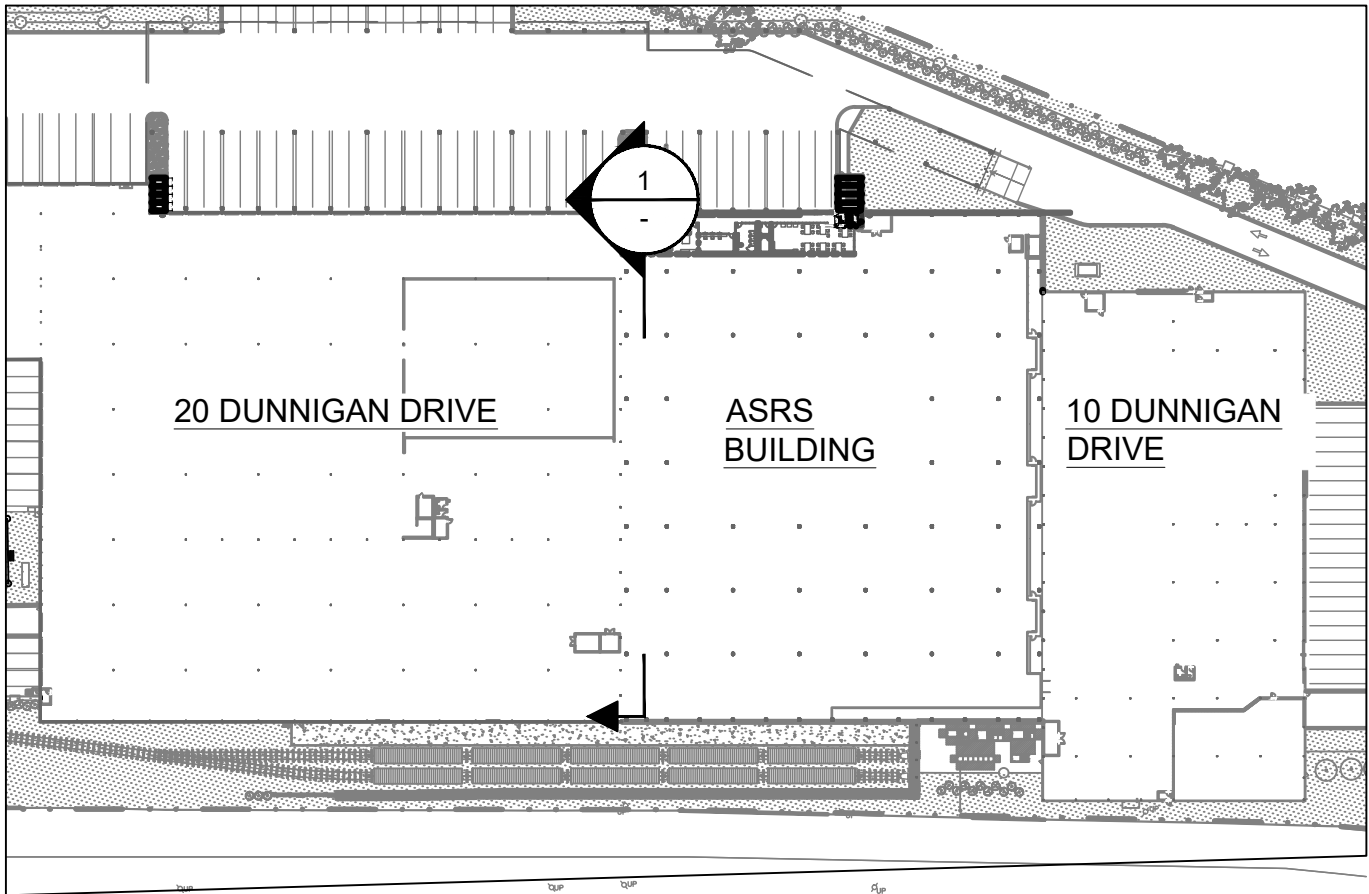


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S-321

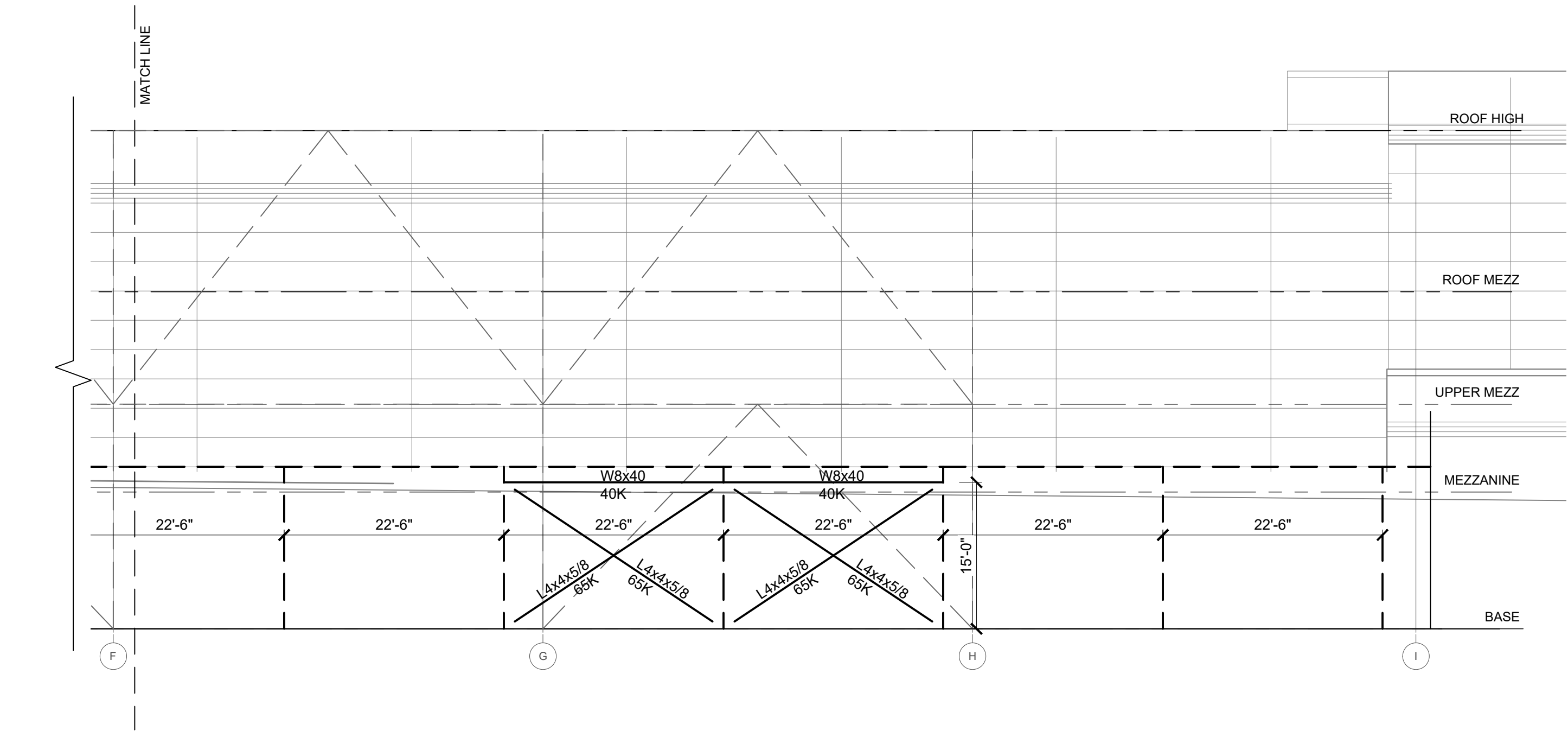
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ASRS BUILDING WEST ELEVATION
EXTERIOR PANEL SUPPORTS
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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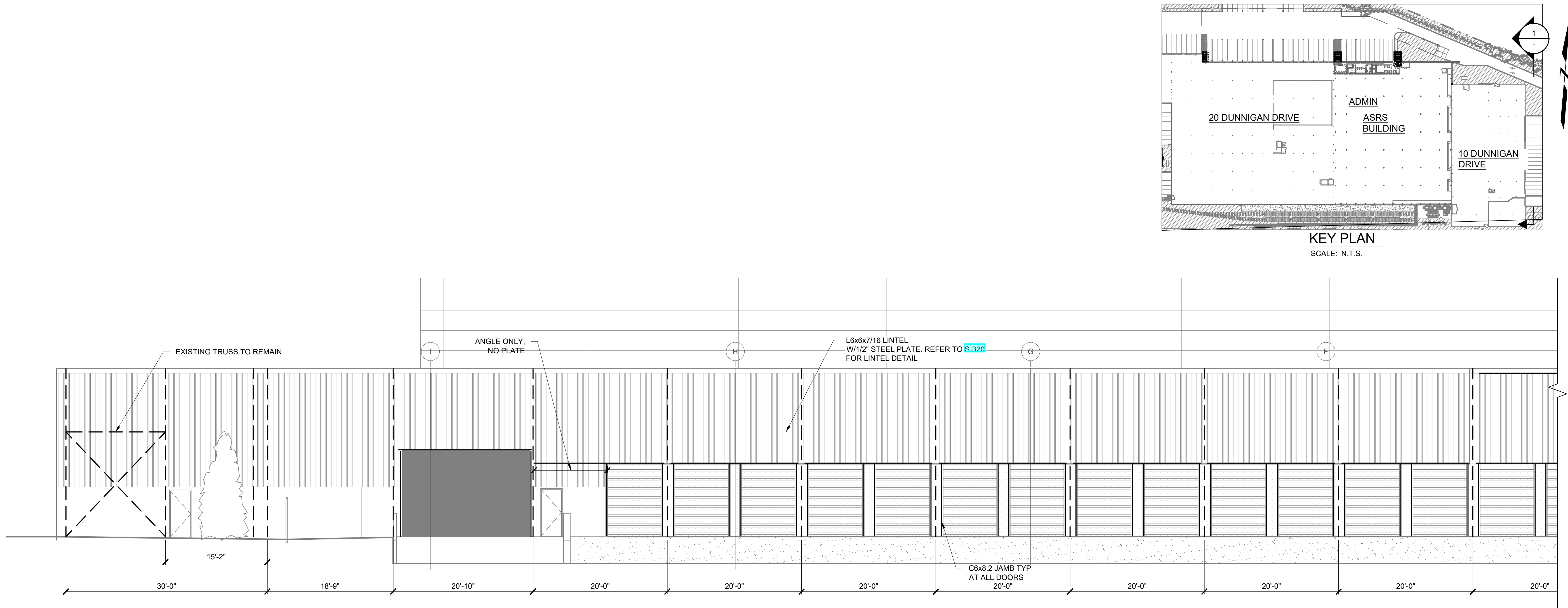
20 DUNNIGAN DRIVE BRACING
FRAME ELEVATION



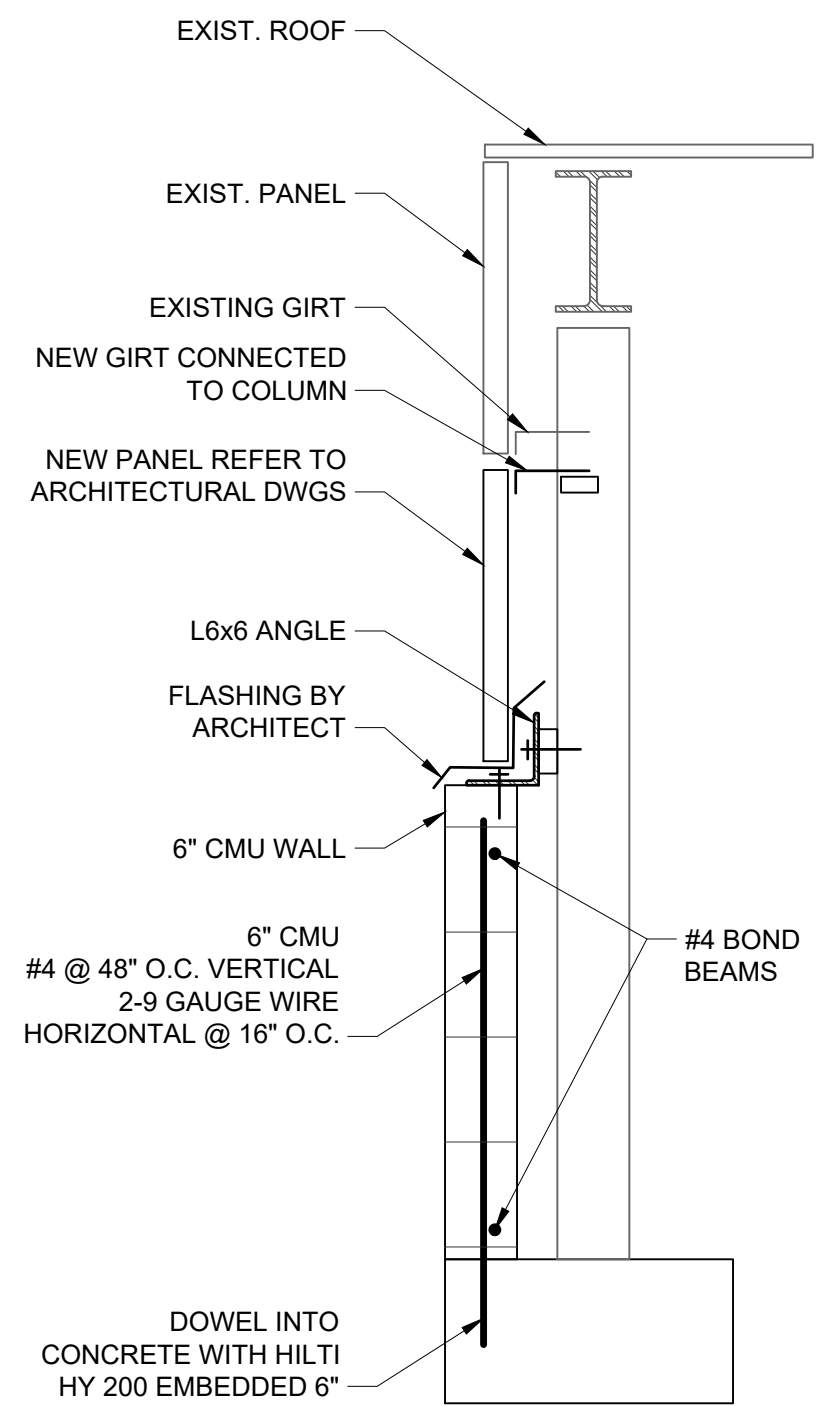
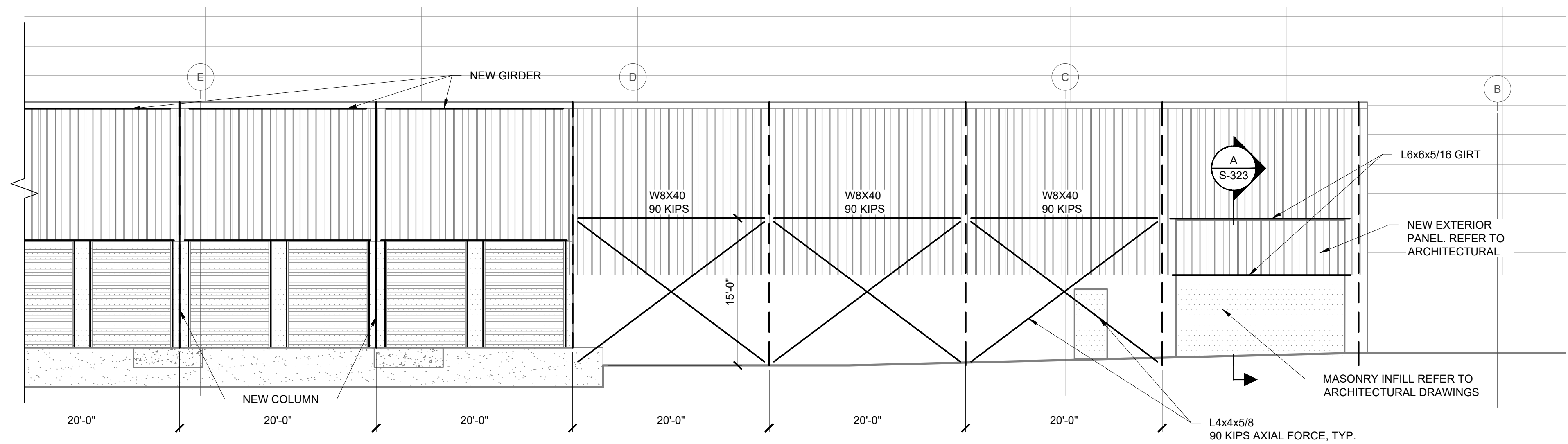
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S-322

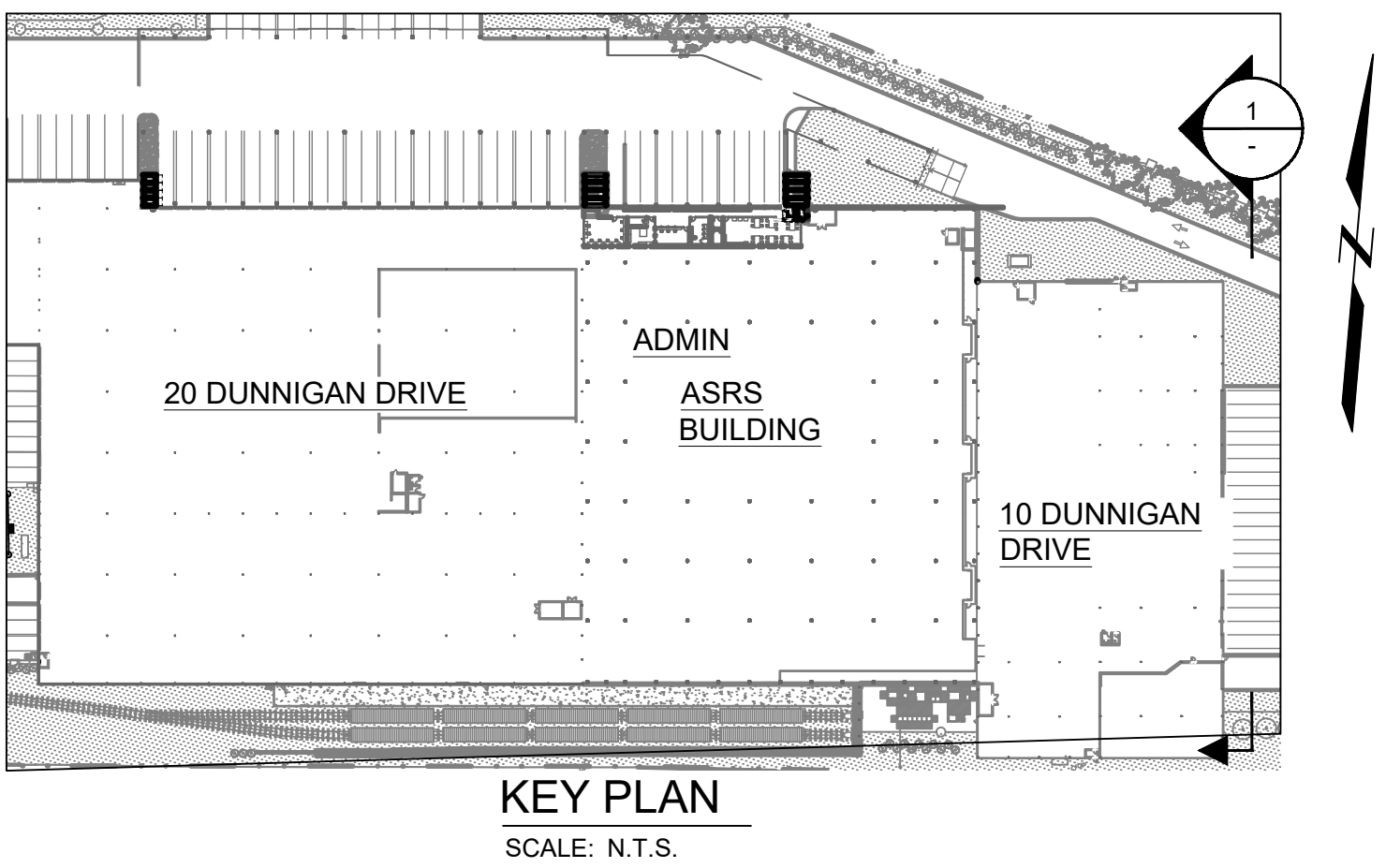
48 OF 61



1 EAST ELEVATION 10 DUNNIGAN DRIVE
SCALE: 1/8" = 1'-0"



A TYPICAL SECTION AT WALL INFILLS
SCALE: 3/4" = 1'-0"



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DRAWING TITLE :

EAST ELEVATION 10
DUNNIGAN



DWG NUMBER :

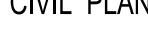
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4/4/22



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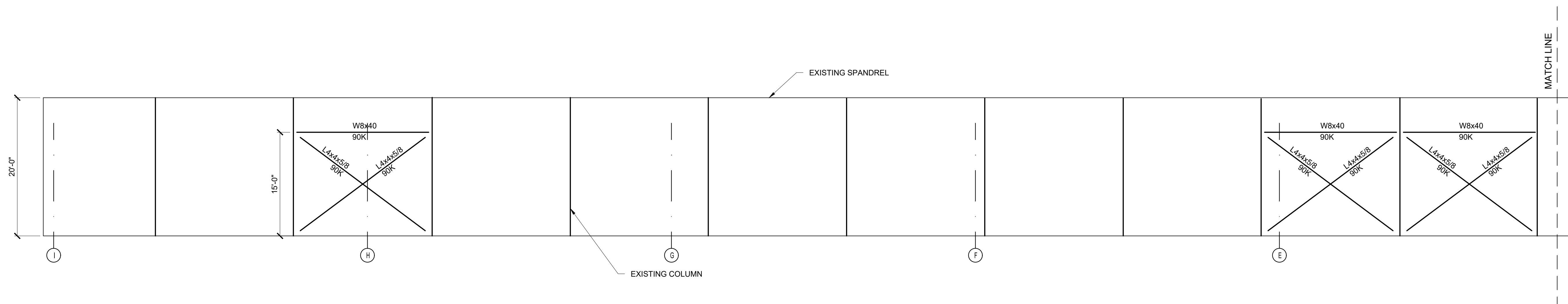
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DRAWING TITLE:

BRACING ELEVATION AT
10 DUNNIGAN WEST WAL

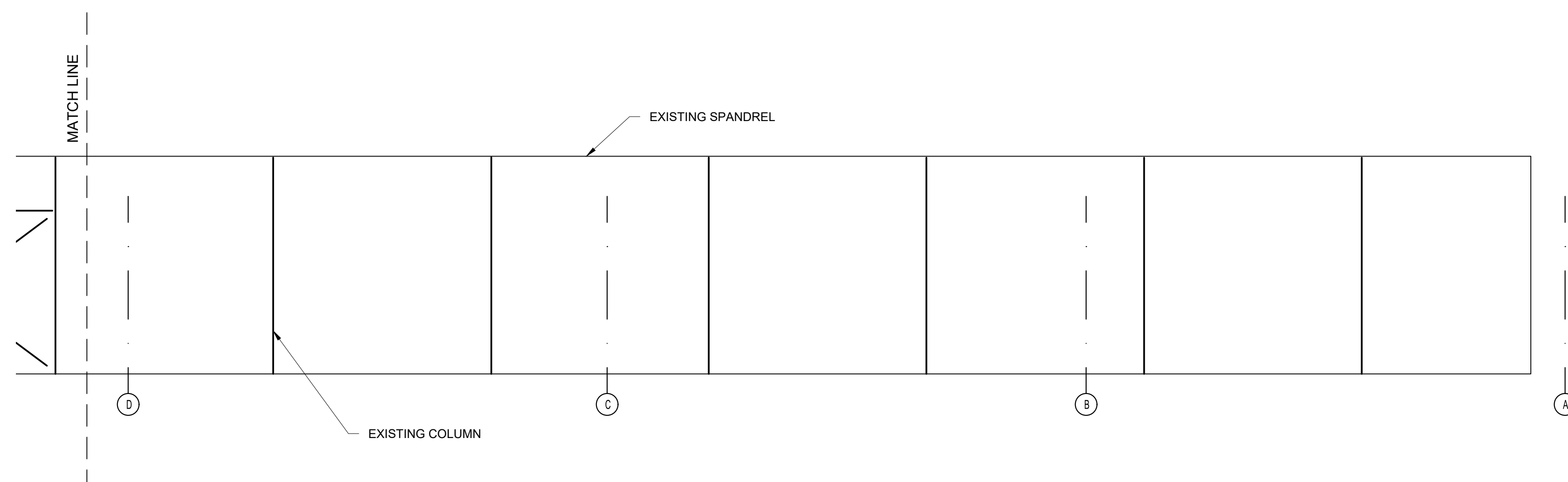
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S-324

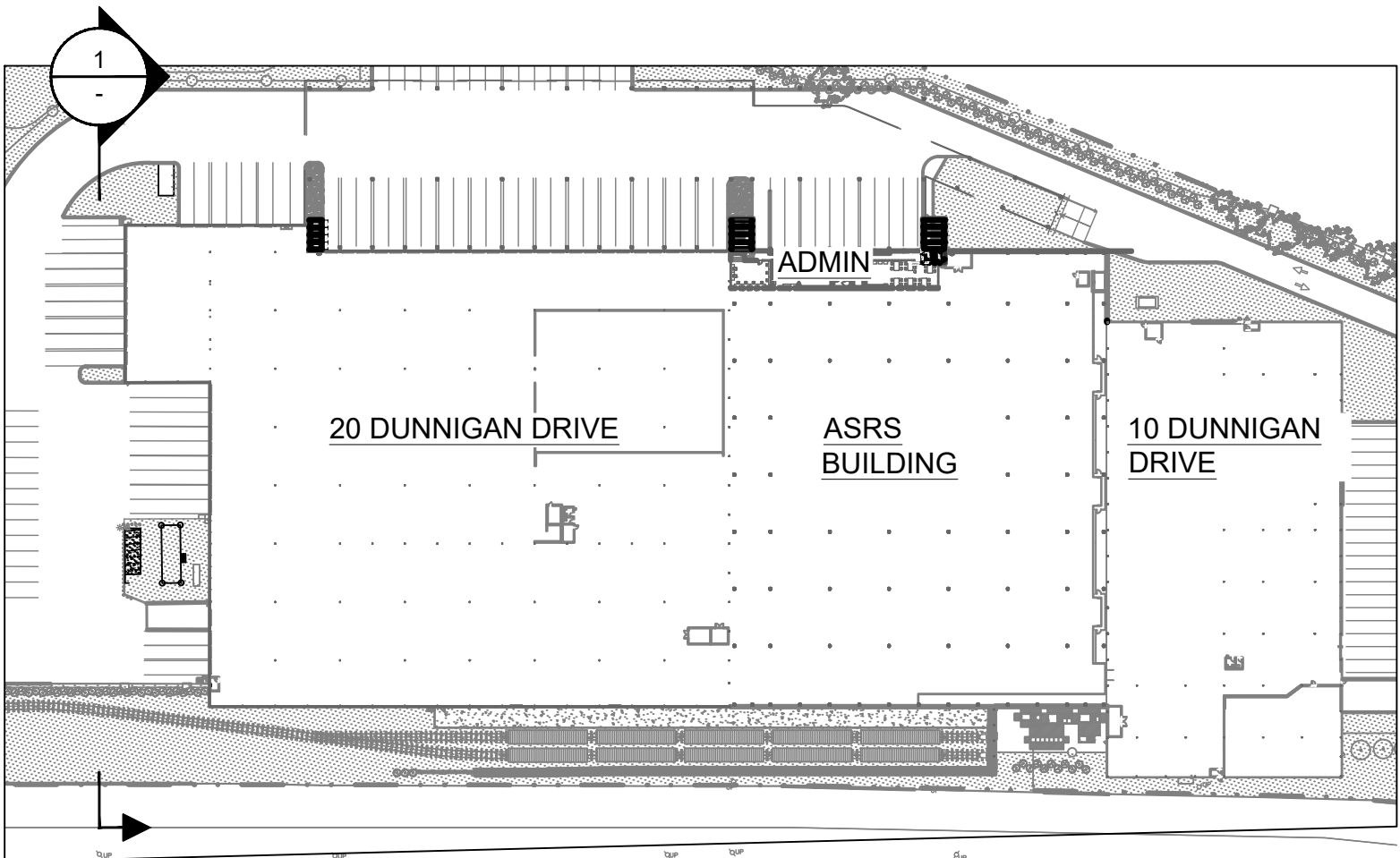
50 OF 61



BRACING ELEVATION AT 10
DUNNIGAN WEST WAL

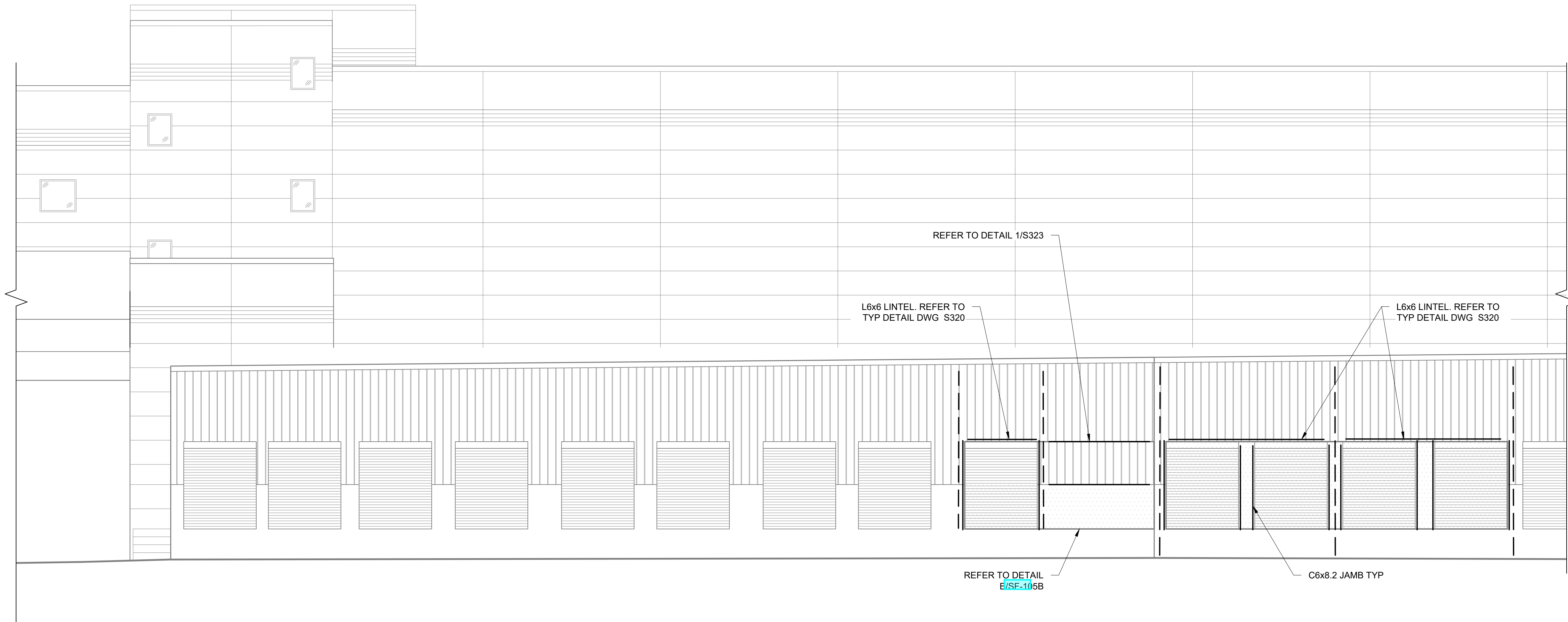
SCALE: 1/8" = 1'-0"





KEY PLAN

SCALE: N.T.S.



1 WEST ELEVATION

SCALE: 1/8" = 1'-0"

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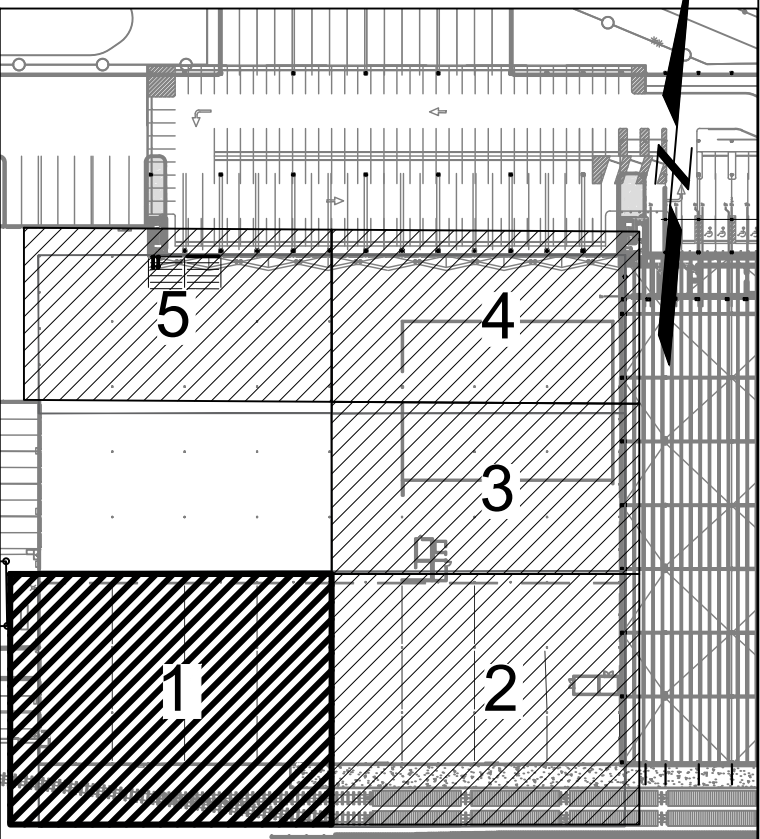
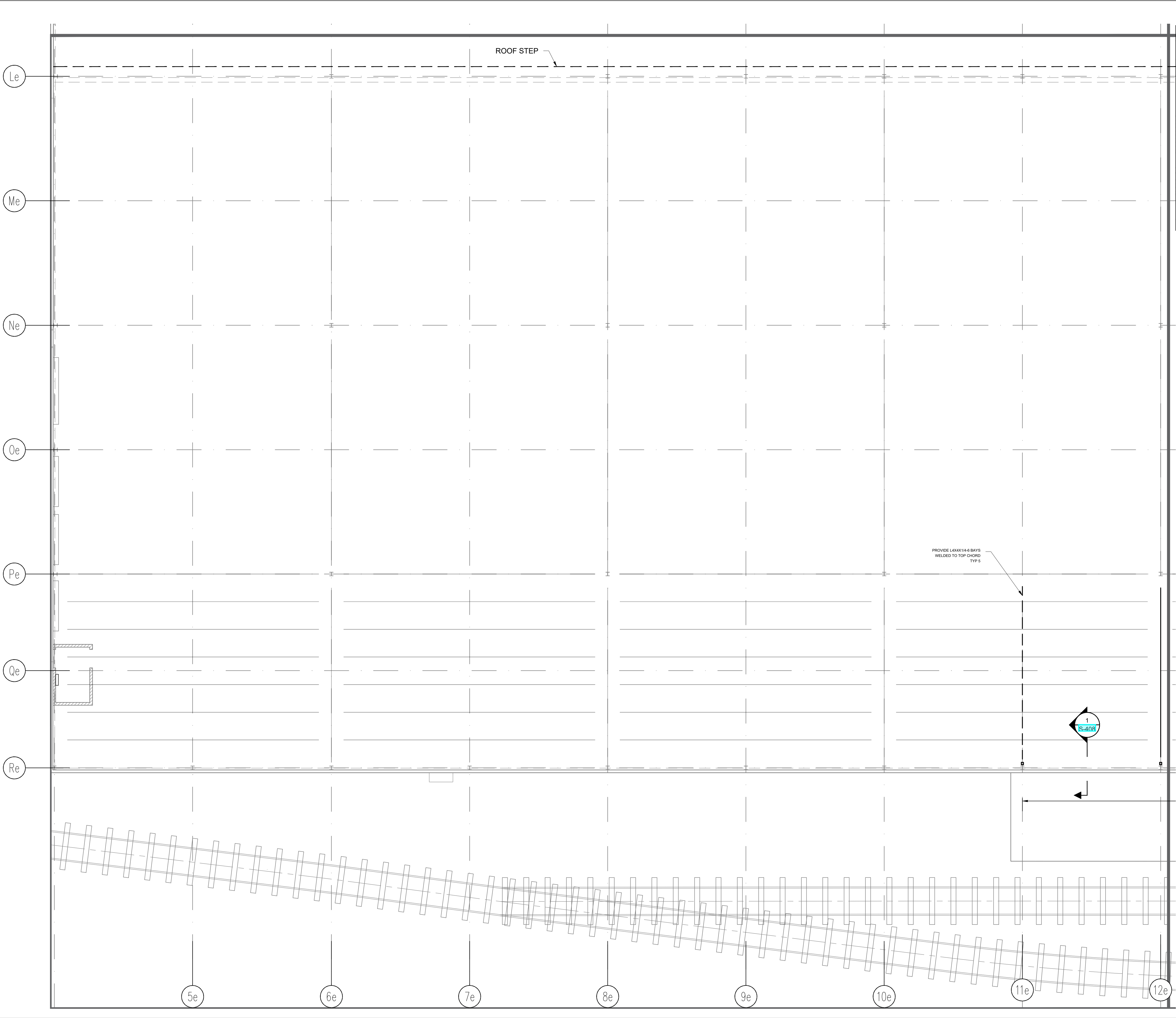
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SCALE :	AS SHOWN

DRAWING TITLE :

EXTERIOR PANEL
SUPPORTS



DWG NUMBER :
S-325
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4/4/22



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SCALE: N.T.S.

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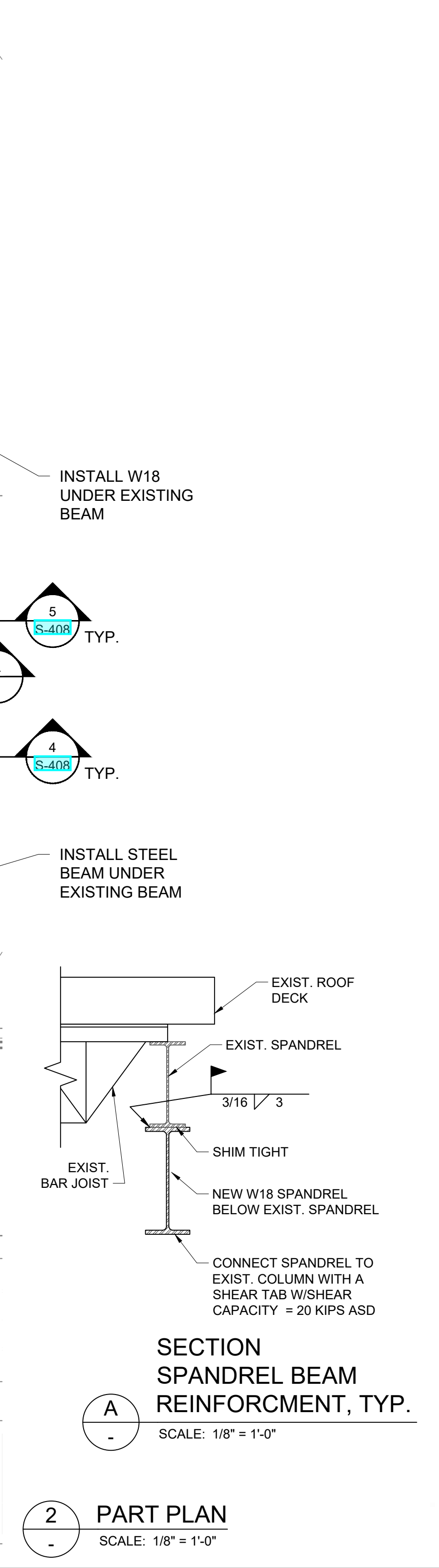
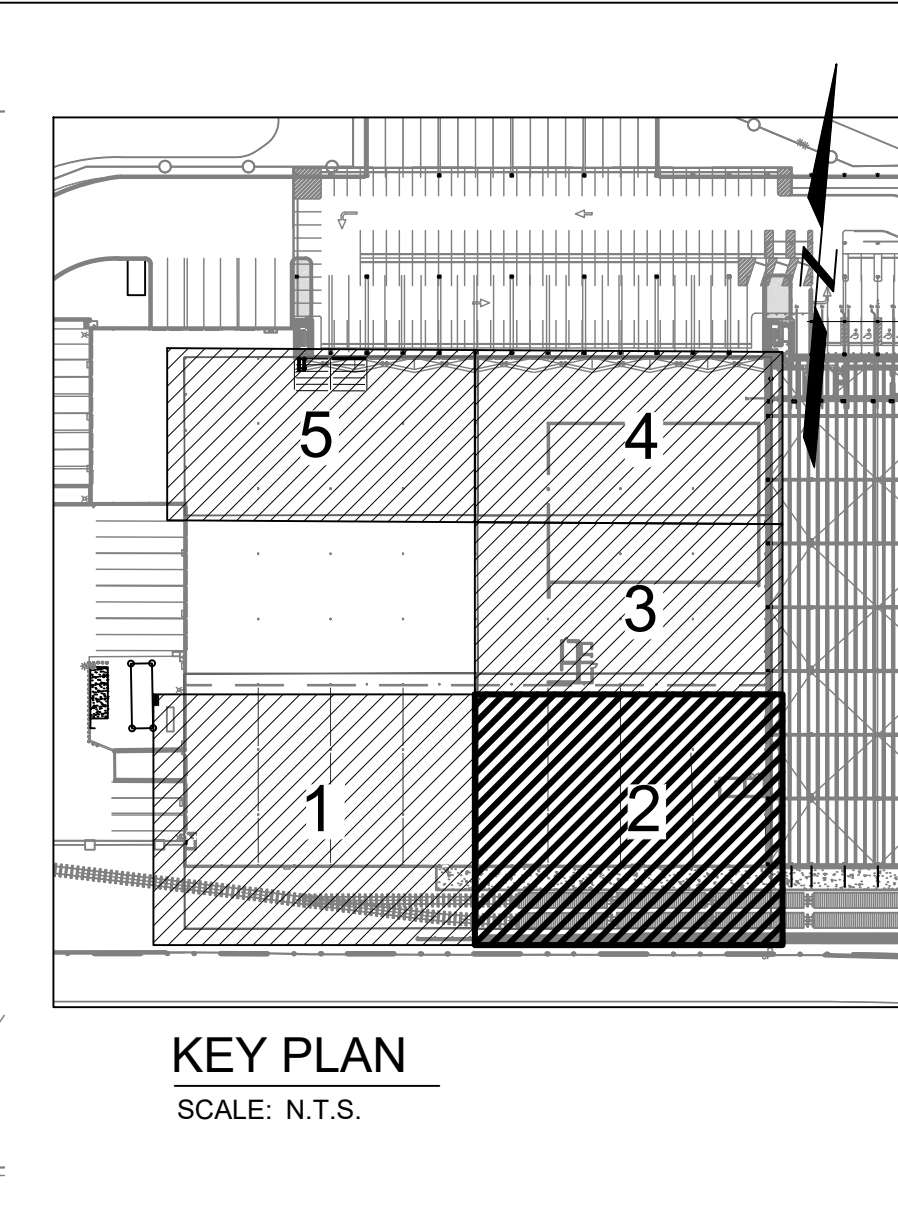
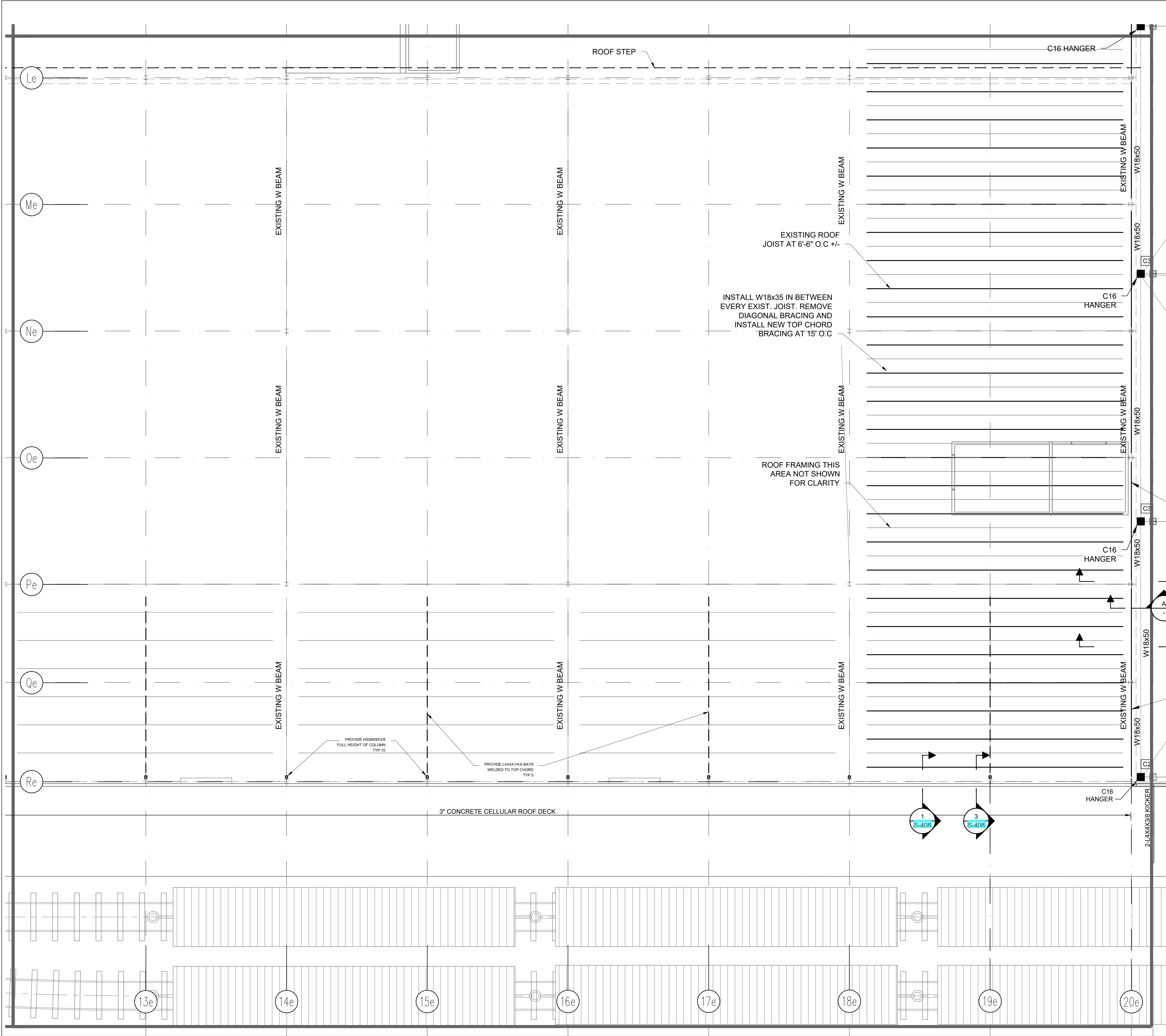
DRAWING TITLE :

20 DUNNIGAN DRIVE ROOF
REINFORCING FRAMING PLAN



DWG NUMBER :
S-400
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1 PART PLAN
SCALE: 1/8" = 1'-0"



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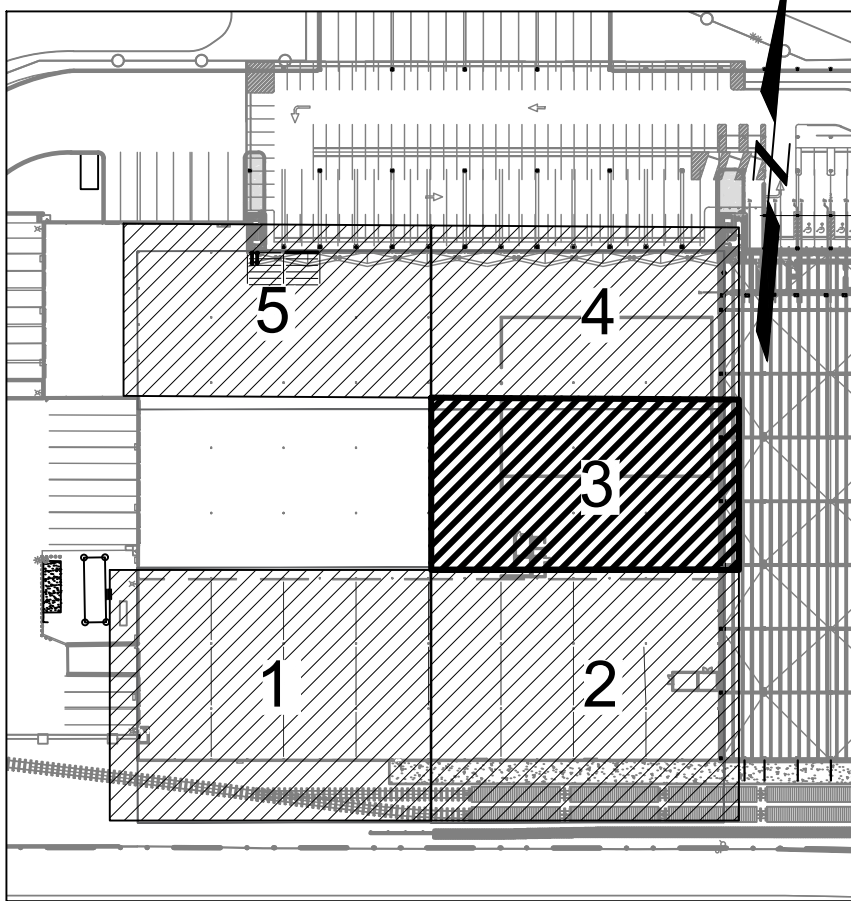
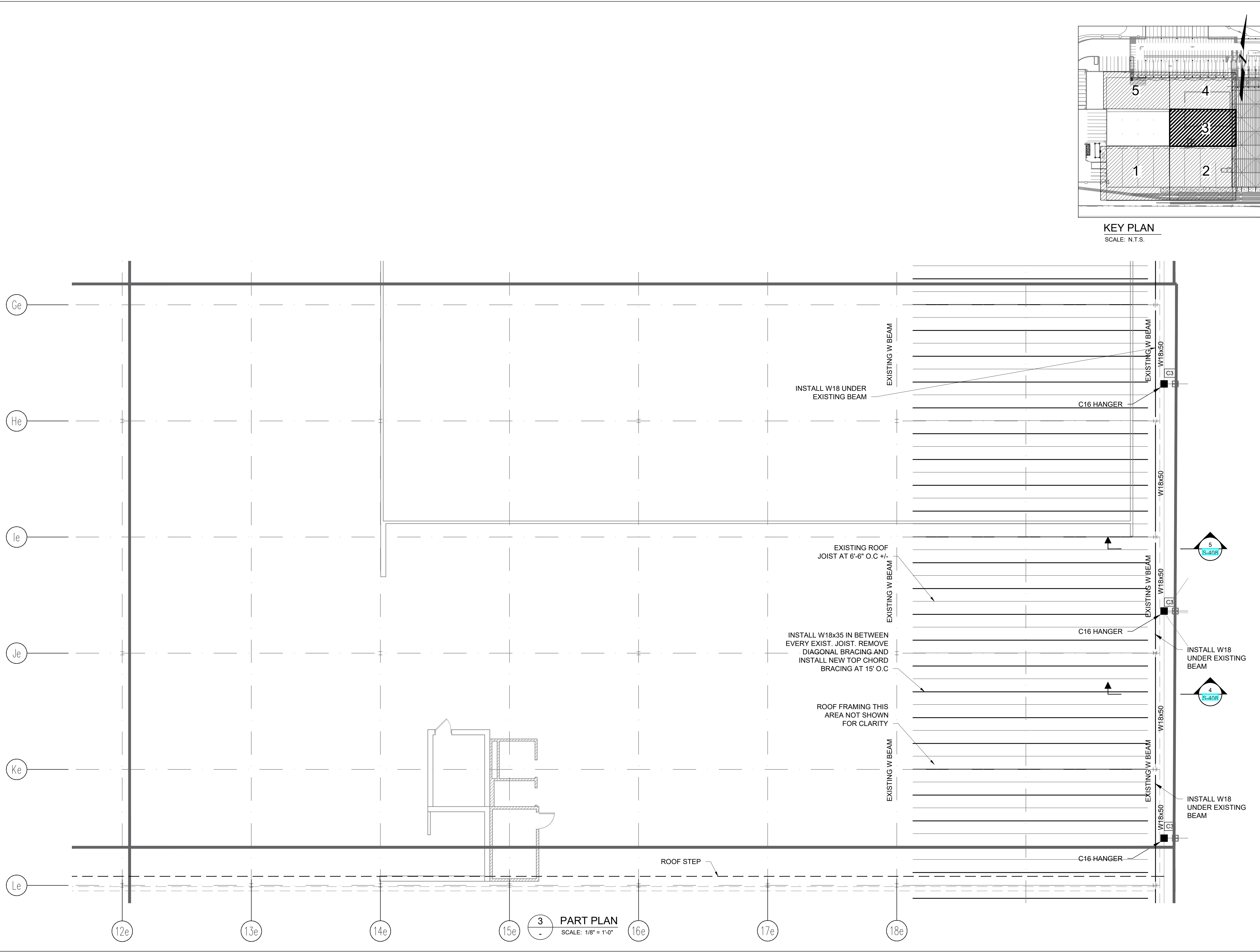
DRAWING TITLE :

20 DUNNIGAN DRIVE ROOF REINFORCING FRAMING PLAN

**PART PLAN**
SCALE: 1/8" = 1'-0"

DWG NUMBER :
S-401

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KEY PLAN
SCALE: N.T.S.

