

GENERAL CONSTRUCTION NOTES:

- 1. Reference Standards: Unless noted otherwise, all standards shall be current edition, with latest addenda, if applicable.
2. Contractor shall verify all existing dimensions, member sizes, and field conditions prior to any fabrication, construction, or installation and notify Structural Engineer of Record if conditions, materials, sizes, and dimensions are different from those shown.
3. The contract structural drawings and specifications represent the finished structure. Unless otherwise indicated, they do not indicate the means or method of construction. The contractor is solely responsible for the protection of the structure during all phases of demolition, construction, and installation.
4. The finished structure has been designed for the loading indicated below. It is the responsibility of the contractor(s) and their specialty Engineer(s) to review and use means and methods to adequately address loading on the structure during construction including, but not limited to, wind, snow, seismic, underpinning, material storage, and equipment.
5. Cross reference all dimensions and details with architectural and mechanical drawings before commencing any fabrication and/or construction.
6. Details and conditions not specifically shown shall be constructed in accordance with details shown for similar conditions and materials.
7. Shop drawings prepared by suppliers, sub-contractors, etc. shall be reviewed, coordinated, and signed/stamped by the contractor prior to submitting to the Structural Engineer of Record. The Structural Engineer of Record's review of shop drawings, product data, design calculations, etc., does not relieve the contractor from complying with the contract documents.
8. Verify location of all box outs and openings. Opening sizes and locations shown for pipes, ducts, mechanical units, etc. are for general information only and shall be verified with all trades before commencing the work.
9. Contractor is solely responsible for protection of the existing building during all phases of construction.
10. No structural repairs, corrections, or alterations of work affecting a structural member shall be made without the approval of the Structural Engineer of Record. Design and/or review may be an additional service.
11. Do not scale the drawings.

DESIGN CRITERIA LOADS AND STRESSES:

CODES:

- 1. 2020 New York Building Code
2. 2018 International Building Code w/ Amendments
3. Minimum Design Loads for Buildings and Other Structures (ASCE 7-16).

DESIGN LOADS:

Table with 2 columns: Risk Category, Seismic Importance Factor, I\_s, Mapped Spectral Response Acceleration Parameters, S\_a and S\_1, Site Class, Design Spectral Response Acceleration Parameters, S\_DS and S\_D1, Seismic Design Category, Basic Seismic Force-Resisting System, Response Modification Coefficients, Seismic Response Coefficients, Design Base Shear, Analysis Procedure Used.

Table with 2 columns: Ultimate Design Wind Speed (3-sec gust), V\_u, Nominal Design Wind Speed (3-sec gust), V\_nom, Risk Category, Wind Exposure, Internal Pressure Coefficients.

Table with 2 columns: Ground Snow Load, P\_g, Snow Exposure Factor, C\_e, Snow Load Importance Factor, I, Thermal Factor, C\_t, Slope Factor, C\_s, Flat Roof Snow Load, P\_f.

\*See Plan for Unbalanced Snow Loads & Snow Drift Loads

THE STRUCTURE HAS BEEN DESIGNED BASED ON THE HOLIDAY INN EXPRESS & SUITES FOUNDATION LOADING DRAWINGS BY CHAMPION COMMERCIAL STRUCTURES, DATED 1/7/2021.

FLOOR LIVE LOADS:

- 40 PSF Residential private rooms, corridors, stairs, and exits serving them
100 PSF Residential public spaces, corridors, stairs, and exits serving them
125 PSF Mechanical, Electrical, Storage Areas
100 PSF First floor corridors, public spaces, stairs, and exits
15 PSF Partition load, office areas

ROOF LIVE LOADS:

- 20 PSF Minimum Roof Live Load

CONCRETE: (F\_c) at 28 Days

- 3000 PSI Footings
3000 PSI Masonry grout corefill with 3/8" max. aggregate (fy ash not permitted, no entrained air)
3500 PSI Slab on grade (max w/c = 0.45, fy ash not permitted, no entrained air)
4000 PSI Slab on steel deck, topping slabs (max w/c = 0.45, fy ash not permitted, no entrained air)
4500 PSI Piers (unless noted otherwise), foundation walls, and exterior slabs [5%-7% air content]
4500 PSI Retaining walls, basement walls, pile caps, and grade beams
7000 PSI Non-shrink grout below baseplates

All exterior concrete work shall have 5% to 7% air entrainment.

DESIGN CRITERIA LOADS AND STRESSES: (CONTINUED)

STEEL: (F\_y)

- 60,000 PSI ASTM A615 grade 60 reinforcing
60,000 PSI ASTM A706 weldable reinforcing
75,000 PSI ASTM A185 welded wire fabric
50,000 PSI ASTM A992 wide-flange shapes
36,000 PSI ASTM A36 plates, channels, and angles, etc.
50,000 PSI ASTM A500 grade C structural tubes (HSS)
35,000 PSI ASTM A53 type E or S, grade B steel pipe
46,000 PSI ASTM A500 grade C structural pipe (HSS)
92,000 PSI ASTM A325 high strength bolts
36,000 PSI ASTM F1554 threaded anchor rods
50,000 PSI ASTM A108 headed studs
55,000 PSI ASTM F1554 threaded rods (as noted at brace frames)
50,000 PSI ASTM A572-50 plates where indicated (brace frame base plates, shear lugs, etc.)

MASONRY: (F\_m)

- 2000 PSI concrete masonry

FOUNDATION LOADS:

4,000 PSF soil bearing, based on soil report prepared by Dynamic Earth, LLC, dated 1/22/2021, (report # 1685-99-001EC).

TEMPORARY BRACING:

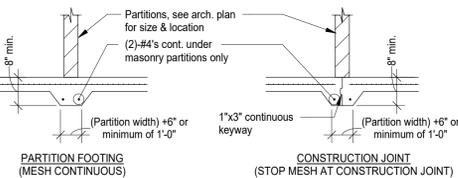
- 1. Provide temporary lateral support for all walls where grade varies on the two sides until slab has reached its design strength.
2. Provide required temporary bracing for structural steel until permanent bracing and walls are in place.
3. Provide temporary bracing for all walls, concrete, masonry, light gate metal, or wood until they are of adequate design strength and are properly anchored in final form.
4. All temporary shoring is to be designed by a specialty shoring contractor, by a Professional Engineer licensed in the state of the project, at the expense of the contractor.
5. Shore all foundation walls as required before backfilling and compacting.
6. Contractor shall provide adequate bracing and shoring during all phases of construction and erection of the structure.

GENERAL FOUNDATION NOTES:

- 1. All foundation excavations, backfill, and compaction shall be inspected and certified by a qualified soils testing firm prior to the construction of any footings. All reports are to be submitted to Structural Engineer of Record in a timely manner.
2. Cross reference all architectural, mechanical, electrical, and structural drawings to assure proper dimensions and placement of all anchor rods, inserts, etc.
3. All footing elevations are shown to top of footings, unless noted otherwise.
4. All footings are centered under walls or columns above, unless noted otherwise.
5. Continuous wall footings up through 1'-8" wide to be 10" thick. Footings over 1'-8" wide to be 12" thick, unless noted otherwise.
6. Provide wall footing reinforcement as follows:
Footings up through 2'-0" wide = (2)-#5 cont.
Footings 2'-1" through 3'-0" wide = (3)-#5 cont.
Footings 3'-1" through 3'-6" wide = (3)-#5 cont. & #5 @ 12" transv.
See details for reinforcing in all footings over 3'-6" wide.
7. Provide 90 degree bend in all footing dowels. Cast dowels in footings for columns, piers, and walls above. Dowels to be the same number and size as the vertical reinforcing, unless noted otherwise. See General Concrete Notes or General Masonry Notes for required lap length.
8. Provide hot dip galvanized welded wire block reinforcing in all masonry foundation walls at 16" o.c. maximum spacing.
9. Rebar and anchor rods to be securely tied in place prior to placing concrete (i.e. no "wet-sticking" is allowed).

CONCRETE SLAB AND JOINT NOTES AND DETAILS:

- 1. Control Joints (C.J.) - Locate saw cut control joints at column centerlines and at the following maximum spacing to create approximately square panels
a. Concrete slabs on grade:
i. 4'-5" thick slab = 12'-0"
ii. 6'-8" thick slab = 15'-0"
b. Coordinate control joint layout with floor finish requirements.
c. Control joint depth to be 1", using an early entry saw.
d. Cut control joints with an early entry saw as soon as possible without damage to the slab surface.
2. Provide 6x6-W1.4XW1.4 W.W.F. in all slabs on grade, unless noted otherwise. All mesh to be lapped a minimum of 12". Provide prefabricated sheets in lieu of rolled mesh. Reinforce with (2)-#5 x 3'-0" long at all re-entrant (inside) corners.
3. Place slab reinforcing between 1/4 and 1/3 of slab thickness down from top of slab.
4. Coordinate all floor finishes, slopes, recesses, floor drains, gutters, etc. with all disciplines (arch., mech., etc.).
5. Provide a preformed isolation joint in concrete slab at columns. The isolation joint can be either a circular or diamond shaped pattern.
6. Do not provide control joints in structural slabs, slabs on metal deck, or precast topping, unless noted otherwise.
7. Provide thickened slabs at masonry partitions and construction joints as detailed below.