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REINFORCING FRAMING PLAN

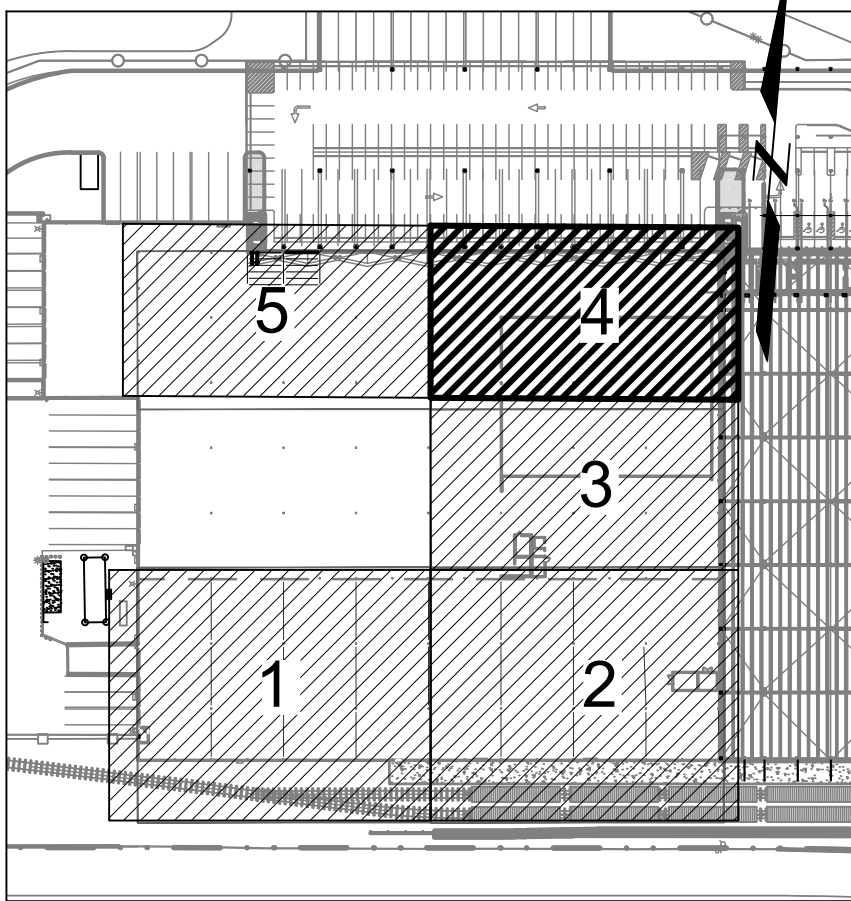
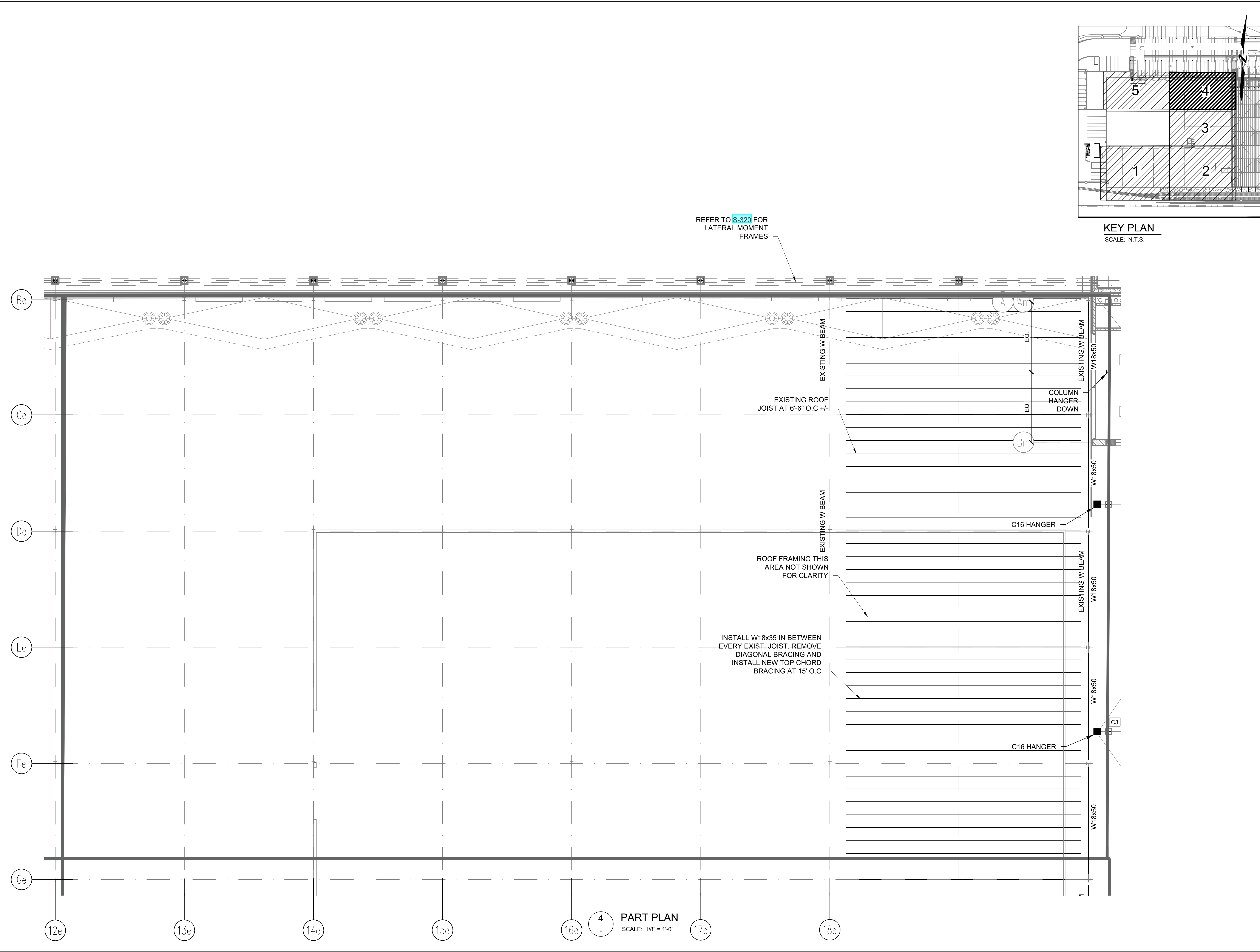


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S-402

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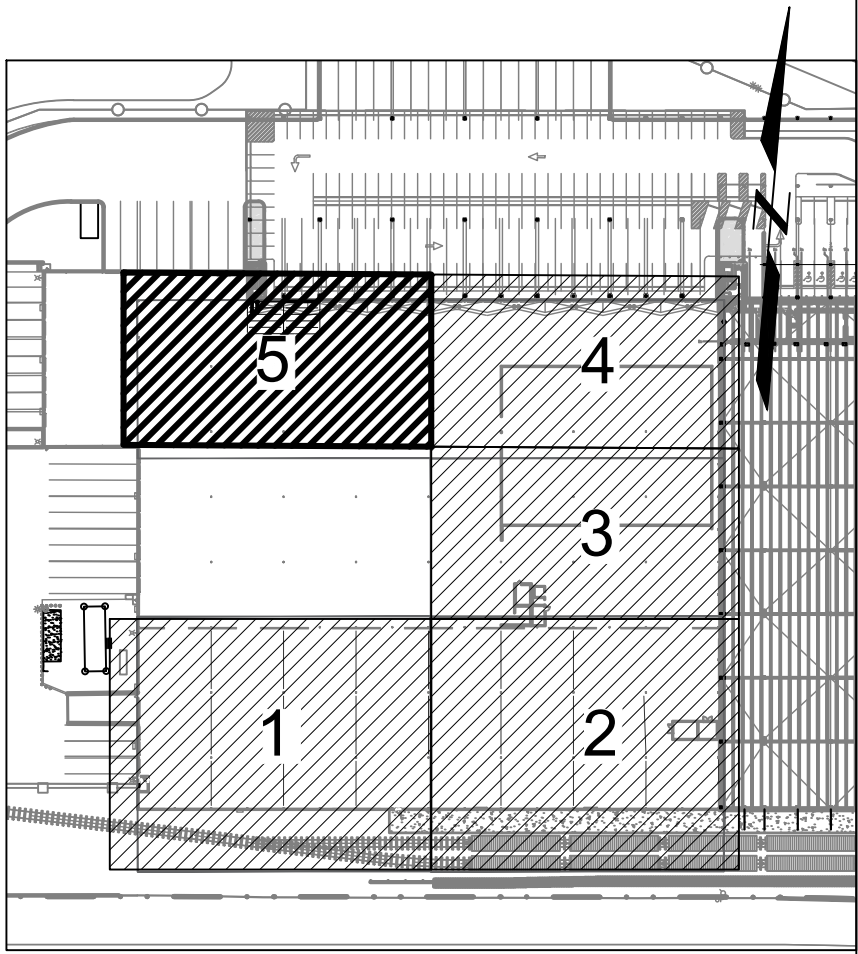
20 DUNNIGAN DRIVE ROOF
REINFORCING FRAMING PLAN



DWG NUMBER :
S-403
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5 PART PLAN
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.

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DRAWING TITLE :

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REINFORCING FRAMING PLAN

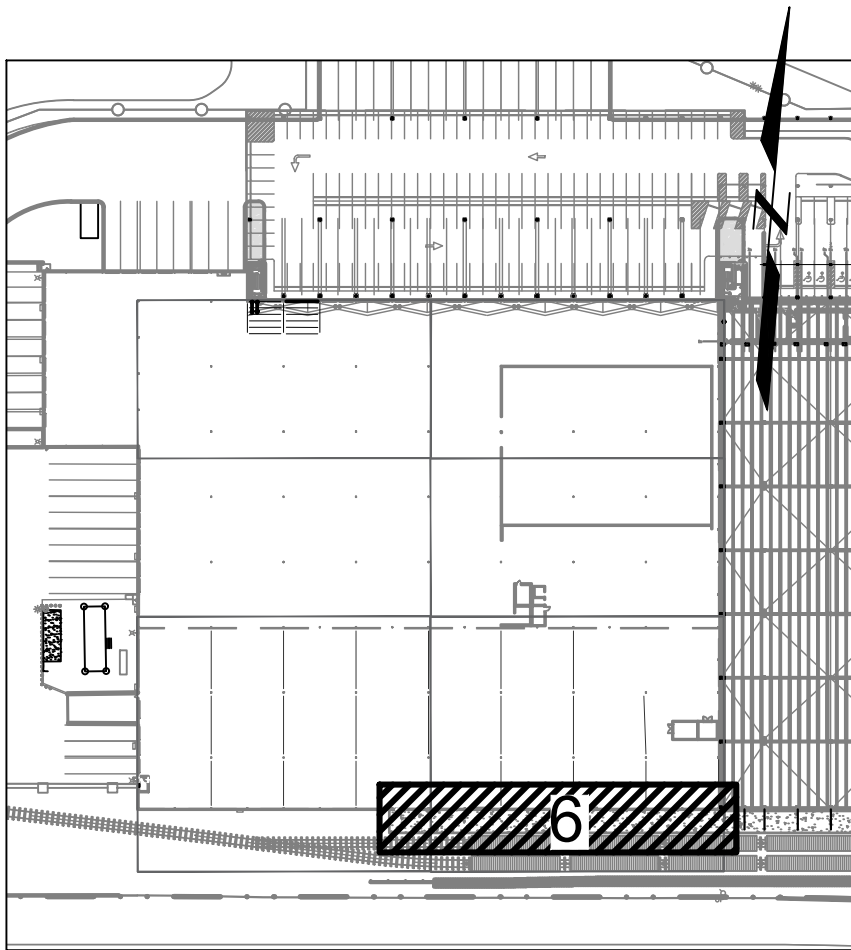


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4/4/22



KEY PLAN
SCALE: N.T.S.

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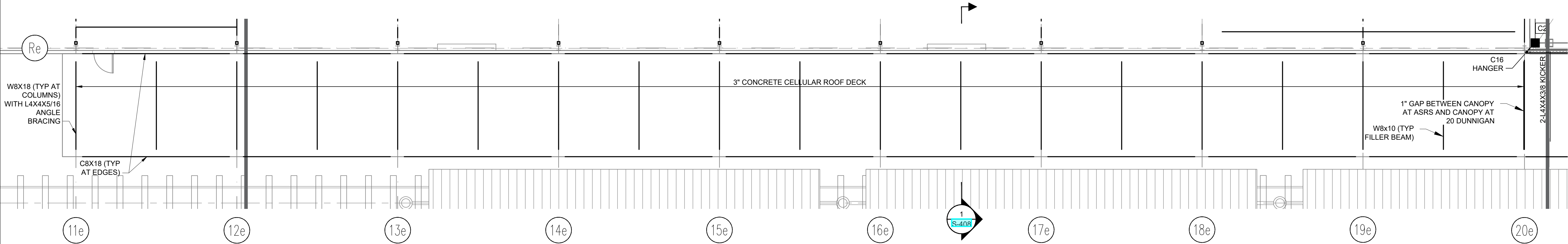
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DRAWING TITLE :

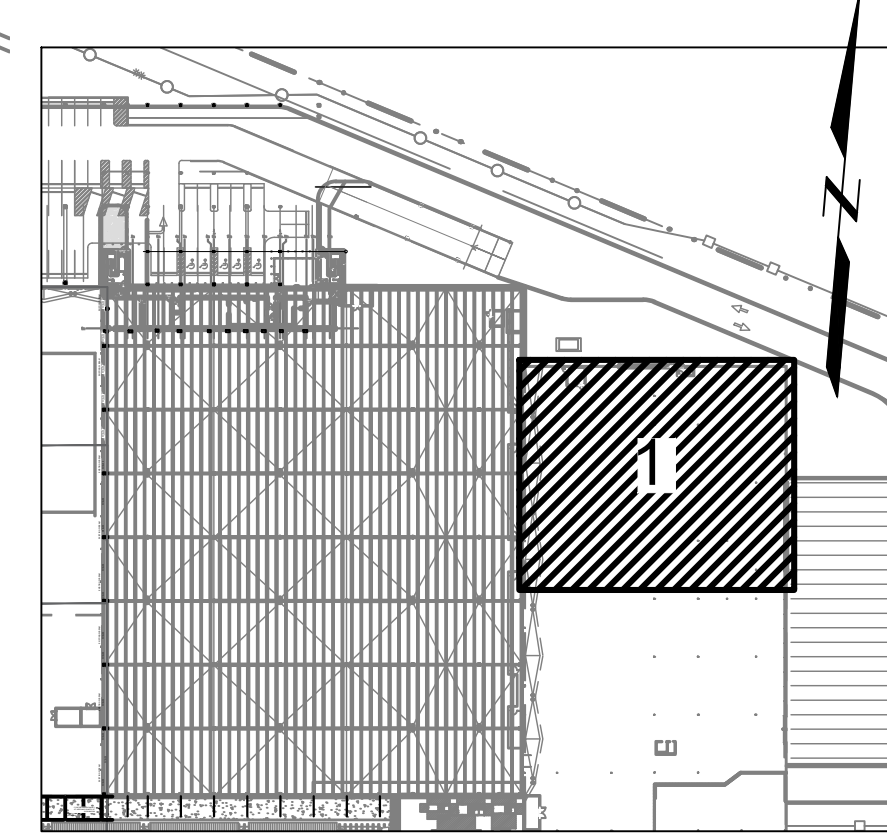
PROPOSED CANOPY FRAMING
PLAN 20 DUNNIGAN DRIVE



DWG NUMBER :
S-405
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6 PART PLAN
SCALE: 1/8" = 1'-0"



NEW DIAGONAL BRACING
REFER TO BUILDING
ELEVATION

— EXISTING ROOF JOISTS
TO REMAIN

1 PART PLAN
SCALE: 1/8" = 1'-0"

dID

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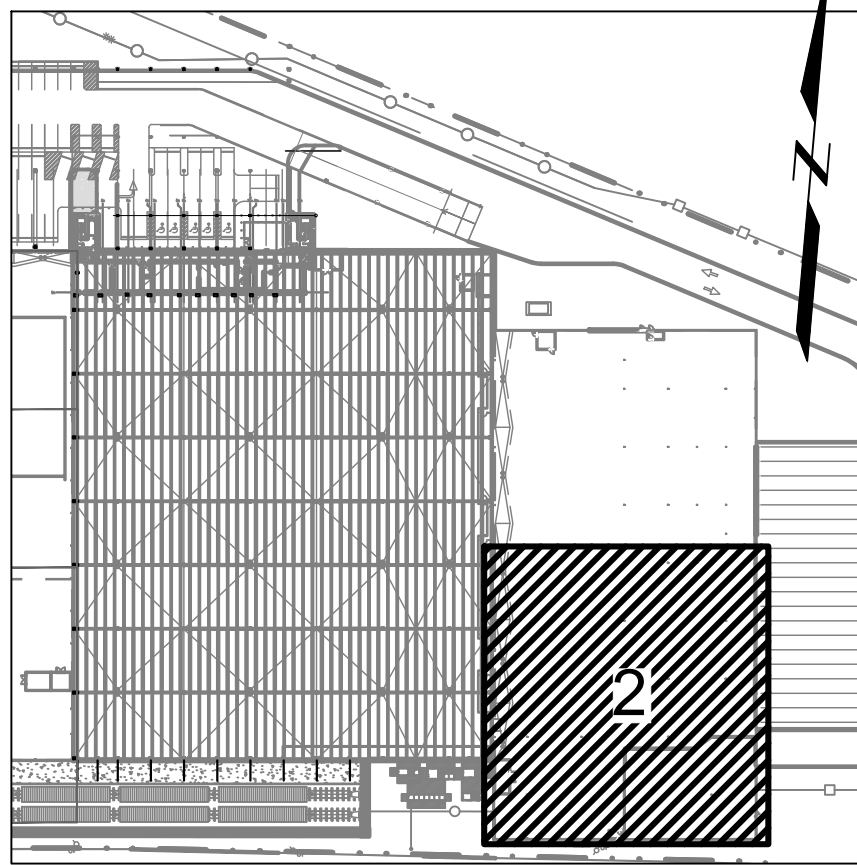
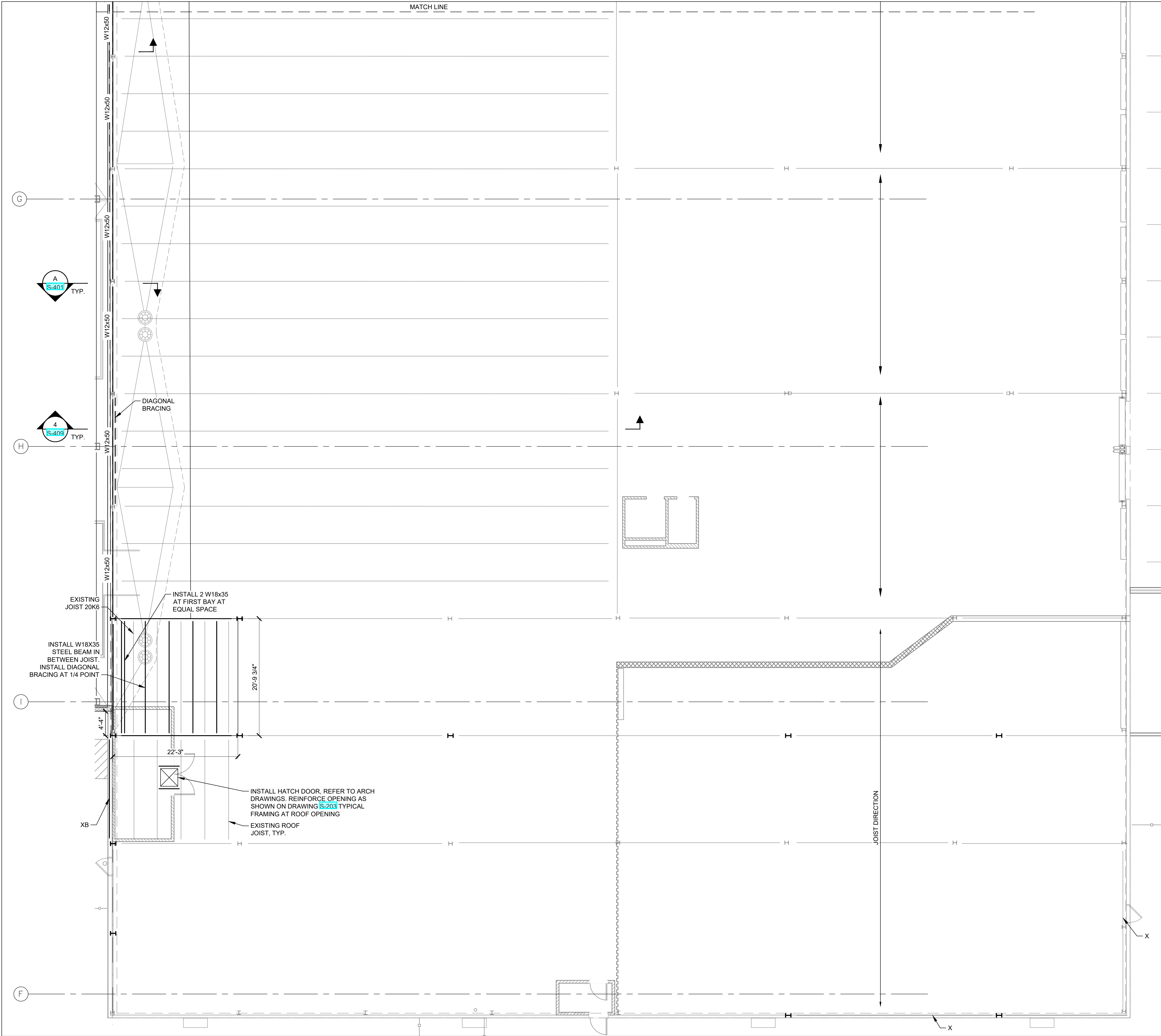
DRAWING TITLE :

10 DUNNIGAN DRIVE
ROOF FRAMING PLAN



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KEY PLAN
SCALE: N.T.S.

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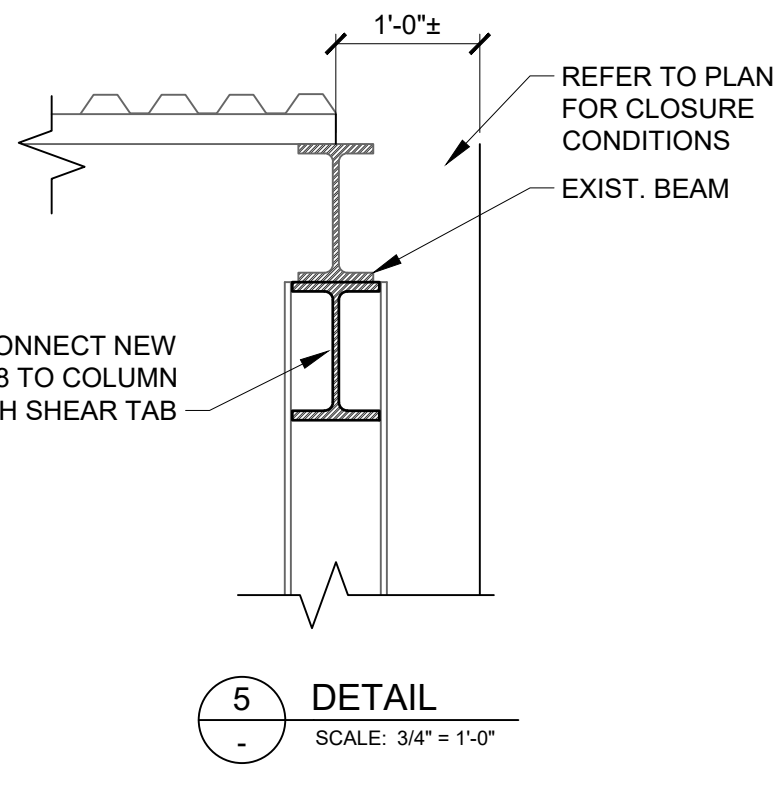
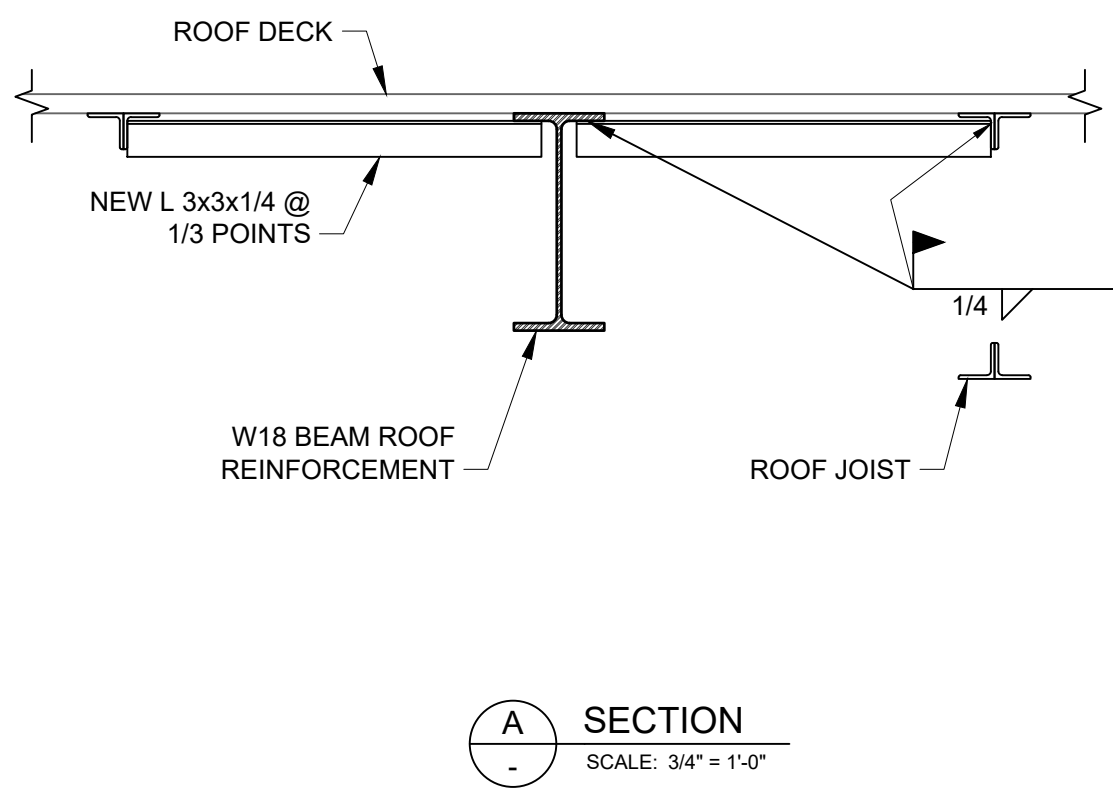
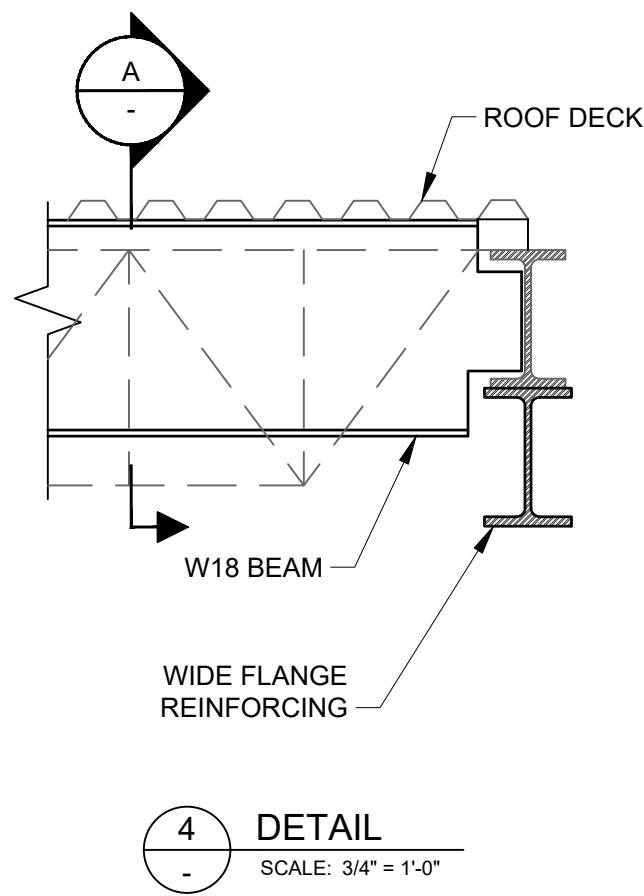
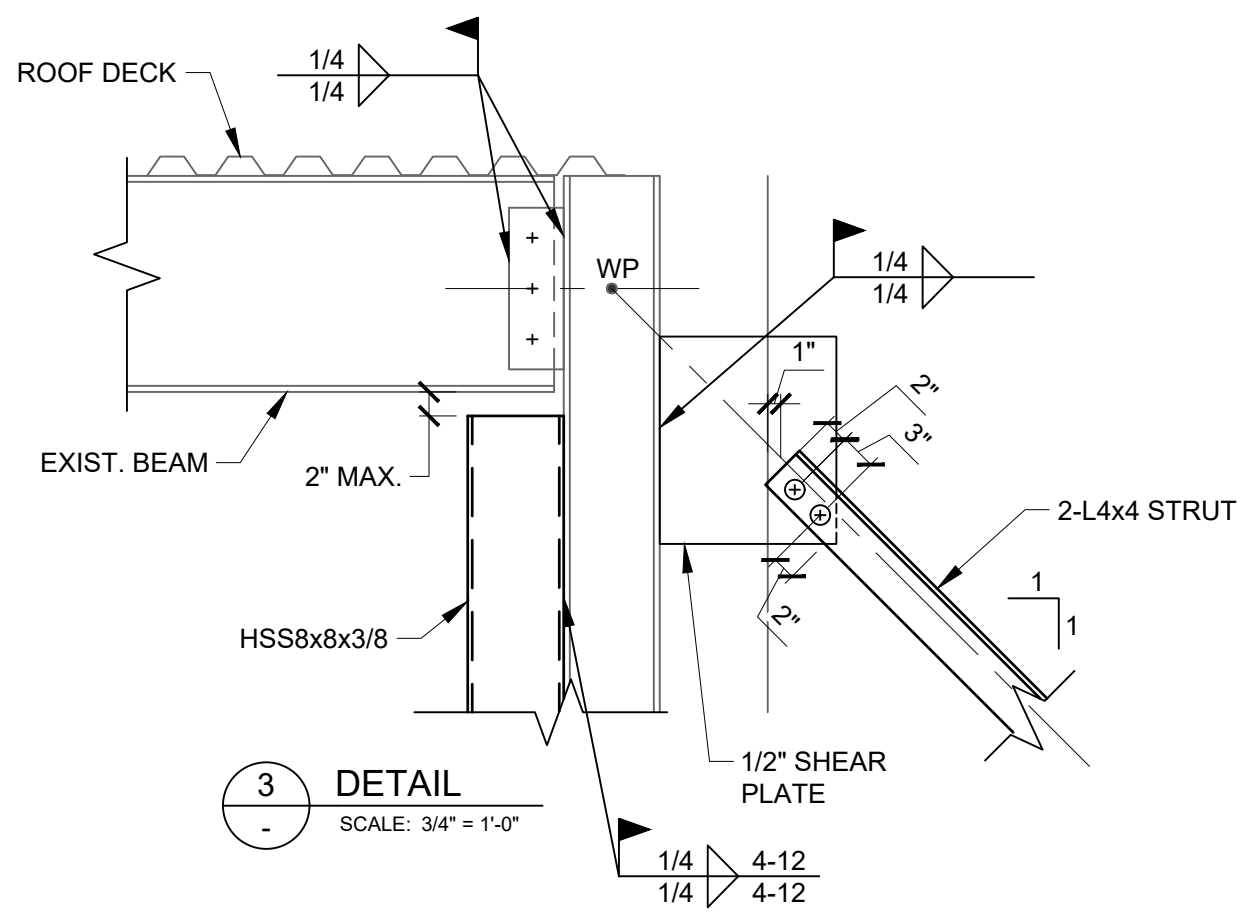
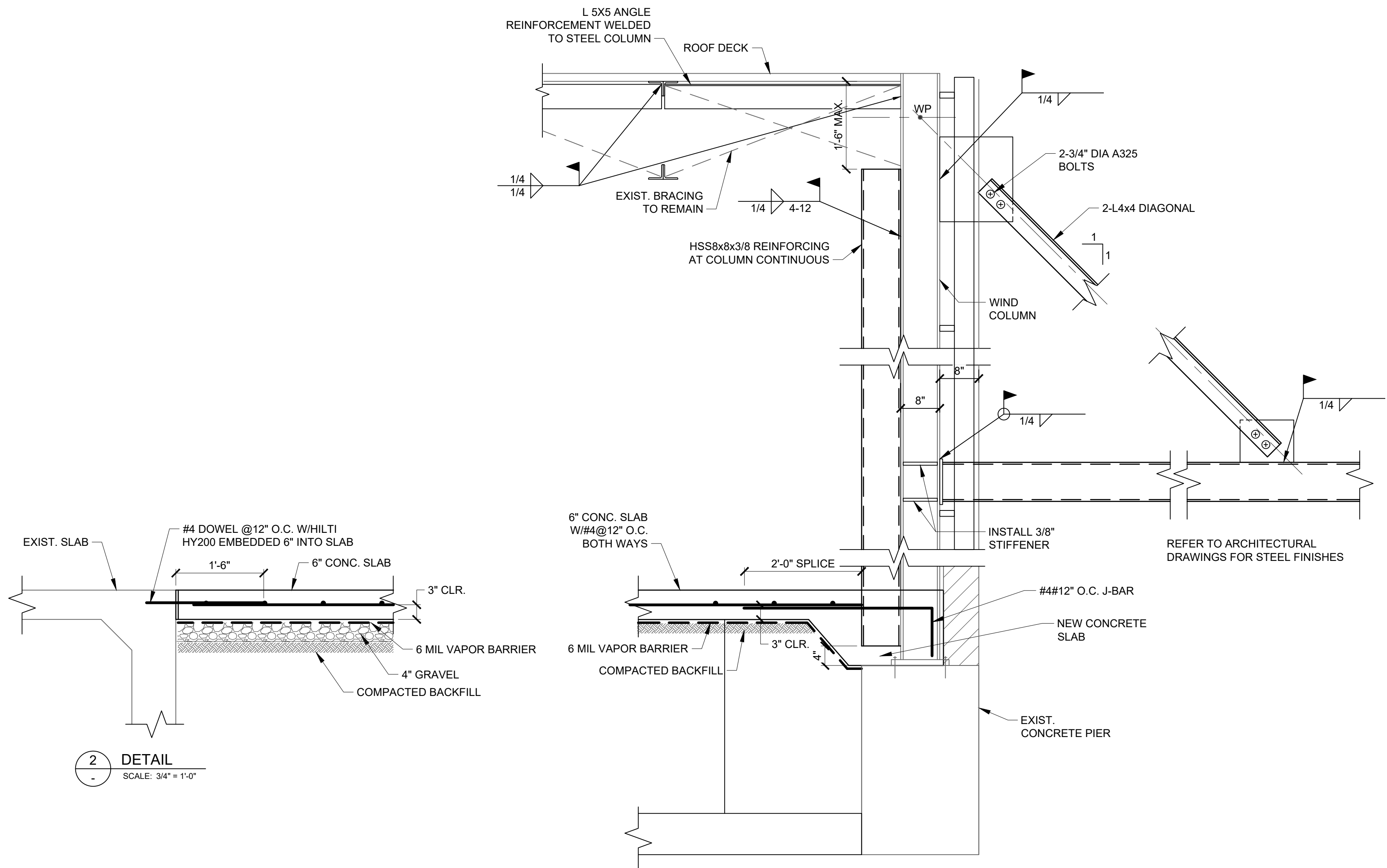
10 DUNNIGAN DRIVE
ROOF FRAMING PLAN



DWG NUMBER :

S-407

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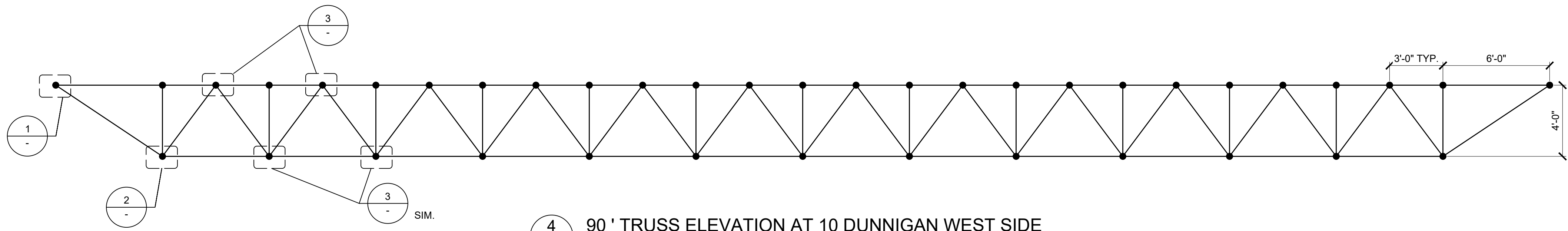
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20 DUNNIGAN DRIVE
DETAILS

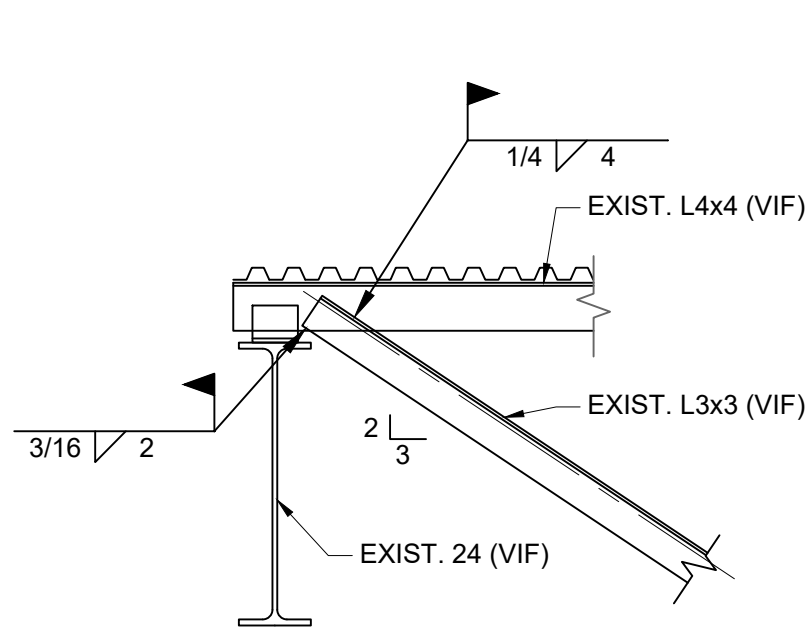


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S-408

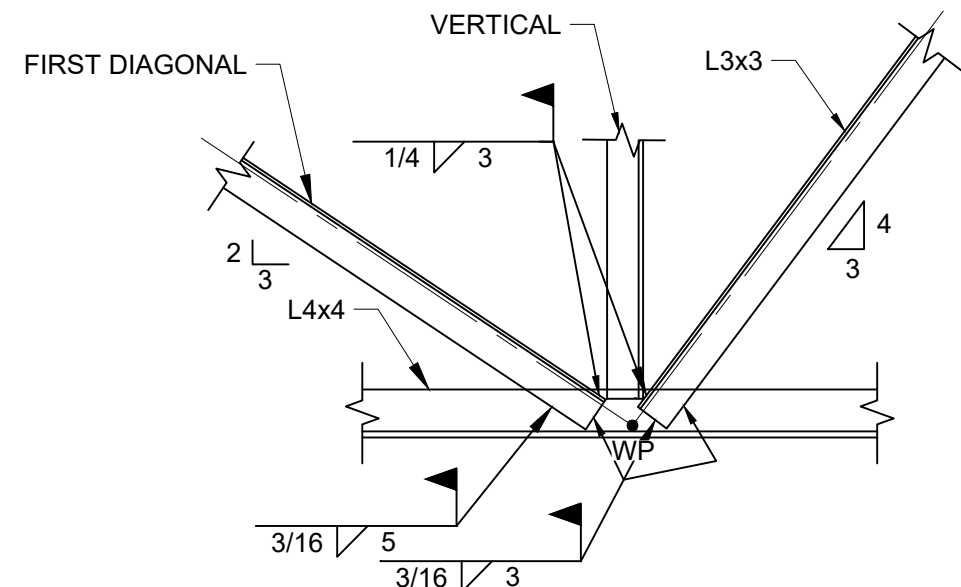
60 OF 61



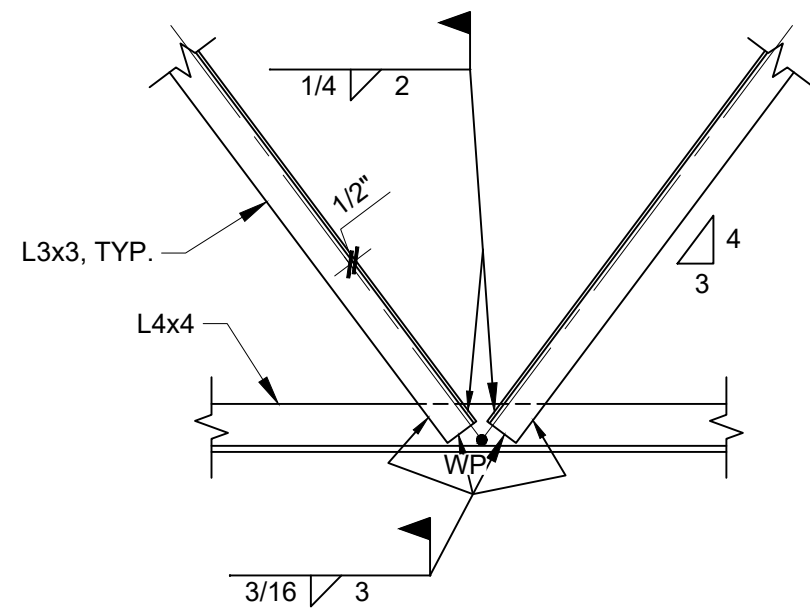
4 90' TRUSS ELEVATION AT 10 DUNNIGAN WEST SIDE
SCALE: 1/4" = 1'-0"



1 DETAIL
SCALE: 3/4" = 1'-0"



2 DETAIL
SCALE: 3/4" = 1'-0"



3 TYP. CONNECTION AT
INTERIOR PANEL POINT WHERE
SHOWN ON ELEVATION
SCALE: 3/4" = 1'-0"



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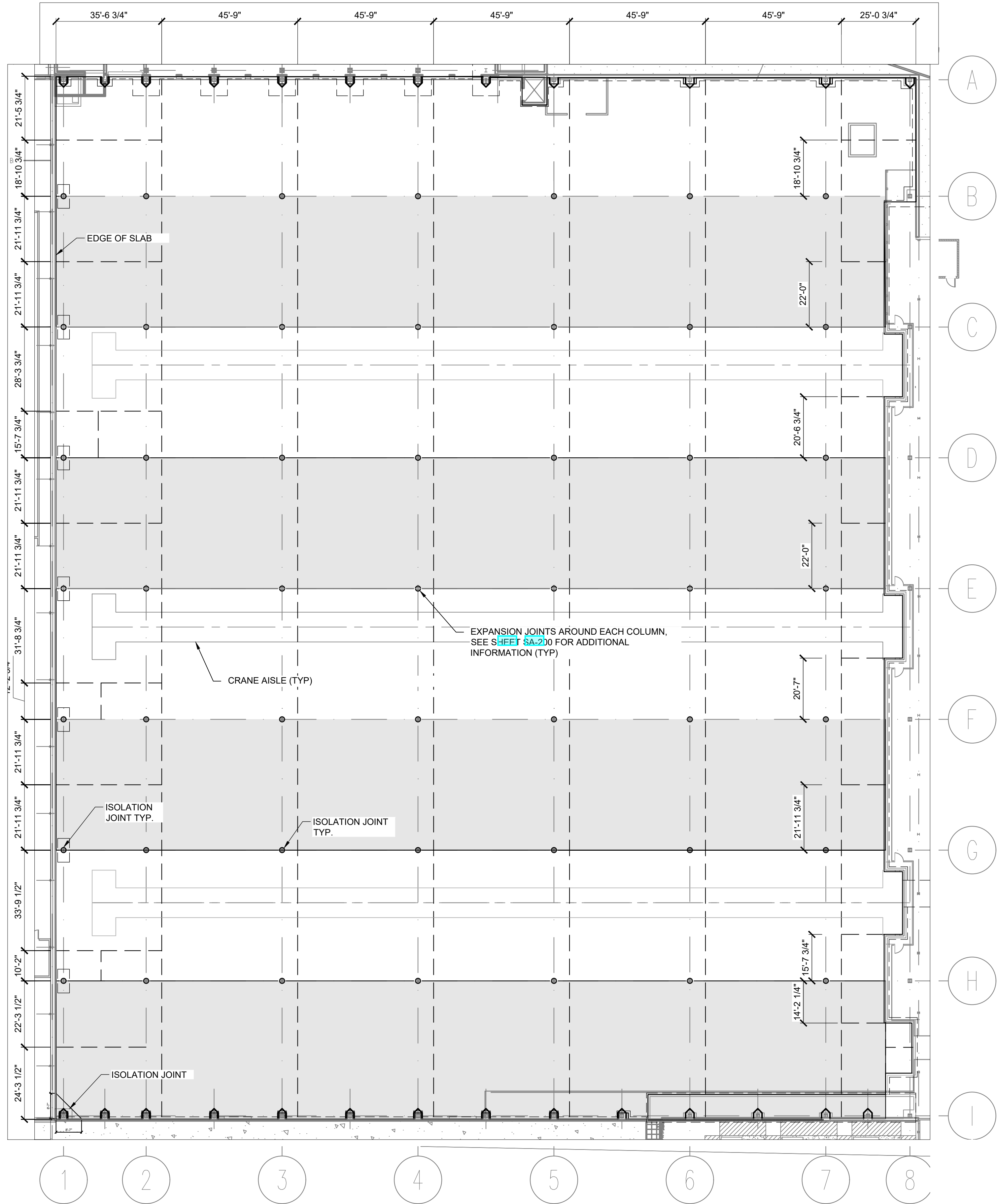
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20 DUNNIGAN DRIVE
DETAILS



DWG NUMBER :
S-409
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- LEGEND
- CONSTRUCTION OR CONTRACTION JOINT
 - - - CONTRACTION JOINT
 - EXPANSION JOINT AROUND COLUMN
 - FIRST POUR SEQUENCE

- NOTES:
1. THERE ARE NO EXPANSION JOINTS IN THE SLAB, EXCEPT AROUND THE COLUMNS. POURS SHALL BE SEQUENCED IN STRIPS AS SHOWN WITH THE SHADED SECTIONS BEING PLACED FIRST. INFILL AT COLUMNS SHALL BE PERFORMED AFTER REMAINDER OF SLAB IS IN PLACE.
 2. AT STRIPS WITH CRANE AISLES, PLACE LOWER SLAB AT CRANE AISLE FIRST, THEN PLACE UPPER SLAB ON EACH SIDE OF THE CRANE AISLE.

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SCALE : AS SHOWN

DRAWING TITLE :

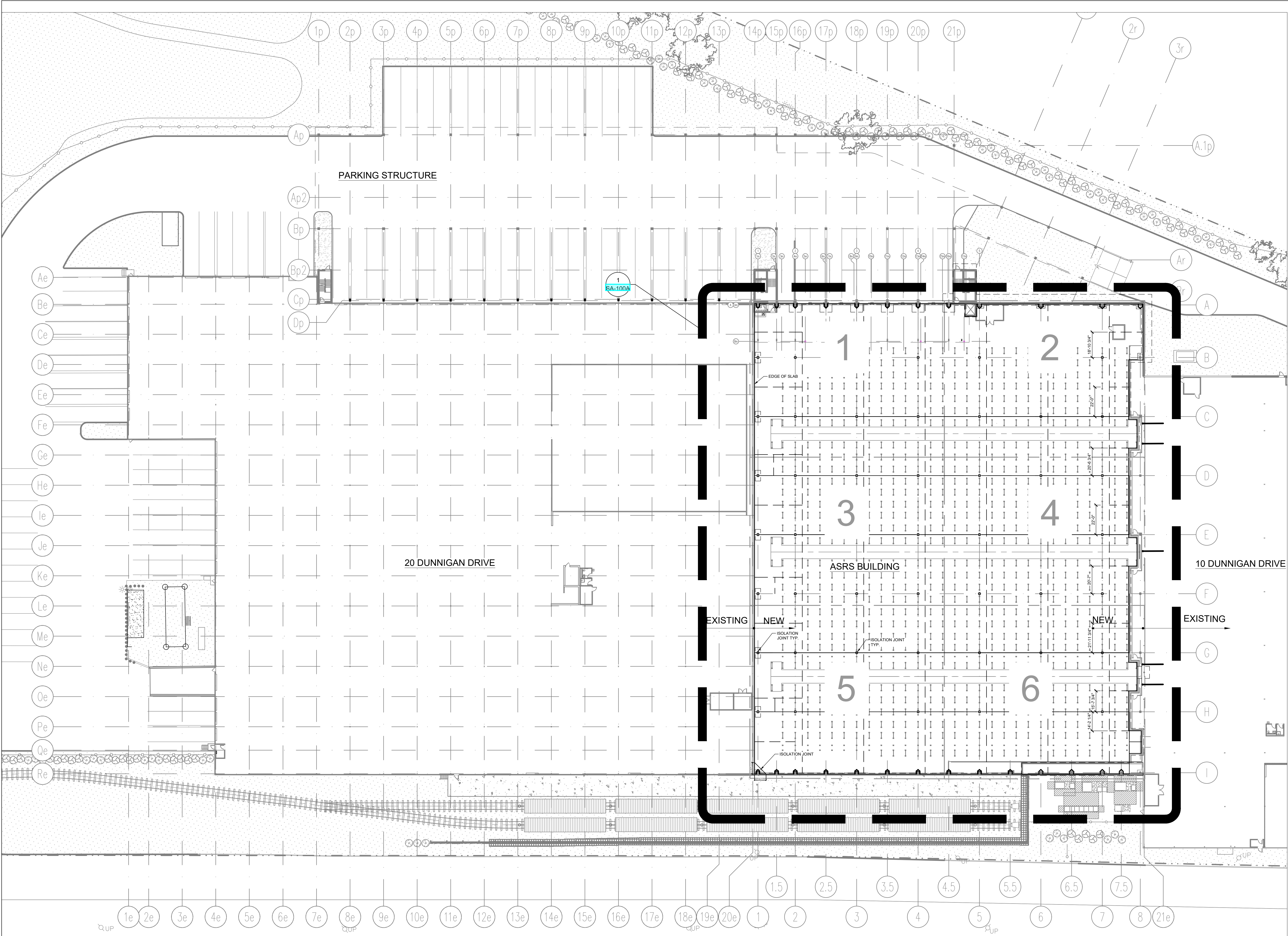
SLAB ON GRADE JOINT
PLAN AND PLACEMENT
SEQUENCE



SA-100A

24 OF 31

SLAB ON GRADE JOINT PLAN AND PLACEMENT SEQUENCE
SCALE: N.T.S.