GENERAL CONCRETE NOTES:

- 1. Concrete construction shall comply with the provisions of the "Building Code Requirements for Structural Concrete," ACI 318-14.
2. The "ACI Detailing Manual" shall govern detailing and fabrication of all reinforcing steel, unless noted otherwise.
3. Reinforcing steel supplier to provide all accessories, chairs, spacing bars, and supports necessary to secure steel in accordance with "Manual of Standard Practice" by the Concrete Reinforcing Steel Institute. Clay brick is not allowed.
4. Provide minimum clear concrete cover for all reinforcement as follows:

Cast against and permanently exposed to earth = 3"

Exposed to earth or weather:
#5 bars and smaller = 1 1/2"
#6 bars and larger = 2"

Not exposed to weather or in contact with ground:
Slabs, walls, & joists (#3 to #11 bars) = 3/4"
Beams, girders and columns, primary reinforcement, ties, stirrups, or spirals = 1 1/2"

- 5. Provide corner bars at all corners and intersections of walls, grade beams, and edge beams. Corner bar to be the same size and spacing as all horizontal bars.
6. At openings in structural slabs or walls, provide a minimum of (2)-#6 bars each side of opening. Bars are to extend a minimum of 3'-0" beyond corners of openings, unless noted otherwise. Provide (1)-#5 x 4'-0" long diagonal bar at each corner of opening in each face of wall or slab.
7. No aluminum of any type shall be allowed in the concrete work, unless coated to prevent reaction with concrete.
8. Maximum outside diameter of embedded conduit shall be no larger than 1/3 of the slab thickness. This restriction applies to the total height at conduit crossings. The conduit shall be placed such that it does not significantly impair the strength of construction.
9. Post-installed anchors in concrete shall be ICC approved for use in cracked concrete. Approved anchors shall be Hilti Kwik Bolt TZ Expansion Anchors (ESR-1917) or a Hilti HIT-HY 200 Adhesive Anchoring System (ESR-3187), unless noted otherwise. Install anchors in strict conformance with anchor manufacturer's instructions. Anchor substitutions shall not be made without written permission from the Structural Engineer of Record.
10. No pipe or conduit of any type shall be placed in structural concrete members without written approval from the Structural Engineer of Record.
11. Composite slabs and beams are designed to support the dead load of the wet concrete plus normal construction loads without requiring temporary shoring. Some deflection of the deck and beams will occur when the wet concrete is placed. The contractor shall include in the bid the cost of the additional concrete quantity caused by the deflection of the beams and deck.
12. Do not weld rebar, unless Weldable Rebar is provided and its use is approved by the Structural Engineer.
13. Lap splice lengths in continuous reinforcing shall be tension lap splices and are shown below, unless noted otherwise on drawings or details:

f\_c = 3000 PSI:

Table for CLASS B TENSION LAP SPLICE LENGTH with columns for Bar Size, Case 1, Case 2, Case 1, Case 2.

f\_c = 4000 PSI:

Table for CLASS B TENSION LAP SPLICE LENGTH with columns for Bar Size, Case 1, Case 2, Case 1, Case 2.

f\_c = 5000 PSI:

Table for CLASS B TENSION LAP SPLICE LENGTH with columns for Bar Size, Case 1, Case 2, Case 1, Case 2.

Notes:

- 1. Tables are for normal weight concrete with Grade 60 uncoated reinforcing bars.
2. Top bars are horizontal bars with more than 12" of concrete cast below the bars.
3. Compression lap splices (only where indicated on drawings) for Grade 60 uncoated reinforcing bars shall be 30 times the bar diameter.
4. Cases 1 and 2 are defined as follows:
Cases and columns:
Case 1: Concrete cover at least 1.0 times the bar diameter and center-to-center spacing of at least 2.0 times the bar diameter.
Case 2: Concrete cover less than 1.0 times or center-to-center spacing less than 2.0 times the bar diameter.
All other members:
Case 1: Concrete cover at least 1.0 times the bar diameter and center-to-center spacing at least 3.0 times the bar diameter.