FOUNDATION KEY PLAN
SCALE: 1/32" = 1'-0"

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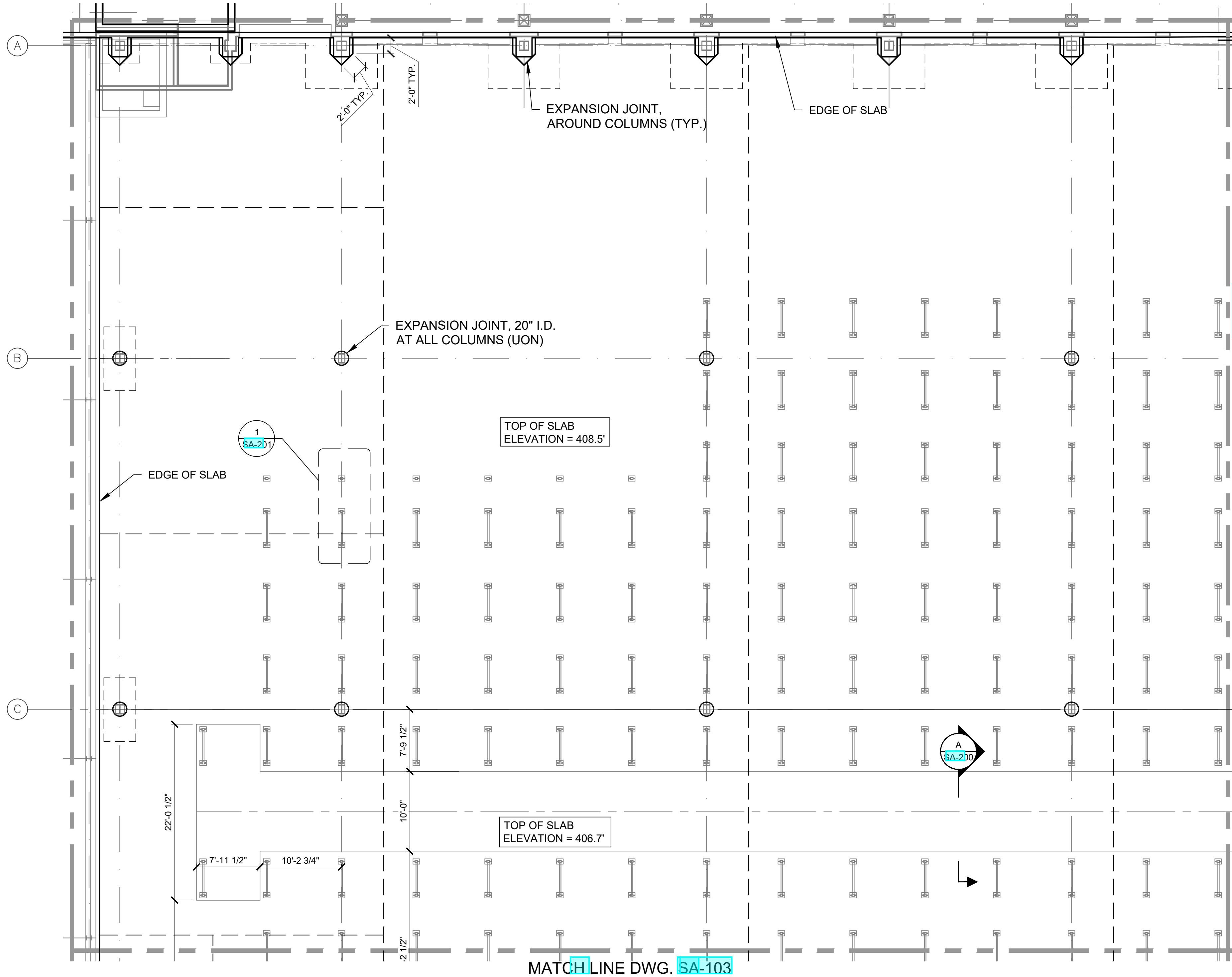
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SLAB ON GRADE - KEY PLAN

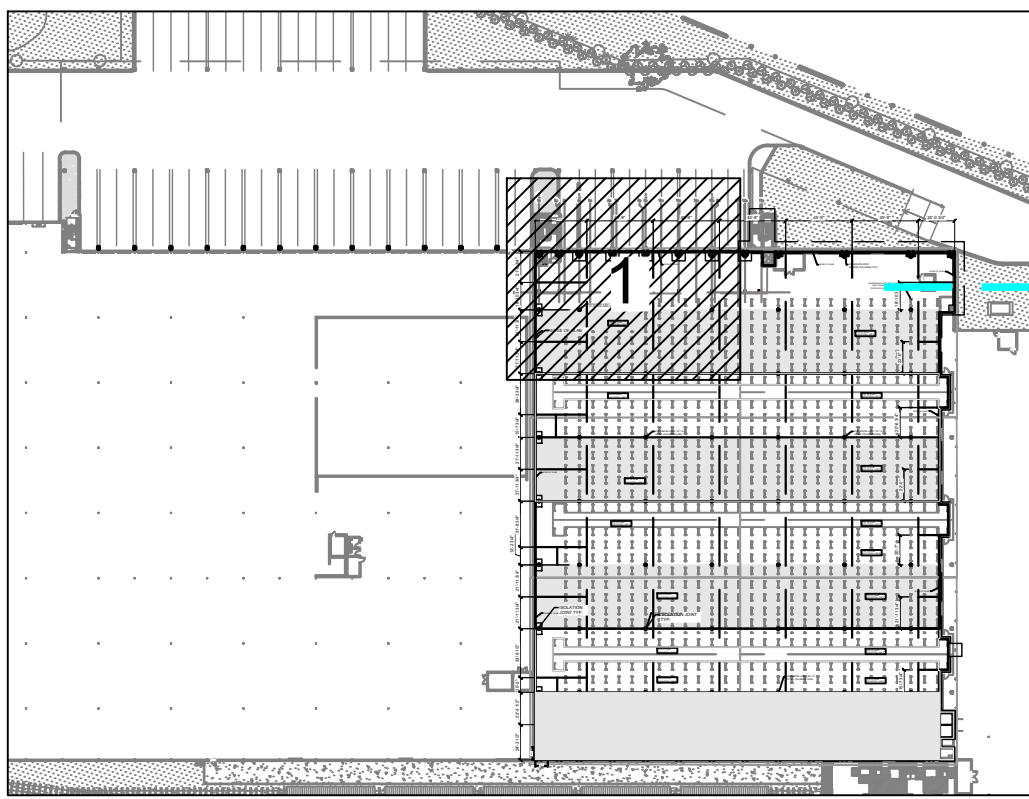


SA-100

20 DUNNIGAN DRIVE



1 PART PLAN
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

LEGEND

- CONSTRUCTION OR CONTRACTION JOINT
- CONTRACTION JOINT
- EXPANSION JOINT AROUND COLUMN

- NOTES:
- FOR SLAB REINFORCING, SEE DRAWING SA-200
 - NO TOP REINFORCING SHALL BE PLACED WITHIN 4 INCHES OF THE ANCHOR BOLTS FOR THE AS/RS BASE PLATES
 - REFER TO ARCH DRAWINGS FOR SLAB FINISH
 - ASRS POST LOADING (ASD)
DL= 3 KIPS
LL=56 KIPS
EQ= 7 KIPS

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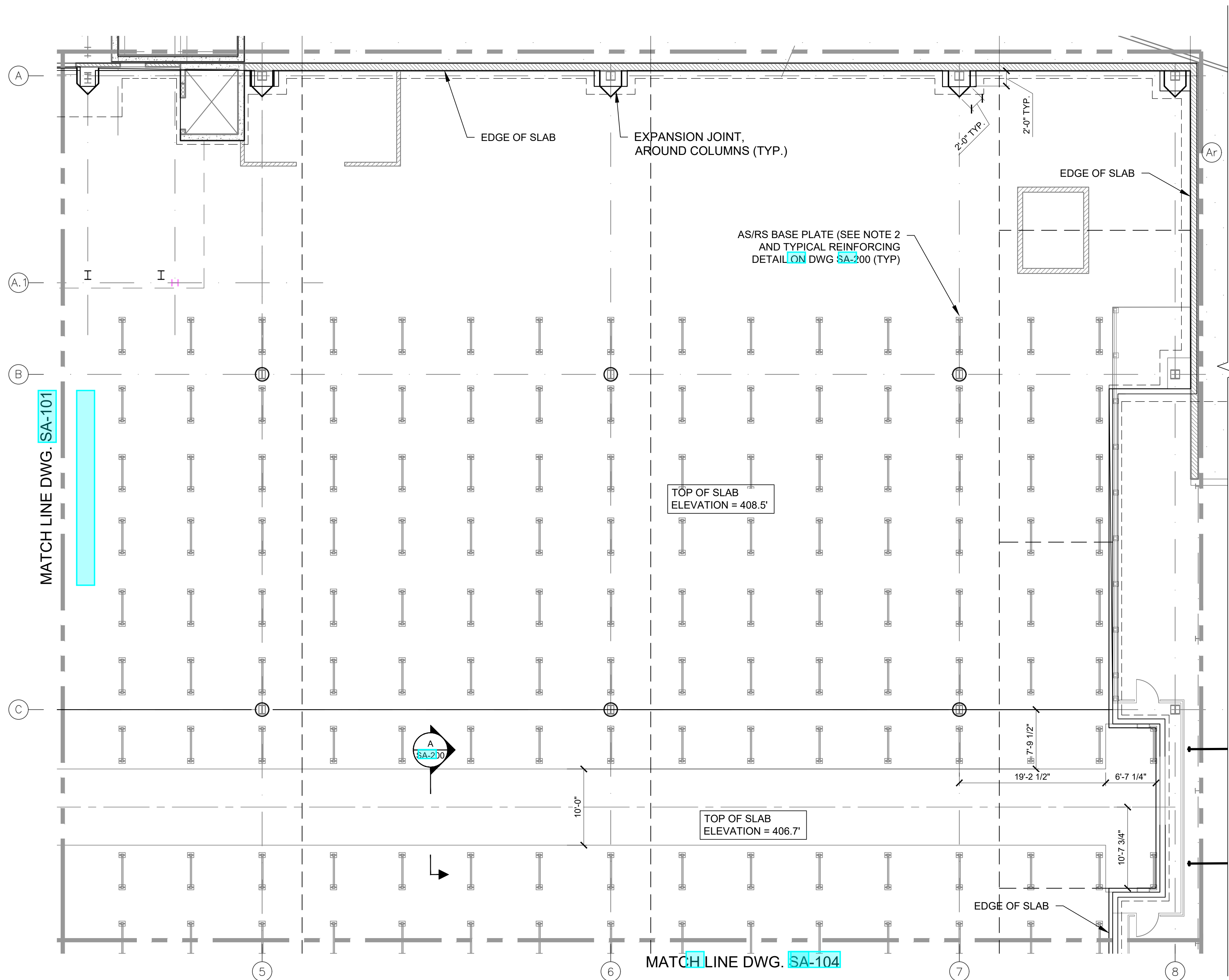
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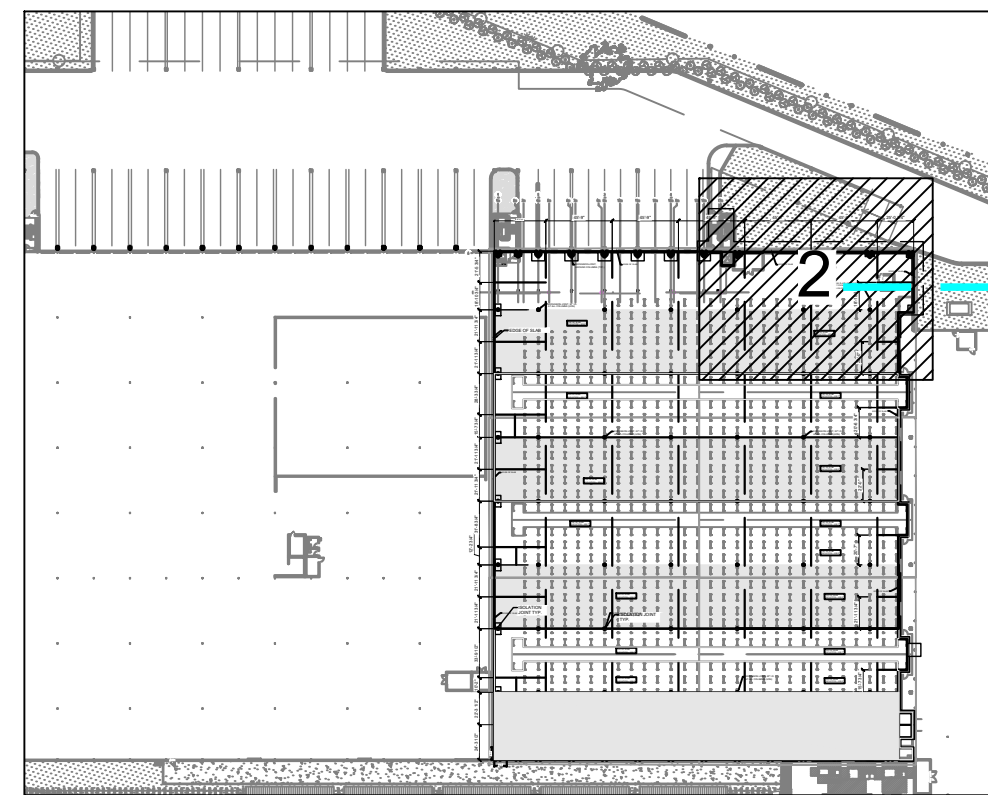
SLAB ON EDGE PART PLAN



SA-101



2 PART PLAN
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

LEGEND

- CONSTRUCTION OR CONTRACTION JOINT
- CONTRACTION JOINT
- EXPANSION JOINT AROUND COLUMN

NOTES:

- FOR SLAB REINFORCING, SEE DRAWING SA-200
- NO TOP REINFORCING SHALL BE PLACED WITHIN 4 INCHES OF THE ANCHOR BOLTS FOR THE AS/RS BASE PLATES
- REFER TO ARCH DRAWINGS FOR SLAB FINISH
- ASRS POST LOADING (ASD)
DL= 3 KIPS
LL=56 KIPS
EQ= 7 KIPS

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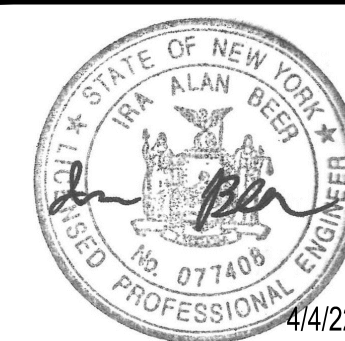
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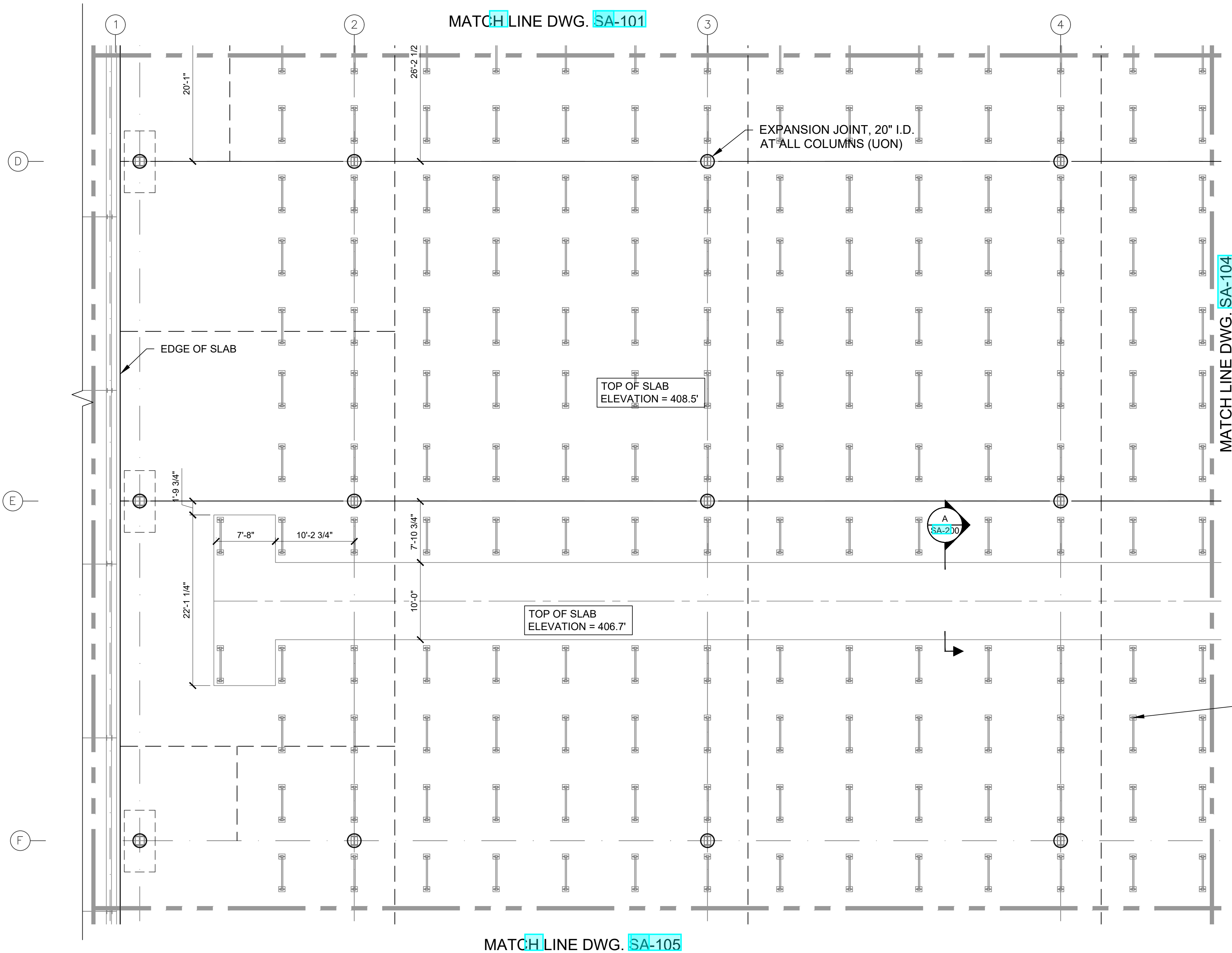
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SLAB ON EDGE PART PLAN

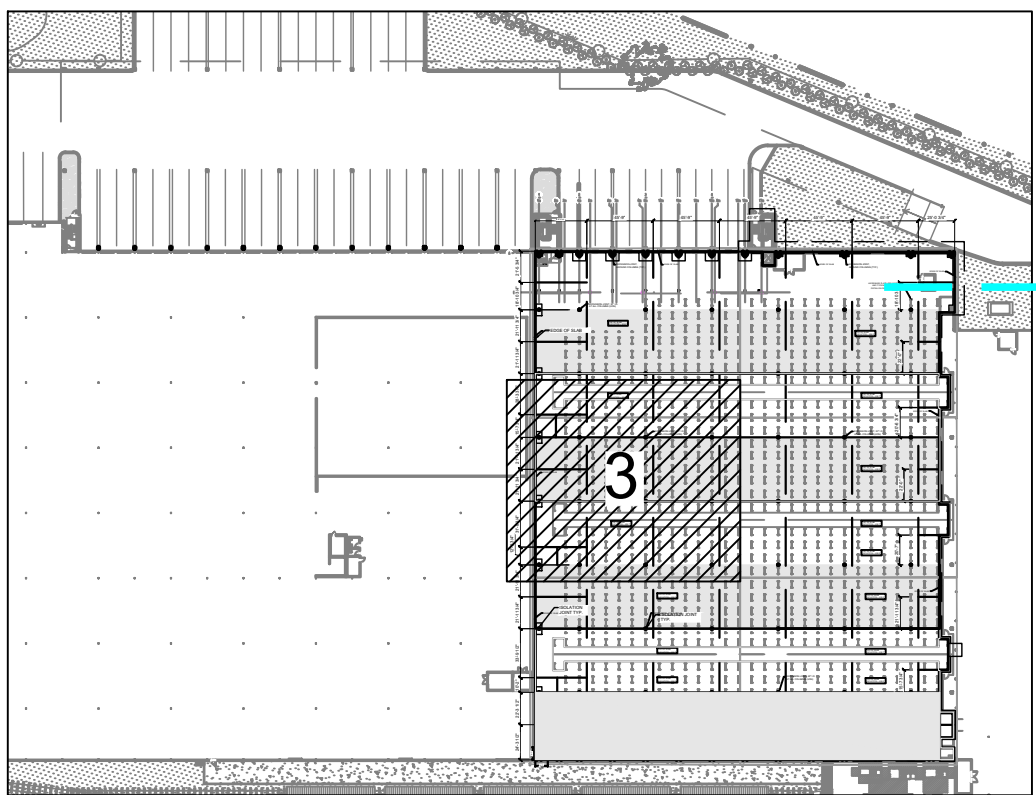


SA-102

20 DUNNIGAN DRIVE



3 PART PLAN
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

LEGEND

- CONSTRUCTION OR CONTRACTION JOINT
- CONTRACTION JOINT
- EXPANSION JOINT AROUND COLUMN

NOTES:

- FOR SLAB REINFORCING, SEE DRAWING SA-200
- NO TOP REINFORCING SHALL BE PLACED WITHIN 4 INCHES OF THE ANCHOR BOLTS FOR THE AS/RS BASE PLATES
- REFER TO ARCH DRAWINGS FOR SLAB FINISH
- ASRS POST LOADING (ASD)
DL= 3 KIPS
LL=56 KIPS
EQ= 7 KIPS

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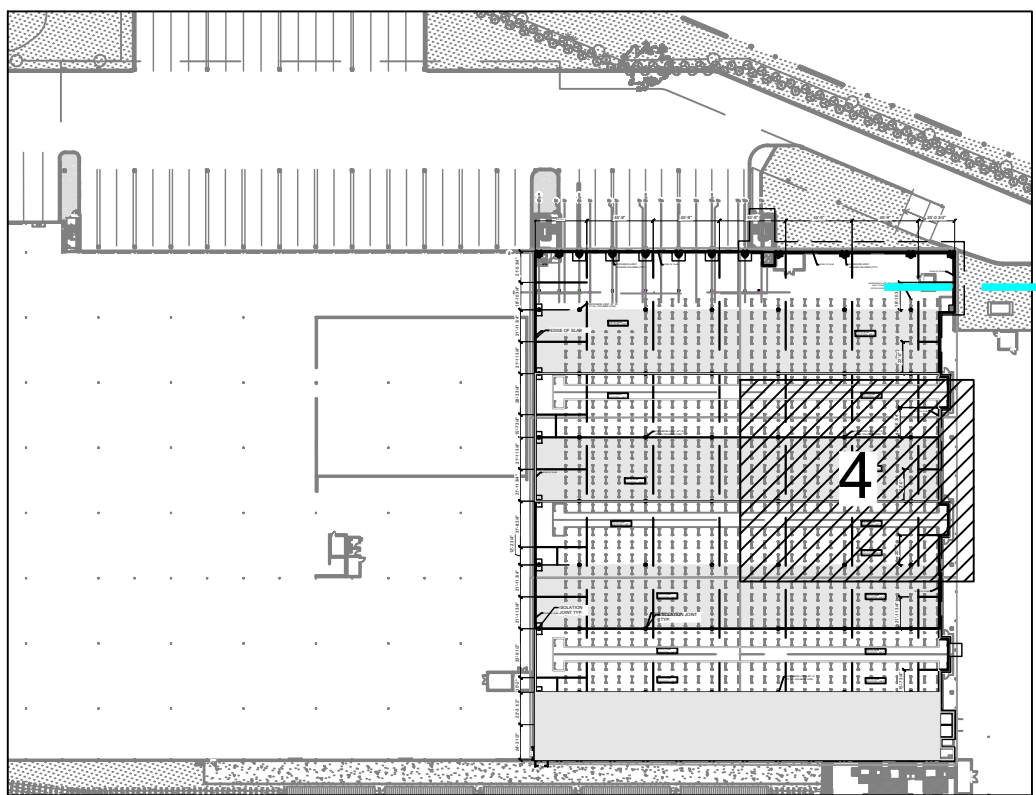
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DRAWING TITLE :

SLAB ON EDGE PART PLAN



SA-103



KEY PLAN
SCALE: N.T.S.

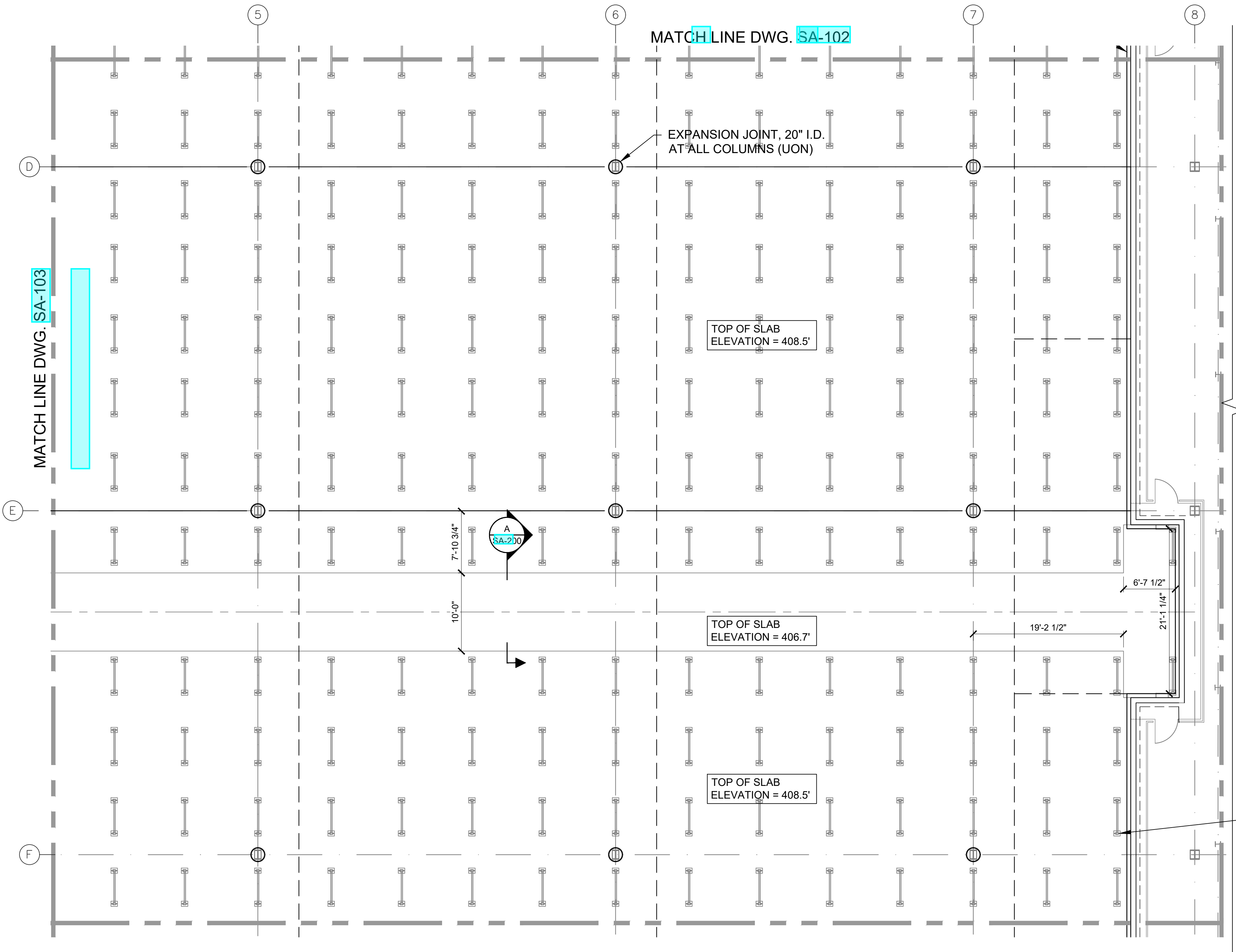
LEGEND

- CONSTRUCTION OR CONTRACTION JOINT
- CONTRACTION JOINT
- EXPANSION JOINT AROUND COLUMN

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- REFER TO ARCH DRAWINGS FOR SLAB FINISH
- ASRS POST LOADING (ASD)
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EQ= 7 KIPS

AS/RS BASE PLATE (SEE NOTE 2 AND TYPICAL REINFORCING DETAIL ON DWG SA-200 (TYP))



MATCH LINE DWG. SA-106

4 PART PLAN
SCALE: 1/8" = 1'-0"

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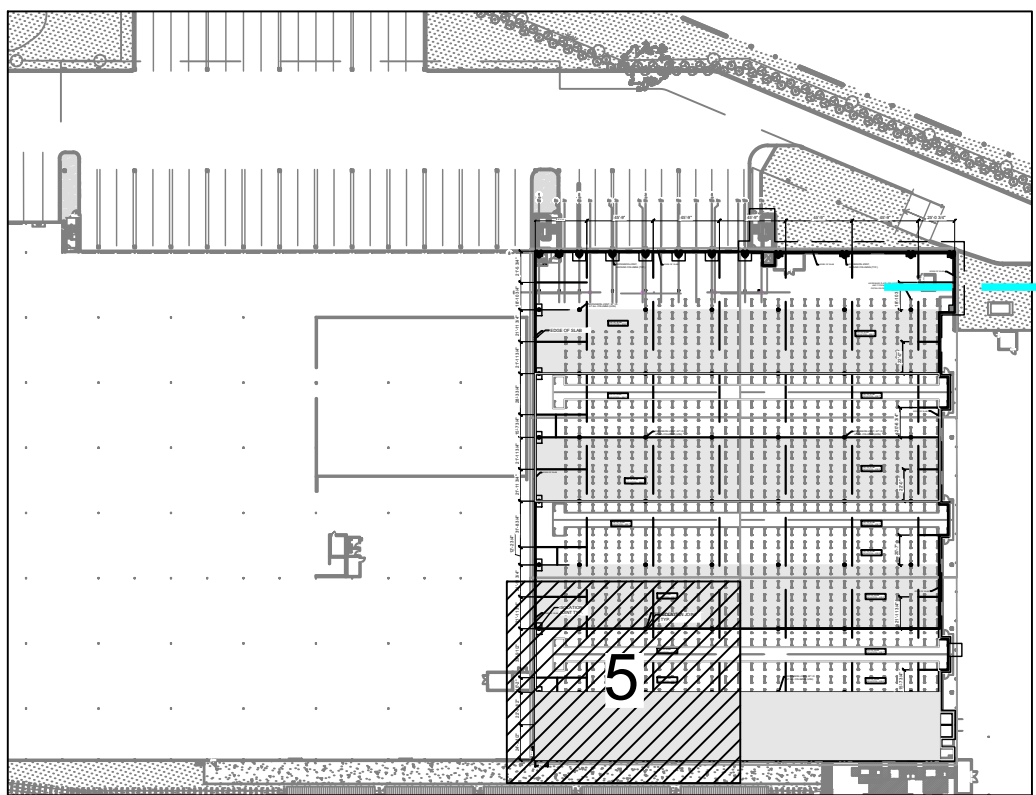
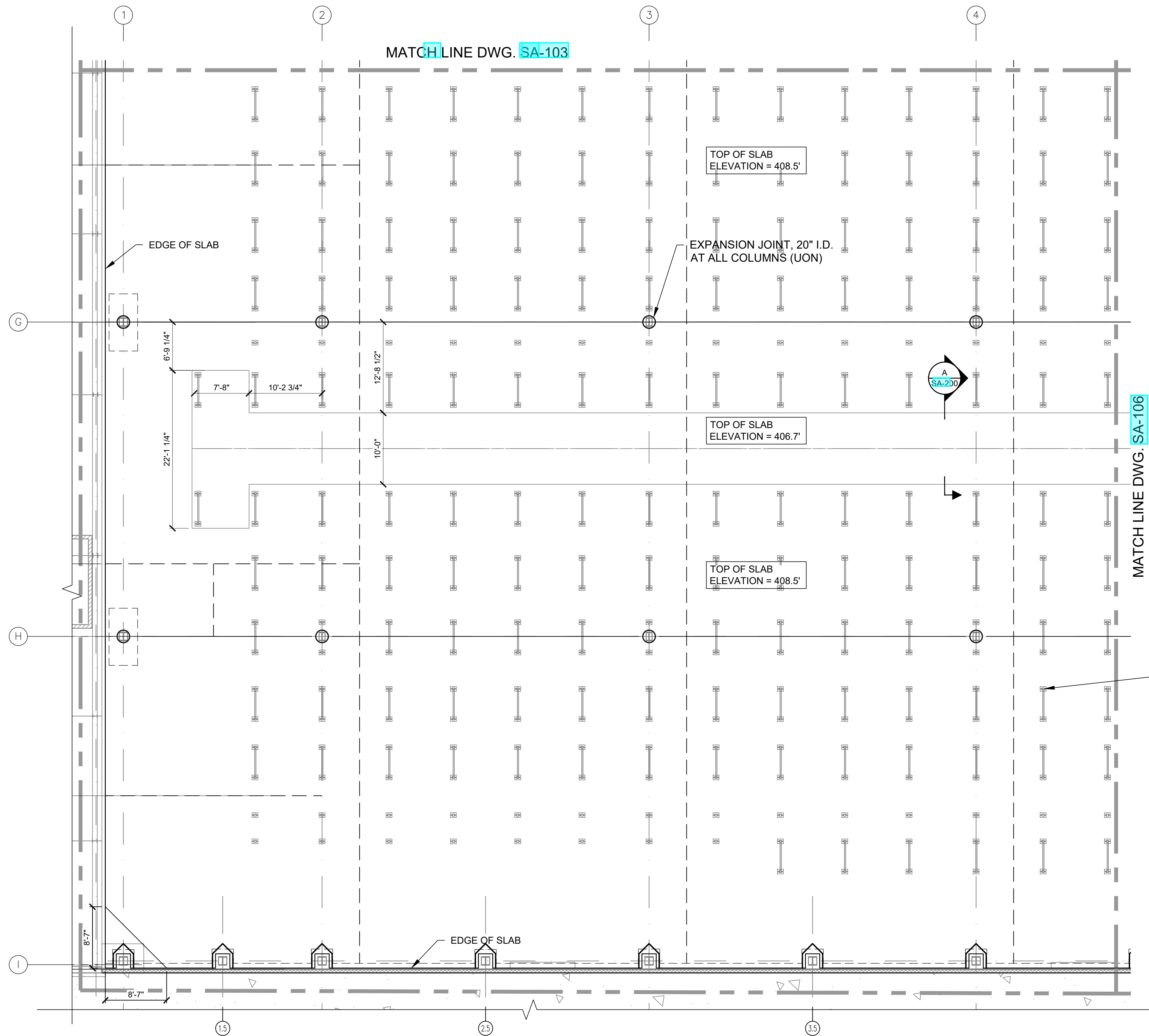
SCALE : AS SHOWN

DRAWING TITLE :

SLAB ON EDGE PART PLAN



SA-104



LEGEND

- CONSTRUCTION OR CONTRACTION JOINT
- CONTRACTION JOINT
- EXPANSION JOINT AROUND COLUMN

NOTES:

- FOR SLAB REINFORCING, SEE DRAWING SA-200
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- ASRS POST LOADING (ASD)
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AS/RS BASE PLATE (SEE NOTE 2 AND TYPICAL REINFORCING DETAIL ON DWG SA-200 (TYP))

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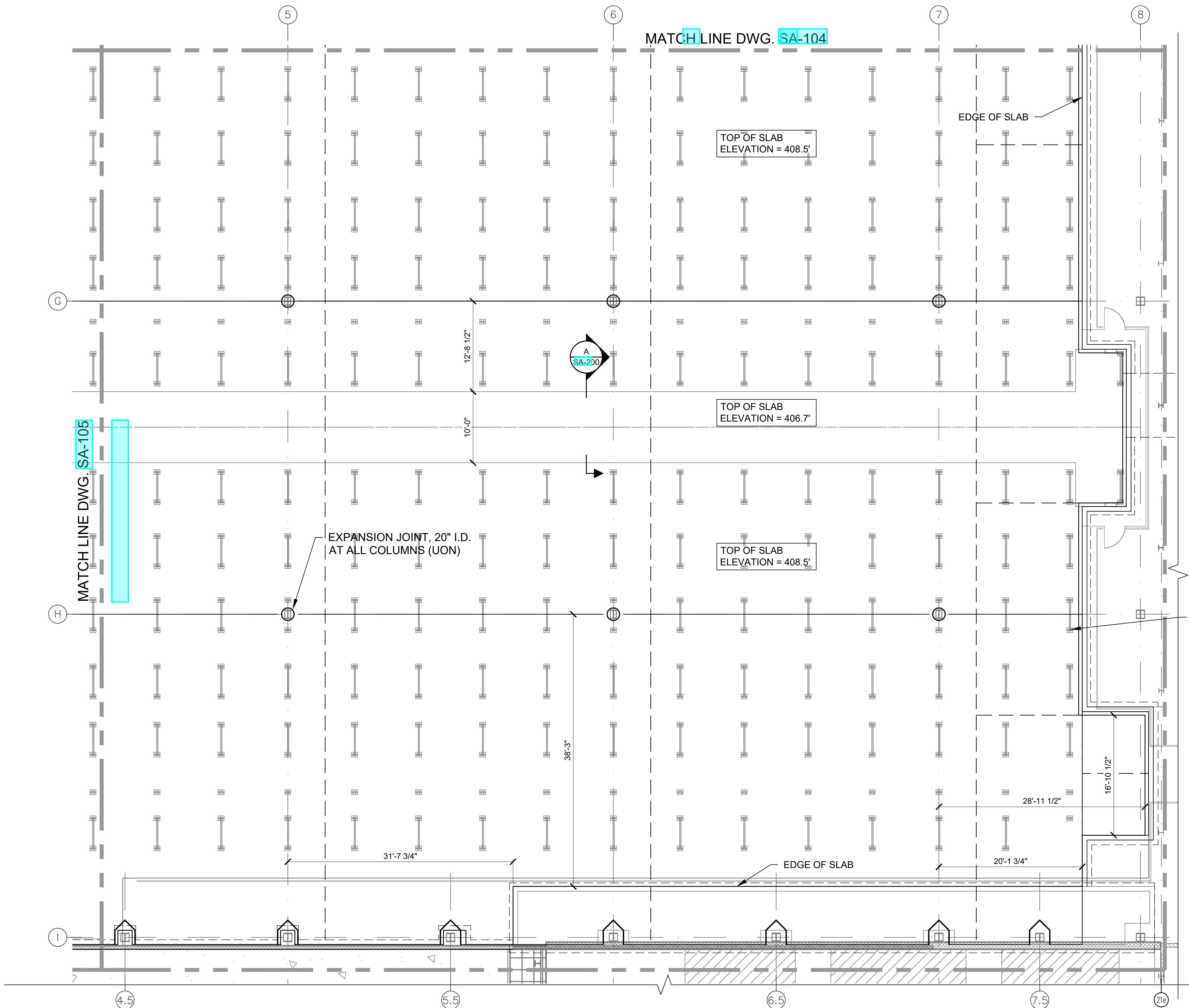
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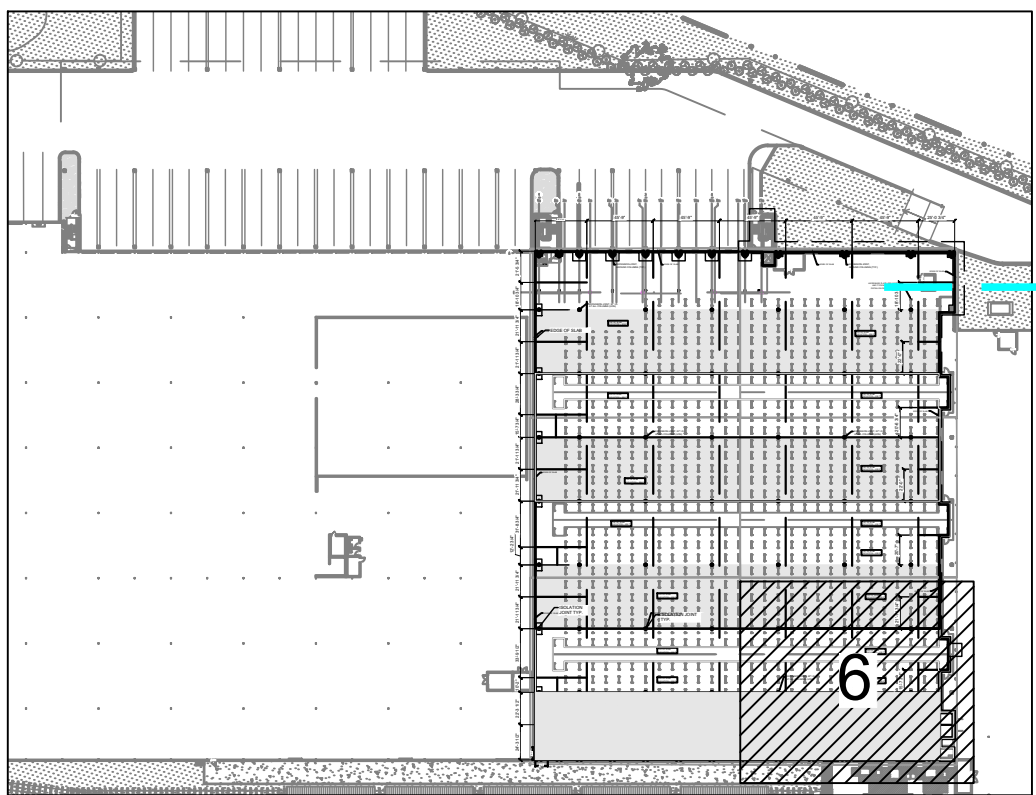


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6 PART PLAN
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

LEGEND

- CONSTRUCTION OR CONTRACTION JOINT
- CONTRACTION JOINT
- EXPANSION JOINT AROUND COLUMN

NOTES:

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AS/RS BASE PLATE (SEE NOTE 2 AND TYPICAL REINFORCING DETAIL ON DWG SA-200 (TYP))

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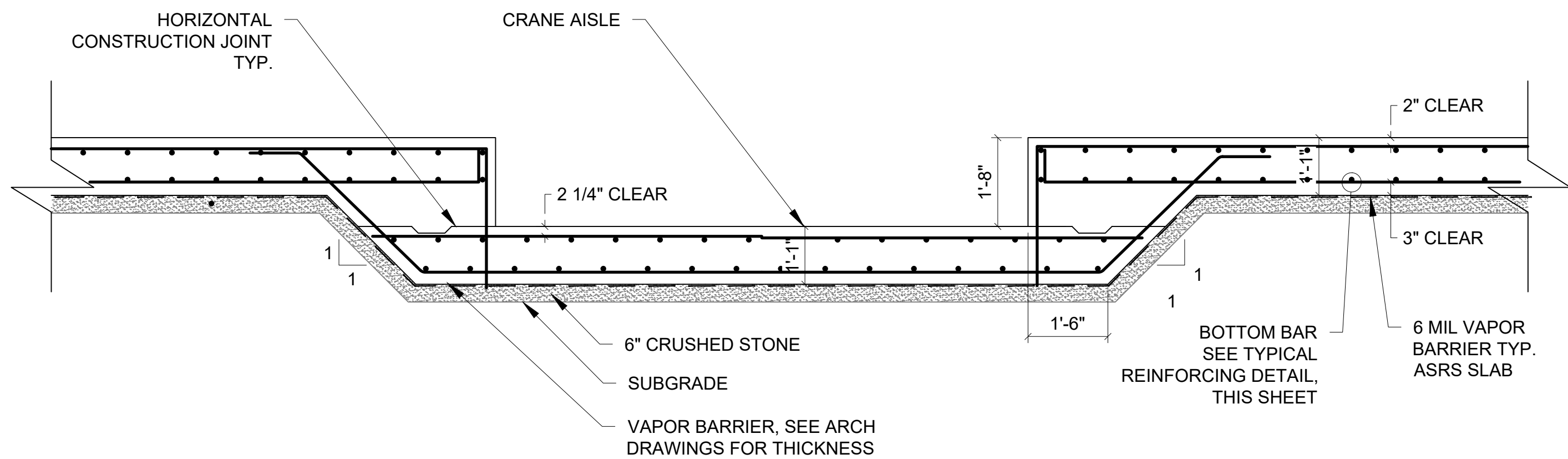
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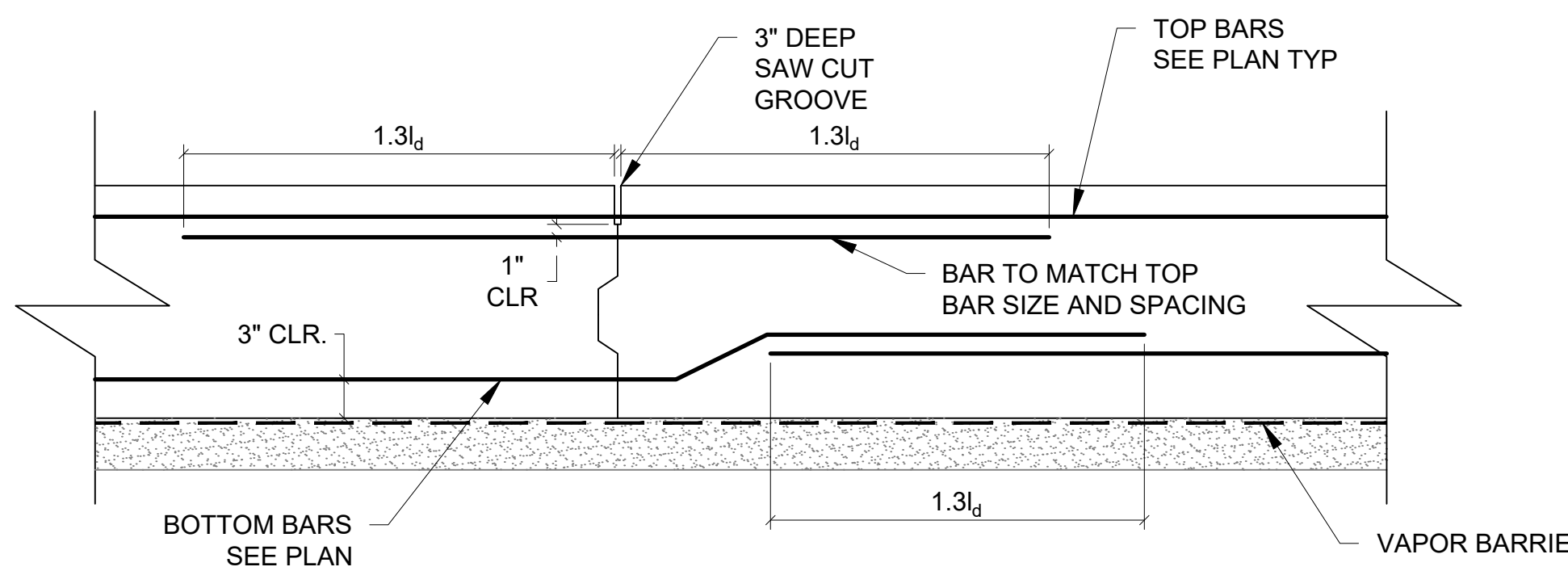
SLAB ON EDGE PART PLAN



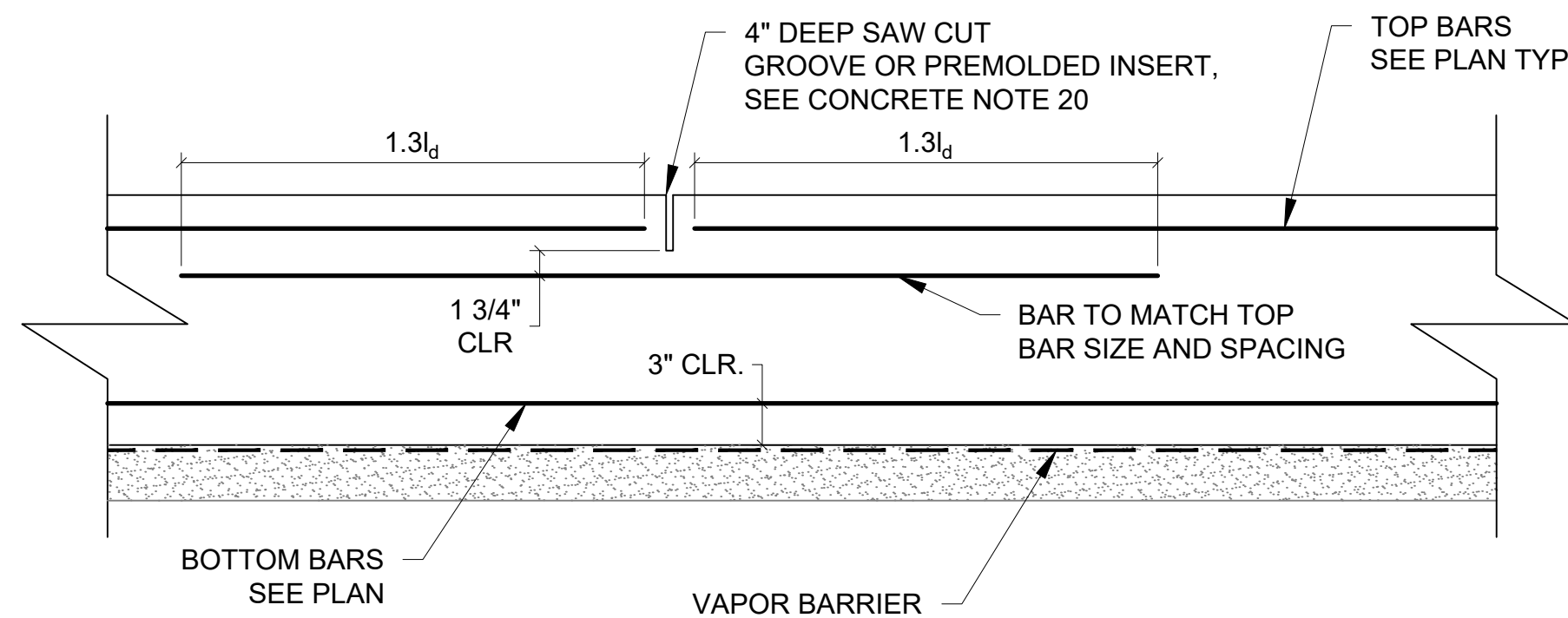
SA-106



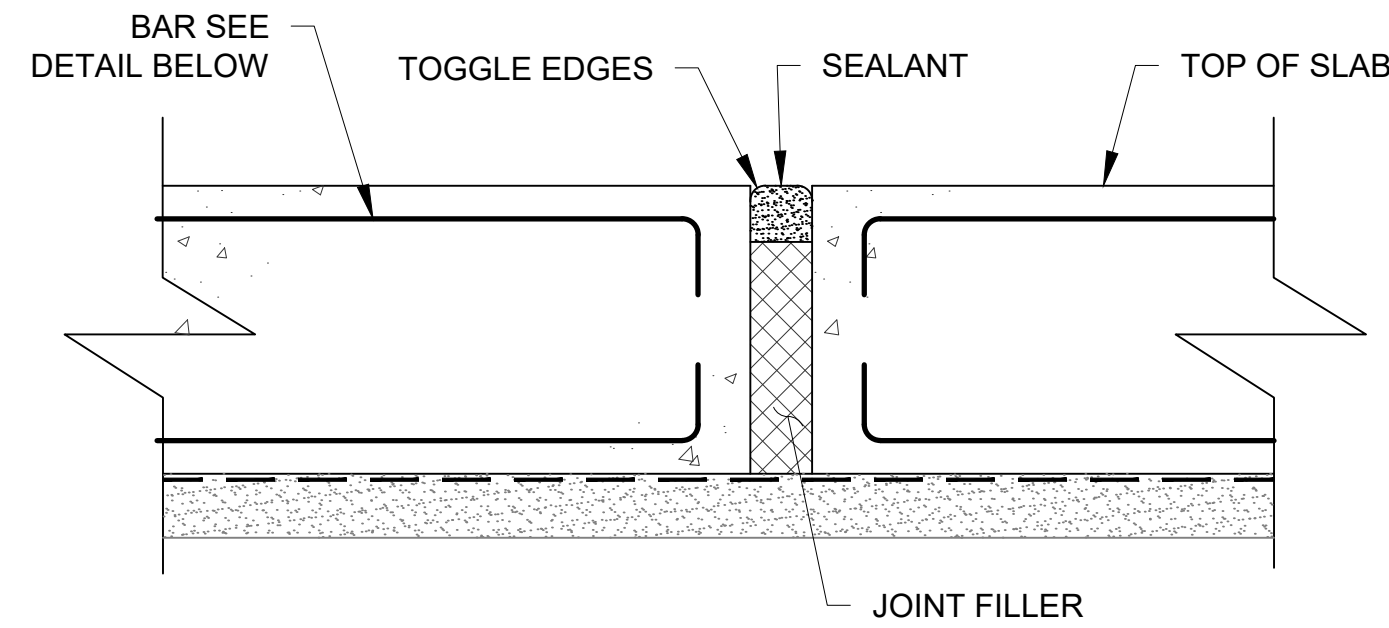
A SECTION AT FOUNDATION SLAB
SCALE: 1/2" = 1'-0"