GENERAL MASONRY NOTES:

- 1. Masonry construction shall comply with the provisions of the "Building Code Requirements and Specification for Masonry Structures," TMS 402-16 and TMS 602-16.
2. Do not apply floor/roof loads until masonry has reached adequate design strength.
3. Masonry wall construction shall be running bond, unless noted otherwise on architectural drawings.
4. All mortar in bearing walls shall be Type S.
5. Provide continuous bond beams with (2)-#5 continuous reinforcing bars at the top of all bearing walls, end walls, and at joint bearing elevations. See details for angles and plates cast in bond beams.
6. Provide grout corefill under all steel beam or lintel bearings a minimum of 5 courses down for a 16" length of wall, unless noted otherwise.
7. Wire reinforcing for single-wythe concrete block walls, masonry cavity walls, and multi-wythe composite masonry walls shall be hot dipped galvanized, corrosion resistant horizontal joint reinforcing with the following gage and vertical spacing:

Any width masonry in running bond:
9 gage @ 16" o.c. (typical wall)
Masonry in other than running bond:
6'-8" CMU - 9 gage @ 16" o.c.
10'-16" CMU - 9 gage @ 8" o.c.

- 8. Provide corefill at all vertical and horizontal reinforcing locations.
9. Consolidate and reconsolidate all grout by puddling or vibrating per ACI 530 specification section 3.5E.
10. Grouting shall be stopped 1 1/2" below the top of a course to form a key at the pour joint.

- 11. Masonry wall cells to be filled with grout shall be filled in lifts not exceeding 4'-0". High lift grouting can be utilized at the contractor's option. Provide clean outs at each bar location.

- 12. Vertical reinforcing bars shall be held in position at top and bottom and as required to maintain intended position of rebar.
13. Vertical reinforcing shall be lapped to all dowels extending from footings and provide vertical wall reinforcing splices as follows:

#4 BAR = 16"
#5 BAR = 24"
#6 BAR = 42"

Unless noted otherwise on the drawings.

- 14. Walls must be guyed and braced until floor and/or roof systems are in place.

- 15. Where a single rebar is specified in a masonry wall, it shall be centered in the core. Where two rebar are specified in one core, one bar shall be located at each face. The distance from the centerline of the rebar to the outside face of the block shall not exceed 3".

- 16. Reinforce all bond beams with (2)-#5 continuous horizontal, unless noted otherwise.

- 17. Provide corner bars in all bond beams to match continuous bar size.

- 18. Where pipe, conduit or other non-structural embed items are desired in reinforced masonry, items shall be placed whenever possible in un-grouted cells. Vertical grouted cores may have up to one (1)-1 inch diameter conduit, pipe, etc positioned in the middle of the core, provide 1" clear to masonry core vertical reinforcing bars. Additional items/configurations only permitted with the written approval from the Structural Engineer of Record.

- 19. Grout corefill shall not contain flyash or entrained air, unless approved by the Structural Engineer.

- 20. Post-installed anchors in masonry shall be ICC approved. Approved anchors shall be Hilti Kwik Bolt 3 Expansion Anchors (ESR-1385) or a Hilti HIT-HY 270 Adhesive Anchoring System (ESR-4143), unless noted otherwise. Grout masonry solid at anchor locations. Install anchors in strict conformance with anchor manufacturer's instructions. Anchor substitutions shall not be made without written permission from the Structural Engineer of Record.

VPH ENGINEERING SERVICES, PC AN AFFILIATE OF IMEG CORP. 623 26TH AVENUE ROCK ISLAND, IL 61201 309.788.0673 FAX: 309.788.5967 PROJECT # 21004052.00



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Consultants:

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Project:

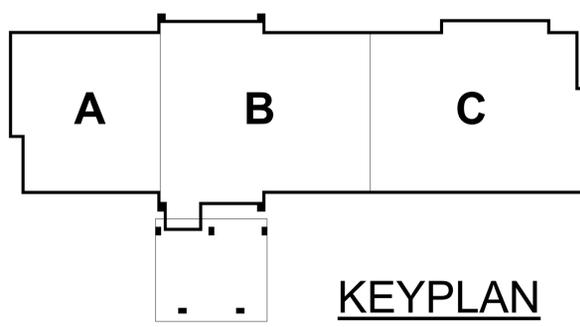
HOLIDAY INN EXPRESS THRUWAY PLAZA OF ROCKLAND ASSOCIATES CLARKSTOWN (NANUET) ROCKLAND COUNTY, NY 10954

Issued for:

BID/PERMITS 04.30.21

SHEET INDEX SCHEDULE

Table with 2 columns: Sheet Number, Description (GENERAL NOTES, TESTING & INSPECTION SCHEDULES, FOUNDATION PLAN - AREA A, etc.)