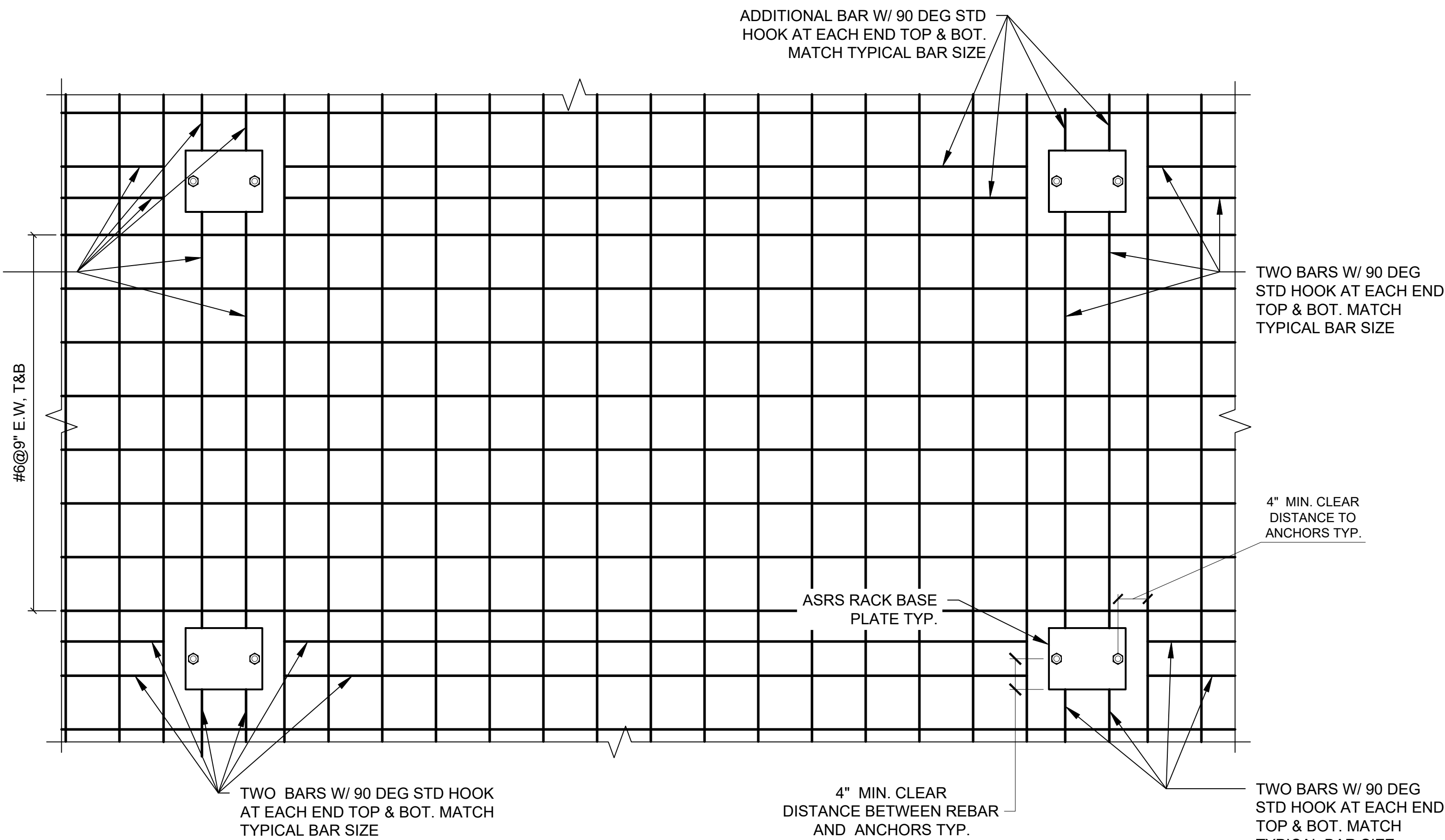
TYPICAL CONSTRUCTION JOINT DETAIL
NOT TO SCALE



TYPICAL CONTRACTION JOINT DETAIL
NOT TO SCALE



TYPICAL COLUMN ISOLATION JOINT DETAIL
(AROUND COLUMNS AND SLAB EDGES ONLY)
NOT TO SCALE



TYPICAL REINFORCEMENT DETAIL
SCALE: 1" = 1'-0"

NOTES:
1. PROVIDE 90 DEG HOOK AT TERMINATIONS
OF ALL BARS AT THE EDGES OF THE SLAB

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MANHATTAN BEER DISTRIBUTORS
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SUFFERN, NEW YORK

REV	DESCRIPTION	DATE
1	ISSUED FOR BID	10.15.21
2	CD SET	11.30.21
3	REVISED BID SET	01.20.22
4	ISSUED FOR CONSTRUCTION	04.04.22

DRAWN BY : D. CHECHURIN
CHECKED BY : P. BRUNO
APPROVED BY : P. BRUNO
DATE : 9/10/2021
SCALE : AS SHOWN

DRAWING TITLE :

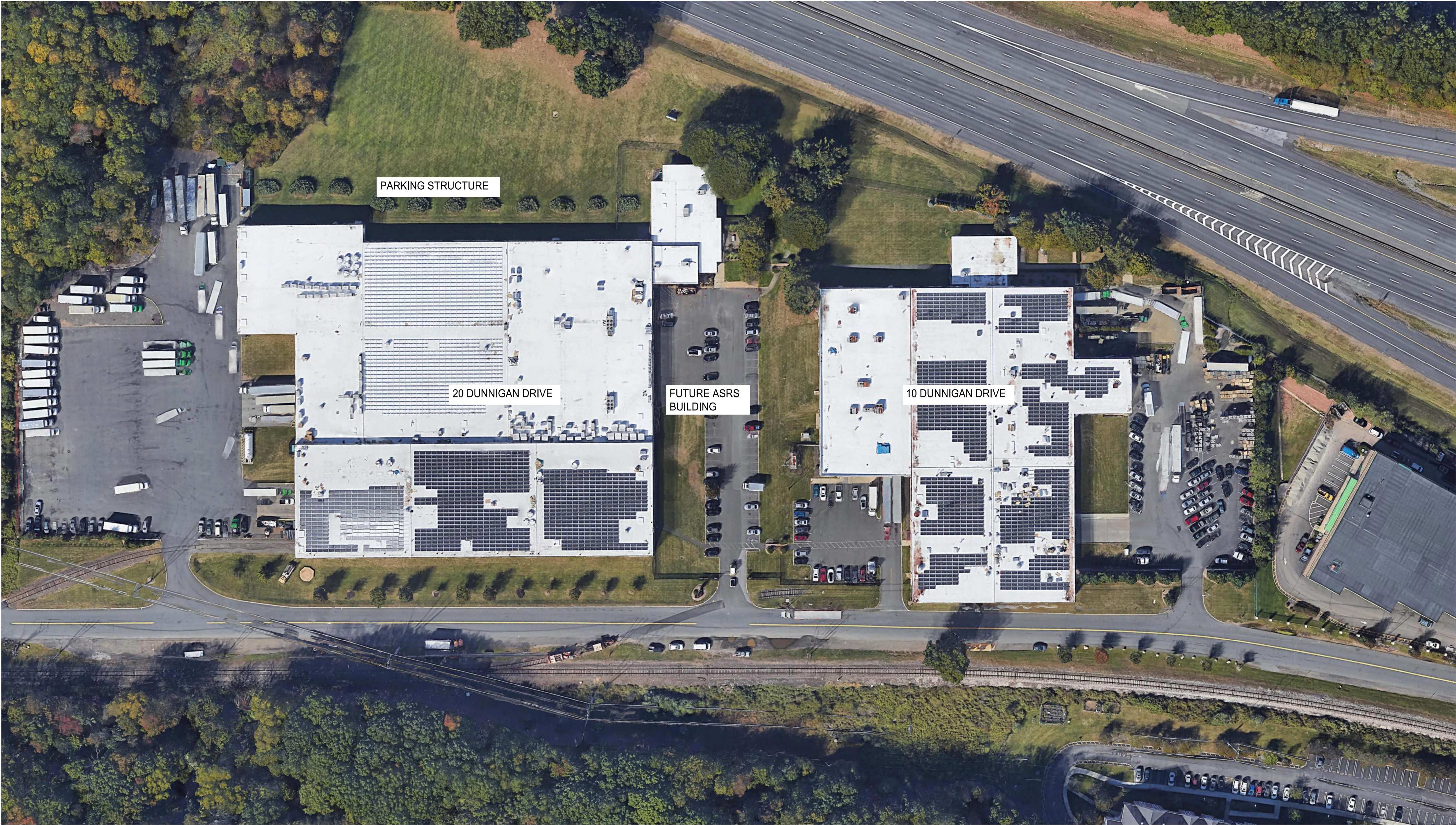
SECTIONS AND DETAILS



SA-200

MANHATTAN BEER DISTRIBUTORS

20 DUNNIGAN DRIVE, SUFFERN, NY
STRUCTURAL REPAIRS



SITE LOCATION MAP - OVERALL LAYOUT
(NOT TO SCALE)



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DRAWN BY : D. CHECHURIN
CHECKED BY : I. BEER
APPROVED BY : I. BEER
DATE : 7/26/2021
SCALE : AS SHOWN

DRAWING TITLE : -

FOUNDATION - KEY PLAN



DWG NUMBER :
SF-000

1 OF 31

GENERAL NOTES

1. DESIGN AND CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE FOLLOWING CODES:
- A. NEW YORK STATE BUILDING CODE.
 - B. AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318).
 - C. AMERICAN CONCRETE INSTITUTE SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI-301)
 - D. AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL AND CODE.
 - E. OCCUPATIONAL HEALTH SAFETY ADMINISTRATION (OSHA) CODE.
 - F. AMERICAN NATIONAL STANDARDS FOR CONSTRUCTION (ANSI).
 - G. USE LATEST EDITIONS INCLUDING ADDENDA, SUPPLEMENTS, AMENDMENTS, REVISIONS AND ERRATA U.N.O., EXCEPT AS MODIFIED BY THE CONTRACT DOCUMENTS.
2. PLANS AND APPLICATION FORMS MUST BE FILED AND APPROVED BY THE LOCAL BUILDING DEPARTMENT BEFORE STARTING ANY WORK OR ORDERING ANY MATERIALS.
3. CONTRACTOR TO PROPERLY NOTIFY BUILDING DEPARTMENT PRIOR TO ANY WORK REQUIRING INSPECTION BY THE BUILDING DEPARTMENT AS CALLED FOR IN THE NEW YORK STATE BUILDING CODE.
4. A TESTING AND INSPECTION AGENCY ACCEPTABLE TO THE STRUCTURAL ENGINEER AND RETAINED BY THE OWNER SHALL PERFORM ALL REQUIRED TESTING AND INSPECTIONS IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE REGULATIONS (SEE SPECIAL INSPECTION NOTES).
5. THE CONTRACTOR IS RESPONSIBLE FOR ARRANGING WITH TESTING AGENCY WHEN INSPECTION SHALL BE PERFORMED. THE CONTRACTOR SHALL PROVIDE AMPLE TIME FOR THE TESTING AGENCY TO COMPLETE THEIR WORK.
6. TESTING AND INSPECTION OF SOILS, CONCRETE, REBARS, STEEL, WELDING AND H.S. BOLTING, ETC., SHALL BE IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE. (SEE SPECIAL INSPECTION NOTES)
7. CONTRACTOR SHALL PROVIDE, INSTALL, AND MAINTAIN ALL NECESSARY FALSEWORK, SHORING, BRACING AND ALL OTHER TEMPORARY CONSTRUCTION AND EQUIPMENT REQUIRED AND SHALL BE RESPONSIBLE FOR SITE SAFETY, STRUCTURAL STABILITY AND PROTECTION OF THE PUBLIC, STRUCTURES, AND PROPERTY INCLUDING PROTECTION FROM THE ELEMENTS DURING ANY PHASE OF THE WORK. SHORING DESIGN SHALL BE PERFORMED BY A NEW YORK STATE LICENSED P.E.
8. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELEVATOR DRAWINGS, AND SPECIFICATIONS. IN ADDITION, REFER TO THESE DRAWINGS FOR ADDITIONAL STRUCTURAL WORK NOT SHOWN ON STRUCTURAL DRAWINGS SUCH AS OPENINGS, INSERTS, SLEEVES, CURBS, ETC.
9. AS PART OF THE CONTRACTOR'S SCOPE OF WORK, THE CONTRACTOR SHALL REPLACE, REPAIR AND PATCH TO ITS ORIGINAL CONDITION ANY AREAS ALTERED OR DAMAGED DURING PROCESS OF WORK AT NO ADDITIONAL COST TO THE OWNER AND WITH ALL NECESSARY GUARANTEES AND WARRANTIES.
10. THE CONTRACTOR SHALL TAKE FIELD MEASUREMENTS, ELEVATIONS, OPENINGS AND VERIFY FIELD CONDITIONS AND SHALL CAREFULLY COMPARE SUCH FIELD MEASUREMENTS AND CONDITIONS AND OTHER INFORMATION KNOWN TO THE CONTRACTOR, WITH THE CONTRACT DOCUMENTS, BEFORE COMMENCING WORK AND PRIOR TO PREPARING SHOP DRAWINGS AND ORDERING MATERIALS AND DETAILS. ERRORS, DISCREPANCIES, INCONSISTENCIES, OR OMISSIONS DISCOVERED WHICH MAY INTERFERE WITH THE PROPER EXECUTION OF THE WORK, SHALL BE REPORTED TO THE ENGINEER IMMEDIATELY. DEVIATIONS FROM THESE PLANS MAY OCCUR DUE TO FIELD CONDITIONS.
11. THE CONTRACTOR SHALL FILE AND OBTAIN AND PAY FOR ALL REQUIRED PERMITS, FEES AND LICENSES BEFORE STARTING ANY WORK OR ORDERING ANY MATERIAL. DO NOT BEGIN CONSTRUCTION UNLESS AND UNTIL A WRITTEN PERMIT HAS BEEN ISSUED IN ACCORDANCE WITH THE REQUIREMENTS OF THE BUILDING CODE. SUBMIT ORIGINAL COPIES TO OWNER BEFORE PROCEEDING.
12. THESE DRAWINGS REPRESENT STRUCTURAL COMPONENTS IN THEIR FINAL AND FINISHED STATE. CONSTRUCTION PROCEDURE, METHODS, SAFETY PRECAUTIONS OR MECHANICAL REQUIREMENTS USED TO ERRECT THEM ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR OR SUB-CONTRACTOR DOING THE WORK, AND SHOULD FOLLOW CHAPTER 33 OF THE NEW YORK STATE BUILDING CODE.
13. THE CONTRACTOR IS CAUTIONED TO MAKE CONTINUOUS OBSERVATIONS AND SAFEGUARDS OF THE EXISTING STRUCTURES DURING THE PERFORMANCE OF HIS WORK. SHOULD HE BECOME AWARE OF ANY EXISTING UNFORESEEN STRUCTURAL CONDITIONS OR SITUATIONS THAT REQUIRE FURTHER INVESTIGATION OR STUDY, (SUCH AS EXCESSIVE DETERIORATION, CRACKS IN MASONRY OR SLABS, MOVEMENT, ADDITIONAL DEFLECTION, ETC.), HE SHALL IMMEDIATELY NOTIFY THE ENGINEER/ARCHITECT OF RECORD.
14. SHOP DRAWINGS AND SAMPLE PRODUCTS ARE REQUIRED FOR ALL WORK AND SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY ARE STAMPED 'APPROVED' AND INITIALED BY THE CONTRACTOR ATTESTING THAT IT CONFORMS TO THE CONTRACT DOCUMENTS. SHOP DRAWINGS SHALL BE PREPARED, SIGNED AND SEALED BY A NEW YORK STATE LICENSED P.E. RETAINED BY THE CONTRACTOR.
15. THE CONTRACTOR SHALL RETAIN HIS OWN CONSULTANTS, INCLUDING STRUCTURAL CONSULTANTS, REGARDING CONTRACTORS DEMOLITION AND CONSTRUCTION PROCEDURES, SHORING AND BRACING PROTECTION, STRUCTURAL STABILITY REVIEWS DURING CONSTRUCTION AGAINST DAMAGE COLLAPSE, DISTORTION AND OFF-ALIGNMENT, ETC., AND ON SITE SAFETY RESPONSIBILITIES AND BUILDING DEPARTMENT REPORTING AND FILING REQUIREMENTS.
16. THE CONTRACTOR SHALL SAFEGUARD ALL ELECTRICAL AND MECHANICAL CONDUITS AND UTILITIES AND COORDINATE ALL EXISTING LINES, PIPES, DUCTS AND OTHER UTILITIES WHICH ARE TO BE RELOCATED AND REINSTALLED AS A RESULT OF THE NEW WORK.
19. EXISTING FOUNDATIONS, WALLS, COLUMNS, SLABS, BEAMS, FLOORS, (CONCRETE, STEEL, TIMBER, ETC.), ARE ASSUMED TO BE IN GOOD CONDITION. THIS MUST BE VERIFIED IN THE FIELD BY THE CONTRACTOR. CONTRACTOR SHALL NOTIFY ENGINEER-OF-RECORD OF ANY CONDITIONS REQUIRING REPAIRS PRIOR TO CONSTRUCTION.
20. THE ENGINEER OF RECORD SHALL NOT HAVE CONTROL OR CHARGE OF AND SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURE, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK FOR THE ACTS OR OMISSIONS OF THE CONTRACTORS, SUBCONTRACTOR, OR ANY OTHER PERSON PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

EXCAVATION NOTES

1. BUILDING DEPARTMENT TO BE NOTIFIED AT LEAST 24 HOURS IN ADVANCE, BUT NOT MORE THAN 48 HOURS PRIOR TO COMMENCEMENT OF EARTHWORK OPERATIONS.
2. CONTRACTOR SHALL LOCATE EXISTING SANITARY SEWERS AND OTHER UNDERGROUND PIPING. BEFORE EXCAVATING THE CONTRACTOR SHALL VERIFY ACTUAL FIELD CONDITIONS, EXPOSED OR CONCEALED, ALL ELECTRICAL, MECHANICAL CONDUITS AND UTILITIES. THE CONTRACTOR SHALL CALL "DIG-SAFELY-NEW YORK" FOR A COPY OF CODE 53 FOR LOCATION OF UTILITIES (NYS INDUSTRIAL CODE RULE 53-12). THE CONTRACTOR SHALL ARRANGE AND PAY FOR THE UTILITY COMPANIES SHUT-OFF OF UTILITIES SERVING THE STRUCTURE.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE EXCAVATION AND CONSTRUCTION DURING THE INSTALLATION OF GRADE BEAMS, FOOTINGS AND FOUNDATION WALLS. THE ADJACENT STREETS, SIDEWALKS, AND PROPERTIES SHALL BE PROTECTED, SHORED, BRACED, ETC. AS REQUIRED.

STRUCTURAL CONCRETE NOTES

1. MATERIALS SHALL CONFORM WITH THE FOLLOWING STANDARDS:
- A. PORTLAND CEMENT AS PER ASTM C-150, TYPE I / II.
 - B. CONCRETE AGGREGATES AS PER ASTM C-33 AND C-330.
 - C. WATER SHALL BE CLEAN AND FREE FROM INJURIOUS OILS, ACIDS, ALKALIS, SALT, ORGANIC MATERIALS AND DELETERIOUS MATERIALS.
 - D. REBARS SHALL BE DEFORMED BARS CONFORMING TO ASTM A-615, GRADE 60. TIES MAY BE A-615, GRADE 40. FOR BARS REQUIRING WELDING CONFORM TO ASTM A-706.
 - E. WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM 185 WITH ULTIMATE TENSILE STRENGTH OF 70 K.S.I. WIRE FABRIC SHALL CONFORM TO ASTM 475.

2. CONCRETE DESIGN STRENGTH: CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES FOR THE VARIOUS COMPONENTS, UNLESS NOTED OTHERWISE:
- | | 28-DAY COMPRESSIVE STRENGTH
(F'C) (PSI) | ENTRAINED
AIR CONTENT | DENSITY
(PCF) | MAX.
W/C RATIO |
|-----------------------------|--|--------------------------|------------------|-------------------|
| GRADE BEAMS, | | | | |
| FOUNDATION FOOTINGS & WALLS | 5,000 | 6% | 145 | 0.45 |
| ASRS SLAB | 5,000 | 0% | 145 | 0.45 |
- THE REQUIRED AVERAGE CONCRETE MIX DESIGN (F'CR) USED AS A BASIS OF SELECTION OF CONCRETE PROPORTIONS SHALL BE DETERMINED FROM A CURVE OF CONCRETE STRENGTH VERSUS WATER-CEMENT RATIO WHERE THE AVERAGE CONCRETE MIX DESIGN SHALL COMPLY WITH ACI 318.
3. CONCRETE SHALL BE PRODUCED FROM APPROVED BATCH PLANTS, BASED ON PRELIMINARY TEST DESIGN AND RESULTING INTO MIX STRENGTH SPECIFIED. PRODUCER SHALL CERTIFY CONFORMANCE OF QUALITY AND CONDITION OF MATERIALS TO ACI-318 AND THAT INGREDIENTS ARE THE SAME OR EQUAL TO THOSE USED FOR THE PRELIMINARY TESTS. ATTESTATION OF QUALITY INSPECTION AT THIS BATCH PLANT SHALL APPEAR ON THE TICKET ACCOMPANYING EACH LOAD OF CONCRETE.
4. CONTRACTOR SHALL SUBMIT PRELIMINARY TESTS AND DESIGN MIX REPORTS IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE FOR REVIEW BY THE STRUCTURAL ENGINEER BEFORE COMMENCING WORK. NO CONCRETE SHALL BE PLACED UNTIL THE DESIGN MIXES WITH ACCOMPANYING COMPRESSIVE TEST RESULTS HAVE BEEN APPROVED BY THE ENGINEER OF RECORD.
5. COMPRESSION TEST SAMPLES SHALL BE TAKEN FROM THE MIXER IN ACCORDANCE WITH ASTM C172, CURED IN ACCORDANCE WITH ASTM C-31, AND TESTED AT 28-DAYS IN ACCORDANCE WITH ASTM C-39.
6. THE APPROVED INSPECTION AGENCY WILL CONDUCT ALL NECESSARY TESTS AT THE LABORATORY, AND WILL HAVE ONE QUALIFIED CONCRETE TECHNICIAN TO BE STATIONED AT THE MIXING PLANT AND A MINIMUM OF ONE QUALIFIED CONCRETE TECHNICIAN AT THE JOB SITE. THE TECHNICIAN AT THE JOB SITE WILL PERFORM THE TESTS AND PREPARE THE SPECIMENS AS REQUIRED UNDER THE CODE.
7. THE TESTING AGENCY SHALL PROVIDE ACI CERTIFIED TECHNICIANS.
8. ALL CONCRETE USED IN THE STRUCTURE SHALL CONFORM IN ALL RESPECTS TO THE MATERIAL AND PROPORTIONS OF THESE MATERIALS IDENTIFIED IN THE APPROVED DESIGN MIX. THE USE OF ANY ADDITIVES NOT PRESENT IN THE APPROVED DESIGN MIX IS PROHIBITED.
9. WHEN CHANGES TO BRAND, TYPE, SIZE OR SOURCE OF CEMENTITIOUS MATERIALS, AGGREGATES, WATER, ICE OR ADMIXTURES ARE PROPOSED, SUBMIT NEW FIELD DATA, DATE FROM NEW TRIAL MIXTURES, OR OTHER EVIDENCE THAT THE CHANGE WILL NOT ADVERSELY AFFECT THE RELEVANT PROPERTIES OF THE CONCRETE. DATA SHALL BE SUBMITTED FOR ACCEPTANCE BEFORE CHANGES ARE MADE.
10. CONCRETE TEST CYLINDERS AND SLUMP. INSPECTION OF REINFORCING BARS AND PLACING OF CONCRETE TO BE TAKEN AND TESTED IN ACCORDANCE WITH ACI-318 AND AS REQUIRED BY THE NEW YORK STATE BUILDING CODE. (SEE SPECIAL INSPECTION NOTE)
11. CONTRACTOR TO PROVIDE A LOCATION WHICH IS LEVEL, SAFE AND FOR THE STORING OF CURING BOXES IN ORDER FOR THE TESTING LABORATORY TO PREPARE TEST CYLINDERS.
12. THE TEST CYLINDERS IMMEDIATELY AFTER MOLDING AND FINISHING, SHALL BE STORED IN CURING BOXES (SUPPLIED BY THE TESTING AGENCY) ON-SITE FROM 24-HOURS TO 48-HOURS IN A TEMPERATURE RANGE FROM 60F TO 80F IN AN ENVIRONMENT PREVENTING MOISTURE LOSS FROM THE SPECIMENS BEFORE BEING TRANSPORTED TO THE TESTING LABORATORY.
13. NO CHLORIDES SHALL BE INTRODUCED INTO THE CONCRETE OR GROUT, ETC., UNDER ANY CIRCUMSTANCES.
14. ALL CONTINUOUS REINFORCING BARS SHALL BE EMBEDDED AT ENDS OF MEMBERS AND LAPPED AT SPLICES AND CORNERS AS PER THE TO LATEST EDITION OF ACI 318. SPLICES FOR BOTTOM STEEL TO BE LOCATED AT SUPPORTS, SPLICES FOR TOP STEEL TO BE LOCATED AT MID-SPAN. NO SPLICES ARE PERMITTED IN TENSION ZONE. CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT. ALL DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS SHALL COMPLY WITH THE REQUIREMENTS OF ACI 315 AND ACI 318-02, CHAPTERS 7 AND 12, AND THE NEW YORK STATE BUILDING CODE.
- A. ALL SPLICES FOR SLABS AND BEAMS SHALL BE TENSION SPLICES, U.O.N.
 - B. COMPRESSION SPLICES SHALL BE USED IN PIERS AND COLUMNS, U.O.N.
 - C. EMBEDMENT FOR DOWELS, ETC. SHALL BE TENSION DEVELOPED, U.O.N.
14. MECHANICAL SPLICES MAY BE USED. WHERE THESE SPLICES ARE USED, SPLICES SHOULD BE STAGGERED AND SHALL DEVELOP 133 PERCENT OF THE SPECIFIED YIELD STRENGTH IN TENSION OR COMPRESSION.
15. MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE:
- | | |
|---|-----------|
| UNFORMED SURFACE IN CONTACT WITH THE GROUND | 3 IN. |
| FORMED SURFACES EXPOSED TO EARTH | |
| #6 BARS AND LARGER | 2 IN. |
| #5 BARS AND SMALLER | 1-1/2 IN. |
| FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER | |
| BEAMS, GIRDERS, AND COLUMNS | 1-1/2 IN. |
| SLABS, WALLS, AND JOISTS: | |
| #11 BARS AND SMALLER | 3/4 IN. |
| #14 BARS #18 BARS | 1-1/2 IN. |
- SURVEY REBAR ELEVATIONS BEFORE PLACING CONCRETE.
16. ALL BAR SIZES DESIGNATED ARE BASED ON NUMBER OF EIGHTHS OF AN INCH NOMINAL DIAMETER, NOT METRIC UNITS.
17. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL THE NECESSARY CHAIRS, REBARS, TIES, SPACERS, ETC., TO SECURE AND SUPPORT THE REINFORCING. PROPERLY INSTALLED SPACERS SHALL BE UTILIZED TO PROPERLY MAINTAIN APPROPRIATE CONCRETE COVER WHILE PLACING CONCRETE. LIFTING OF BARS OR MESH DURING PLACEMENT OF CONCRETE IS NOT PERMITTED.
- NOTE: PROVIDE PLASTIC TIPPED BOLSTERS AND CHAIRS AT THOSE LOCATIONS WHERE THE CONCRETE SURFACE IN CONTACT WITH THE BOLSTERS OR CHAIRS ARE EXPOSED.
18. THE CONTRACTOR SHALL SUBMIT CHECKED SHOP DRAWINGS CONSISTING OF COMPLETE PLANS AND DETAILS OF REINFORCEMENT, LOCATIONS OF POUR LINES, CONSTRUCTION JOINTS, ETC., FOR APPROVAL BEFORE PROCEEDING WITH THE WORK. SUCH DRAWINGS SHALL BE OF SUFFICIENT DETAIL TO PERMIT PLACING OF REINFORCEMENT WITHOUT REFERENCE TO THE STRUCTURAL DRAWINGS.
19. NO COLD WEATHER CONSTRUCTION OR HOT WEATHER CONSTRUCTION IS PERMITTED WITHOUT WRITTEN APPROVAL OF THE ARCHITECT/ENGINEER. SUBMIT PROCEDURES FOR REVIEW.
20. LOCATION OF ALL CONSTRUCTION AND CONTRACTION JOINTS, NOT SHOWN IN DRAWINGS, SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL PRIOR TO DETAILING OF REINFORCEMENT. ALL CONSTRUCTION JOINTS TO BE CLEARLY INDICATED ON REBAR DETAIL DRAWING. MAXIMUM CONTRACTION JOINT SPACING = 400" O.C.
21. ALL STRUCTURAL MEMBERS SHALL BE POURED FOR THEIR FULL DEPTHS IN ONE OPERATION. CONSTRUCTION JOINTS SUCH AS DAY'S END POUR SHALL BE LOCATED IN THE MIDDLE THIRD OF THE SPAN. MAIN REINFORCING TO RUN THROUGH THE JOINT. KEY AND ROUGHEN JOINTS TO EXPOSE AGGREGATE FOR CHEMICAL BOND. NO HORIZONTAL CONSTRUCTION JOINTS PERMITTED IN BEAMS OR WALLS WITHOUT THE EXPRESS CONSENT OF ENGINEER-OF-RECORD. NO CONSTRUCTION JOINTS ARE PERMITTED IN BEAMS CARRYING COLUMN LOADS OR OTHER CONCENTRATED LOADS. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS. ALL REINFORCING IS TO BE CONTINUOUS THROUGH JOINT.
- ROUGHENED SURFACE AT INTERFACE OF SEPARATE CONCRETE POURS (JOINTS) SHALL BE PREPARED AS FOLLOWS:
- A)ROUGHEN SURFACE TO A FULL AMPLITUDE OF APPROXIMATELY 1/4" WITH STIFF BROOM AFTER INITIAL SET.
- B)BEFORE PLACING FRESH CONCRETE, CLEAN SURFACE AND REMOVE LAITANCE WITH WIRE BRUSH.
- C)IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, WET SURFACE AND REMOVE STANDING WATER.
22. ALL OPENINGS IN WALLS, UNLESS OTHERWISE NOTED, SHALL HAVE TWO #6 BARS ON SIDES AND SHALL EXTEND 2'-0" BEYOND EDGES OF OPENINGS, U.O.N.
23. ANCHOR BOLTS TO BE ASTM F1554 (WELDABLE) FY=55KSI UNLESS OTHERWISE NOTED.
24. NO PIPES OR CONDUITS EXCEEDING 1/3 SLAB THICKNESS IN OUTSIDE DIAMETER SHALL BE EMBEDDED IN THE STRUCTURAL CONCRETE FLOOR OR ROOF SLAB. WHERE CONDUITS OR PIPES ARE PERMITTED, THEY SHALL BE PLACED NOT CLOSER THAN 3 DIAMETER O.C. AND SHALL BE LOCATED SO AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE. ALUMINUM CONDUITS ARE NOT PERMITTED.

25. PROVIDE 4-#6 DOWELS X 3'-0" LONG BETWEEN ALL PIERS, FOOTINGS, GRADE BEAMS, ETC., U.O.N.
26. CONCRETE DIMENSIONAL TOLERANCE SHALL CONFORM TO ACI 117.
27. ALL SLAB-ON-GROUND SHALL HAVE THICKENING, DEPRESSIONS, OPENINGS, ETC., AS REQUIRED OR AS SHOWN HEREIN, OR ON ARCHITECTURAL, ELECTRICAL OR MECHANICAL DRAWINGS.
28. THE METHOD FOR CONVEYING CONCRETE TO THE PLACE OF DEPOSIT SHALL COMPLY WITH THE NEW YORK STATE BUILDING CODE. CONCRETE MAY BE CONVEYED BY PUMPING. PUMPING METHODS SHALL COMPLY WITH DEPARTMENT OF BUILDING REGULATIONS. ALUMINUM OR ALUMINUM ALLOY PIPE SHALL NOT BE USED.
29. THE MEANS AND METHODS OF CONCRETE PLACEMENT, INCLUDING POUR HEIGHTS, TEMPERATURE CONTROL, LATERAL PRESSURE AND UPLIFT PRESSURE CONTROL, FORMWORK DESIGN, AND OTHER REQUIRED CONSTRUCTION METHODS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR DOING THE WORK. THE CONTRACTOR SHALL EMPLOY ALL NECESSARY PRECAUTIONS IN ORDER TO AVOID ANY MOVEMENT OR DISLOCATION OF ALL M/E SYSTEMS, PIPING PENETRATIONS, ETC., INCLUDING THE CONCRETE PLACEMENT. ALL M/E EQUIPMENT SHALL BE INSTALLED AS INDICATED ON THE M/E DRAWINGS.
30. SEE ARCHITECTURAL DRAWINGS FOR FINISH OF ALL EXPOSED CONCRETE.

FOUNDATION NOTES

1. ALL MATERIAL, FABRICATION , INSTALLATION, AND INSPECTION REQUIREMENTS RELATING TO FOUNDATIONS SHALL CONFORM TO CHAPTER 18 OF THE NEW YORK STATE BUILDING CODE.
2. ALL FOOTINGS TO BEAR ON 6" OF CRUSHED STONE THAT IS TO BE PLACED ON UNDISTURBED SOIL WITH A MINIMUM BEARING CAPACITY OF 3 TONS/SQ.FT. AND TO BE VERIFIED IN THE FIELD BY INSPECTING AGENCY. NO CRUSHED STONE SHALL BE PLACED UNTIL THE SOIL BEARING CAPACITY HAS BEEN VERIFIED BY THE INSPECTING AGENCY.
4. PLACEMENT OF THE CRUSHED STONE SHALL BE PERFORMED UNDER THE OBSERVATION OF THE INSPECTION AGENCY.
5. FOR ADDITIONAL BACKFILL REQUIREMENTS, SEE THE CIVIL DRAWINGS.
6. ALL ELEVATIONS OF NEW FOOTINGS INDICATED ARE SUBJECT TO CHANGE UPON INSPECTION OF SOIL CONDITIONS DURING EXCAVATION OF SITE.
7. PRIOR TO COMMENCEMENT OF WORK, CONTRACTOR SHALL CALL (CALL-BEFORE-YOU-DIG.) FOR LOCATION OF UTILITIES
8. THE CONTRACTOR SHALL CONFORM TO SAFETY REQUIREMENTS DURING EXCAVATION AND COMPLY WITH NEW YORK STATE BUILDING CODE.
9. ALL FOOTINGS SHALL BE PLACED ON DRY SOIL. ALL EXCAVATION SHALL BE ADEQUATELY DEWATERED PRIOR TO POURING OF CONCRETE.
- 10.NO CONCRETE SHALL BE PACED ON FROZEN GROUND.

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REV	DESCRIPTION	DATE
1	ISSUED FOR BID	10.15.21
2	CD SET	11.30.21
3	REVISED BID SET	01.20.22
4	ISSUED FOR CONSTRUCTION	04.04.22

DRAWN BY :	D. CHECHURIN
CHECKED BY :	I. BEER
APPROVED BY :	I. BEER
DATE :	7/26/2021
SCALE :	AS SHOWN
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NOTES

DWG NUMBER :

SF-001

2 OF 31

LOADING SCHEDULE

SUPERIMPOSED DEAD LOADING: SELF WEIGHT OF CONCRETE SLAB AND METAL DECK IS NOT INCLUDED IN THE FOLLOWING VALUES.

- FIRST FLOOR SLAB ON GRADE 17 PSF
- SECOND FLOOR 15 PSF
- ROOF

- LIVE LOADING:
- FIRST FLOOR 100 PSF
 - SECOND FLOOR 80 PSF @ CORRIDORS 50PSF @ ROOMS
 - ROOF 20 PSF
 - DUNNAGE 100 PSF

- SNOW LOADING:
- GROUND SNOW LOAD PG= 30 PSF
 - SNOW EXPOSURE FACTOR CE= .9
 - IMPORTANCE FACTOR IS= 1
 - THERMAL FACTOR CT= 1
 - FLAT ROOF SNOW LOAD PF= 20 PSF
 - SLOPE ROOF FACTOR CS= 1.0
 - SLOPE ROOF SNOW LOAD PS= 20 PSF
 - DRIFT
 - SNOW DENSITY = 17.9 PCF
 - DRIFT HEIGHT HD = 3.52 FT
 - DRIFT HEIGHT PD = 63 PSF
 - MAXIMUM DRIFT LOAD = 83 PSF
 - DRIFT LOAD AT END = 20 PSF
 - DRIFT WIDTH W= 14 FT
 - FROST DEPTH = 4'-0" BELOW GRADE

- WIND LOADING:
- BASIC WIND VELOCITY V= 115 MPH
 - EXPOSURE FACTOR B
 - IMPORTANCE FACTOR IW= 1
 - ASCE 7-10 SIMPLIFIED PROCEDURE WIND LOADS
 - BUILDING OCCUPANCY II
 - g = 4.0
 - TERRAIN FACTOR KZT= 1.0
 - ROOF MEAN HEIGHT HR= 30 FT

- WIND FORCE MWFRS HORIZONTAL PRESSURE:
 - ZONE = 20.16 PSF
- WIND FORCE MWFRS VERTICAL PRESSURE:
 - ZONE = -22 PSF
- TOTAL BASE SHEAR OFFICE BUILDING.
 - N-S = 29 KIPS
 - E-W = 77 KIPS
- TOTAL BASE SHEAR ASRS BUILDING.
 - N-S = 186 KIPS
 - E-W = 160 KIPS

- COMPONENTS AND CLADDING
- DISTANCE a= 21.0 FT
 - EFFECTIVE WIND AREA OF C&C = 50 SF.
 - WIND FORCE C&C HORIZONTAL PRESSURE.
 - WALL = 26 PSF
 - WIND FORCE C&C VERTICAL PRESSURE.
 - ROOF = -28 PSF (UP)
 - ROOF = 16 PSF (DOWN)

- SEISMIC LOADING OFFICE BUILDING:
- SEISMIC LOADING CALCULATED AS PER ASCE 7-10.
 - SEISMIC OCCUPANCY II
 - SS= 0.26g
 - S1= 0.07g
 - SPECTRAL RESPONSE COEFFICIENT SDS= 0.27 g
 - SPECTRAL RESPONSE COEFFICIENT SD1= 0.114 g
 - SITE CLASS C

- SEISMIC DESIGN CATEGORY B
- BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL ORDINARY BRACED FRAMES
- RESPONSE MODIFICATION COEFFICIENT 'R' = 3.25
- SYSTEM OVER STRENGTH FACTOR 'Wo' = 2.00
- DEFLECTION AMPLIFICATION FACTOR 'Cd' = 3.25
- CS= 0.084%g
- TOTAL BASE SHEAR = 536 KIPS

- SEISMIC LOADING ASRS BUILDING:
- BASIC SEISMIC RESISTING FROM SYSTEM: ORDINARY CONCENTRIC BRACED FRAMES
 - R = 3.25
 - = 2.0
 - Cd = 3.25
 - CS = 0.084g
 - TOTAL BASE SHEAR = 284 KIPS

NEW YORK STATE SPECIAL INSPECTION NOTES

- THE DESIGNATED INSPECTING AGENCY FOR SPECIAL INSPECTION APPROVED BY THE CODE ENFORCEMENT OFFICIAL AND ACCEPTABLE TO THE STRUCTURAL ENGINEER RETAINED BY THE OWNER/CONTRACTOR SHALL PERFORM ON SITE INSPECTION AND TESTING IN ACCORDANCE WITH NEW YORK STATE BUILDING CODE REGULATIONS UNDER THE SUPERVISION OF A NEW YORK STATE LICENSED PROFESSIONAL. SPECIAL INSPECTION ITEMS INCLUDE THE FOLLOWING LISTED BELOW.
- THE SPECIAL INSPECTION ENGINEERS ARE RESPONSIBLE FOR FILING AND OBTAINING APPROVAL OF ALL STATEMENTS, TEST AND INSPECTION REPORTS, INCLUDING STEEL, CONCRETE, MASONRY PRODUCER'S CERTIFICATES. COPIES TO BE SENT TO ENGINEER OF RECORD.
- ALL TESTING AGENCY REPORTS SHALL BE SIGNED AND SEALED BY A NEW YORK STATE LICENSED PROFESSIONAL ENGINEER. TECHNICAL REPORTS SHALL BE FILED WITH THE BUILDING DEPARTMENT FOR APPROVAL OF SAID REPORTS.
- CONTRACTOR TO NOTIFY THE SPECIAL INSPECTION ENGINEERS AT LEAST 72 HOURS BEFORE THE SPECIFIC WORK COMMENCES. THE CONTRACTOR SHALL PROVIDE AMPLE TIME FOR THE TESTING AGENCY TO COMPLETE THEIR WORK.
- CONTRACTOR SHALL NOT CONCEAL ANY WORK REQUIRING SPECIAL INSPECTION UNTIL SUCH WORK IS INSPECTED OR TESTED AND DEEMED TO MEET PROJECT CRITERIA AND CODE REQUIREMENTS.

SPECIAL INSPECTION	CODE SECTION	EMPLOYED BY
SOILS SITE PREPARATION	1704.7.1	OWNER
SOIL-FILL PLACEMENT AND IN-PLACE DENSITY	1704.7.2, 1704.7.3	OWNER
CONCRETE - CAST IN PLACE	TABLE 1704.4	OWNER
CONCRETE DESIGN MIX	1904 & 1905	CONTRACTOR
CONCRETE TEST CYLINDERS *	1905.6	OWNER
ADHESIVE ANCHOR AS REQ'D BY MANUFACTURER	N/A	OWNER
EXCAVATION - SHEETING, SHORING AND BRACING	3304.4.1	OWNER
UNDERPINNING		OWNER

UNDERPINNING NOTES

- BEFORE THE START OF UNDERPINNING WORK THE CONTRACTOR SHALL TAKE PHOTOGRAPHS TO DOCUMENT THE CONDITION OF THE EXISTING BUILDING AT 10 DUNNIGAN DRIVE FOR TWO COLUMN BAYS IN EVERY DIRECTION FORM THE FOUNDATION BEING UNDERPINNED. A REPORT CONTAINING THE PHOTOS AND DESCRIPTIONS OF ANY EXISTING CRACKS OR OTHER DAMAGE SHALL BE SUBMITTED TO THE OWNER BEFORE THE START OF UNDERPINNING WORK.
- CONTRACTOR SHALL SUBMIT A DETAILED UNDERPINNING PROCEDURE FOR REVIEW BY THE ENGINEER PRIOR TO THE START OF THE UNDERPINNING WORK. UNDERPINNING WORK SHALL NOT BEGIN UNTIL THIS PROCEDURE HAS BEEN APPROVED.
- THE SPECIAL INSPECTOR SHALL BE PRESENT AT ALL TIMES DURING ALL UNDERPINNING OPERATIONS.
- CONCRETE FOR UNDERPINNING PIERS SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 4000PSI. TYPE III CEMENT MAY BE USED.
- TIMBER LAGGING SHALL HAVE A MINIMUM ALLOWABLE BENDING STRESS OF 1,200 PSI AND A MINIMUM ALLOWABLE SHEAR STRESS OF 175 PSI. USE 2" LAGGING ABOVE 6'-0" DEPTH AND 3" L
- SIZES OF THE PIT PIERS ARE SHOWN ON THE CONTRACT DRAWINGS.
- EXCAVATE TO THE TOP OF THE ADJACENT FOOTING. DURING EXCAVATION OF EACH PIT, RECORD THE ELEVATION OF THE BOTTOM OF THE FOUNDATION. DEMOLITION OF THE OUTER EDGE OF THE FOOTINGS SHALL BE PERFORMED AFTER THE UNDERPINNING PIERS ARE FULLY INSTALLED.
- ONLY PITTS HAVING THE SAME LETTER SHALL BE OPEN AT THE SAME TIME.
- PITS OF THE NEXT LETTER SHALL NOT BE OPENED UNTIL THE PREVIOUS PIT IS WEDGED AND DRYPACKED, THE DOWELS THROUGH THE FOOTING ARE INSTALLED, AND THE APPROACH PIT FOR THE CONSTRUCTION OF THE PREVIOUS PIT IS BACKFILLED.
- NO EXCAVATION FACE SHALL BE LEFT EXPOSED (UNLAGGED) FOR MORE THAN 3 HOURS.
- EXCAVATION BENEATH THE EXISTING STRUCTURE SHALL BE PERFORMED WITH HAND-HELD SHOVELS. CAREFULLY HAND TRIM THE EXCAVATION FACES. USE OF BACKHOES IS PROHIBITED FOR EXCAVATION OF UNDERPINNING PIT PIERS.
- EXCAVATION SHALL NOT EXTEND MORE THAN ONE FOOT BENEATH THE LOWEST INSTALLED LAGGING BOARD.
- ALL FOUR SIDES OF THE EXCAVATION SHALL BE LAGGED AND CLEATED BEFORE PROCEEDING WITH SUBSEQUENT EXCAVATION.
- PROVIDE LOUVERS BETWEEN LAGGING BOARDS AS SHOWN ON SE-300.
- IMMEDIATELY BACKPACK ANY VOIDS BEHIND THE LAGGING BOARDS.
- EXCAVATION OF UNDERPINNING PITTS BELOW THE WATER TABLE IS PROHIBITED.
- IF GROUND WATER IS ENCOUNTERED IN A PIT, BACKFILL TO ABOVE THE GROUND WATER TABLE. PROVIDE A WELL POINT NEAR THE PIT TO LOWER THE WATER TABLE BENEATH THE REQUIRED PIT SUBGRADE ELEVATION. RESUME PIT EXCAVATION AFTER THE GROUNDWATER LEVEL HAS BEEN LOWERED.
- ALL UNDERPINNING PIERS SHALL BEAR ON UNDISTURBED NATURAL SAND. CONCRETE SHALL NOT BE PLACED UNTIL PIT SUBGRADE IS INSPECTED AND APPROVED BY THE SPECIAL INSPECTOR.
- THE UNDERPINNING SHALL BE INSTALLED IN A MANNER SUCH THAT THE EXPOSED FACE OF THE CONCRETE IS VERTICAL, CLEAN AND NEAT.
- EACH UNDERPINNING PIER SHALL BE A CONSTRUCTED IN ONE LIFT WITHOUT INTERMEDIATE HORIZONTAL CONSTRUCTION JOINTS (COLD JOINTS)
- THE UNDERSIDE OF THE FOOTING SHALL BE CLEANED OF SOIL AND LOOSE MATERIAL PRIOR TO INSTALLING WEDGES AND DRYPACKING.
- WEDGING SHALL BE PERFORMED AFTER CONCRETE HAS ATTAINED A MINIMUM STRENGTH OF 2,000 PSI. STRENGTH SHALL BE CONFIRMED BY BREAKING LABORATORY TEST SAMPLES PRIOR TO WEDGING.
- TACK WELD WEDGES AFTER DRIVING. INSTALL DRYPACK AFTER TACK WELDING IS PERFORMED.
- DRYPACK SHALL CONSIST OF A MIXTURE OF 1 PART CEMENT AND 1.5 PARTS MOIST SAND.
- DRYPACK SHALL BE RAMMED INTO SPACE BETWEEN TOP OF PIER CONCRETE AND THE UNDERSIDE OF THE FOOTING USING THE END OF A 2x4 OR SIMILAR INSTRUMENT. THE ENTIRE VOID BETWEEN THE PIER CONCRETE AND THE BOTTOM OF THE FOOTING SHALL BE FILLED WITH DRYPACK IN THIS MANNER.
- DRYPACK THICKNESS SHALL NOT EXCEED 3".
- DO NOT PLACE BACKFILL AGAINST NEW UNDERPINNING SEGMENT UNTIL A MINIMUM OF 24 HOURS HAS PASSED SINCE COMPLETION OF THE CONCRETE PLACEMENT. BACKFILL CURRENT EXCAVATION TO THE TOP OF THE FOOTING PRIOR TO STARTING EXCAVATION FOR NEXT PIER.
- TIMBER SHEETING FOR UNDERPINNING AND APPROACH PITTS SHALL REMAIN IN PLACE UNTIL GENERAL EXCAVATION IS PERFORMED.

TIEBACK NOTES

- TIBEBACKS SHOWN ON THE CONTRACT DRAWINGS UNDER 10 DUNNIGAN DRIVE SHALL BE DESIGNED BY THE CONTRACTOR FOR THE LOADS SHOWN ON THE CONTRACT DRAWINGS IN ACCORDANCE WITH THE REQUIREMENTS OF PTI DC35.1-14, "RECOMMENDATIONS FOR PRESTRESSED ROCK AND SOIL ANCHORS".
- A COMPLETE SET DESIGN CALCULATIONS DOCUMENTING THE STRUCTURAL AND GEOTECHNICAL CAPCITY OF THE TIEBACKS, SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NY SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW.
- TIEBACKS SHALL BE TESTED IN ACCORDANCE WITH THE REQUIREMENTS OF PTI DC35.1-14, "RECOMMENDATIONS FOR PRESTRESSED ROCK AND SOIL ANCHORS".