PROJECT NAME: L 19180 HOLIDAY INN EXPRESS & SUITES NANUET, NY LOCATION: 19180 INN CODE: NYCNT PROJECT: 32435 HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

Drawn by: MAQ

Checked by: JDG, DJH, LJS

Sheet Title: GENERAL NOTES

Project No.: 21004052.00

Sheet No.: S000

E

D

C

B

A

TYPICAL LINTEL TYPES AND NOTES:

- Verify size and location of all mechanical, U.V., U.H., louver, and duct openings with mechanical contractor.
- For all openings through masonry walls not shown, including mechanical and electrical openings, provide one of the following: (unless noted otherwise)
  - Steel angle lintels:
    - (1) L 3 1/2" x 3 1/2" x 1/4" for each 4" thickness of wall for spans up to 4'-0".
    - (1) L 5" x 3 1/2" x 5/16" (LLV) for each 4" thickness of wall for spans up to 5'-0".
    - (1) L 6" x 3 1/2" x 5/16" (LLV) for each 4" thickness of wall for spans up to 6'-0".
  - Block lintels
    - Use only U-shaped lintel block for masonry lintels. The centerline of the reinforcing is to be located 3" maximum from the bottom of the lintel block.
    - Non-bearing wall up to 3'-4" span:
 

12" Block	10" Block	8" Block	6" Block
(2)-#4 Bot.	(2)-#4 Bot.	(2)-#4 Bot.	(1)-#4 Bot.
    - Non-bearing wall 3'-5" to 6'-4" span:
 

12" Block	10" Block	8" Block	6" Block
(2)-#5 Bot.	(2)-#5 Bot.	(2)-#5 Bot.	(1)-#5 Bot.
    - Bearing wall up to 4'-0" span (8" deep lintel):
 

12" Block	10" Block	8" Block	6" Block
(2)-#5 Bot.	(2)-#5 Bot.	(2)-#5 Bot.	(2)-#5 Bot.
    - Bearing wall 4'-1" to 6'-4" span (16" deep lintel):
 

12" Block	10" Block	8" Block	6" Block
(2)-#6 T&B	(2)-#6 T&B	(2)-#6 T&B	(2)-#6 T&B
- Fill lintel blocks solid with 3000 PSI grout (3/8" maximum aggregate). Provide 8" minimum bearing each end of masonry lintel, unless noted otherwise.
- All steel lintel beams to bear a minimum of 8" on grouted or solid masonry, unless noted otherwise. All steel lintel angles to bear a minimum of 6" on solid or grouted masonry, unless noted otherwise.
- Bottom plate of steel lintels shall be welded to the beam with 3/16" fillet weld, 3" long at 12" o.c., staggered both sides.
- All lintels in exterior walls to be hot-dipped galvanized, unless noted otherwise.

GENERAL STEEL NOTES:

- Construction of structural steel shall comply with the provisions of "AISC 360-16 Specification for Structural Steel Buildings."
- All shop connections shall be welded or bolted, field connections shall be bolted, unless noted otherwise. Bolted connections shall be Bearing Type (snug-tightened) and shall be made with a minimum of 3/4" ASTM A325-N Bolts. Direct-Tension Indicators are acceptable substitutions.
- All welds as per latest specifications of the AWS - E70xx electrodes.
- Before encasing steel columns in concrete or masonry, paint column bases and tops of anchor rods with asphaltic paint.
- The structural fabricator shall furnish all plates and angles cast in bond beams, concrete walls, or columns to support steel joists, beams, and steel deck.
- 'C' denotes beam is continuous over columns, 'S' denotes beam simple shear splice.
- All steel beams shall be true to line and elevation, column base plates grouted, and anchor rods tight before any loads are placed.
- All column base and cap plates to be welded around all sides.
- All welds not specified are 3/16" fillet weld, continuous and/or all around.
- Structural fabricators shall show all welding requirements on structural steel shop drawings.
- Fabricator shall select AISC simple shear connections for composite beams capable of carrying the reaction load indicated or the reaction load calculated and based on tributary area or at a minimum 75% of the total shear capacity.
- Cuts, holes, or openings required in structural steel members for the work of other trades shall be shown on the shop drawings. Burning of holes and cuts in structural steel members in the field shall not be allowed, except by written permission from the Structural Engineer of Record.
- The top of all beams receiving shear studs shall not be painted.
- All connections not specifically detailed shall be designed by a Professional Engineer licensed in the state where the project is located. Detailing shall be performed using rational engineering design and standard practice in conformance with the contract documents. The general details shown on the drawings are approximate only and do not indicate the required number of bolts, weld requirements, etc., unless specifically noted.
- Shear stud connectors shall be manufactured by Nelson Stud Welding Co. or equal conforming to ASTM A108, and shall be field applied with automatic welding equipment through the composite steel deck with the use of a proper ferrule. Remove ferrules after welding.
- Location, type, diameter, length, and spacing of shear stud connectors shall be detailed on the shop drawings.