OPTICAL MONITORING NOTES:

- CONTRACTOR SHALL PREPARE AN OPTICAL MONITORING PLAN SHOWING THE LOCATIONS OF THE MONITORING POINTS, AND MONITORING POINT DESIGNATIONS AND SUBMIT THE PLAN FOR REVIEW BY THE ENGINEER PRIOR TO THE START OF WORK.
- PRIOR TO THE START OF WORK, INSTALL A DEFORMATION MONITORING POINT, MIDWAY UP THE COLUMN AT EACH FOUNDATION THAT IS TO BE UNDERPINNED AND AT 25-FOOT INTERVALS ALONG THE NORTH AND WEST WALL OF 20 DUNNIGAN DRIVE, AND ALONG THE EAST AND WEST WALLS OF 10 DUNNIGAN DRIVE.
- TAKE TWO BASELINE READINGS OF ALL MONITORING POINTS PRIOR TO THE START OF EXCAVATION.
- DURING UNDERPINNING OPERATIONS, DEFORMATION MONITORING POINTS THE COLUMNS AT THE FOOTINGS BEING UNDERPINNED SHALL BE OPTICALLY MONITORED FOR VERTICAL AND LATERAL MOVEMENTS AT LEAST TWICE DAILY. ALL MEASUREMENTS SHALL BE REFERENCED TO FIXED BACK-SITE REFERENCE POINTS ESTABLISHED AND AGREED TO WITH THE ENGINEER. MEASUREMENTS SHALL BE PROVIDED TO THE ENGINEER AT THE END OF EACH DAY. AFTER UNDERPINNING IS COMPLETE, TAKE READINGS WEEKLY FOR 4 MORE WEEKS.
- DEFORMATION MONITORING POINTS ON ADJACENT BUILDINGS SHALL BE OPTICALLY MONITORED DAILY DURING ACTIVE EXCAVATION IN FRONT OF BUILDING AND WEEKLY AND UNTIL FOUNDATION CONSTRUCTION IS COMPLETE. MONITORING REPORTS SHALL BE SUBMITTED TO THE ENGINEER AT THE END OF EACH WEEK.
- AT THE END OF EACH DAY DURING UNDERPINNING OPERATIONS, THE SPECIAL INSPECTOR AND THE UNDERPINNING CONTRACTOR SHALL VISUALLY REVIEW THE CONDITION OF THE BUILDING IN EACH COLUMN BAY ADJACENT TO THE UNDERPINNING TO CHECK FOR SIGNS OF SETTLEMENT OR DAMAGE.
- IF MOVEMENT OF ¼" IS RECORDED AT ANY MONITORING POINT, INFORM THE ENGINEER AND PROJECT OWNER IMMEDIATELY. A MEETING SHALL BE HELD WITH THE OWNER, CONTRACTOR AND ENGINEER TO REVIEW THE CONSTRUCTION PROCEDURES AND DETERMINE IF MODIFICATIONS ARE REQUIRED.
- IF MOVEMENT OF ½" IS RECORDED AT ANY MONITORING POINT, STOP ALL WORK, BACKFILL ANY INCOMPLETE UNDERPINNING PITTS. DO NOT RESUME WORK UNTIL REVISED PROCEDURES ARE DETERMINED AND AGREED UPON.

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DRAWN BY :	D.CHECHURIN
CHECKED BY :	I. BEER
APPROVED BY :	I. BEER
DATE :	7/26/2021
SCALE :	AS SHOWN

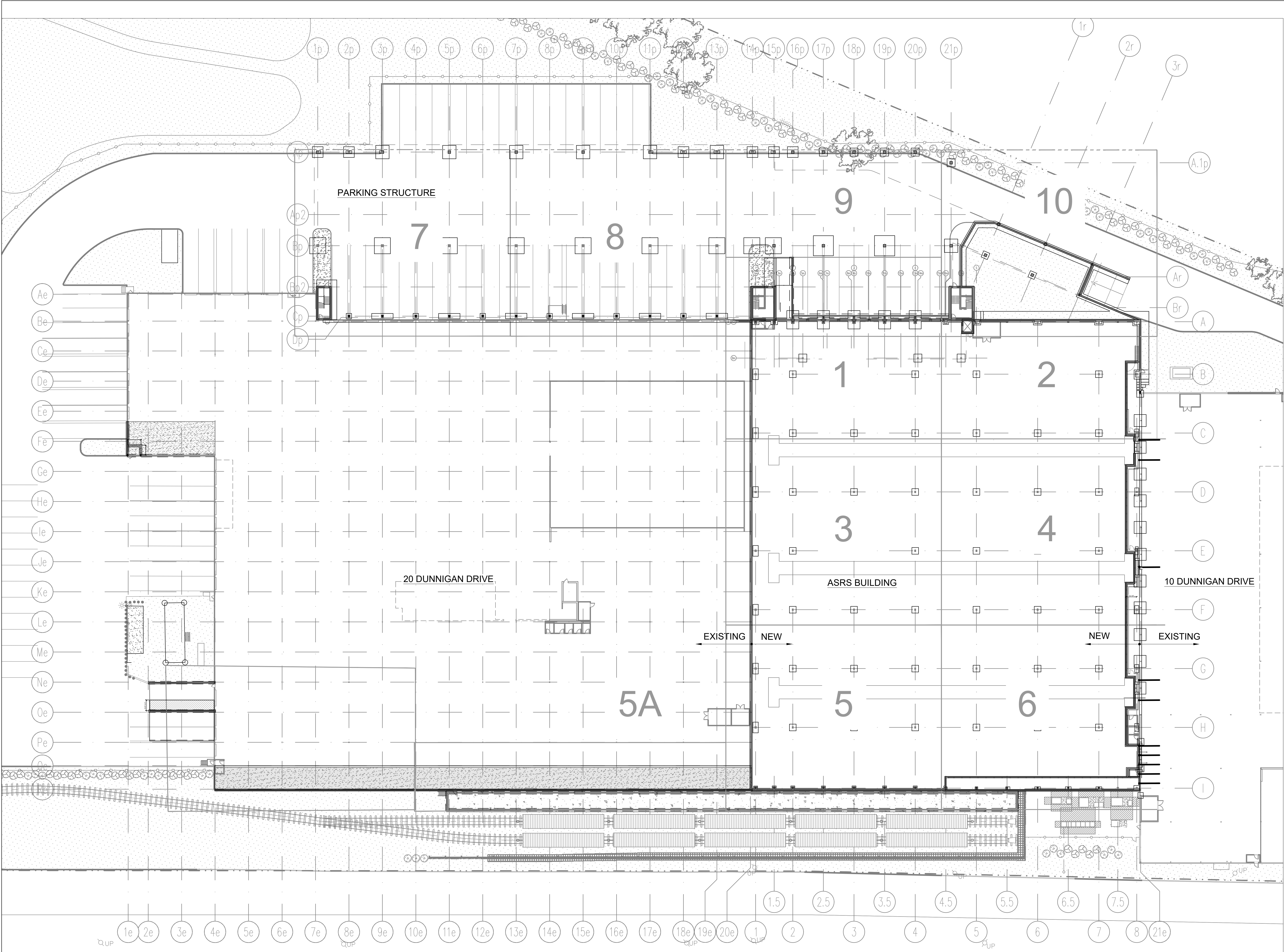
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NOTES



DWG NUMBER :
SF-002

3 OF 31



FOUNDATION KEY PLAN
SCALE: 1/32" = 1'-0"

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FOUNDATION - KEY PLAN



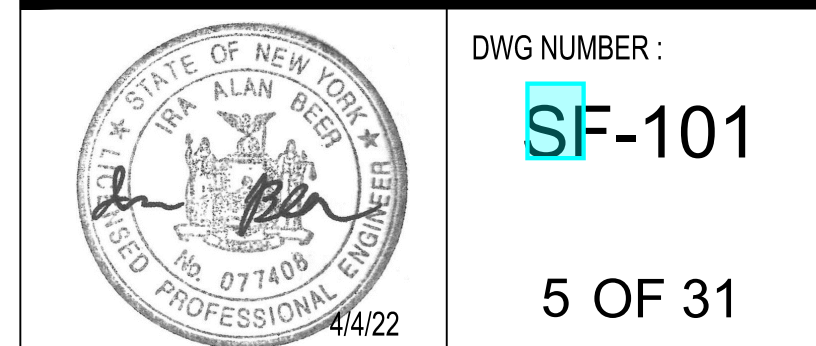
DWG NUMBER :
SF-100

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1	ISSUED FOR BID	10.15.
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SCALE :	AS SHOWN

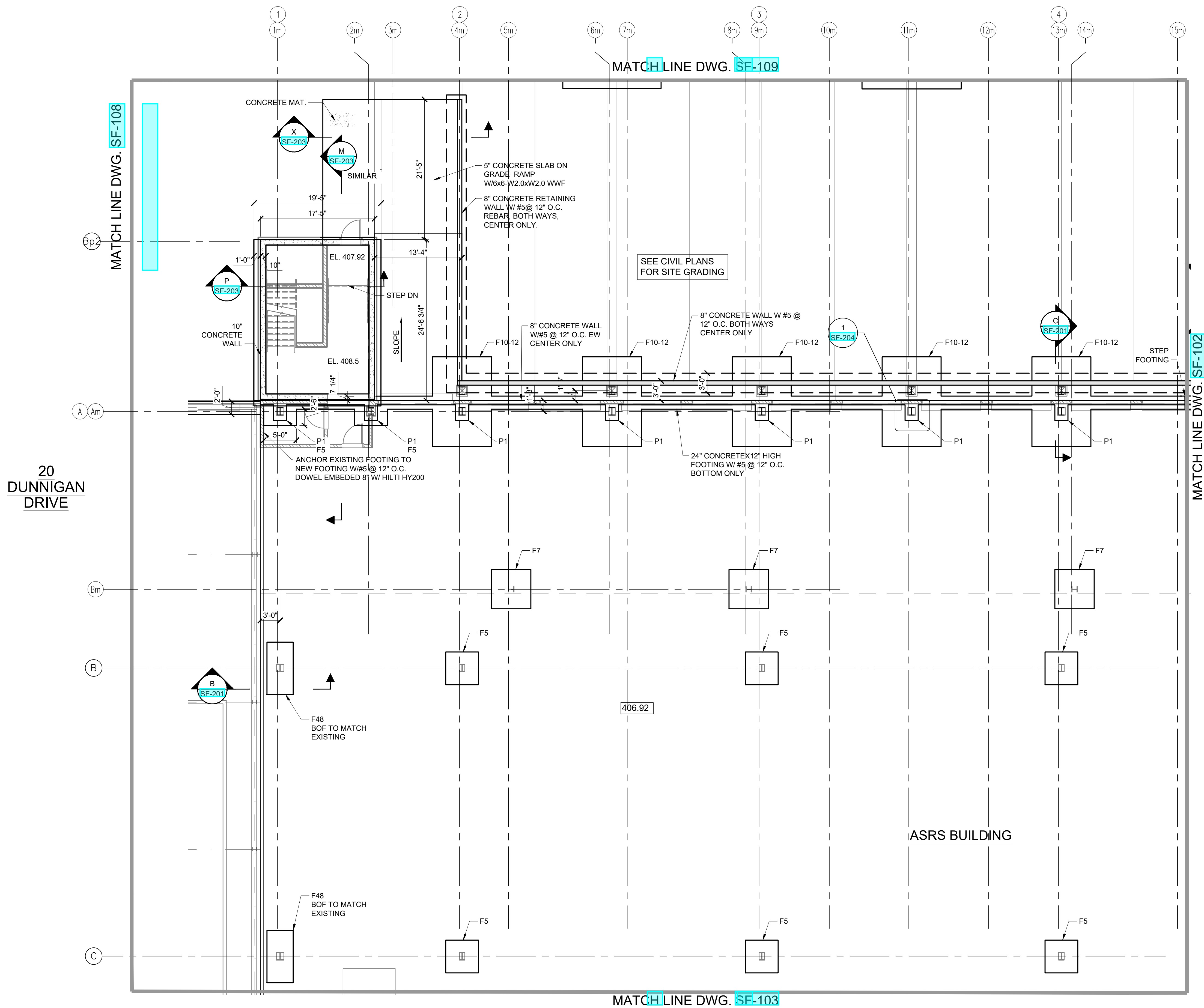
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FOUNDATION PART PLAN

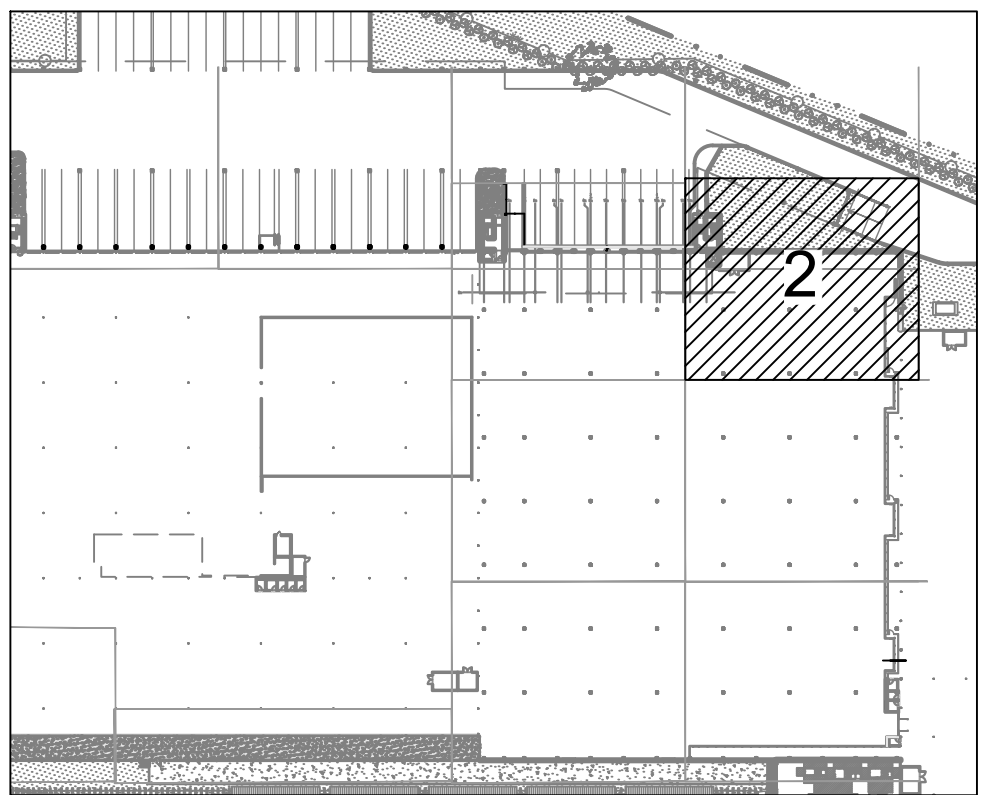
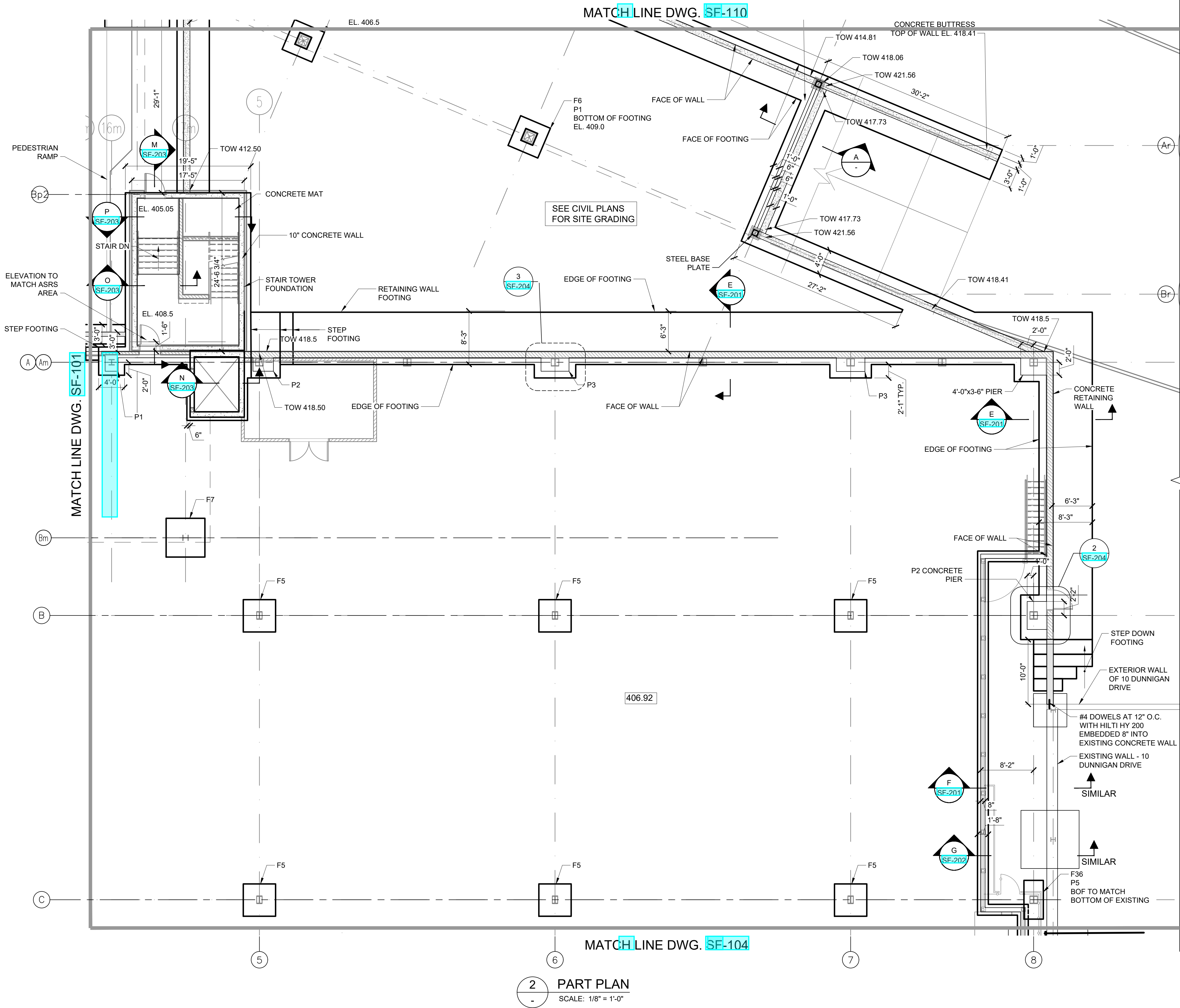


- NOTES:**
1. FOR FOUNDATION SCHEDULE, SEE DWG **SF-203**
 2. FOR PIER DETAILS, SEE DRAWING **SF-204**
 3. GENERAL EXCAVATION SUBGRADE ELEVATION BETWEEN FOUNDATION LOCATIONS IS NOTED BY NUMBERS SHOWN AS **XX.X**
 4. TOP OF PIER ELEVATIONS SHALL BE AT EL. 407.08.

FOOTING ELEVATIONS DWG. SF-102	
COLUMN LINE	BOT. OF FOOTING EL.
A	402.5
A.1	405.67
B	405.83
C	405.83



1 PART PLAN
- SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

- NOTES:
- 1. FOR FOUNDATION SCHEDULE, SEE DWG. SF-203
 - 2. FOR PIER DETAILS, SEE DRAWING SF-204
 - 3. GENERAL EXCAVATION SUBGRADE ELEVATION BETWEEN FOUNDATION LOCATIONS IS NOTED BY NUMBERS SHOWN AS XX.X
 - 4. TOP OF PIER ELEVATIONS SHALL BE AT EL. 407.08.

FOOTING ELEVATIONS DWG. SF-102

COLUMN LINE	BOT. OF FOOTING EL.
A-LINE AND 8-LINE RETAINING WALL	405.5
A LINE FOOTING	402.5
A.1	405.67
B5 TO B9	405.83
C5 TO C7	405.83
8 LINE SOUTH OF LINE B	SEE SECTIONS

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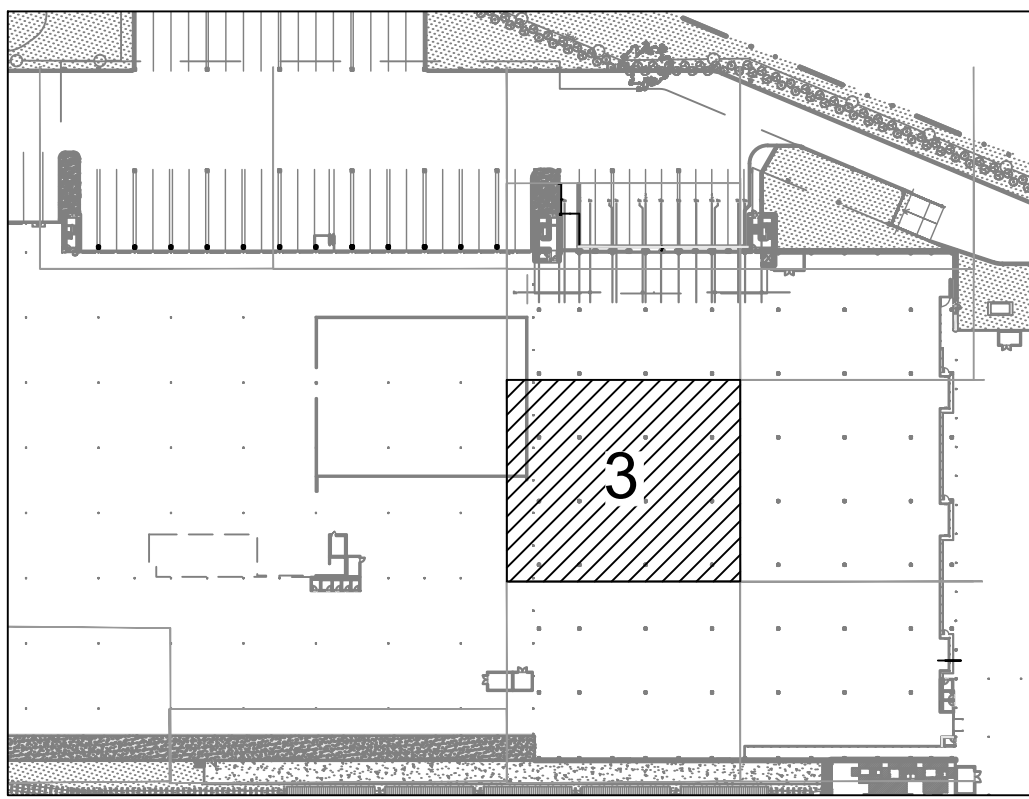
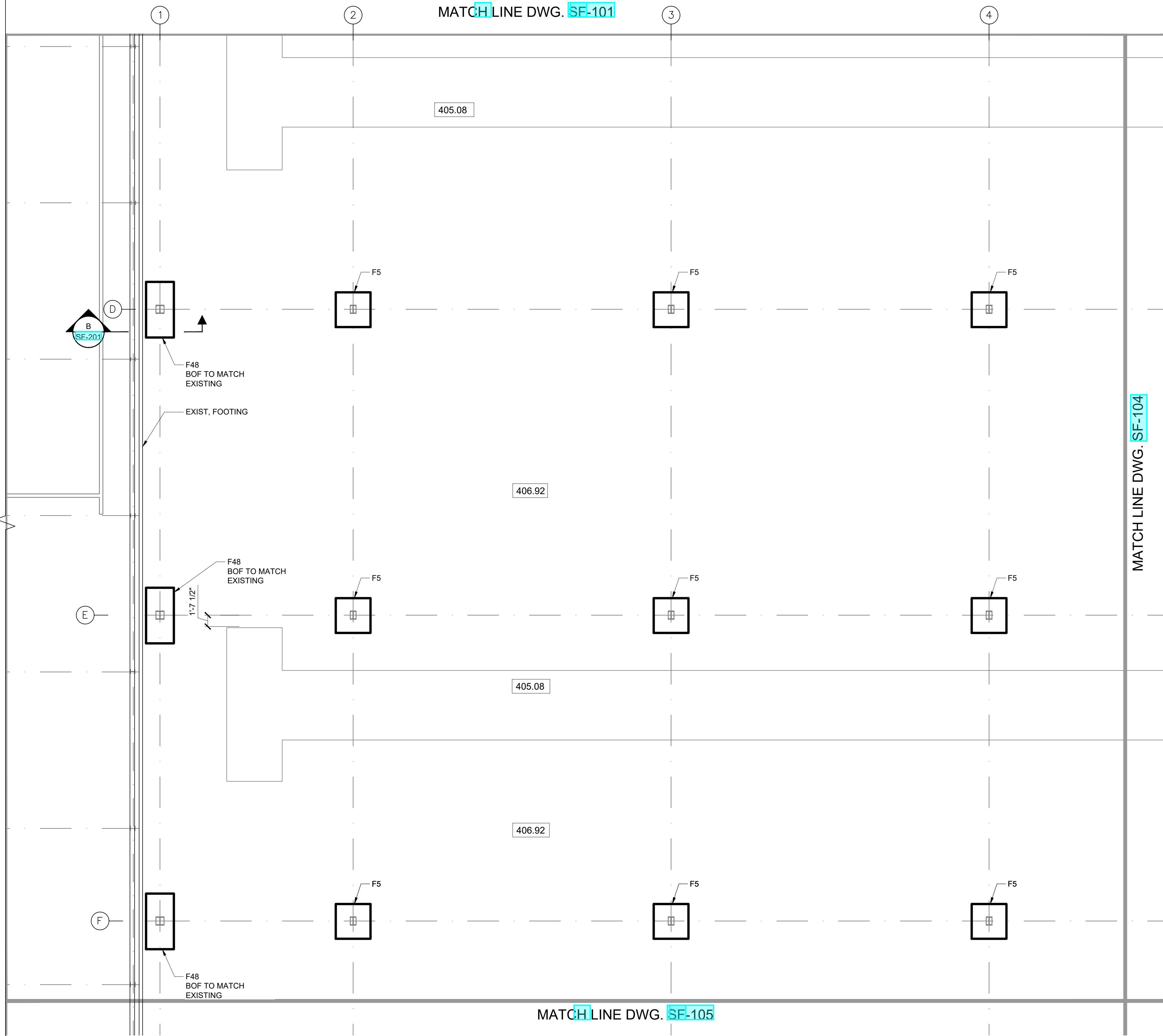
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FOUNDATION PART PLAN

DWG NUMBER :
SF-102

6 OF 31

20 DUNNIGAN DRIVE



KEY PLAN
SCALE: N.T.S.

- NOTES:
1. FOR FOUNDATION SCHEDULE, SEE DWG. SF-203
 2. FOR PIER DETAILS, SEE DRAWING SF-204
 3. GENERAL EXCAVATION SUBGRADE ELEVATION BETWEEN FOUNDATION LOCATIONS IS NOTED BY NUMBERS SHOWN AS [XX.X]
 4. TOP OF PIER ELEVATIONS SHALL BE AT EL. 407.08.

FOOTING ELEVATIONS DWG. SF-103	
COLUMN LINE	BOT. OF FOOTING EL.
D2 TO D4	405.83
E2 TO E4	405.83
F2 TO F4	405.83

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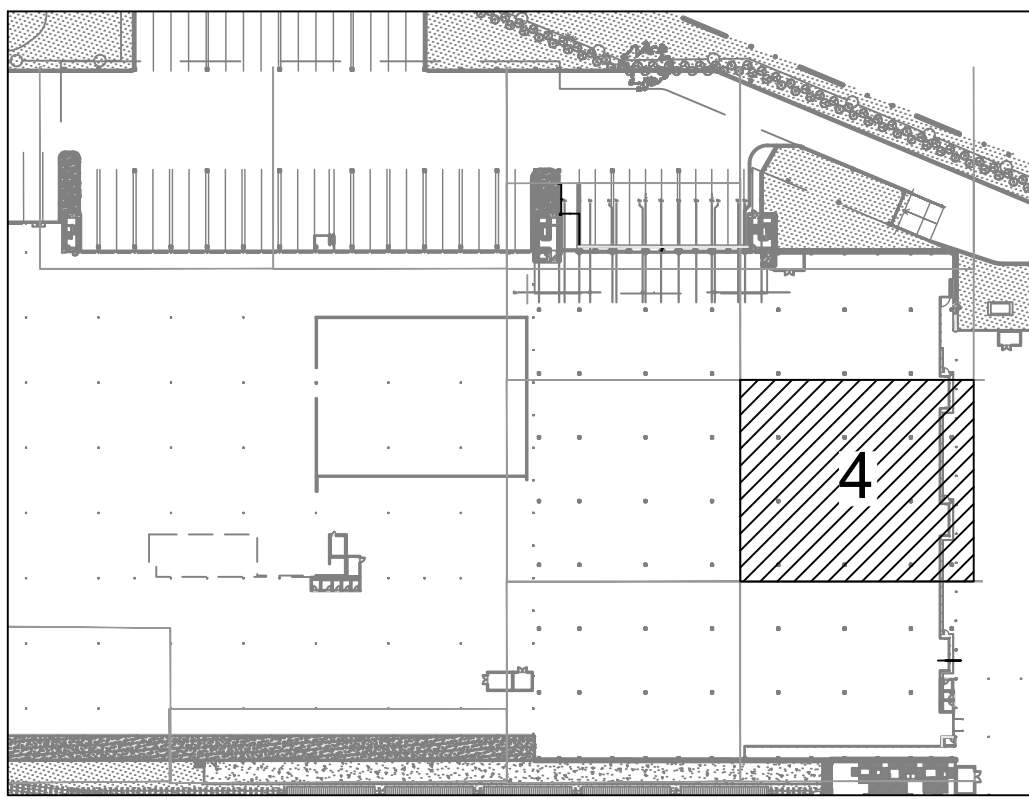
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FOUNDATION PART PLAN



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SF-103

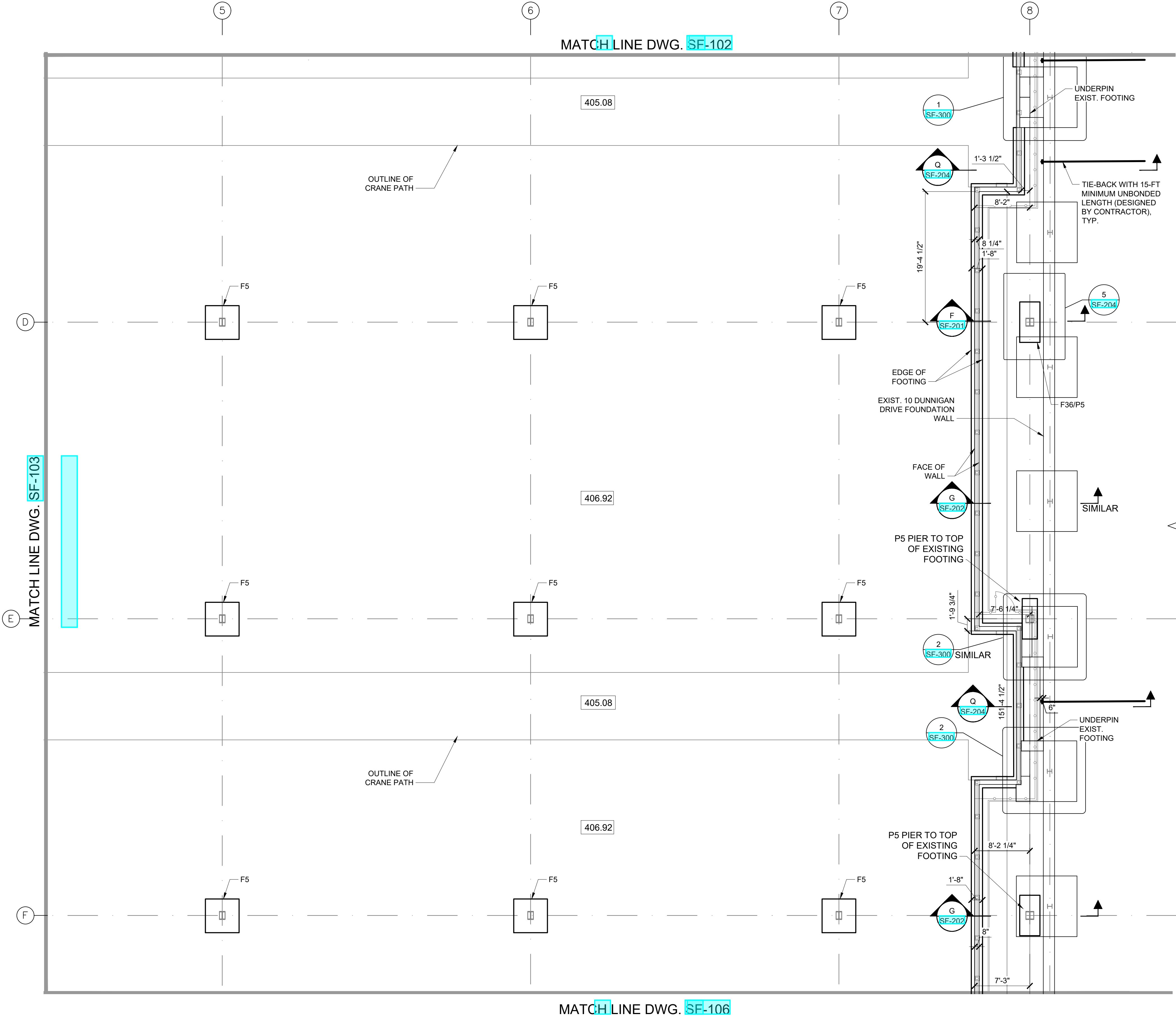


KEY PLAN
SCALE: N.T.S.

- NOTES:
1. FOR FOUNDATION SCHEDULE, SEE DWG. SF-203
 2. FOR PIER DETAILS, SEE DRAWING SF-204
 3. GENERAL EXCAVATION SUBGRADE ELEVATION BETWEEN FOUNDATION LOCATIONS IS NOTED BY NUMBERS SHOWN AS XX.X
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FOOTING ELEVATIONS DWG. SF-104	
COLUMN LINE	BOT. OF FOOTING EL.
D5 TO D7	405.83
E5 TO E7	405.83
F5 TO F7	405.83
8 LINE	SEE SECTIONS

10 DUNNIGAN DRIVE



4 PART PLAN
SCALE: 1/8" = 1'-0"

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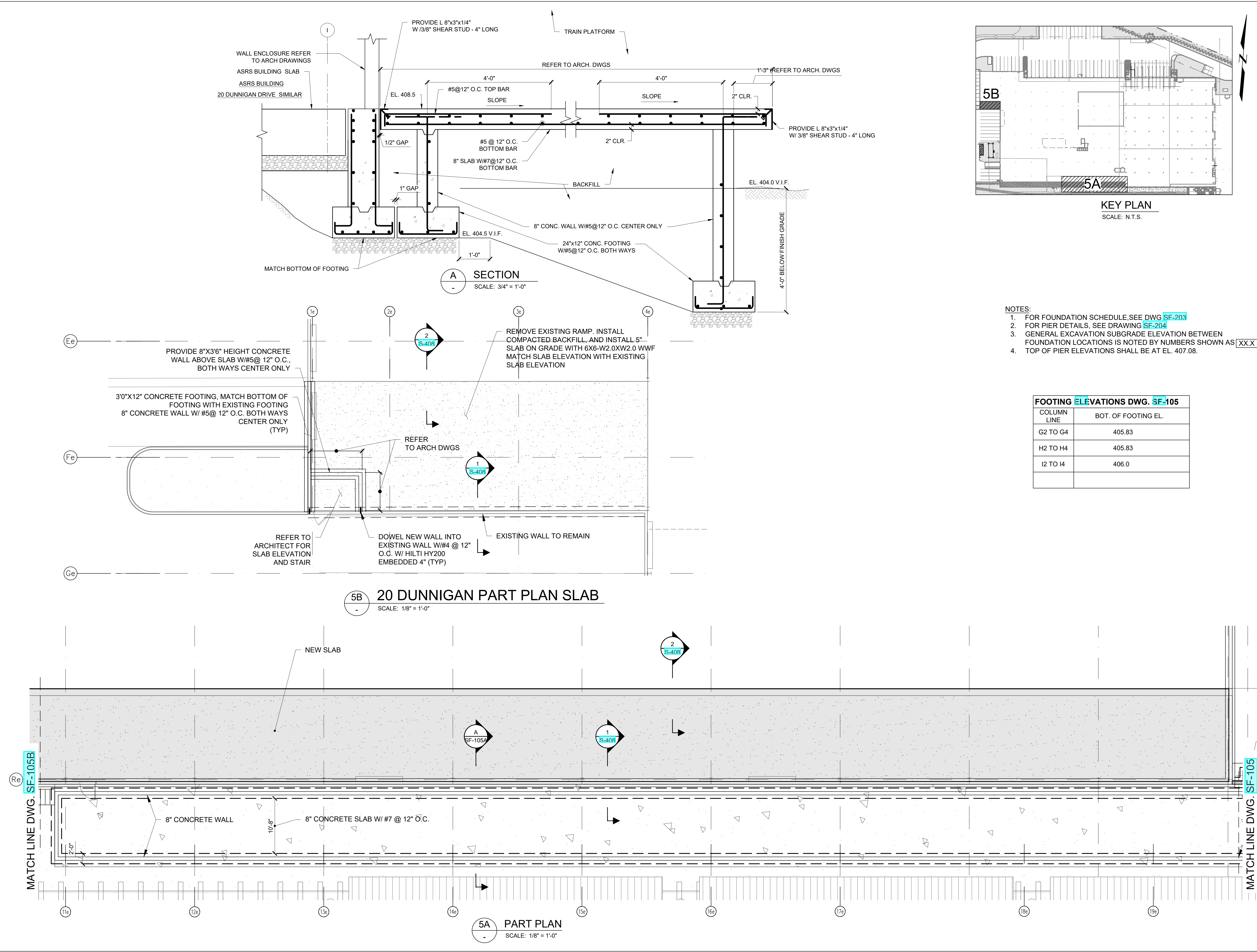
FOUNDATION PART PLAN



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4	ISSUED FOR CONSTRUCTION	04.04.22

DRAWN BY : D. CHECHURIN

CHECKED BY : I. BEER

APPROVED BY : I. BEER

DATE : 7/26/2021

SCALE : AS SHOWN

DRAWING TITLE : FOUNDATION PART PLAN

STATE OF NEW YORK
JULIA ALAN BEER
No. 077409
LICENSED PROFESSIONAL ENGINEER

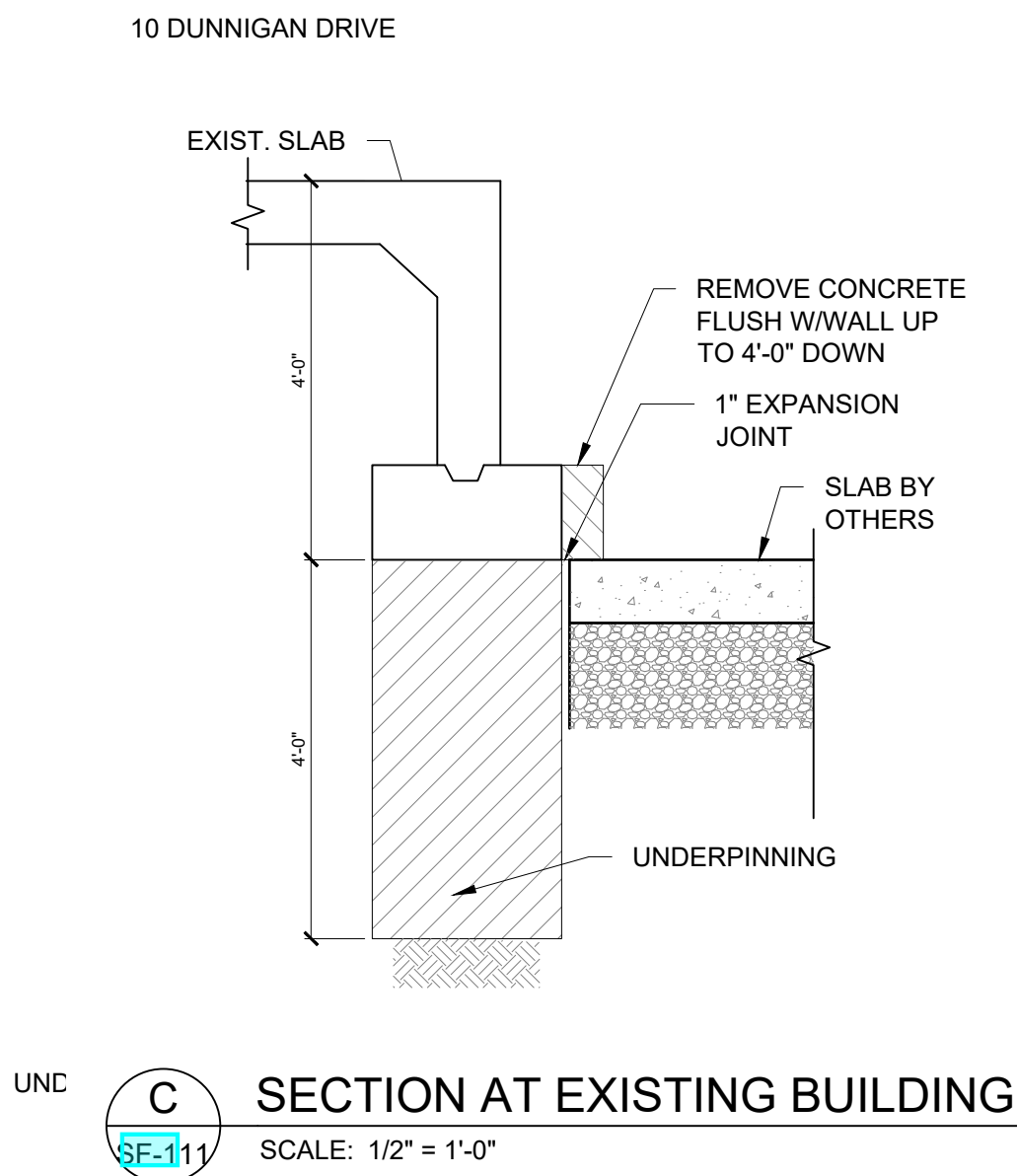
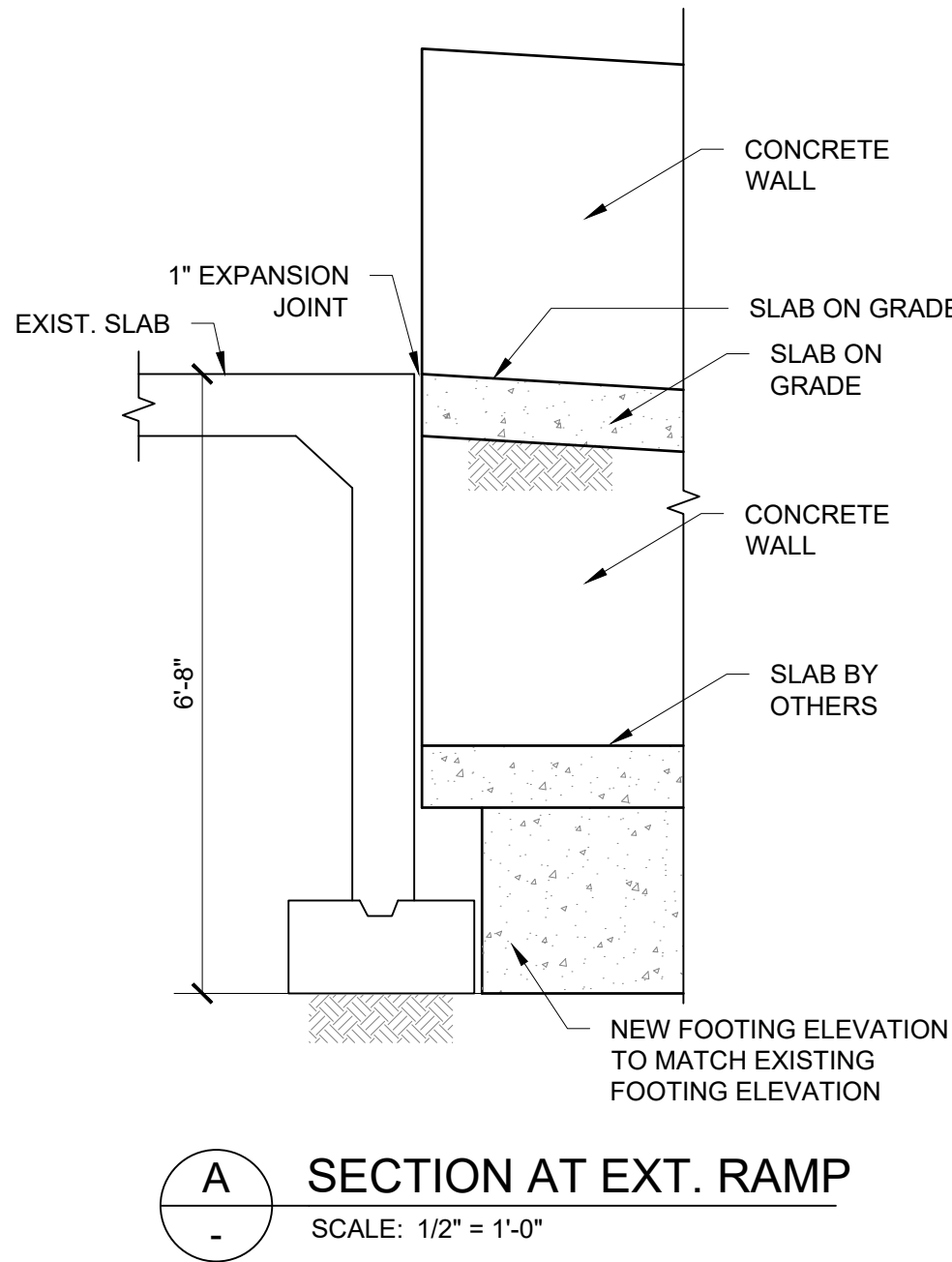
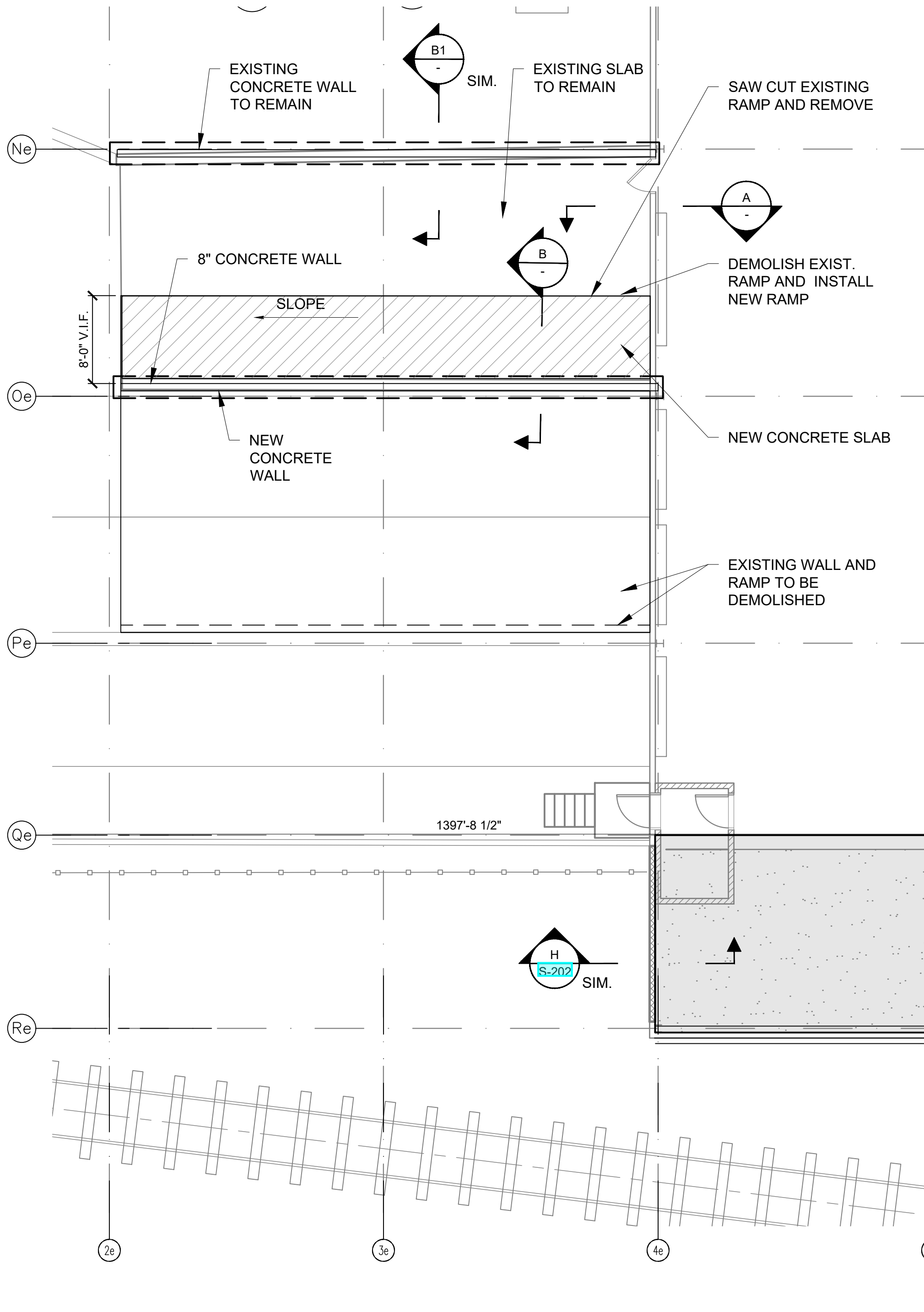
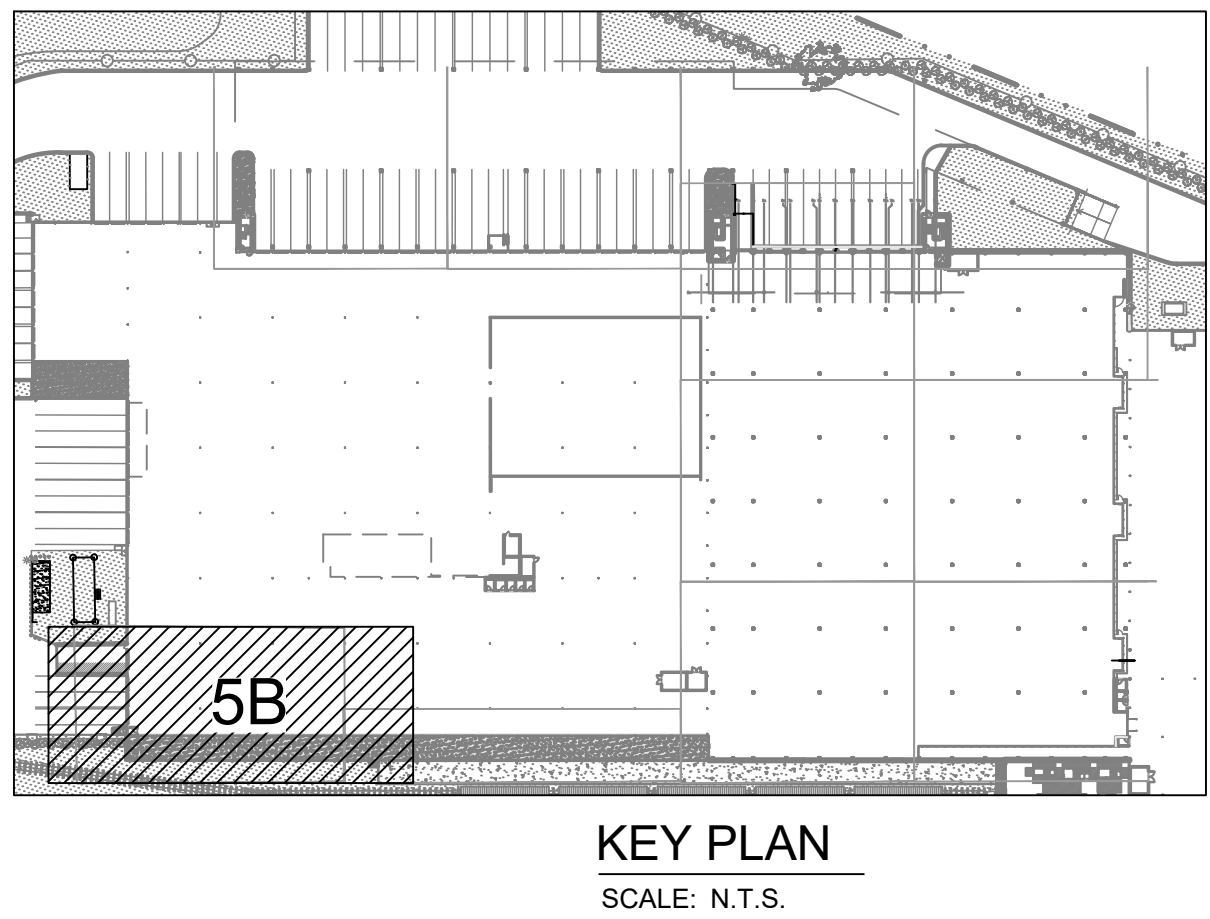
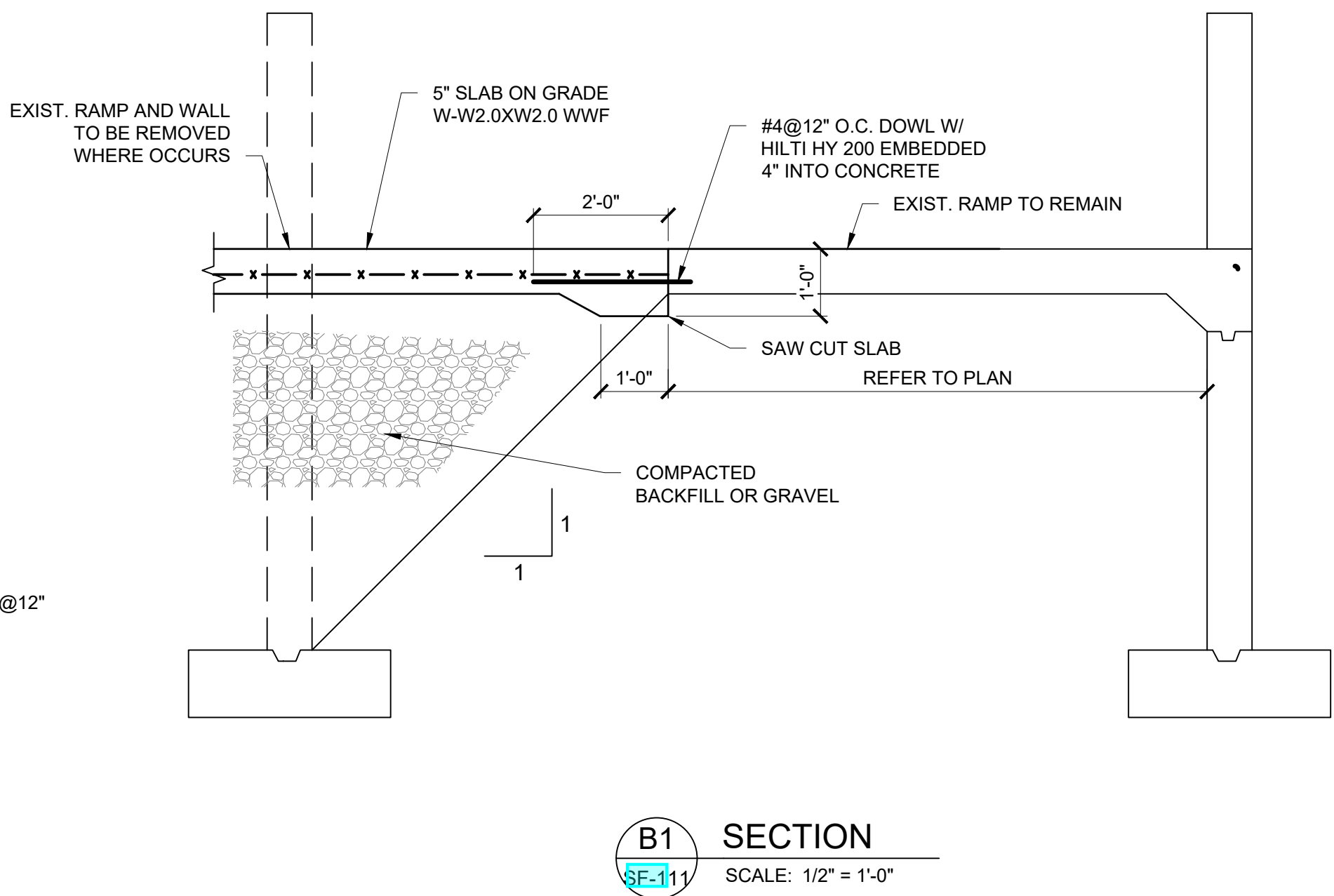
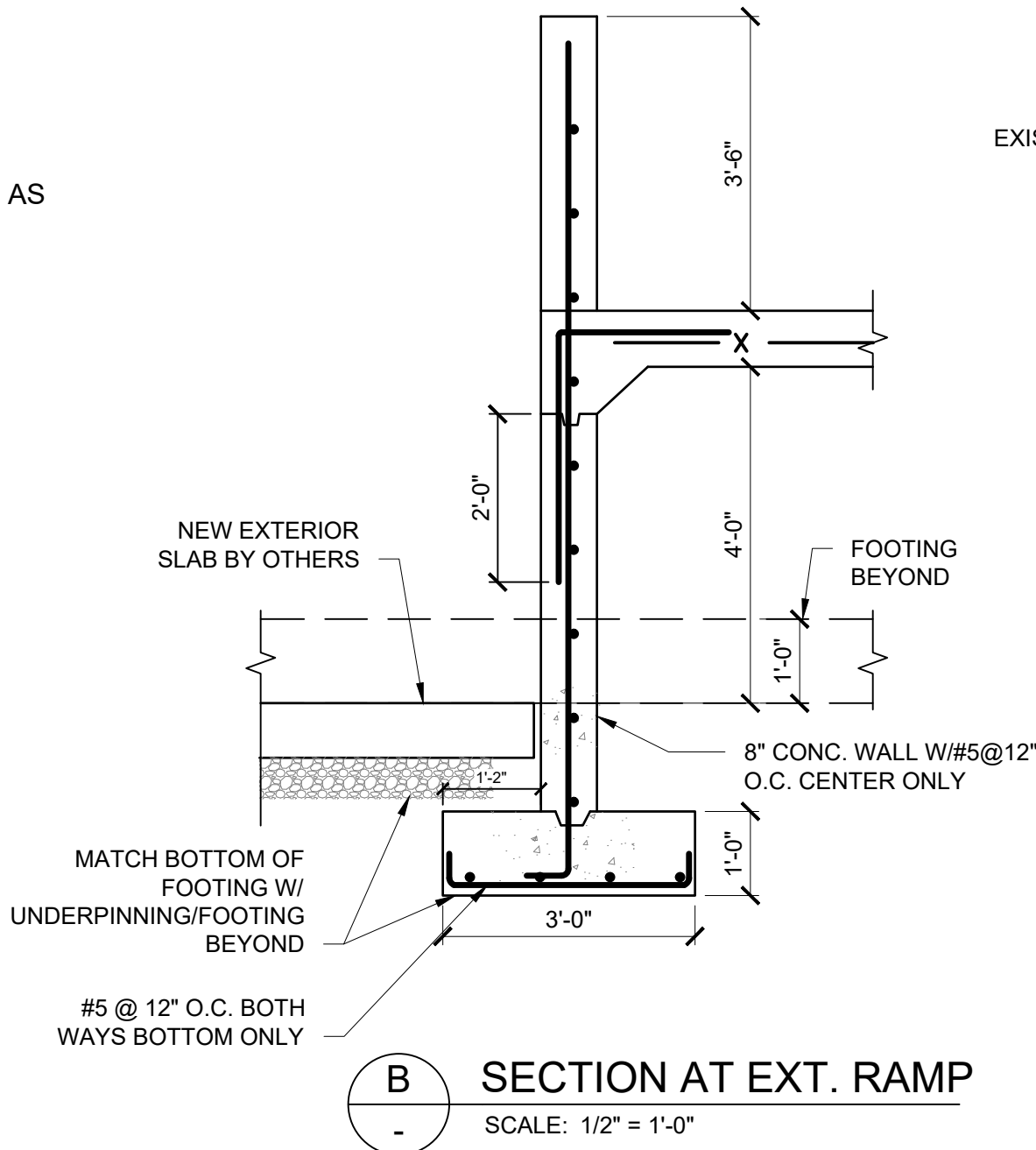
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DWG NUMBER : SF-105A

10 OF 31

- NOTES:
1. FOR FOUNDATION SCHEDULE, SEE DWG. [SF-203](#)
 2. FOR PIER DETAILS, SEE DRAWING [SF-204](#)
 3. GENERAL EXCAVATION SUBGRADE ELEVATION BETWEEN FOUNDATION LOCATIONS IS NOTED BY NUMBERS SHOWN AS
 4. TOP OF PIER ELEVATIONS SHALL BE AT EL. 407.08.