STEEL DECK NOTES:

- All steel decking shall comply with the specifications of the Steel Deck Institute (SDI). Thickness, type, and properties of decks shall be as shown on the drawings.
- All steel deck shall span a minimum of three spans, unless otherwise approved.
- Field weld 1 1/2" steel roof deck to supporting members with 5/8"ø puddle welds at 36/4 pattern. Where areas of warped deck occur, field weld steel deck maximum 6" o.c. at all supports. Typical, unless noted otherwise.
- 1 1/2" steel roof deck shall have, (1)-#10 TEK screw (1) side lap connector installed between adjacent supports (unless noted otherwise).
- Composite steel deck with concrete slabs shall be welded to all supporting members with 5/8"ø puddle welds at 36/4 pattern. For deck units with spans greater than 5'-0", sidelaps and perimeter edges of units between span supports shall be fastened at intervals not exceeding 36" o.c., using one of the following methods:
  - #10 self-drilling screws
  - Arc puddle welds 5/8" minimum visible diameter, or minimum 1" long fillet weld.
- See plans and details for composite deck thickness, depth, and profile. All composite steel deck to be galvanized with G-60 coating.
- Steel conform deck shall be attached at all supports sufficiently to prevent movement. Steel deck fasteners are not required for conform decks supporting concrete stoop slabs.

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PROJECT # 21004052.00

New York Design Firm Registration #0012979

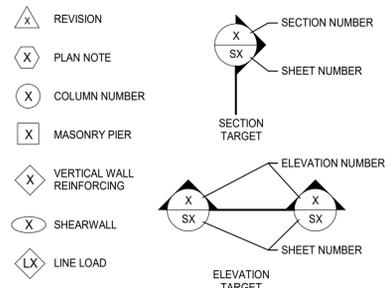
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REFERENCE SCALE IN INCHES

COLD FORMED METAL FRAMING NOTES:

- All cold formed metal framing shall conform to the AISI specification for the design of Cold Formed Structural Metals (AISI S100).
- All welds shall comply with the requirements of the North American Specification for the Design of Cold-Formed Steel Structural Members (AISI S100) and the Structural Welding Code - Sheet Steel (AWS D1.3).
- All framing components not specifically detailed and designed on these structural documents shall be designed by the supplier and sealed by a Professional Engineer licensed in the state of the project.
- All steel studs, joists, and accessories shall be ASTM A653/A653M, Grade 33 (Fy = 33 KSI) or Grade 50 (Fy = 50 KSI), either as indicated on plans, details, or required by design.
- All steel stud and joist fasteners shall be TEK screws, manufactured by ITW Buildex, or approved equal.
- Studs shall have full bearing against inside track web, prior to stud and track attachment.
- Splices in axially loaded studs shall not be permitted.
- Framing components may be preassembled into panels prior to erecting. Prefabricated panels shall be square, with components attached in a manner as to prevent racking.
- All framing components shall be cut square for attachment to perpendicular members. Members shall be held positively in place until properly fastened.
- Erect framing and panels plumb, level, and square in accordance with the shop drawings.
- Handling and lifting of prefabricated panels shall be done in a manner as to not cause distortion in any member.
- Track shall be securely anchored to the supporting structure as shown on the fabrication and erection drawings.
- At track butt joints, abutting pieces of track shall be securely anchored to a common structural element, or they shall be butt-welded or spliced together.
- Studs shall be plumbed, aligned, and securely attached to the flange or webs of both upper and lower tracks.
- Jack studs or cripples shall be installed below window sills, above window and door heads, and shall be securely attached to supporting members.
- Wall stud bridging shall be attached in a manner to prevent stud rotation. The minimum bridging shall be 5'-0" o.c. for wind loaded walls and 3'-4" o.c. for axial loaded walls.
- Cutouts, holes, or notches are not permitted in cold-formed steel roof and floor joists, headers, or beams, without prior written approval of the Structural Engineer of Record.

DRAWING SYMBOL LEGEND:



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PROJECT # 21004052.00



Project:

HOLIDAY INN EXPRESS  
THRUWAY PLAZA OF  
ROCKLAND ASSOCIATES  
CLARKSTOWN (NANUET)  
ROCKLAND COUNTY, NY  
10954

Issued for:

BID/PERMITS 04.30.21

PROJECT NAME: L 19180 HOLIDAY INN EXPRESS & SUITES NANUET, NY  
LOCATION: 19180  
INN CODE: NYCNT  
PROJECT: 32435  
HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

Drawn by: MAQ

Checked by: JDG, DJH, LJS

Sheet Title:  
GENERAL NOTES

Project No.:  
21004052.00

Sheet No.:  
S001

**STATEMENT OF SPECIAL INSPECTION:**

Special Inspections and Testing requirements per Chapter 17 of the IBC in addition to Section 110 of the IBC (Inspection performed by the Building Official). See Specs. for additional information.

Structural Testing & Special Inspection Program Summary Schedule			
IBC Section	Material	Type of Inspector	Report Frequency
1704.2.5	Shop Fabrication	SI-S	Upon Completion
1705.2	Steel	SI-S	Weekly
1705.3	Concrete	SI-S	Weekly
1705.4	Masonry	SI-S	Weekly
1705.5	Wood	SI-S	Weekly
1705.6	Soils/Earthwork	SI-T	Upon Completion

SI-S Special Inspector-Structural  
SI-T=Special Inspector-Technical

**1703.1 - APPROVALS**

- Agency must be approved by the Building Official or AHJ.
- Agency must be independent of the contractor responsible for work and disclose possible conflicts of interests.

**1704.2.4 - SPECIAL INSPECTOR RESPONSIBILITIES:**

- Submit inspection reports to the Building Official, Architect, Engineer of Record (EOR), and Contractor, stating the work was or was not in conformance with construction documents.
- Discrepancies shall be brought to the immediate attention of the contractor for correction.
- If discrepancy is not corrected, it shall be brought to the attention of the building official and EOR in a timely manner to provide remediation or acceptance prior to the completion of work.
- Submit a final report documenting required special inspections and correction of any discrepancies noted.

**1704.2.5 - FABRICATION:**

Where fabrication of structural members and assemblies are being fabricated on the premises of a fabricator's shop, special inspection is required of the fabricated item.

Note: Where Special Inspection and Testing of Shop Fabricated Components is required, it shall conform to the Special Inspection and Testing required in the field for the material specific section the component is fabricated from.

Exception: Special Inspection of the Fabricator's shop is not required if approved per Section 1704.2.5.2.

TABLE 1705.6 REQUIRED VERIFICATION AND INSPECTION SOILS		
Verification and Inspection Task	Continuous During Task Listed	Periodically During Task Listed
1. Verify materials below footings are adequate to achieve the design bearing capacity.	---	X
2. Verify excavations are extended to proper depth and have reached proper material.	---	X
3. Perform classification and testing of controlled fill materials.	---	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.	X	---
5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.	---	X

TABLE 1705.3 REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION				
Verification and Inspection	Continuous	Periodic	Referenced Standard (a)	IBC Reference
1. Inspection of reinforcing steel and placement.	---	X	ACI 318: 3.5, 7.1 - 7.7	1910.4
2. Inspection of reinforcing steel welding in accordance with Table 1705.2.2, item 2b.	---	---	AWS D1.4, ACI 318: 3.5.2	---
3. Inspection of anchors cast in concrete where allowable loads have been increased or where strength design is used.	---	X	ACI 318: 8.1.3, 21.2.8	1908.5, 1909.1
4. Inspection of anchors post-installed in hardened concrete members (b).	---	X	ACI 318: 3.8.6, 8.1.3, 21.2.8	1909.1
5. Verifying use of required design mix.	---	X	ACI 318: Ch. 4, 5.2 - 5.4	1904.2, 1910.2, 1910.3
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	---	ASTM C 172, ASTM C 31, ACI 318: 5.6, 5.8	1910.10
7. Inspection of concrete and shotcrete placement for proper application techniques.	X	---	ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8
8. Inspection for maintenance of specified curing temperature and techniques.	---	X	ACI 318: 5.11 - 5.13	1910.9
9. Inspect formwork for shape, location and dimensions of the concrete member being formed.	---	X	ACI 318: Ch. 6.1.1	---

For SI: 1 inch=25.4mm

- Where applicable, see also Section 17.05.11, Special Inspections for seismic resistance.
- Specific requirements for special inspection shall be included in the research report for the anchor issued by an approved source in accordance with ACI 355.2 or other qualification procedures. Where specific requirements are not provided special inspection requirements shall be specified by the registered design professional and shall be approved by the building official prior to the commencement of the work.