FOOTING ELEVATIONS DWG. SF-105	
COLUMN LINE	BOT. OF FOOTING EL.
G2 TO G4	405.83
H2 TO H4	405.83
I2 TO I4	406.0



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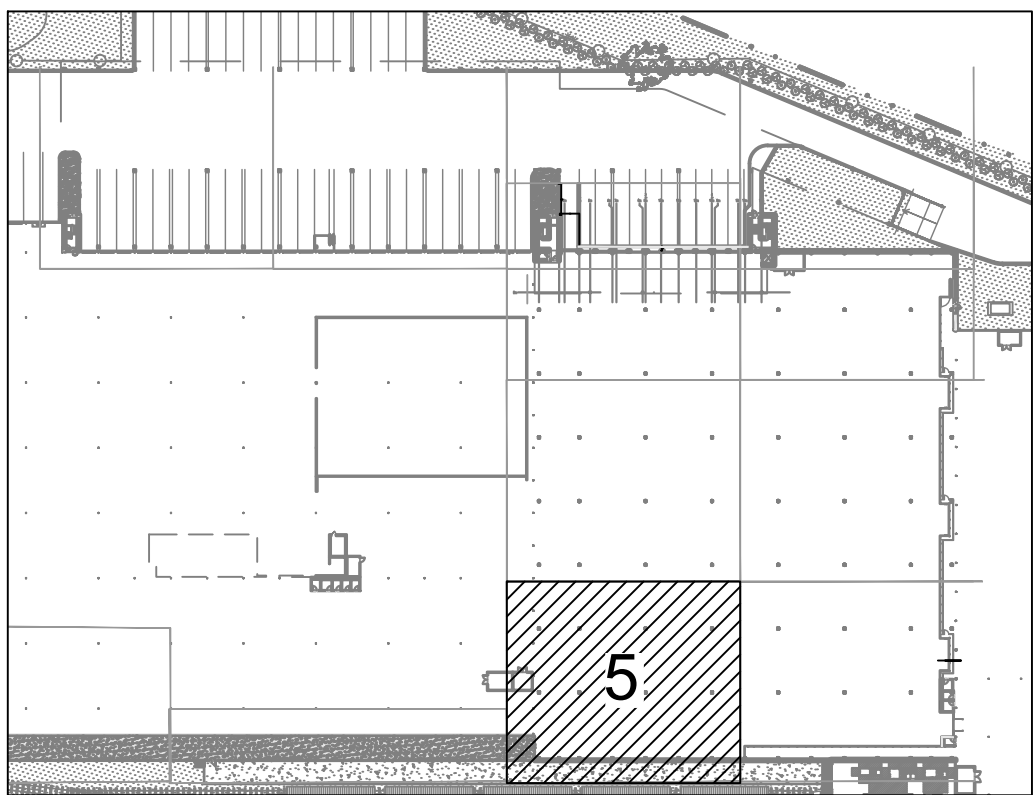
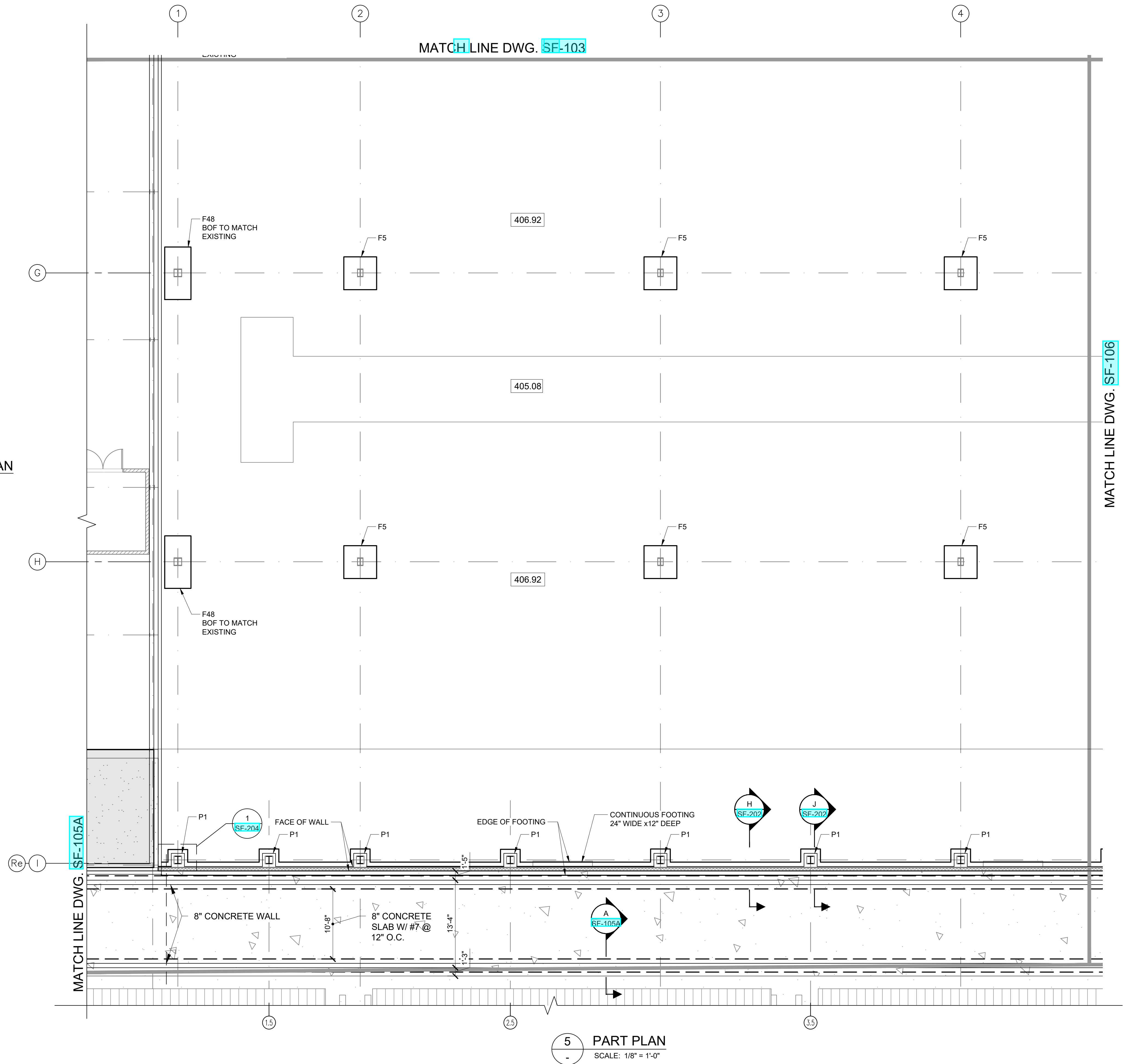
REV	DESCRIPTION	DATE
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3	REVISED BID SET	01.20.22
4	ISSUED FOR CONSTRUCTION	04.04.22

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CHECKED BY : I. BEER
APPROVED BY : I. BEER
DATE : 7/26/2021
SCALE : AS SHOWN
DRAWING TITLE :

FOUNDATION PART PLAN

DWG NUMBER : **SF-105B**
11 OF 31

20 DUNNIGAN
DRIVE



KEY PLAN
SCALE: N.T.S.

- NOTES:
1. FOR FOUNDATION SCHEDULE, SEE DWG. SF-203
 2. FOR PIER DETAILS, SEE DRAWING SF-204
 3. GENERAL EXCAVATION SUBGRADE ELEVATION BETWEEN FOUNDATION LOCATIONS IS NOTED BY NUMBERS SHOWN AS [XX.X]
 4. TOP OF PIER ELEVATIONS SHALL BE AT EL. 407.08.

FOOTING ELEVATIONS DWG. SF-105	
COLUMN LINE	BOT. OF FOOTING EL.
G2 TO G4	405.83
H2 TO H4	405.83
I2 TO I4	406.0

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DRAWN BY : D. CHECHURIN

CHECKED BY : I. BEER

APPROVED BY : I. BEER

DATE : 7/26/2021

SCALE : AS SHOWN

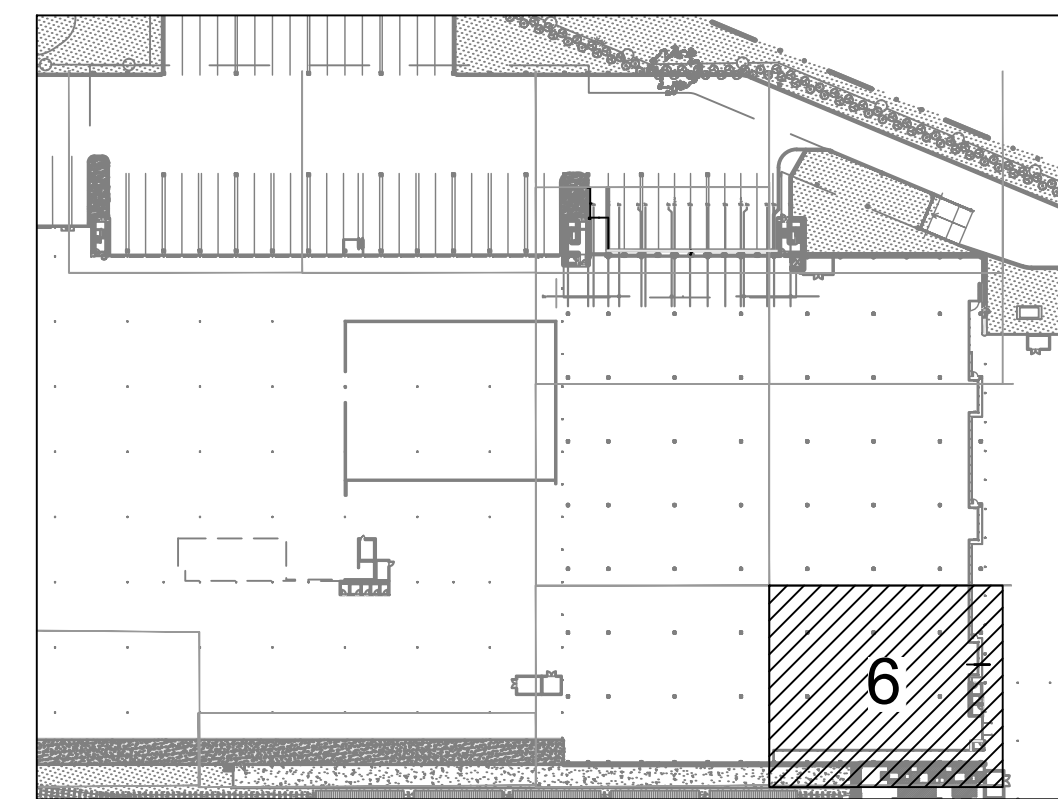
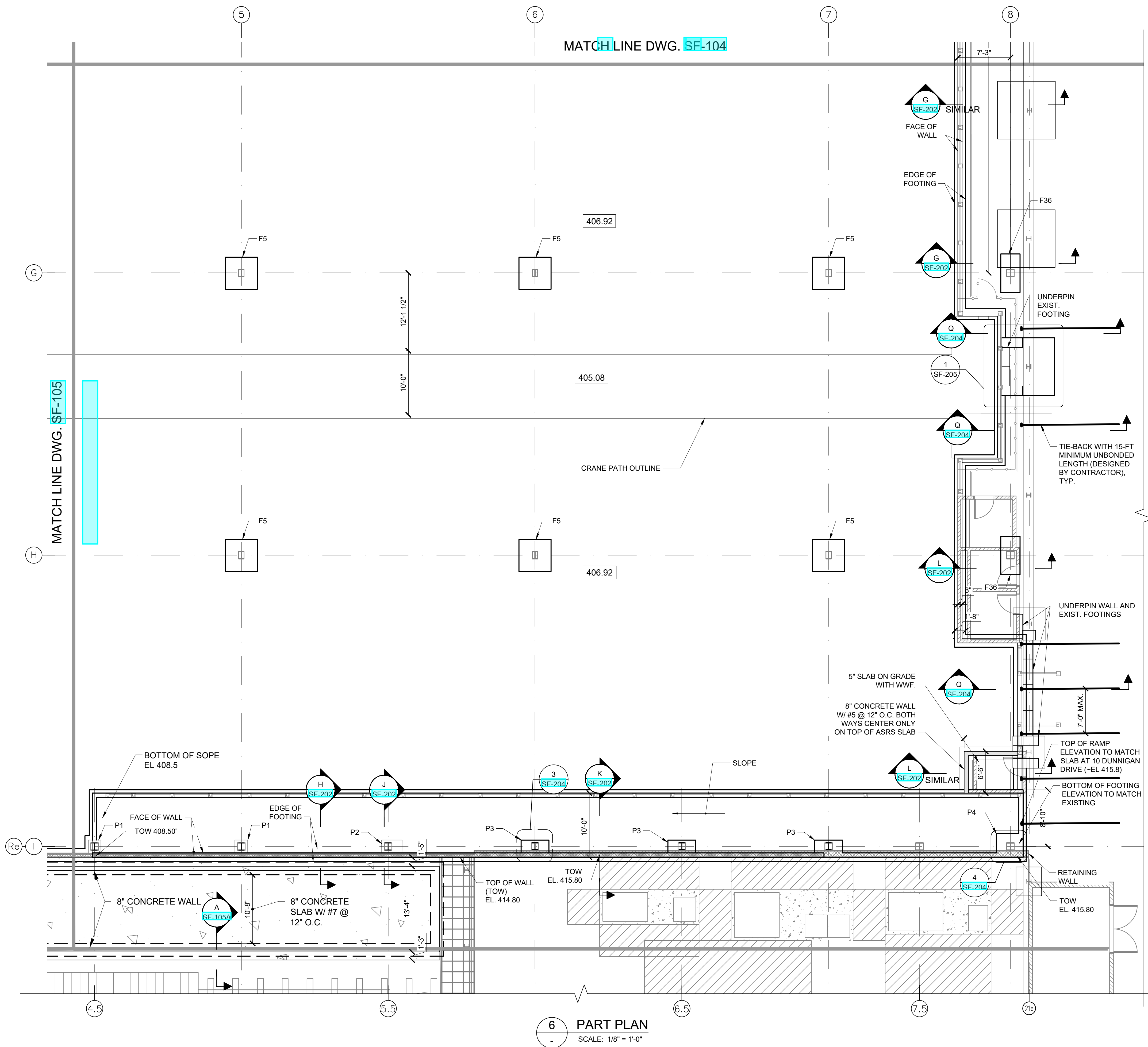
DRAWING TITLE : -

FOUNDATION PART PLAN



DWG NUMBER :
SF-105

9 OF 31



KEY PLAN
SCALE: N.T.S.

- NOTES:
1. FOR FOUNDATION SCHEDULE SEE DWG. [SF-203](#)
 2. FOR PIER DETAILS, SEE DRAWING [SF-204](#)
 3. GENERAL EXCAVATION SUBGRADE ELEVATION BETWEEN FOUNDATION LOCATIONS IS NOTED BY NUMBERS SHOWN AS [XX.X](#)
 4. TOP OF PIER ELEVATIONS SHALL BE AT EL. 407.08.

FOOTING ELEVATIONS DWG. SF-104	
COLUMN LINE	BOT. OF FOOTING EL.
G5 TO G7	405.83
H5 TO H7	405.83
I - WEST OF RAMP	406.0
I - AT RAMP	405.5
8 LINE	SEE SECTIONS

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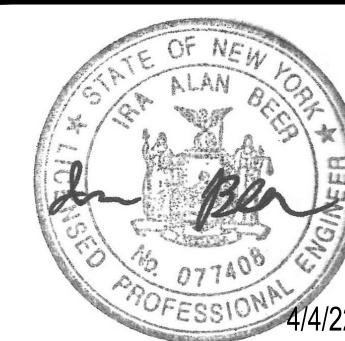
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3	REVISED BID SET	01.20.22
4	ISSUED FOR CONSTRUCTION	04.04.22

DRAWN BY : D. CHECHURIN
CHECKED BY : I. BEER
APPROVED BY : I. BEER
DATE : 7/26/2021
SCALE : AS SHOWN

DRAWING TITLE :

FOUNDATION PART PLAN



DWG NUMBER :
SF-106

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SCALE : AS SHOWN

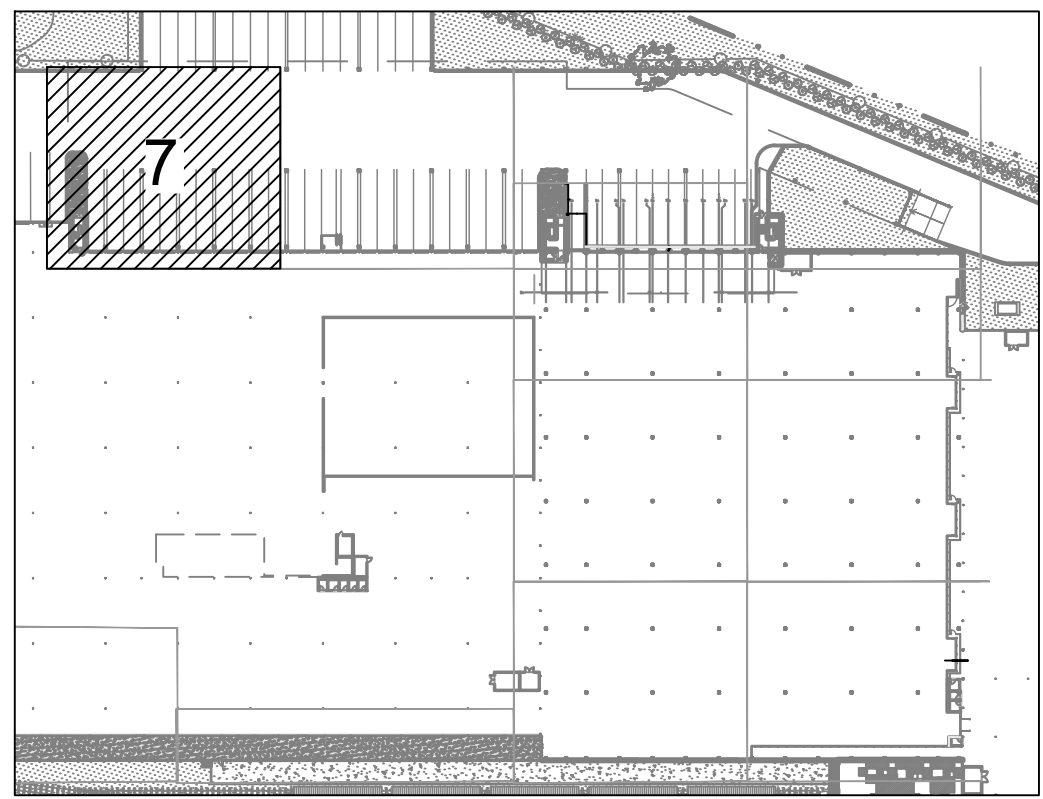
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FOUNDATION PART PLAN



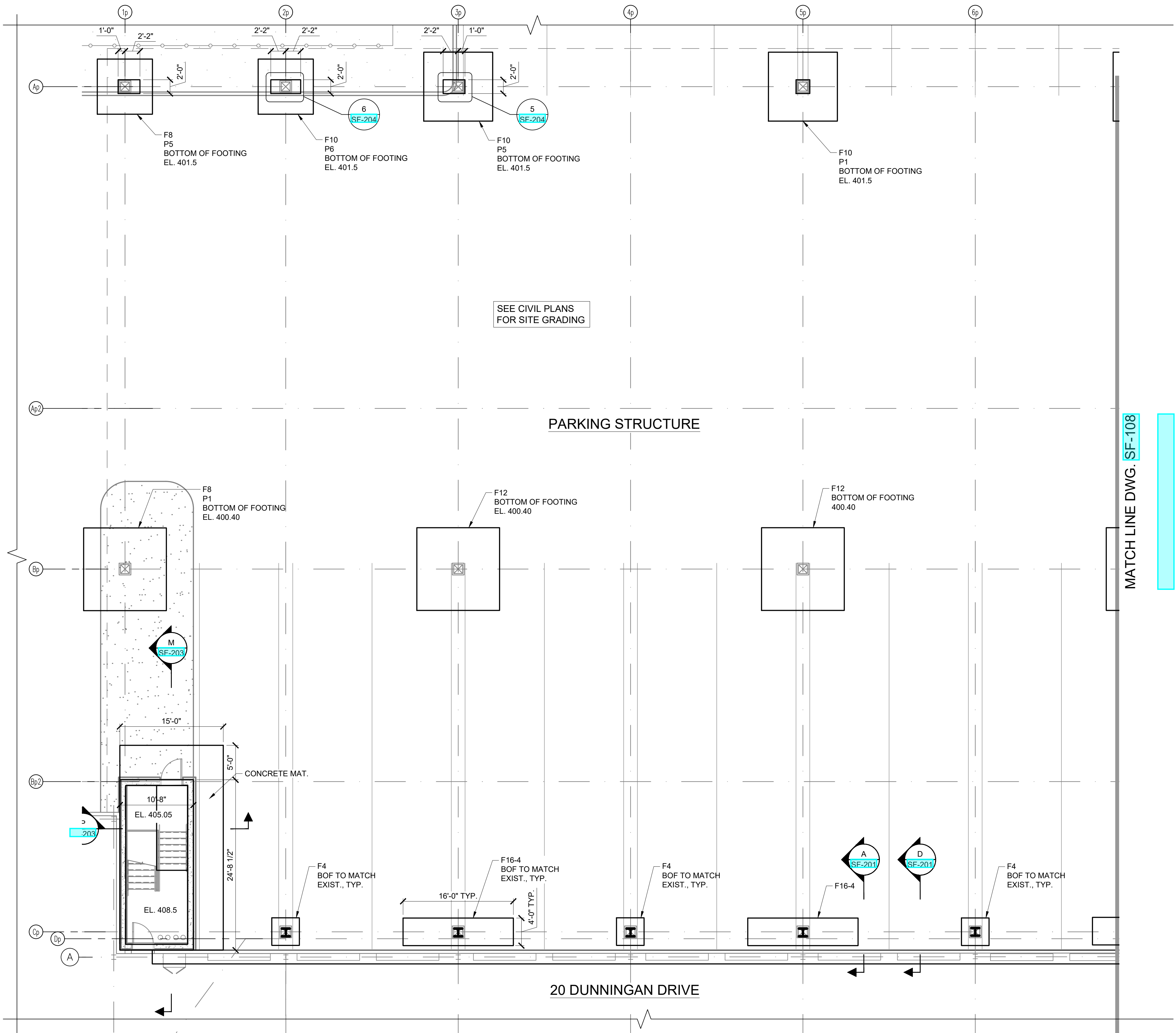
DWG NUMBER :
SF-107

13 OF 31



KEY PLAN
SCALE: N.T.S.

- NOTES:
1. TOPS OF PIERS ON LINE Ap TO BE EL 404.8
 2. FOR FOOTING SCHEDULE SEE DWG SF-203
 3. TOP OF FOOTINGS F14-4 TO BE EL 404.50 (Cp LINE)



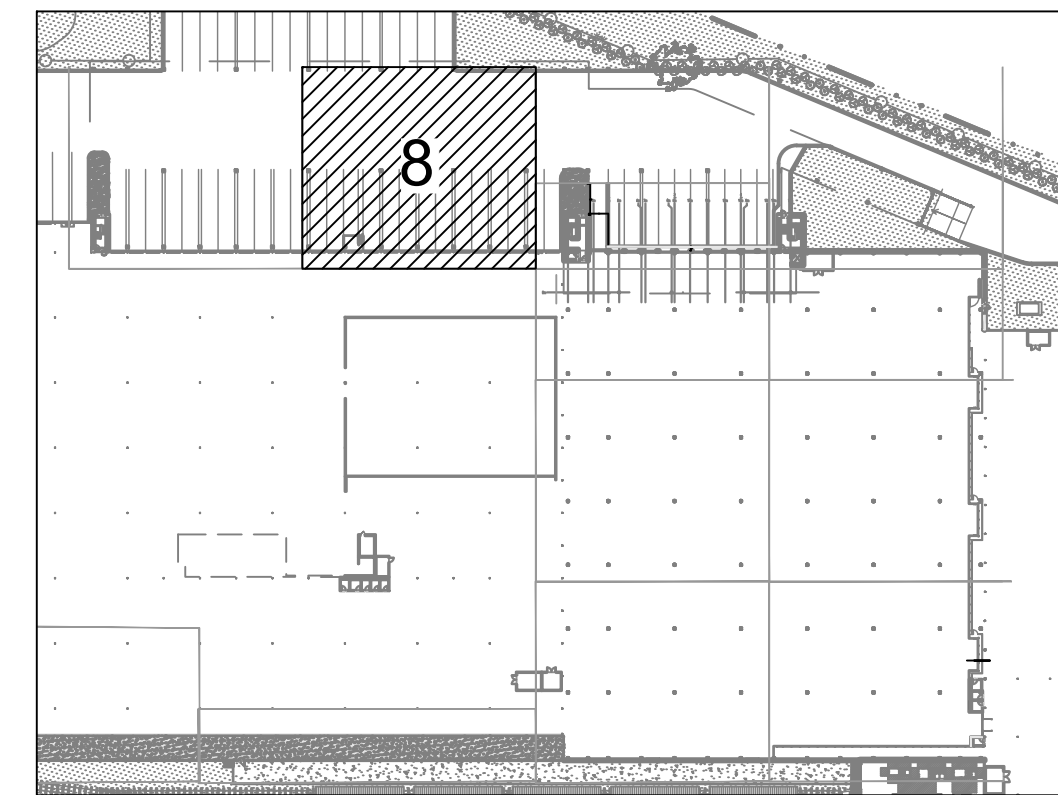
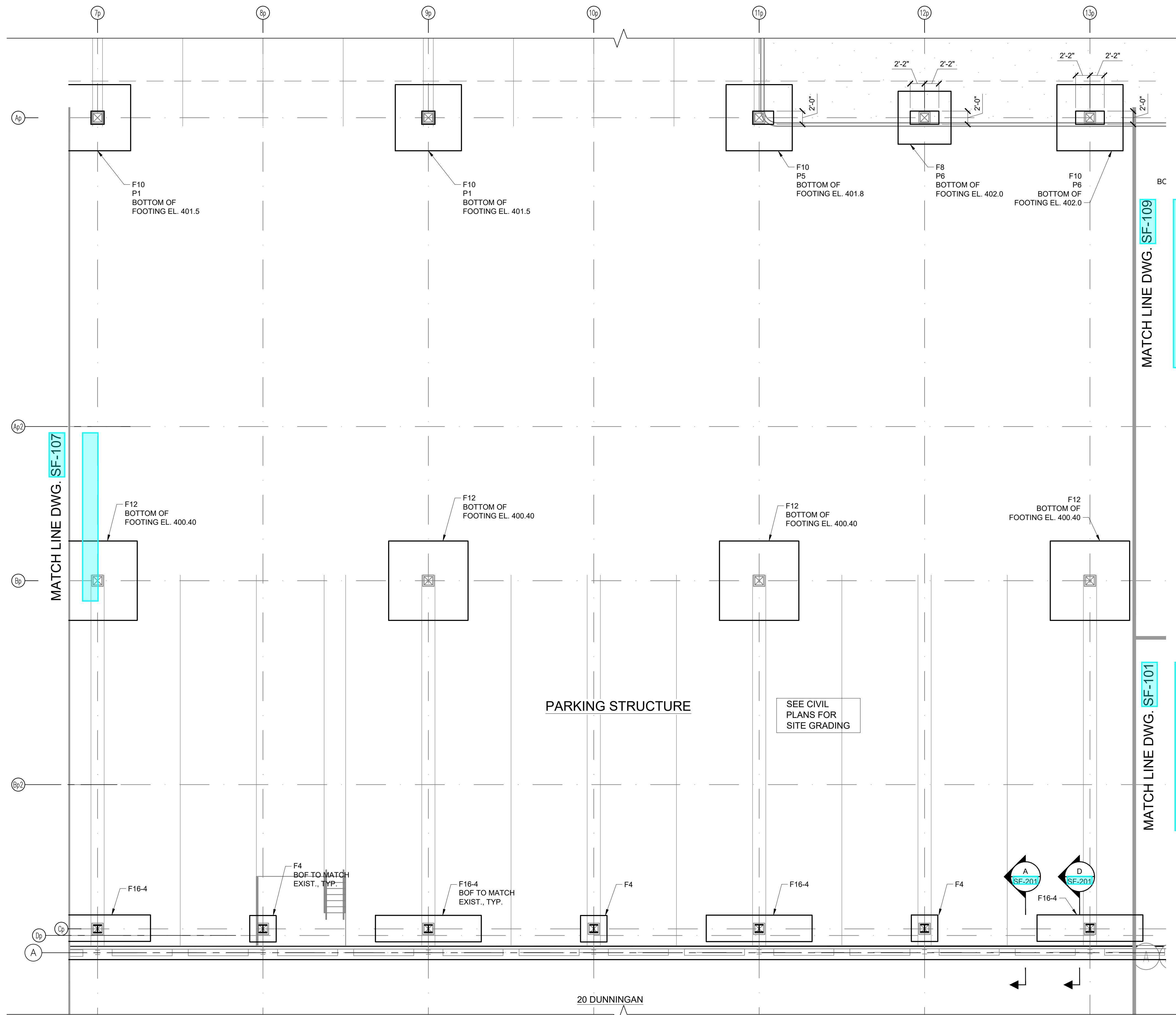
SEE CIVIL PLANS
FOR SITE GRADING

PARKING STRUCTURE

MATCH LINE DWG. SF-108

20 DUNNINGAN DRIVE

7 PART PLAN
SCALE: 1/8" = 1'-0"



- NOTES:
1. TOPS OF PIERS ON LINE Ap TO BE EL 404.8
 2. FOR FOOTING SCHEDULE SEE DWG [SF-203](#)
 3. TOP OF FOOTINGS F14-4 TO BE EL 404.50 (Cp LINE)

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DRAWN BY : D. CHECHURIN
CHECKED BY : I. BEER
APPROVED BY : I. BEER
DATE : 7/26/2021
SCALE : AS SHOWN

DRAWING TITLE : -

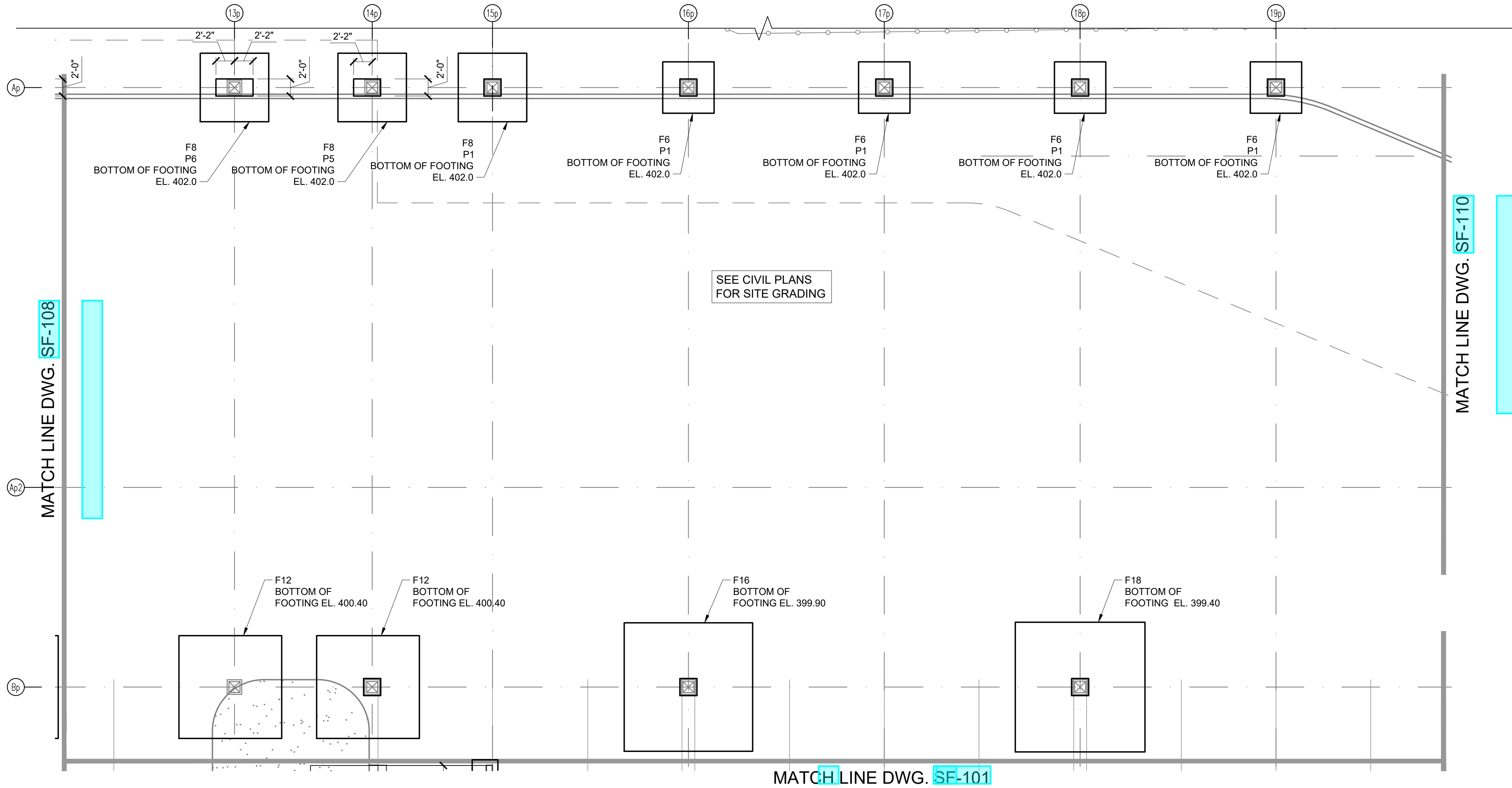
FOUNDATION PART PLAN



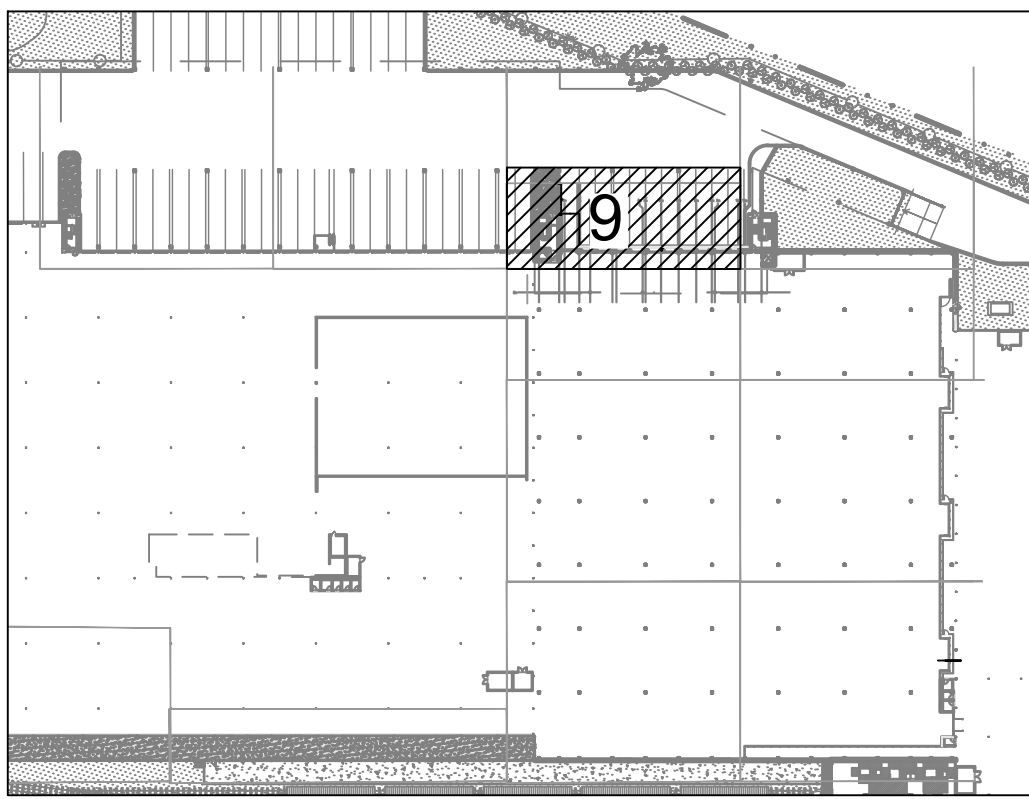
DWG NUMBER :
SF-108

14 OF 31

8 PART PLAN
SCALE: 1/8" = 1'-0"



9 PART PLAN
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

- NOTES:
1. TOPS OF PIERS ON LINE Ap TO BE EL 405.0

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SCALE : AS SHOWN

DRAWING TITLE :

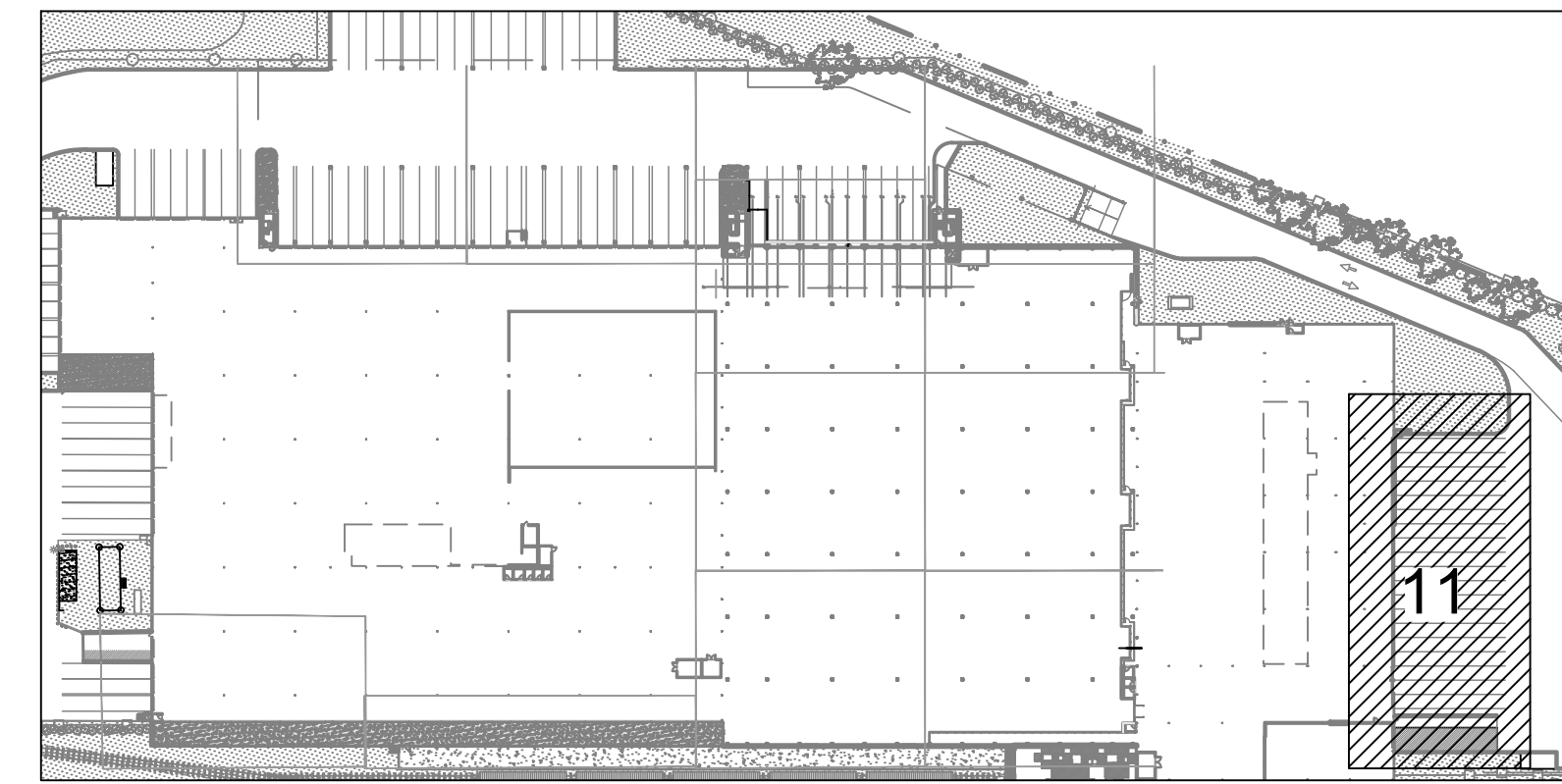
FOUNDATION PART PLAN



DWG NUMBER :
SF-109

15 OF 31

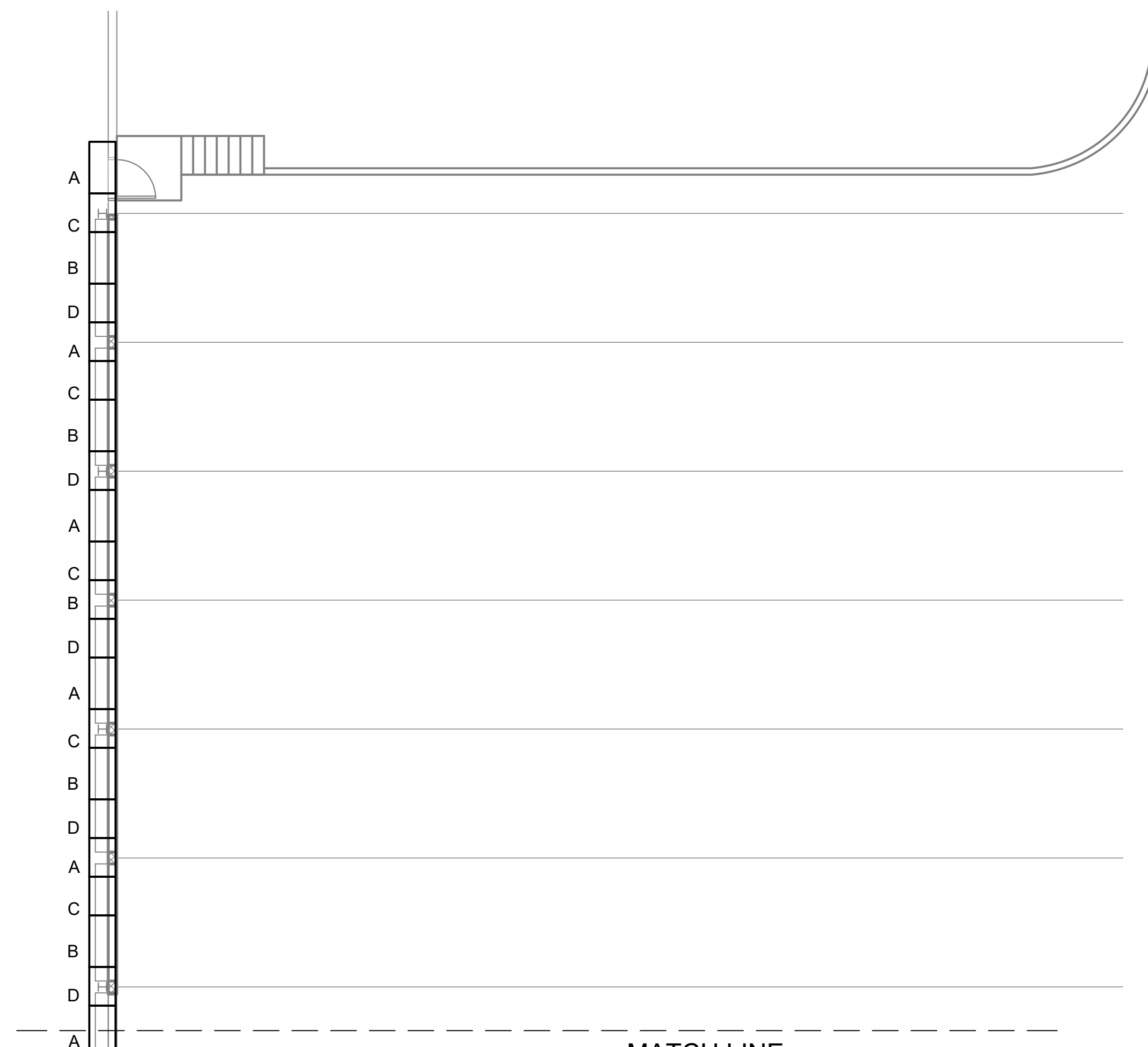
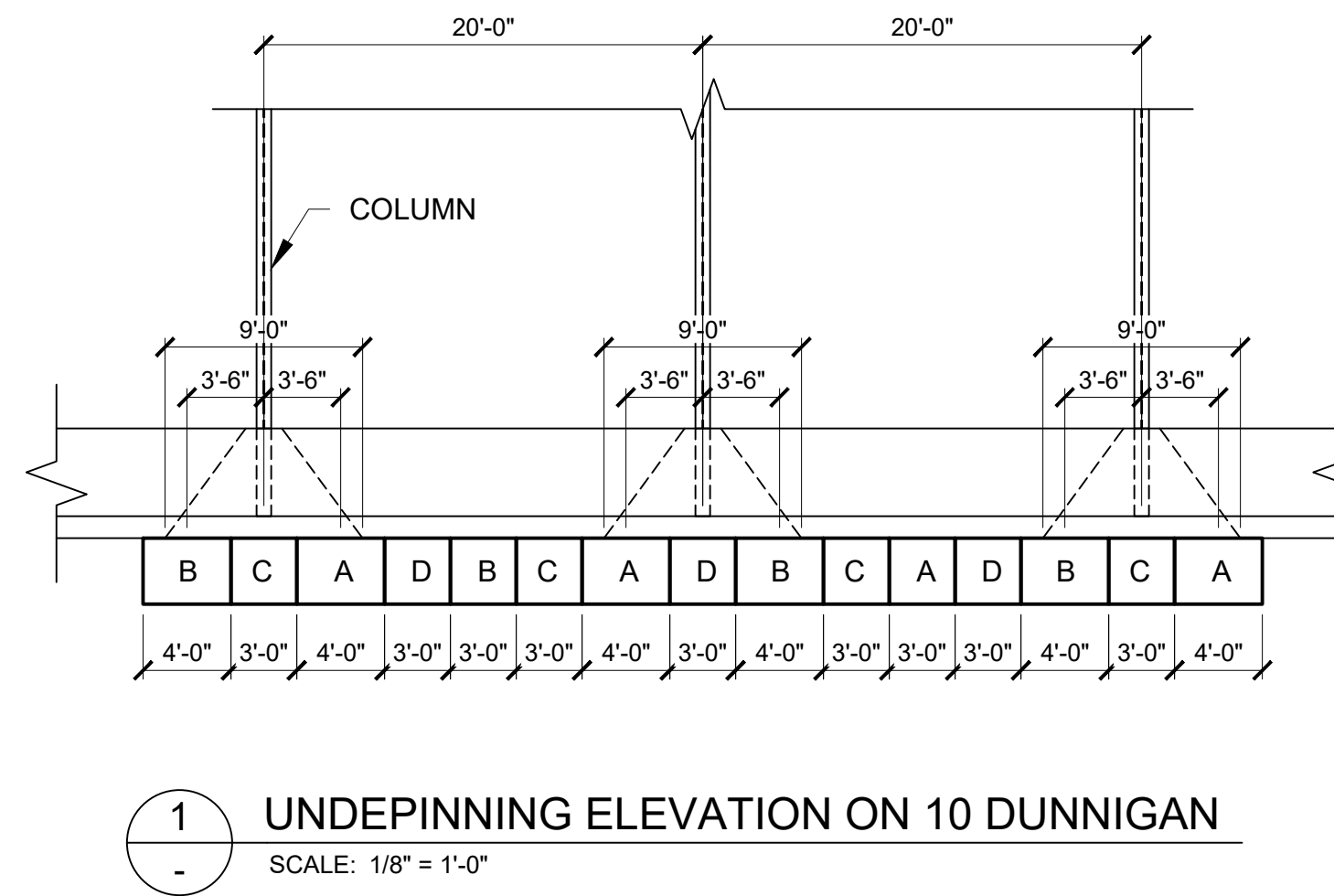
MATCH LINE



NOTES:

1. FOR FOUNDATION SCHEDULE, SEE DWG **SF-203**
2. FOR PIER DETAILS, SEE DRAWING **SF-204**
3. GENERAL EXCAVATION SUBGRADE ELEVATION BETWEEN FOUNDATION LOCATIONS IS NOTED BY NUMBERS SHOWN AS
4. TOP OF PIER ELEVATIONS SHALL BE AT EL. 407.08.

FOOTING ELEVATIONS DWG. SF-105	
COLUMN LINE	BOT. OF FOOTING EL.
G2 TO G4	405.83
H2 TO H4	405.83
I2 TO I4	406.0



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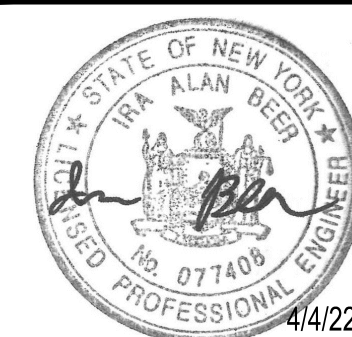
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SCALE :	AS SHOWN

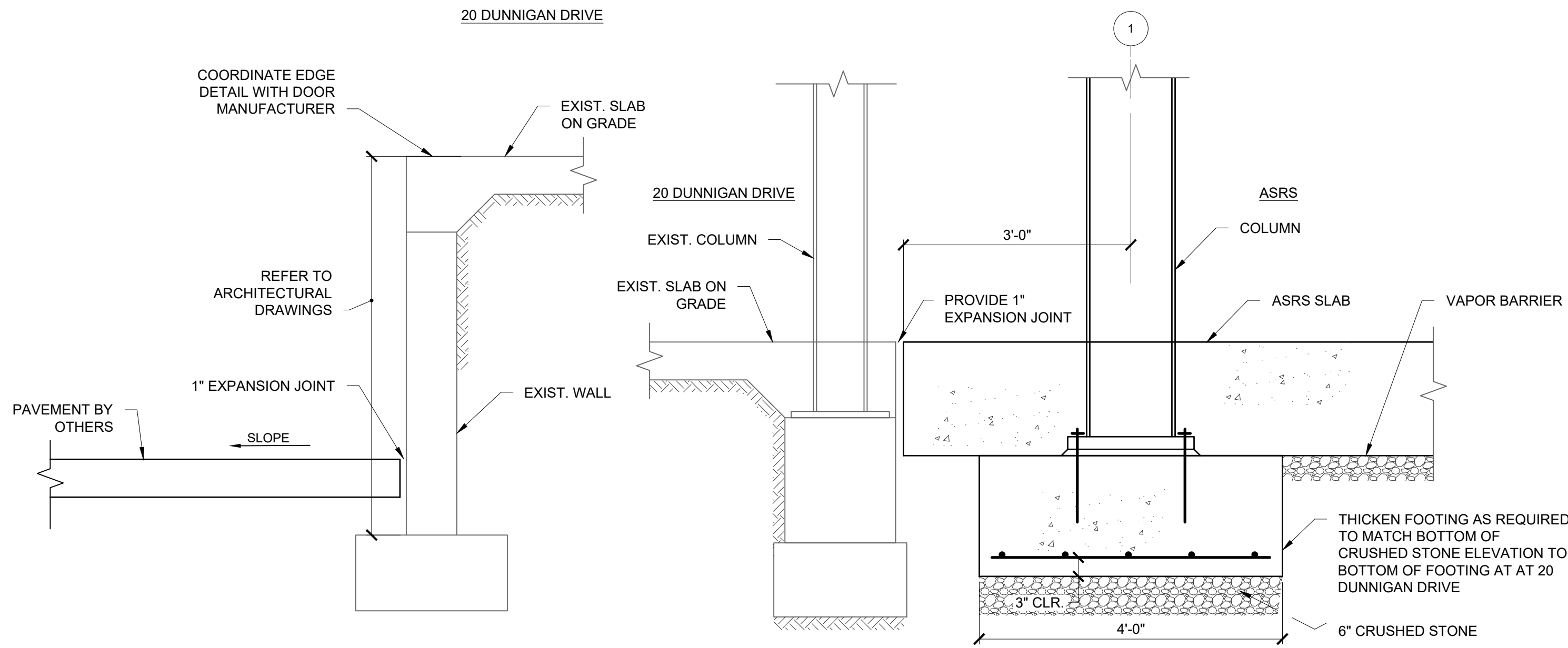
DRAWING TITLE: -

FOUNDATION PART PLAN



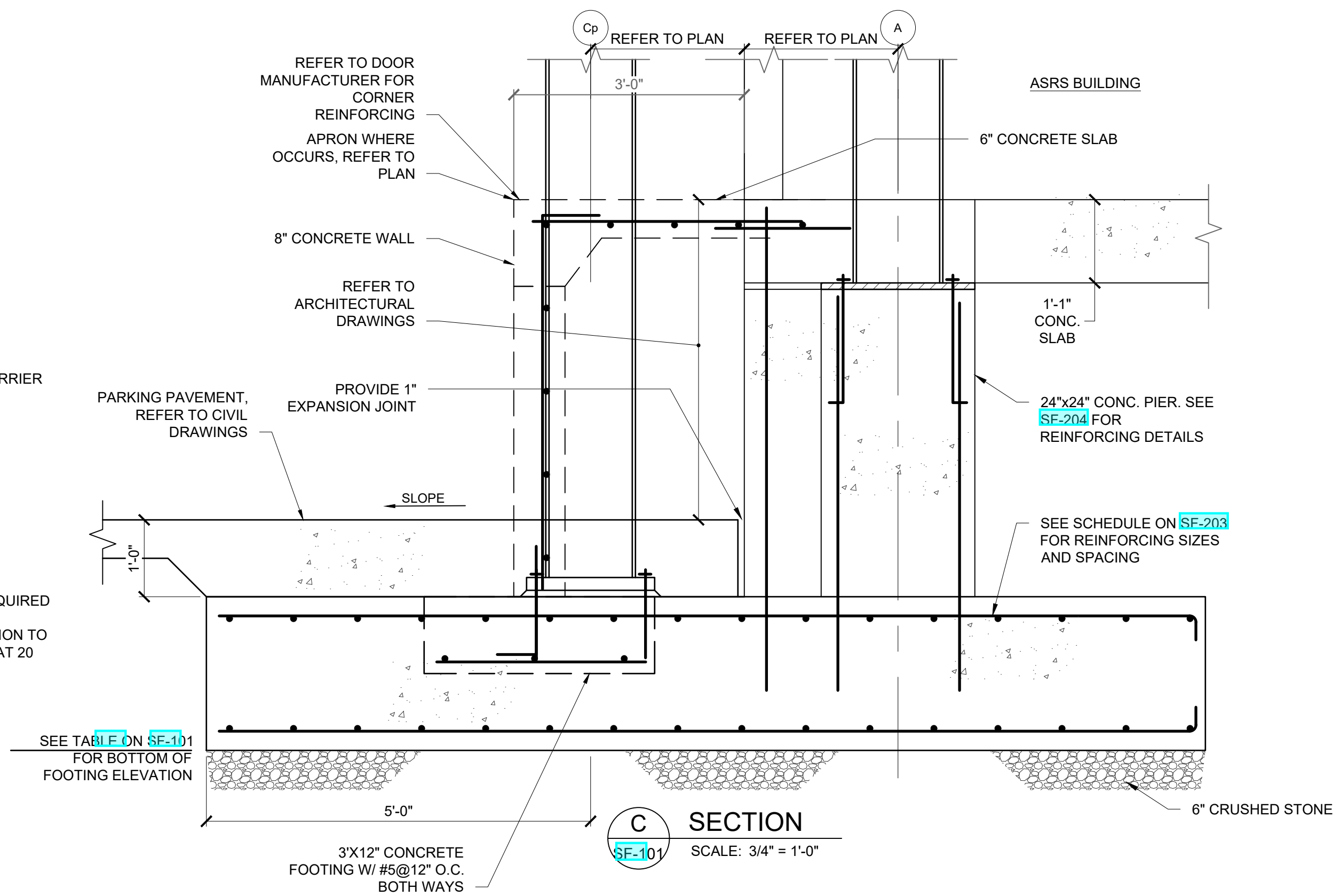
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SF-111

17 OF 31

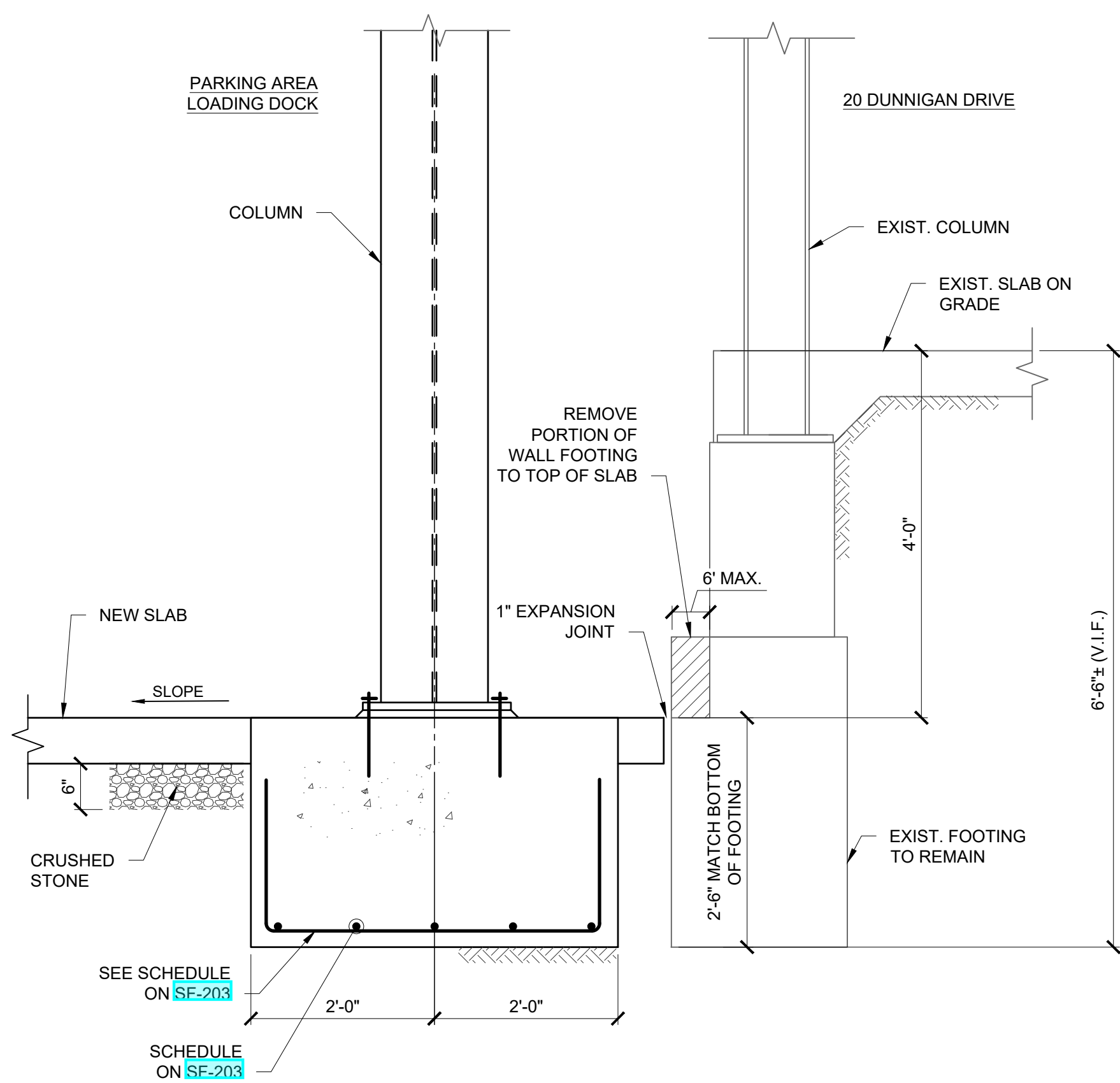


A SECTION
SCALE: 3/4" = 1'-0"

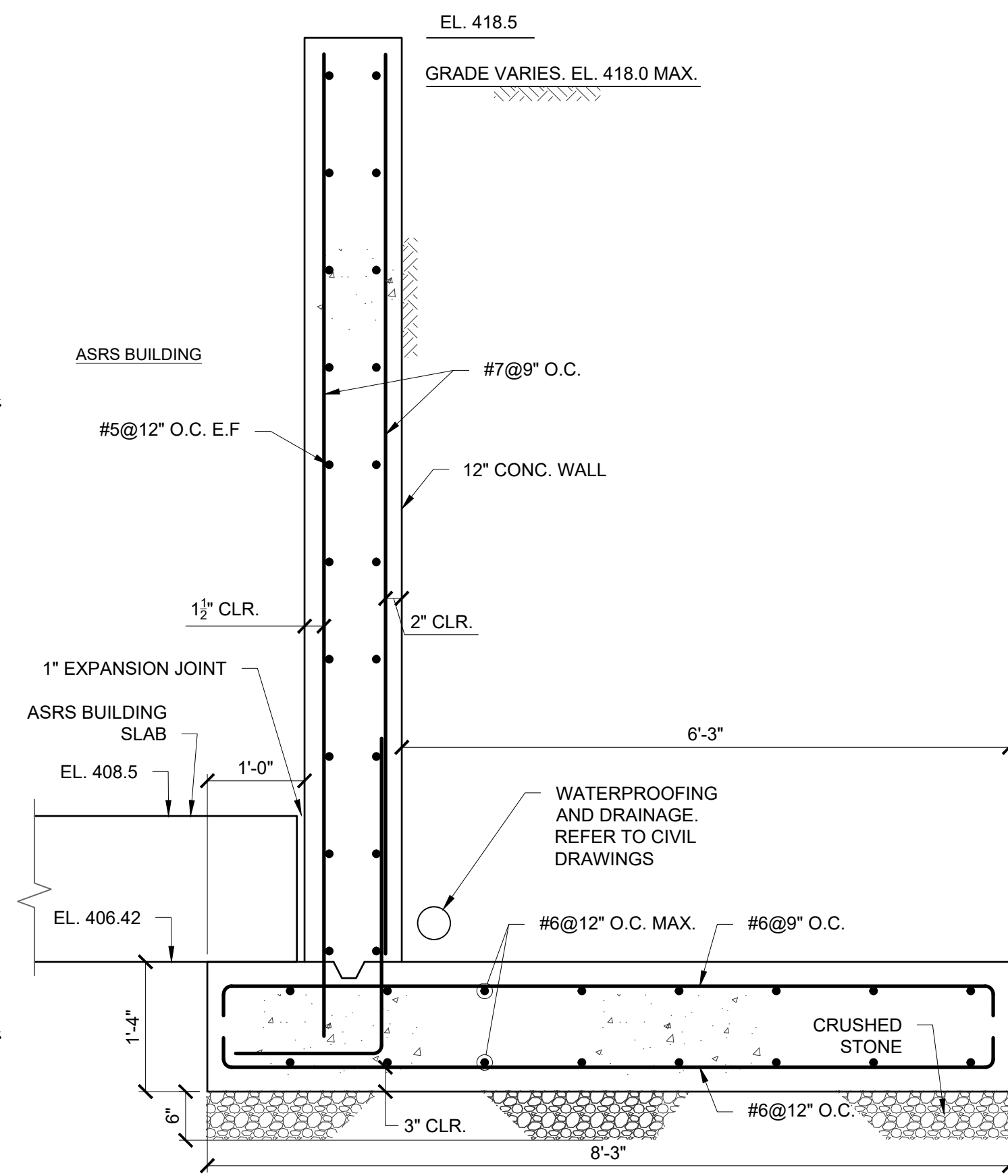
B SECTION
SCALE: 3/4" = 1'-0"



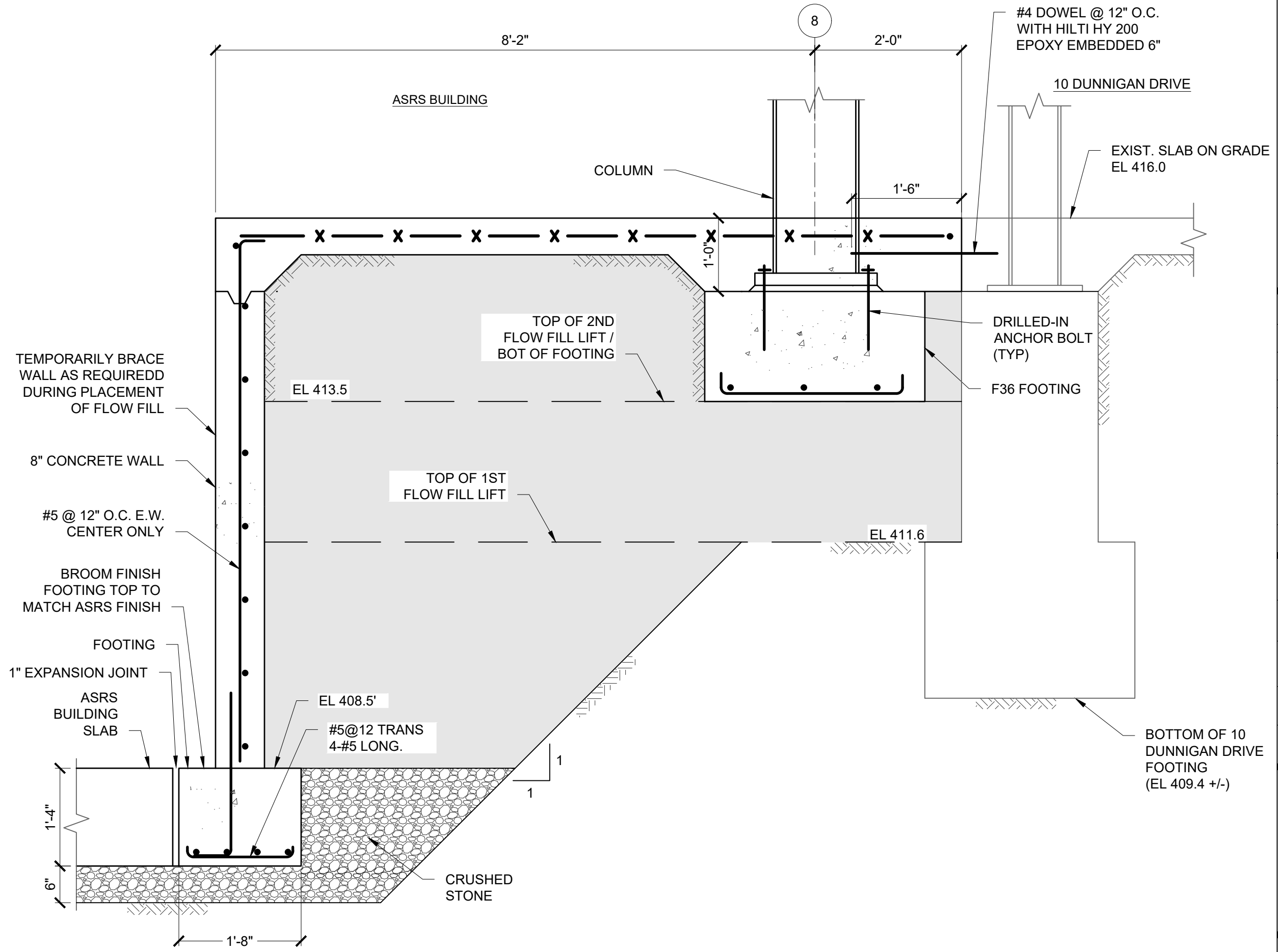
C SECTION
SCALE: 3/4" = 1'-0"



D SECTION
SCALE: 3/4" = 1'-0"



E SECTION
SCALE: 3/4" = 1'-0"



F SECTION
SCALE: 3/4" = 1'-0"

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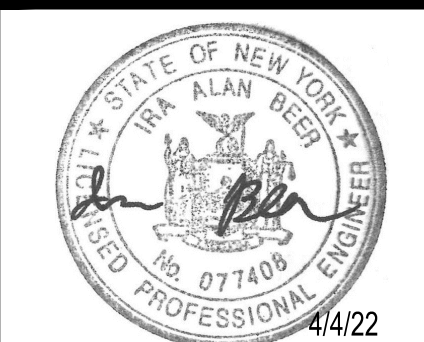
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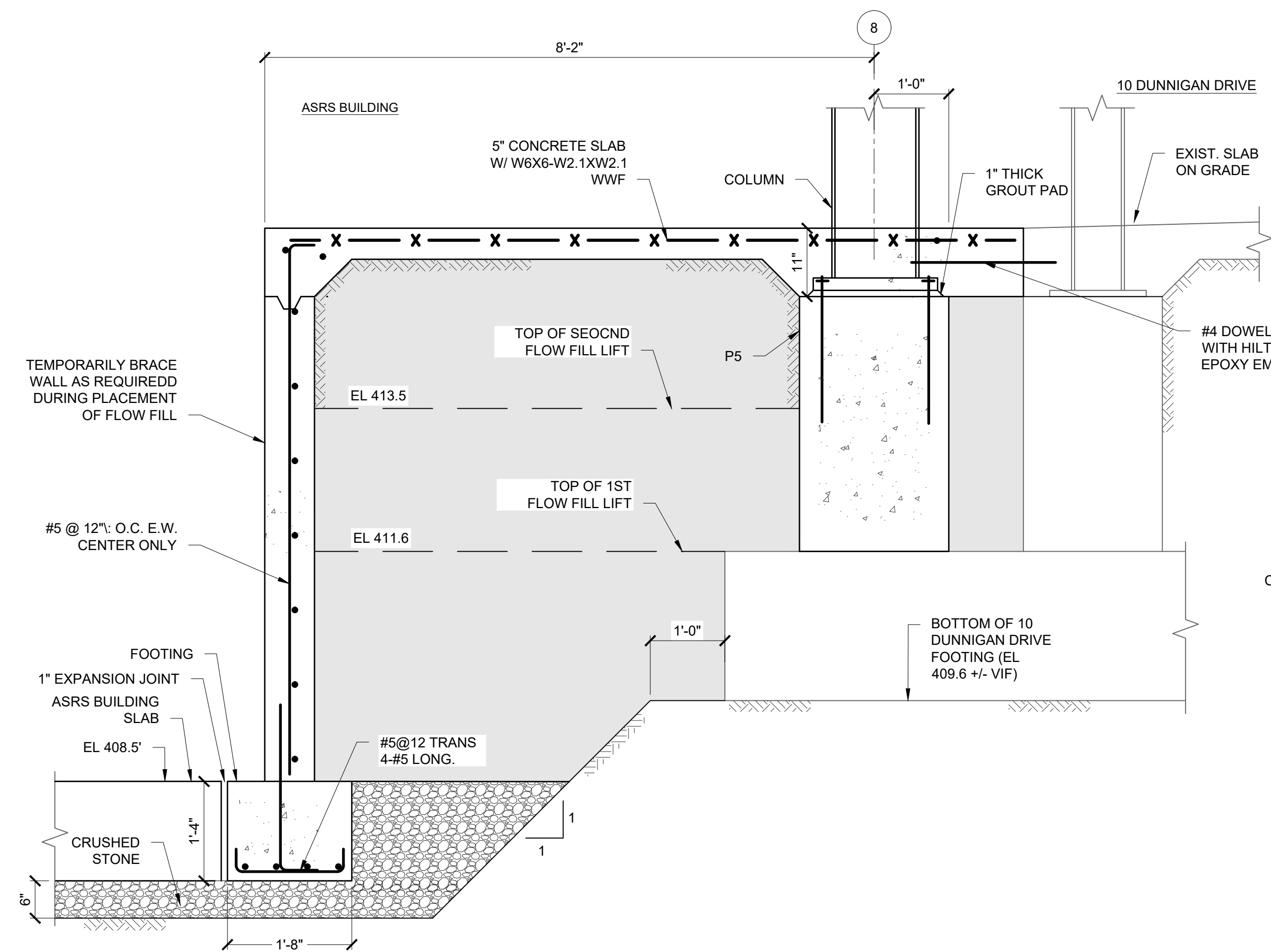
SECTIONS AND DETAILS



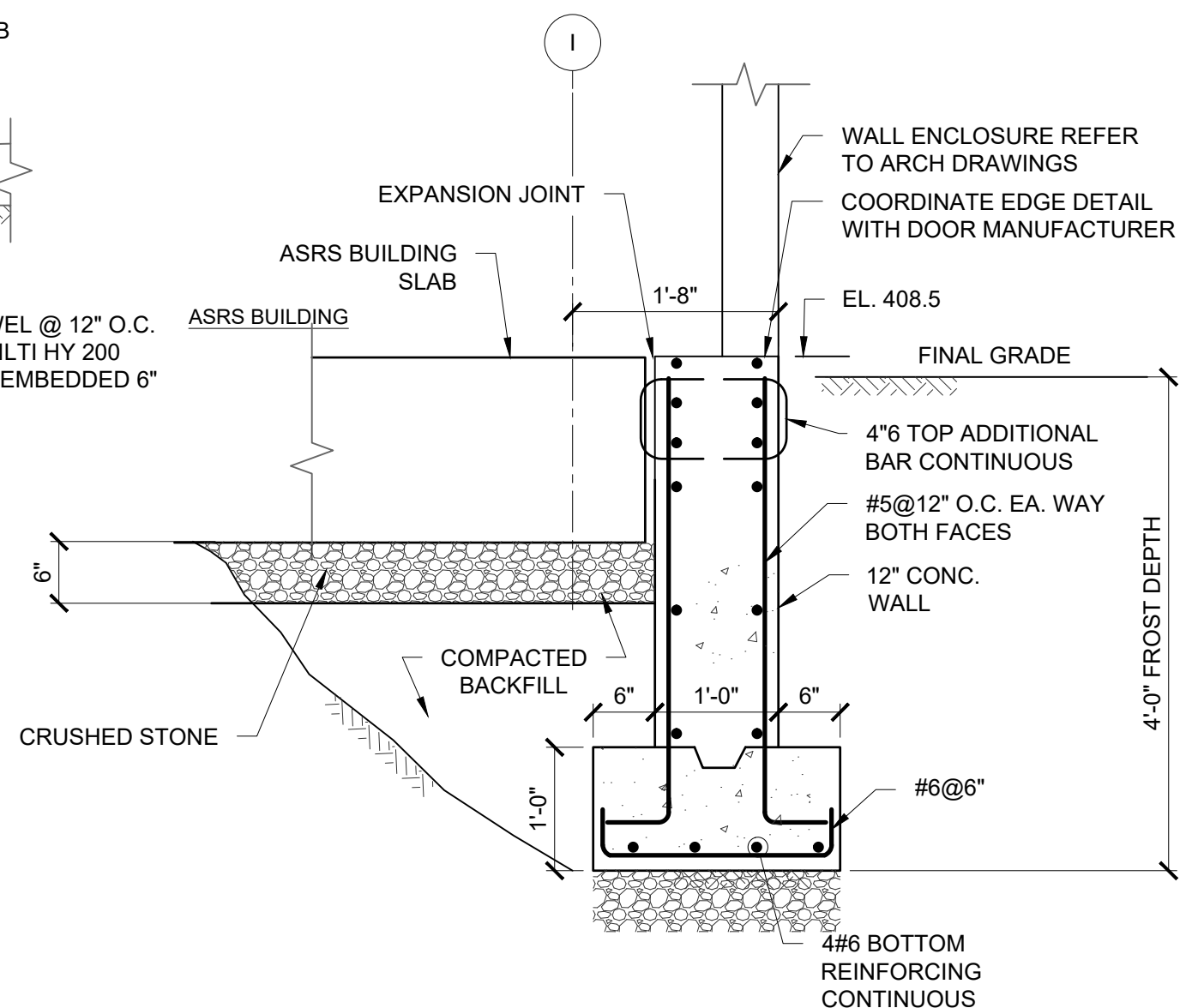
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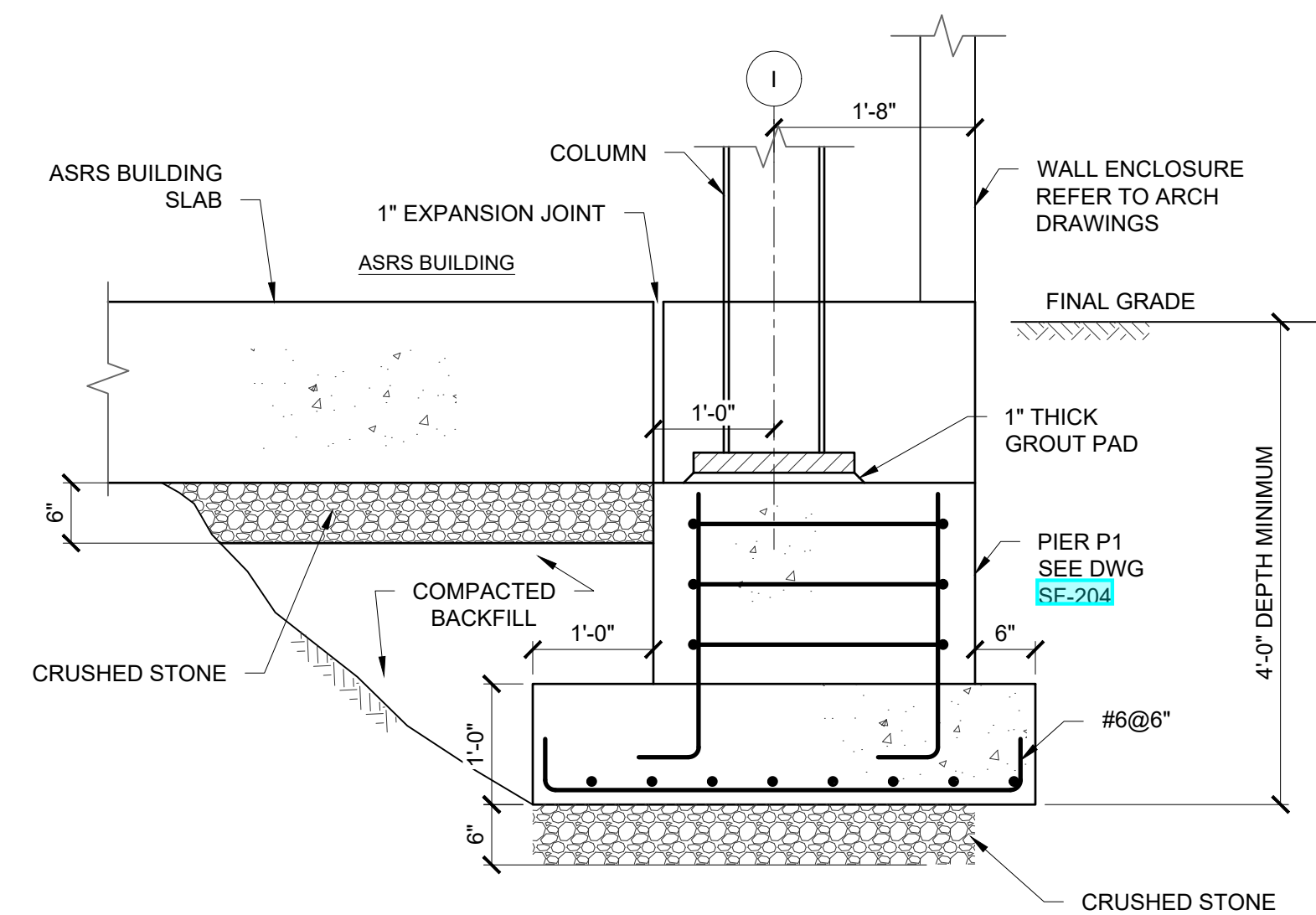
18 OF 31



G SECTION
SCALE: 3/4" = 1'-0"

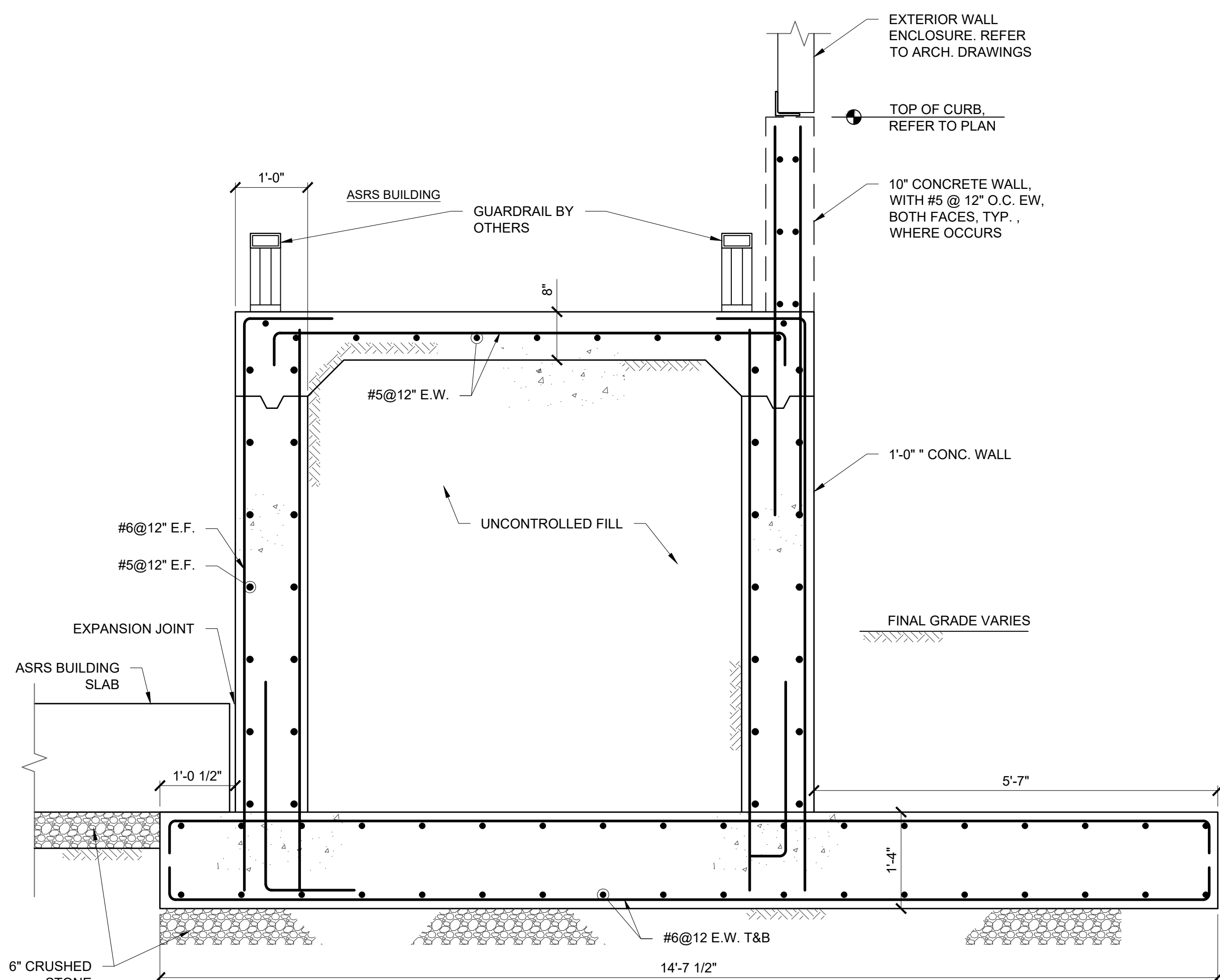


H SECTION
SCALE: 3/4" = 1'-0"

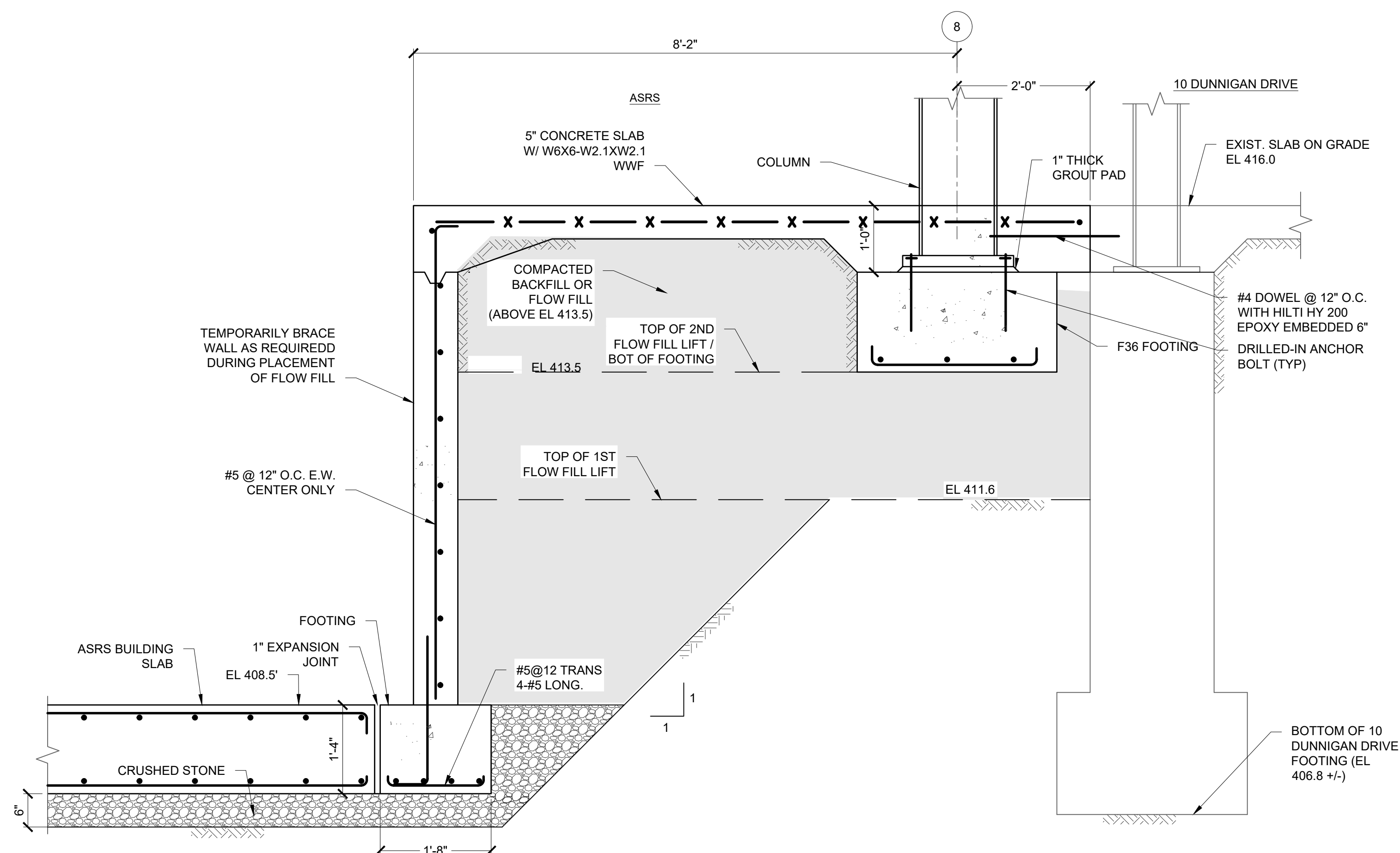


J SECTION
SCALE: 3/4" = 1'-0"

REFER TO DRAWING **S-203**
FOR PIER REINFORCING



K SECTION
SCALE: 3/4" = 1'-0"



L SECTION
SCALE: 3/4" = 1'-0"

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DRAWING TITLE :

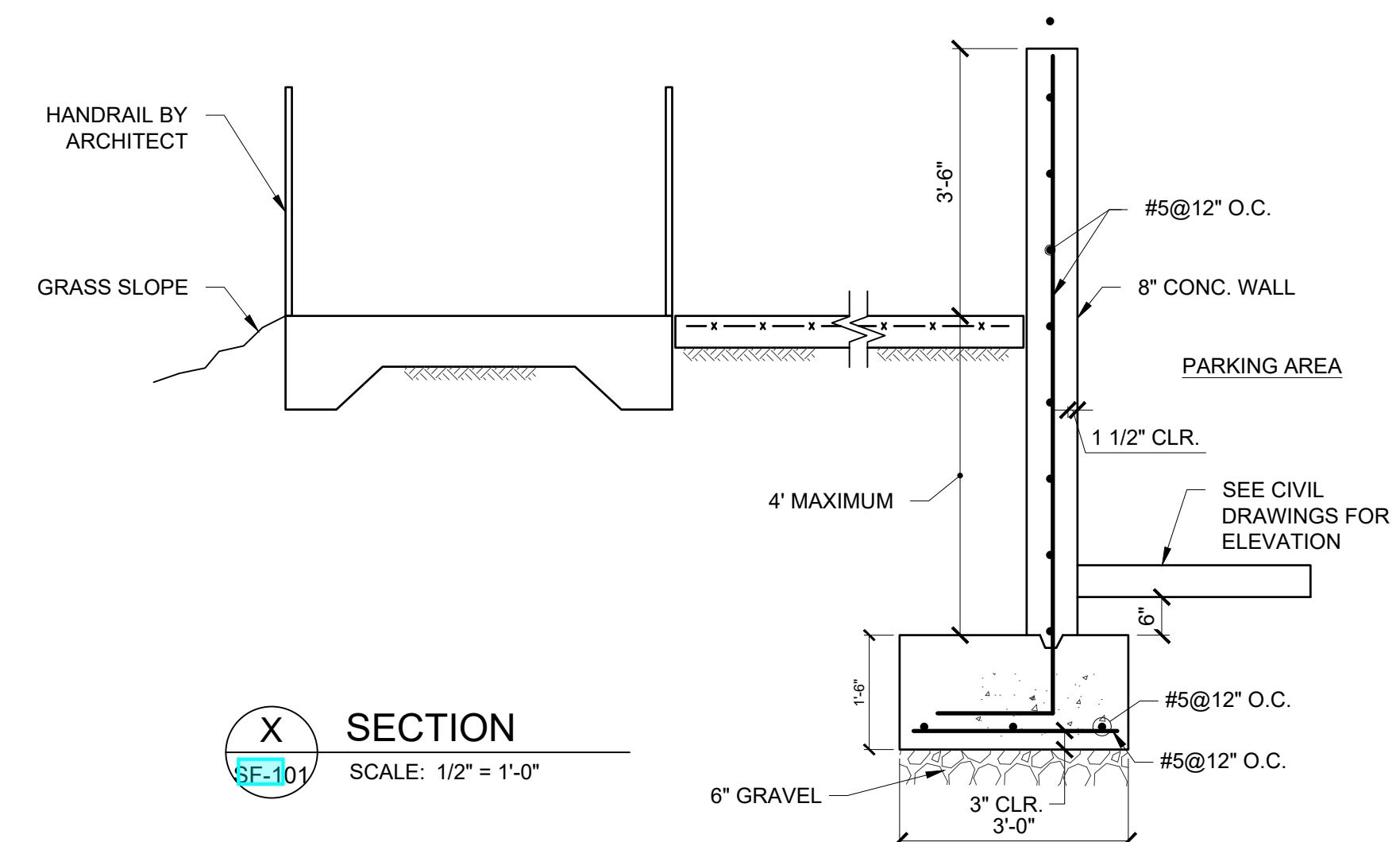
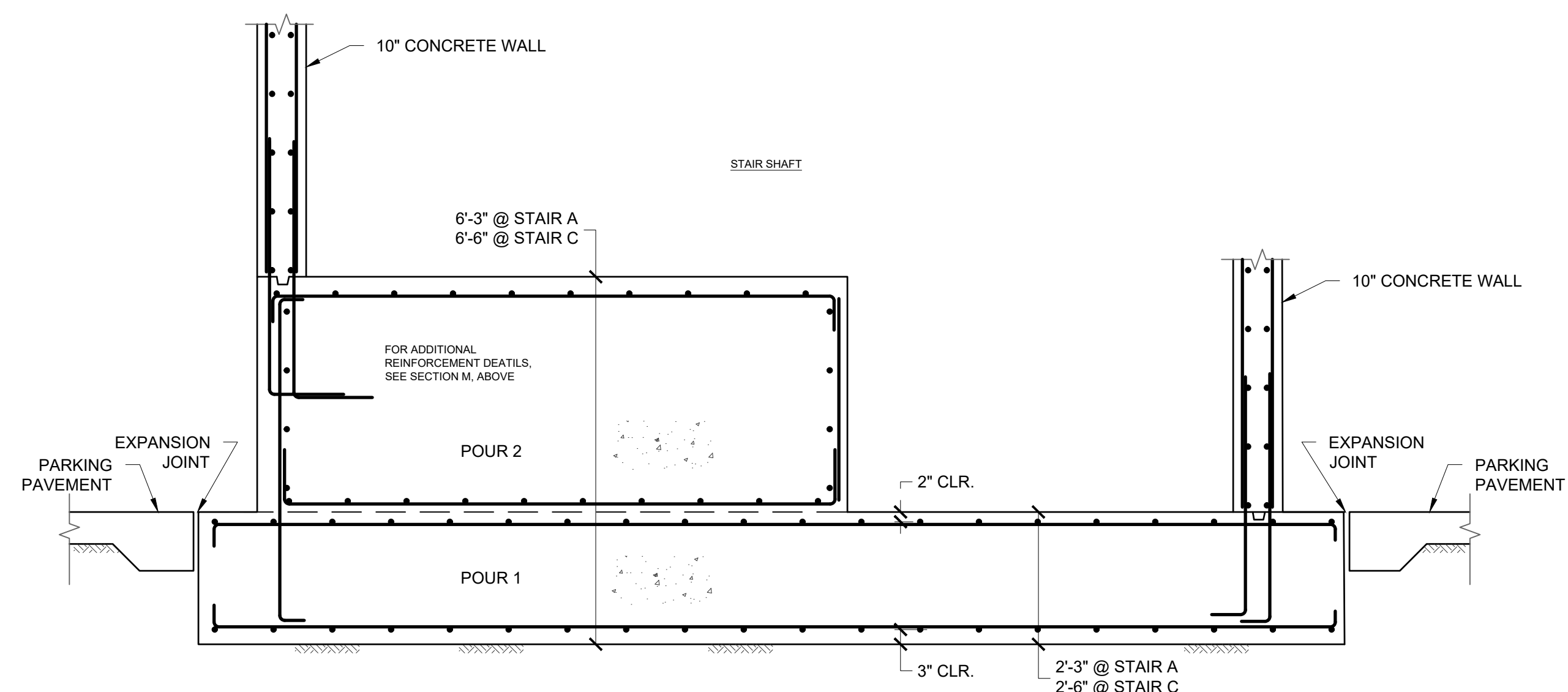
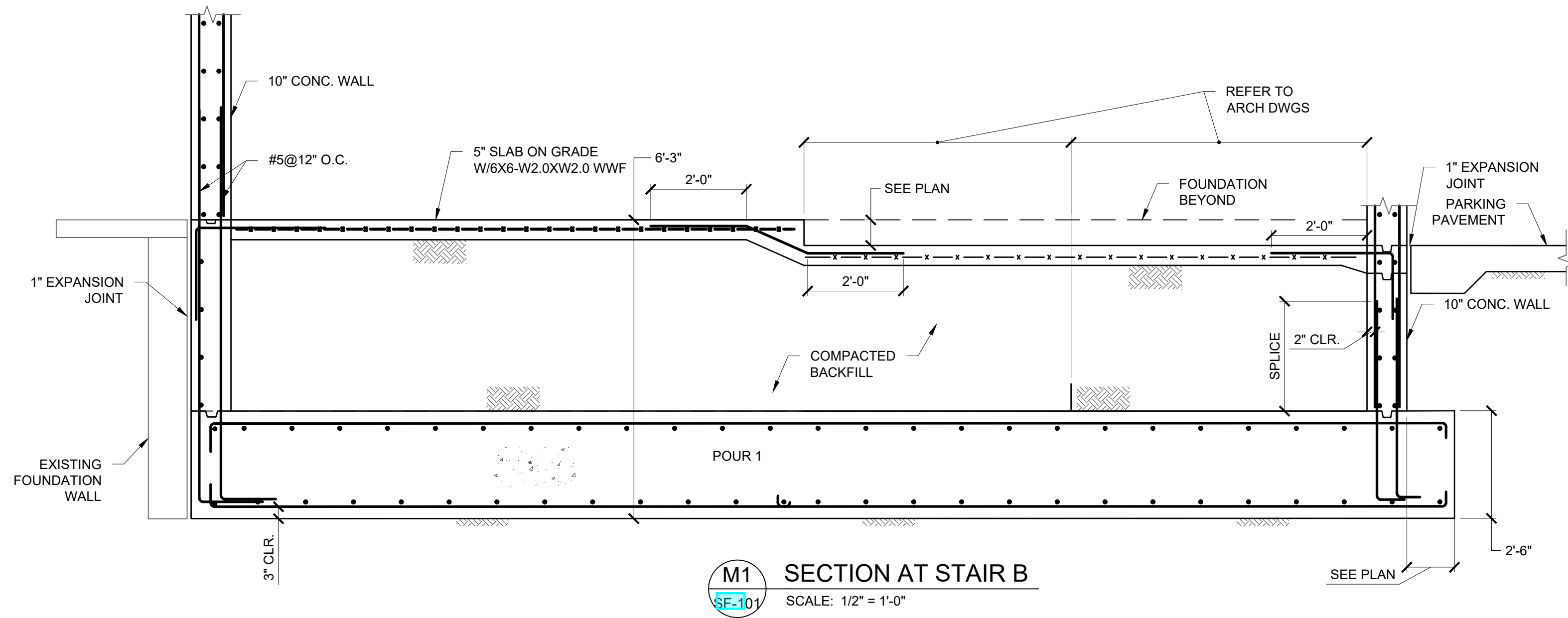
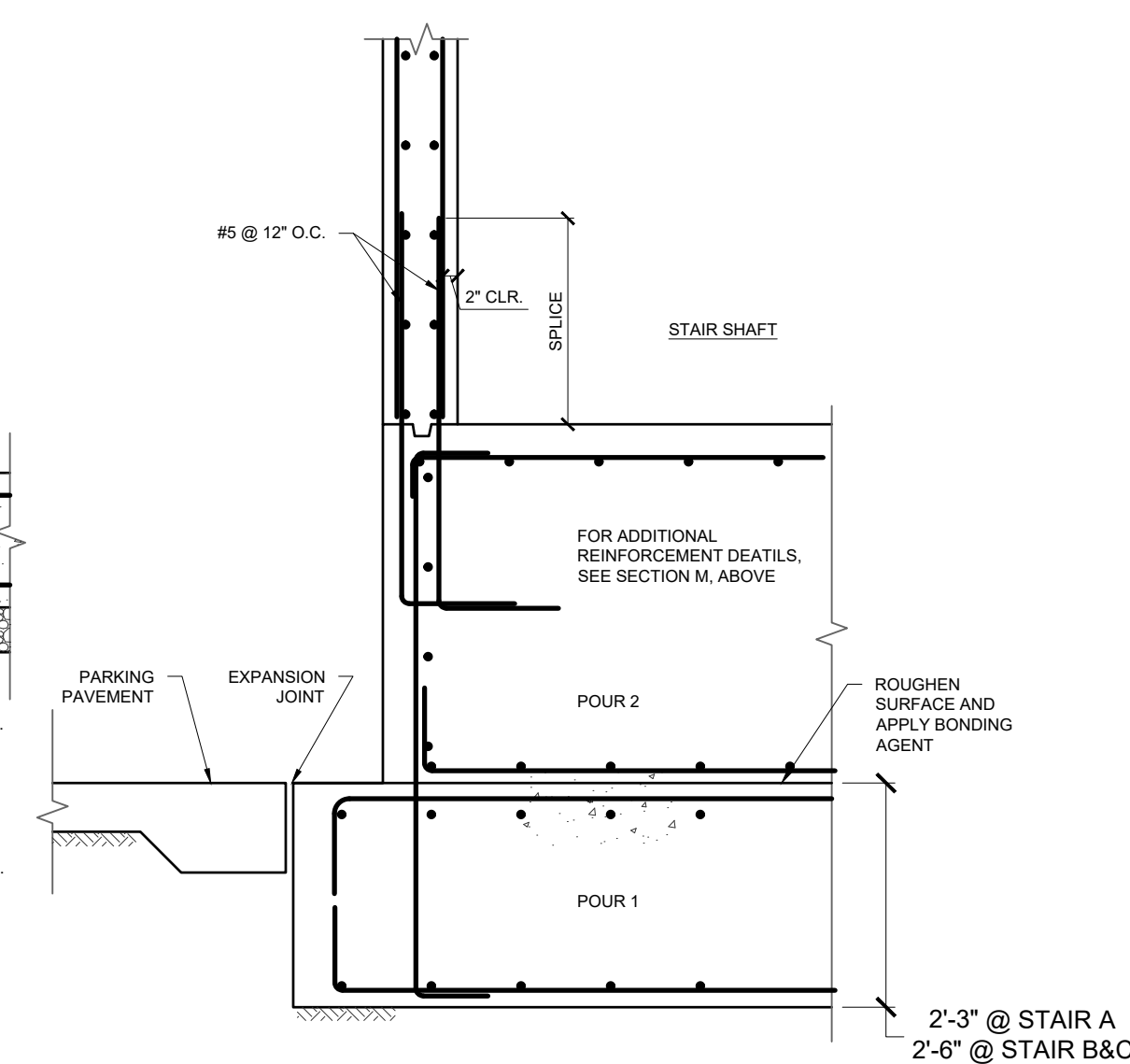
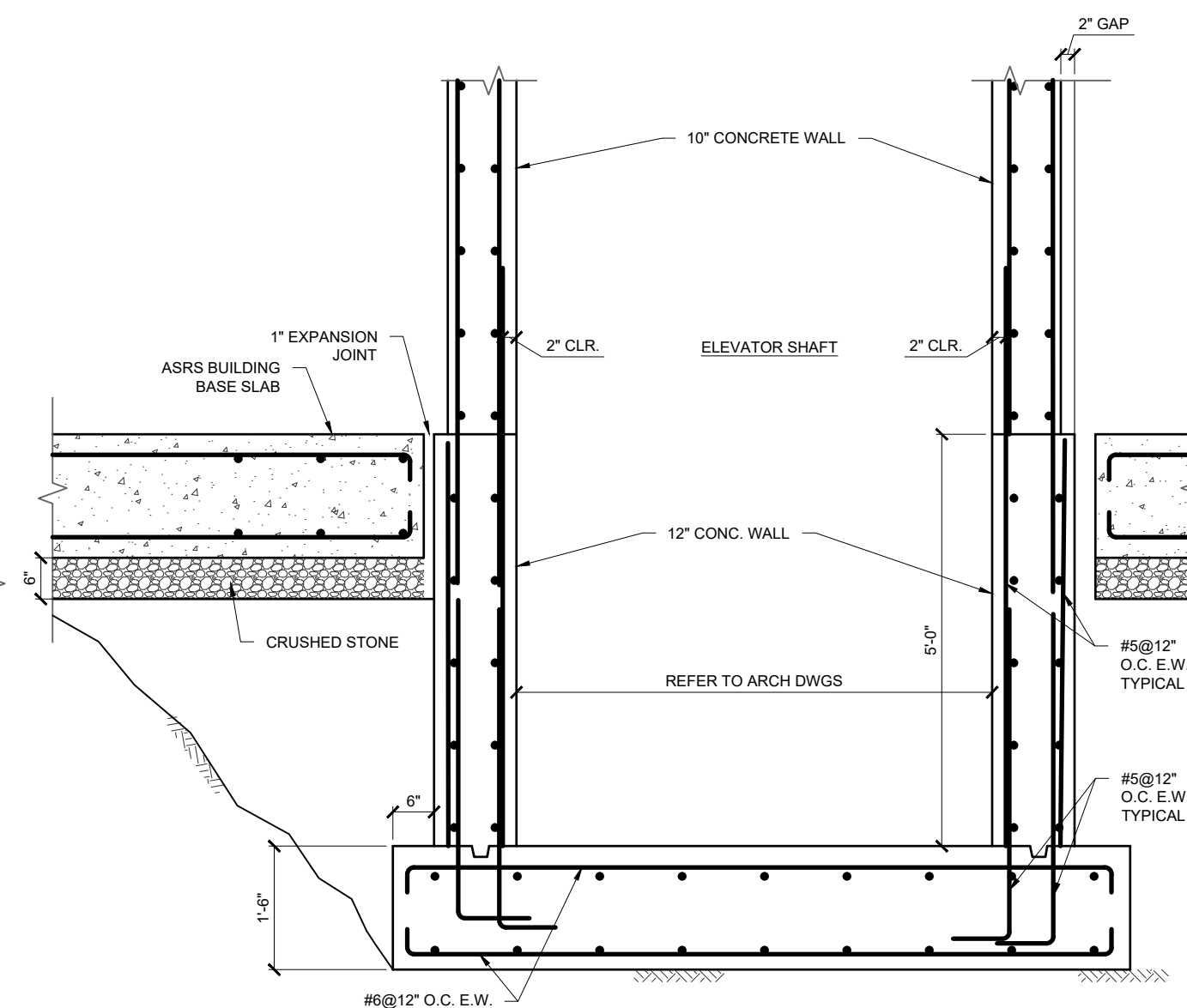
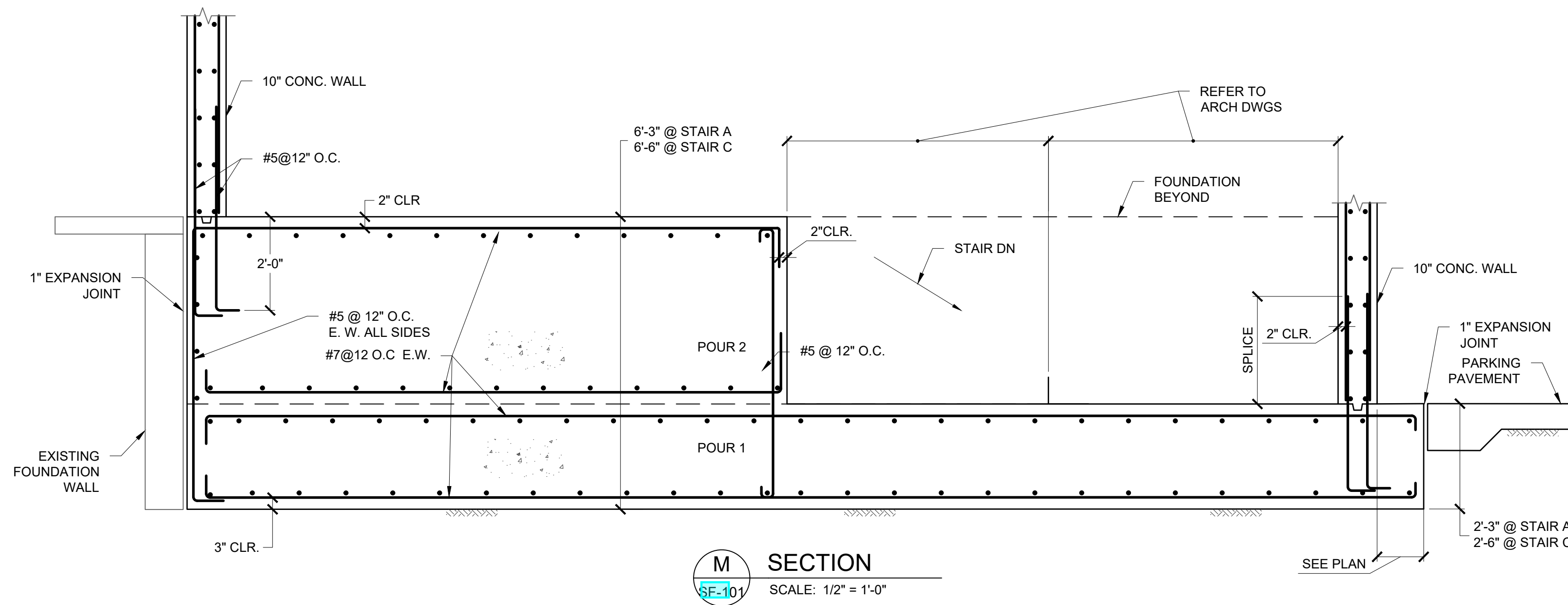
SECTIONS AND DETAILS



DWG NUMBER :

SF-202

19 OF 31



ALL ASRS FOOTING SHALL BE 5000 PSI CONCRETE

FOOTING#	DIMENSIONS			BOTTOM REINFORCING	
	LENGTH		THICKNESS	REINFORCEMENT	
	EW	NS	FT	EW	NS
	FT	FT	FT		
F4	4'-0"	4'-0"	1'-6"	7#6	7#6
F5	5'-0"	5'-0"	1'-6"	8#6	8#6
F7	7'-0"	7'-0"	1'-6"	8#8	8#8
F36	3'-0"	6'-0"	1'-6"	8#6	4#6
F48	4'-0"	8'-0"	1'-6"	9#6	5#6

ALL PARKING AREA FOOTINGS ARE 5950 PSI CONCRETE

FOOTING#	DIMENSIONS			BOTTOM REINFORCING	
	LENGTH		THICKNESS	REINFORCEMENT	
	EW	NS	FT	EW	NS
	FT	FT	FT		
F4	4'-0"	4'-0"	1'-6"	7#6	7#6
F5	5'-0"	5'-0"	1'-6"	8#6	8#6
F6	6'-0"	6'-0"	1'-6"	8#6	8#6
F8	8'-0"	8'-0"	2'-0"	10#8	10#8
F10	10'-0"	10'-0"	2'-6"	11#8	11#8
F12	12'-0"	12'-0"	3'-0"	14#8	14#8
F16	16'-0"	16'-0"	4'-0"	21#8	21#8
F18	18'-0"	18'-0"	4'-0"	26 #8	26#8
F10-12	10'-0"	12'-0"	2'-6"	12#8	12#8
F16-4	16'-0"	4'-0"	2'-6"	10#8	21#7

NOTE: FOR FOOTING F16-4,
PROVIDE 4 LEGS OF #3
STIRRUPS AT 10 " O.C.

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2	CD SET	11.30.21
3	REVISED BID SET	01.20.22
4	ISSUED FOR CONSTRUCTION	04.04.22

DRAWN BY : D. CHECHURIN

CHECKED BY : I. BEER

APPROVED BY : I. BEER

DATE : 7/26/2021

SCALE : AS SHOWN

DRAWING TITLE :

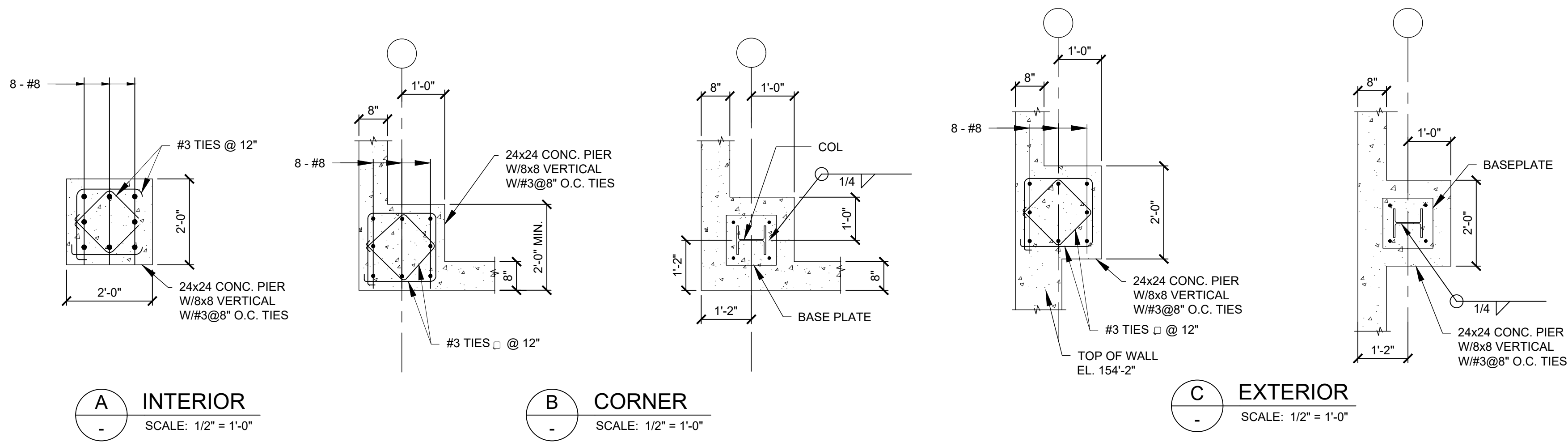
SECTIONS AND DETAILS



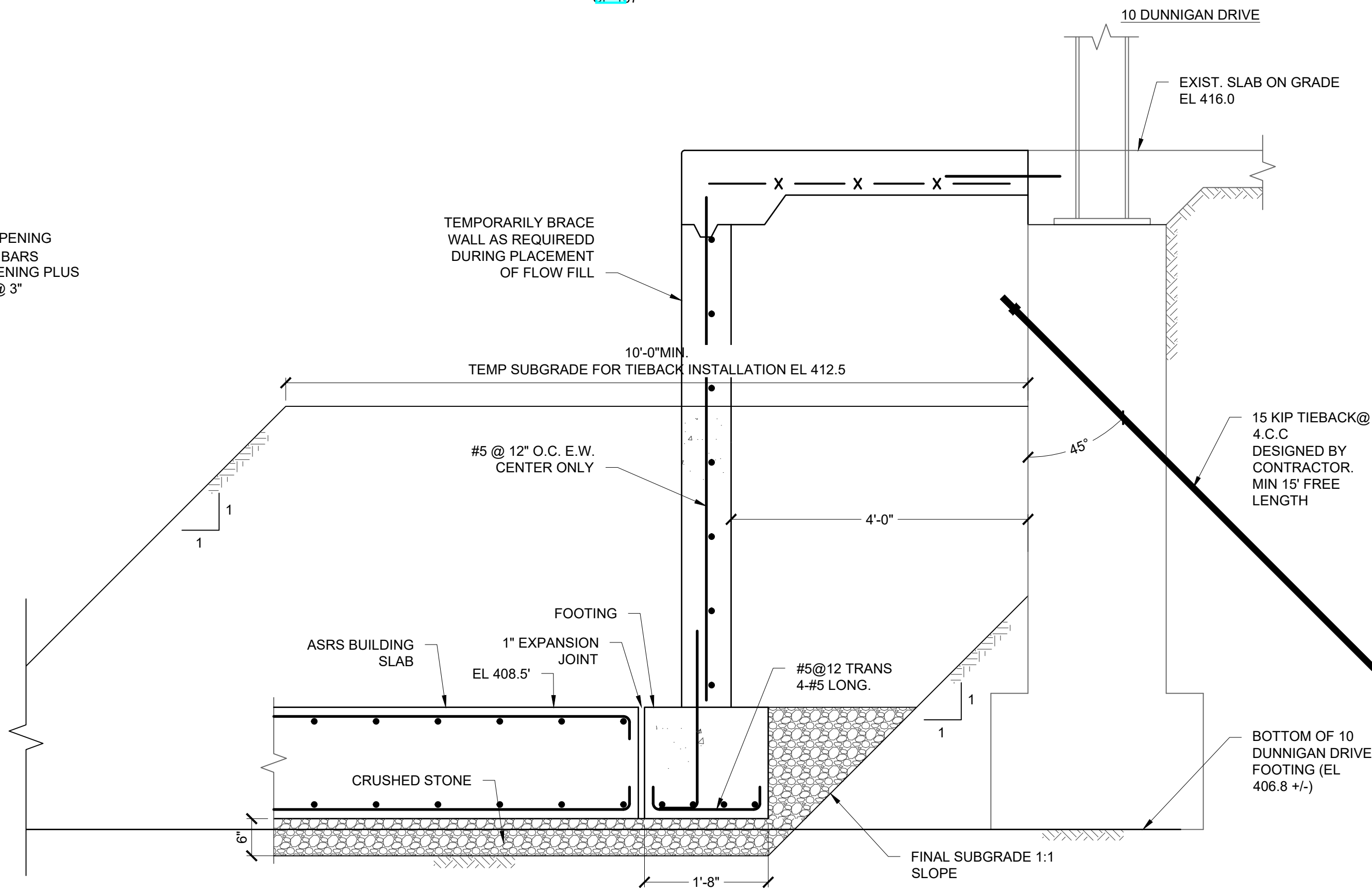
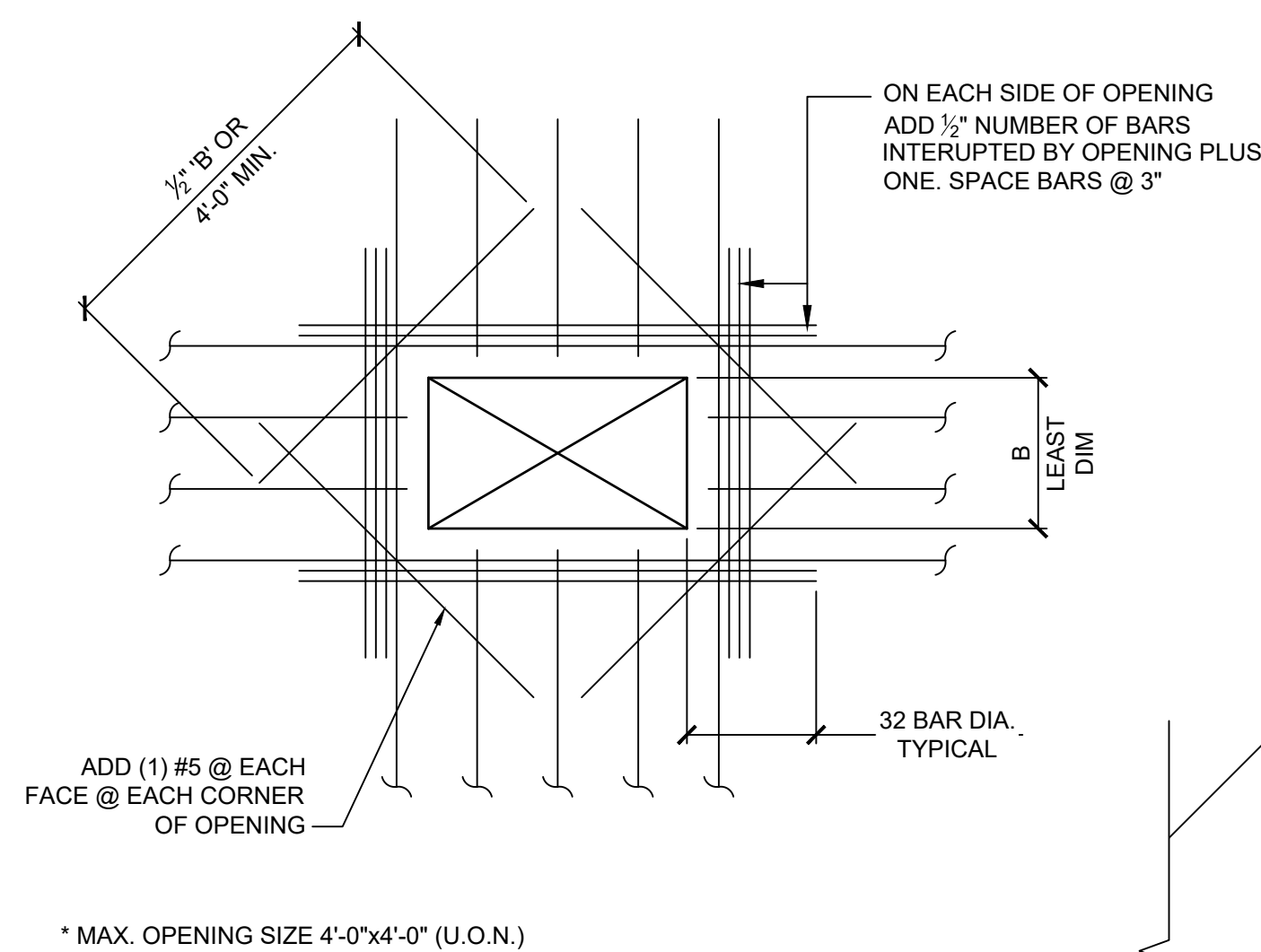
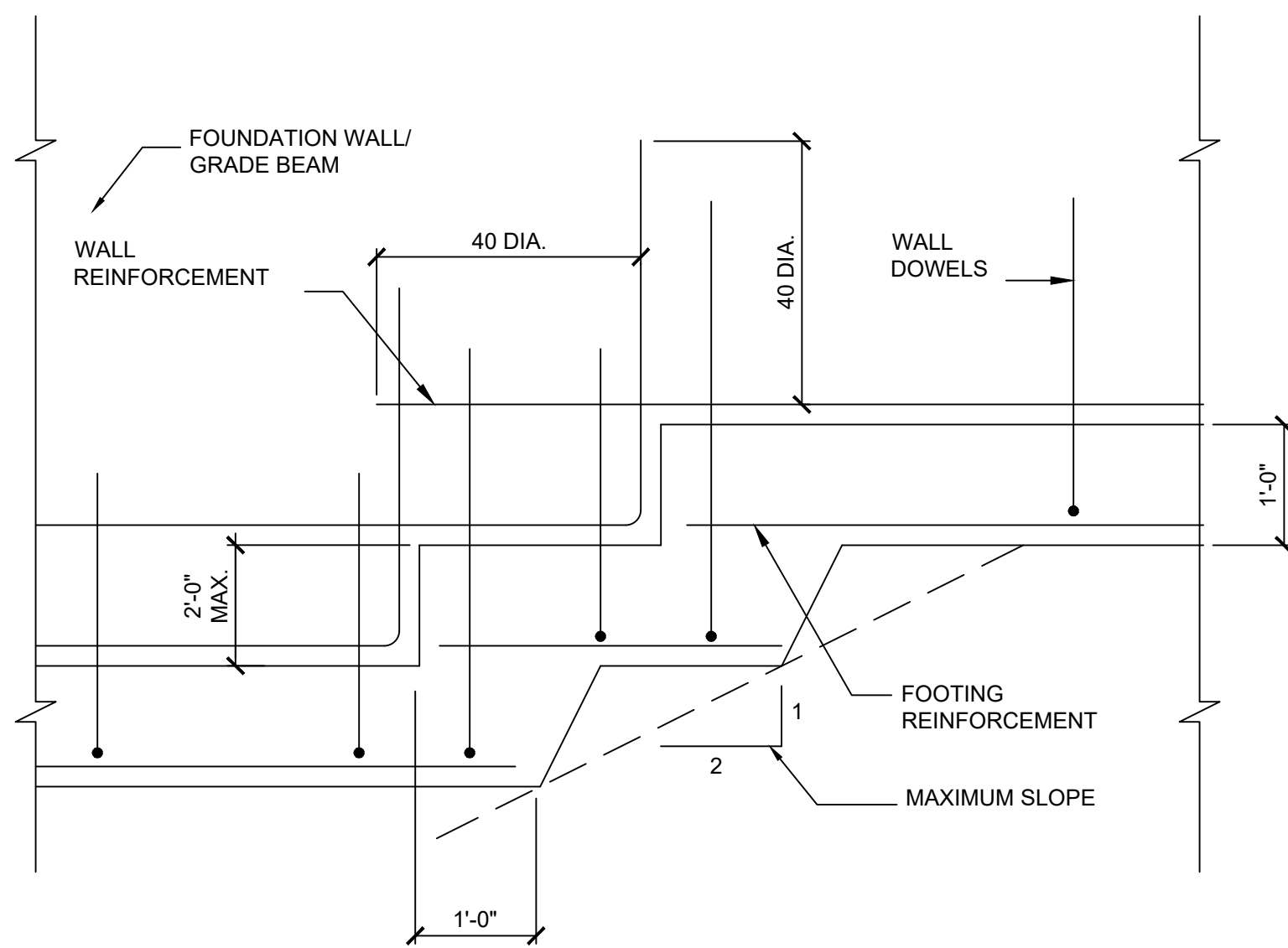
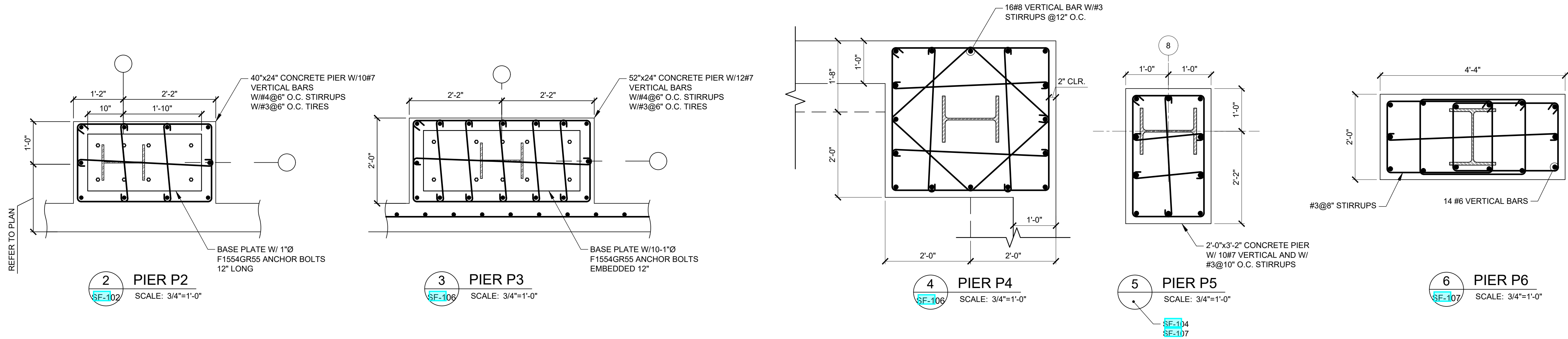
DWG NUMBER :

SF-203

20 OF 31



1 PIER P1 (TYP INTERIOR AND EXTERIOR WALL UNO)
SCALE: 1/2" = 1'-0"



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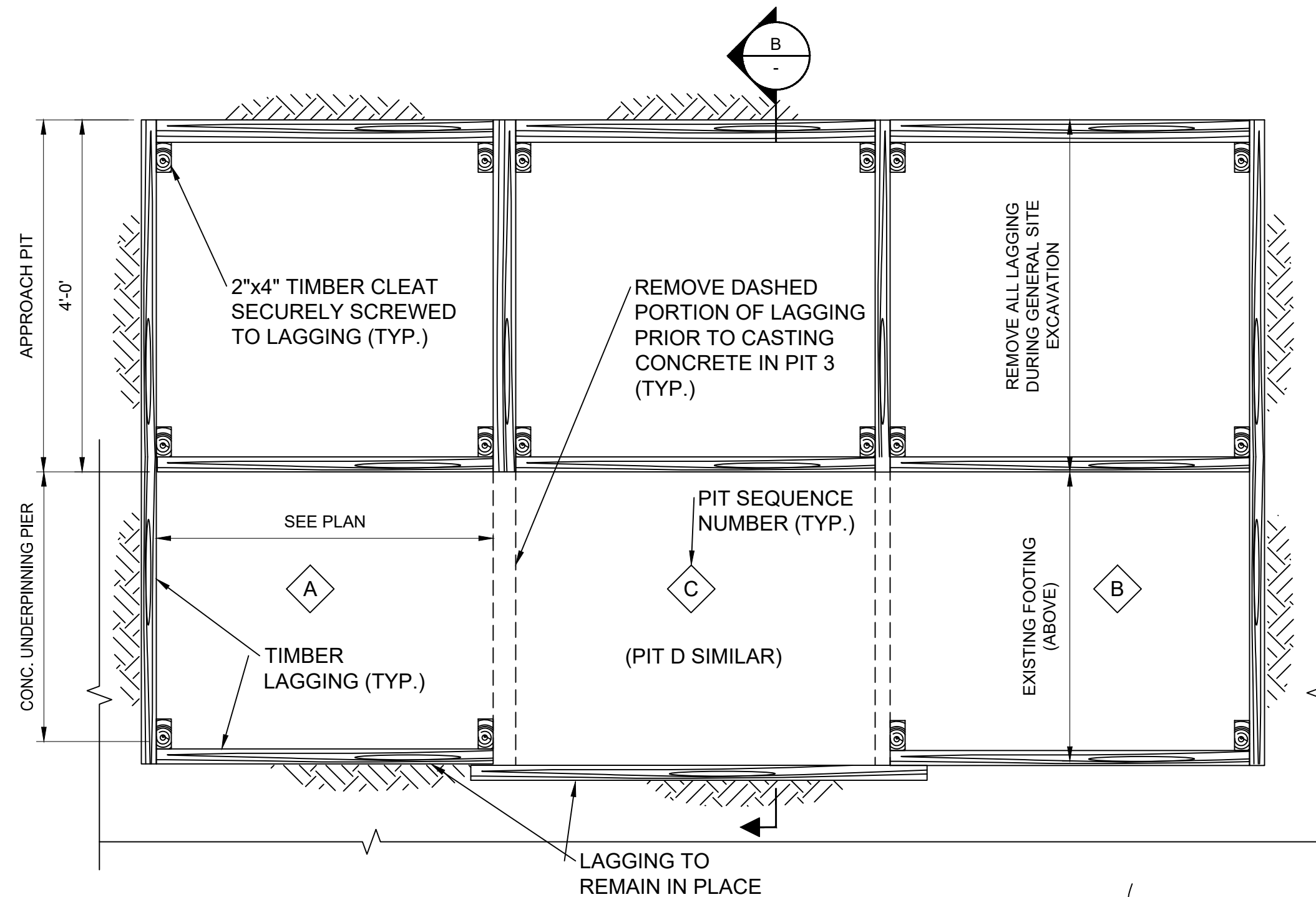
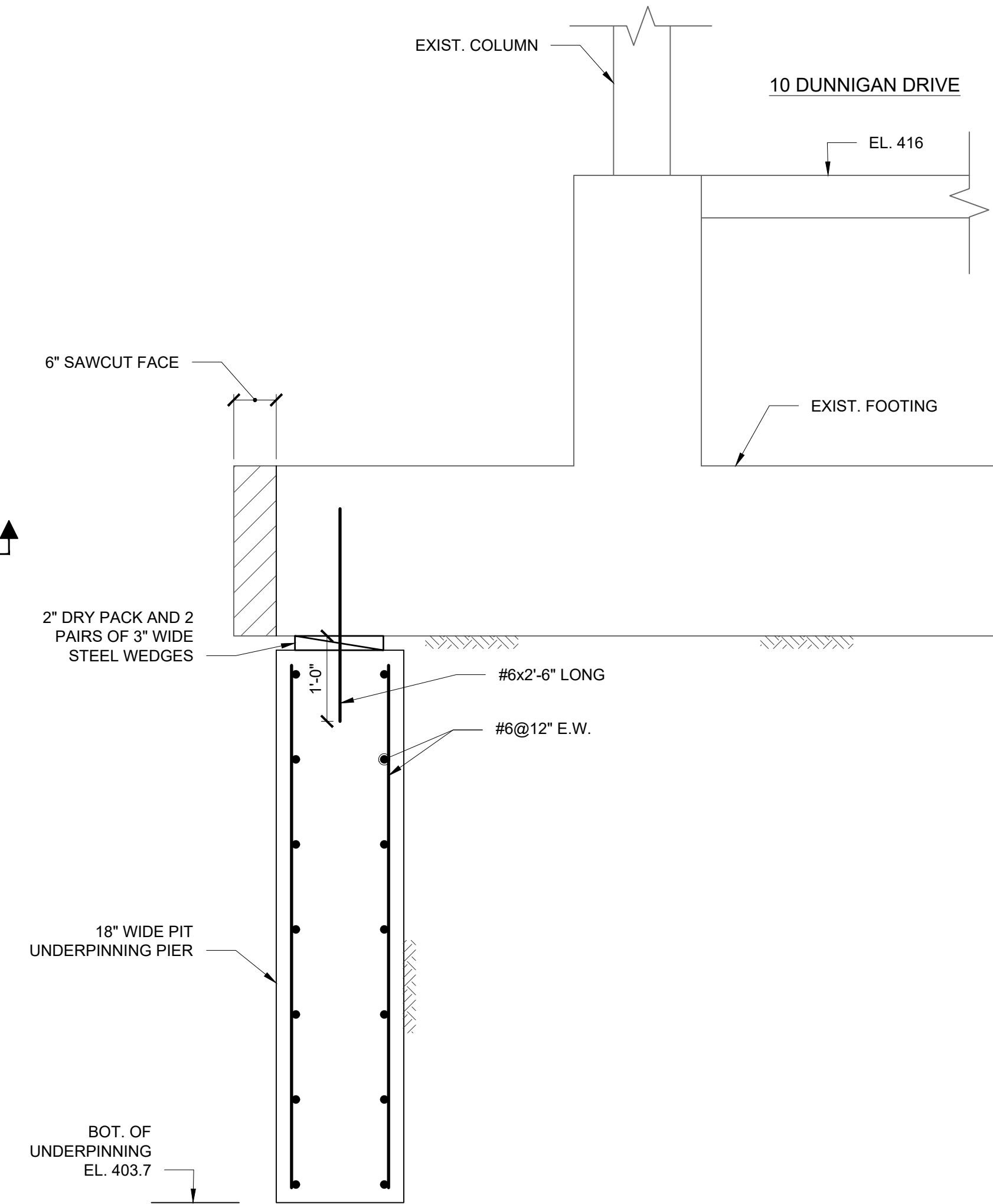
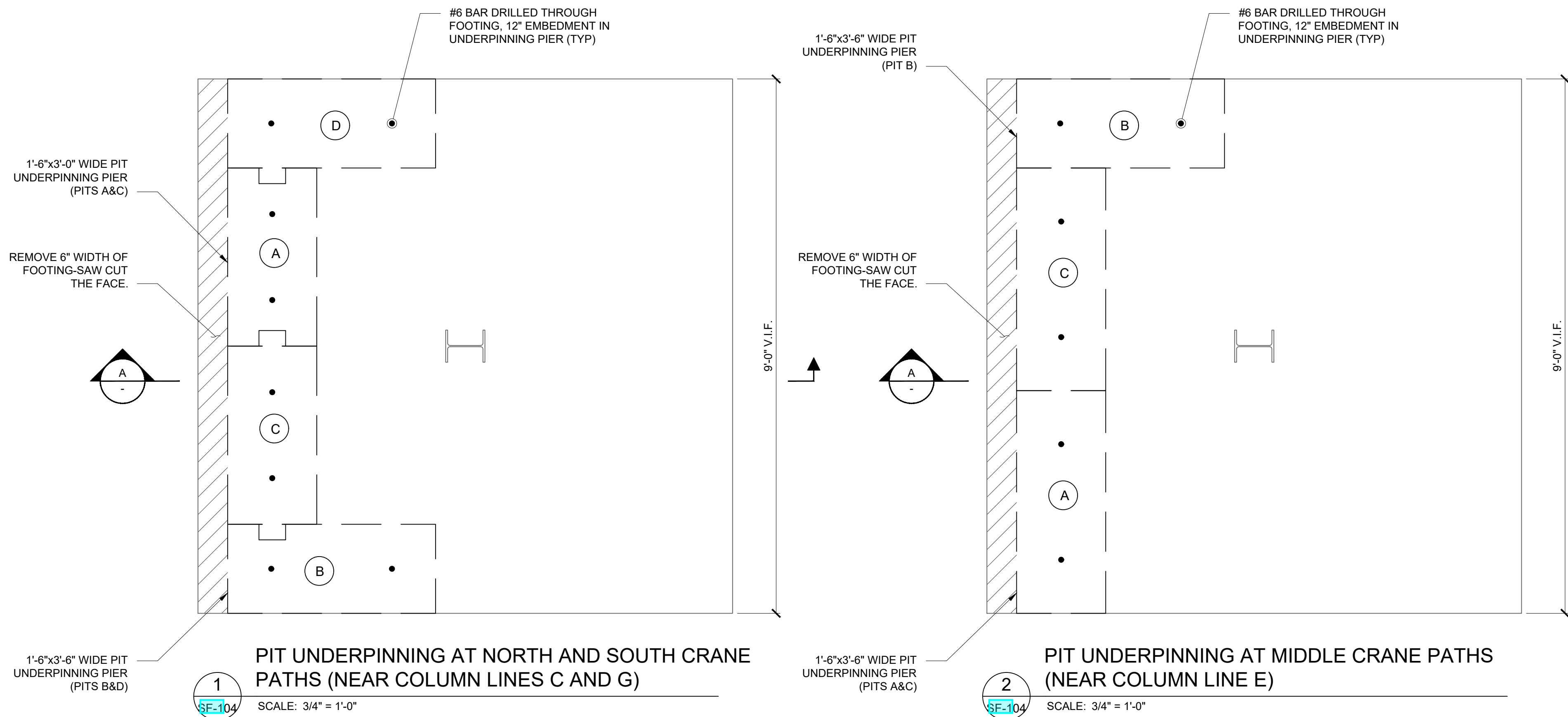
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4	ISSUED FOR CONSTRUCTION	04.04.22

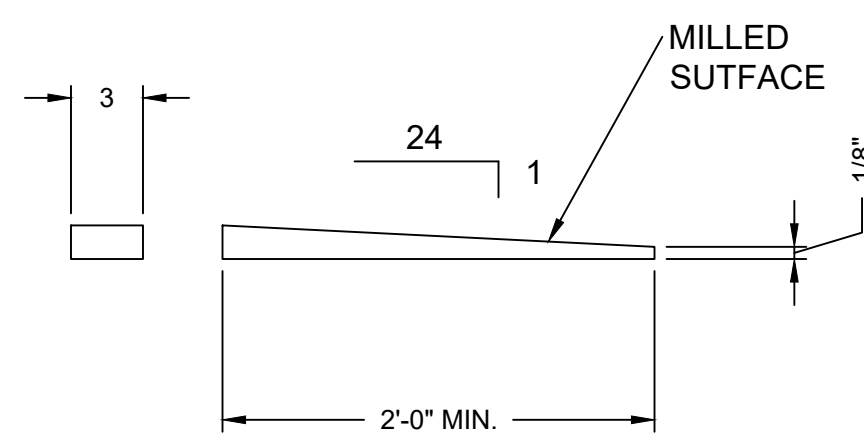
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SCALE: AS SHOWN
DRAWING TITLE: -

SECTIONS AND DETAILS

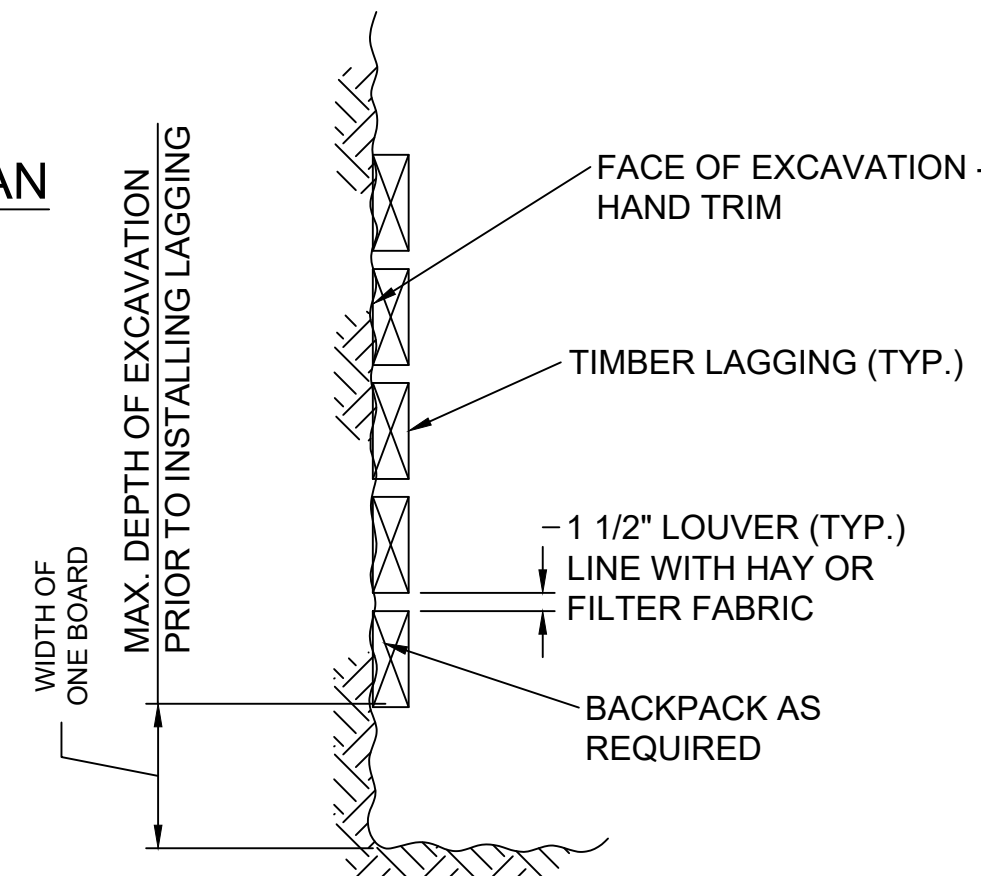
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SF-204
21 OF 31



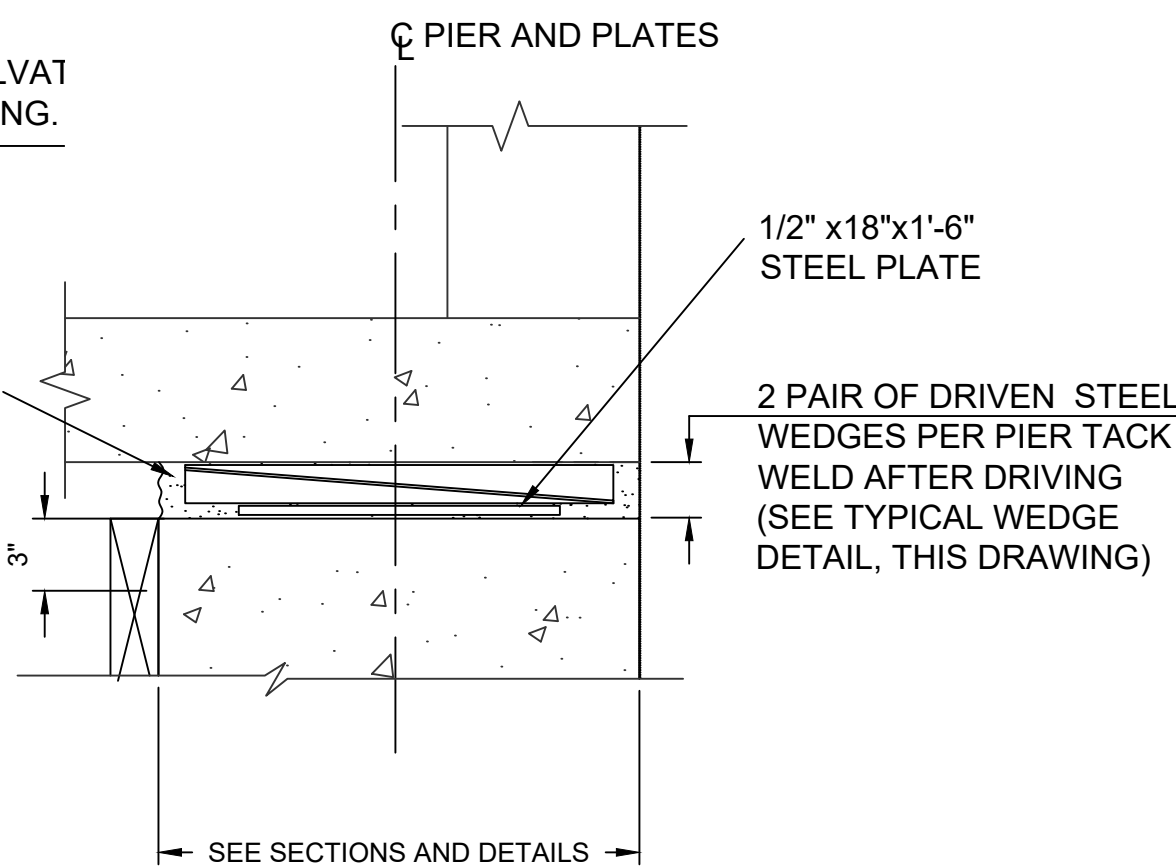
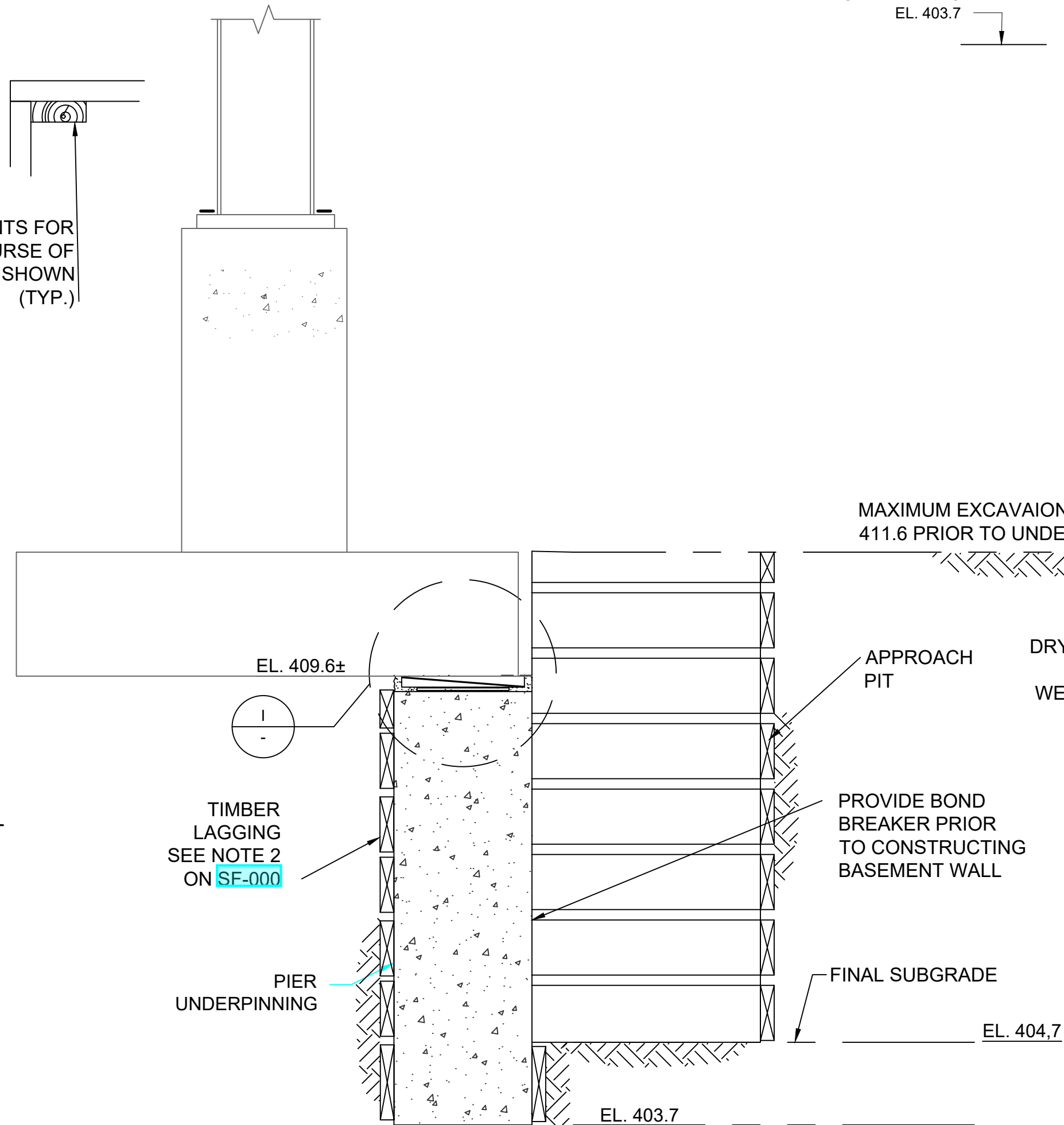
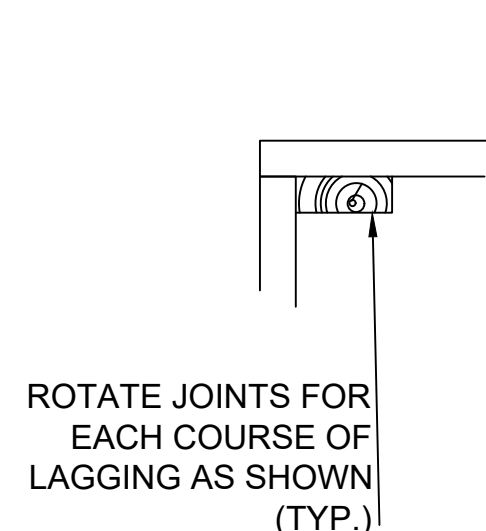
TYPICAL UNDERPINNING/APPROACH PIT PLAN
SCALE: N.T.S.



TYPICAL WEDGE DETAIL
SCALE: N.T.S.



TYPICAL LAGGING INSTALLATION DETAIL
SCALE: 3/4" = 1'-0"



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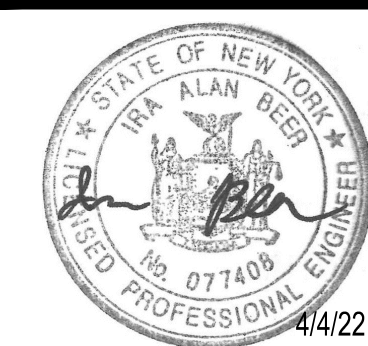
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CHECKED BY: I. BEER
APPROVED BY: I. BEER
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SECTIONS AND DETAILS



DWG NUMBER:
SF-300