**Exceptions:**

- Non-structural concrete slabs supported directly on the ground.
- Concrete patios, driveways and sidewalks on grade.

Building Code Requirements for Masonry Structures				
TABLE 1.19.2 - LEVEL B QUALITY ASSURANCE				
MINIMUM TESTS				
Verification of Slump flow and Visual Stability Index (VSI) as delivered to the project site in accordance with Specification Article 1.5B.1.b.3 for self-consolidating grout				
Verification of fm and fAAC in accordance with Specification Article 1.4B prior to construction, except where specifically exempted by the Code				
MINIMUM INSPECTION				
Inspection Task:	Frequency (a)		Reference for Criteria	
	Continuous	Periodic	TMS 402/ACI 530/ASCE 5	TMS 602/ACI 530.1/ASCE 6
1. Verify compliance with the reviewed submittals		X		Art. 1.5
2. As masonry construction begins, verify that the following are in compliance:				
a. Proportions of site-prepared mortar		X		Art. 2.1, 2.6A
b. Construction of mortar joints		X		Art. 3.3B
c. Grade and size of prestressing tendons and anchorages		X		Art. 2.4B, 2.4H
d. Location of reinforcement, connectors, and prestressing tendons and anchorages		X		Art. 3.4, 3.6A
3. Prior to grouting, verify that the following are in compliance:				
a. Grout		X		Art. 3.2D, 3.2F
b. Grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages		X	Sec. 1.16	Art. 2.4, 3.4
c. Placement of reinforcement, connectors, and prestressing tendons and anchorages		X	Sec. 1.16	Art. 3.2E, 3.4, 3.6A
d. Proportions of site-prepared grout and prestressing grout for bonded tendons		X		Art. 2.6B, 2.4G.1.b
e. Construction of mortar joints		X		Art. 3.3B
4. Verify during construction:				
a. Size and location of structural elements		X		Art. 3.3F
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction		X	Sec. 1.16.4.3, 1.17.1.	
c. Welding of reinforcement	X		Sec. 2.1.7.7.2, 3.3.3.4(c), 8.3.3.4(b)	
d. Preparation, construction, and protection of masonry during cold weather (temperature below 40 degrees F (4.4 degrees C) or hot weather (temperatures above 90 degrees F (32.2 degrees C))		X		Art. 1.8C, 1.8D
f. Placement of grout	X			Art. 3.5, 3.6C
5. Observe preparation of grout specimens, mortar specimens, and/or prisms		X		Art. 1.4B.2.a.3, 1.4B.2.b.3, 1.4 B.2.c.3, 1.4B.3, 1.4 B.4

(a) Frequency refers to the frequency of inspection, which may be continuous during the task listed or periodically during the listed task, as defined in the table.

In addition to the requirements below also comply w/ AISC 360-10 Chapter N

Welding Inspection Tasks	
Welding procedure specifications (WPSs) available	P
Manufacturer certifications for welding consumables available	P
Material identification (type/grade)	O
Welder identifications system (1)	O
Fit-up of groove welds (including joint geometry)	O
Configuration and finish of access holes	O
Fit-up of fillet welds	O
Check welding equipment	---
Use of qualified welders	O
Control and handling of welding consumables	O
No welding over cracked tack welds	O
Environmental conditions	O
WPS followed	O
Welding techniques	O
Welds cleaned	O
Size, length and location of welds	P
Welds meet visual acceptance criteria	P
Arc strikes	P
k-area (2)	P
Backing removed and weld tabs removed (if required)	P
Repair activities	P
Document acceptance or rejection of welded joint or member	P

(1) The fabricator or erector, as applicable, shall maintain a system by which a welder who has welded a joint or member can be identified. Stamps, if used, shall be the low-stress type.

(2) When welding of double plates, continuity plates or stiffeners has been performed in the k-area, visually inspect the web k-area for cracks within 3 in. (75mm) of the weld.

0 - Observe these items on a random basis. Operations need not be delayed pending these inspections.

P - Perform these tasks for each welded joint or member

N5.5	
<b>Non-destructive Testing (NDT) of Welds shall be performed in accordance with AWS D1.1/D1.1M based on the following criteria:</b>	
1. For structures in Risk Category III or IV, Ultrasonic Testing (UT) shall be performed on all Complete Joint Penetration (CJP) groove welds for materials 5/16" thick or greater.	
2. Structures in Risk Category II, UT shall be performed on 10% of CJP groove welds for materials 5/16" thick or greater.	
3. When flange or web thickness exceeds 2", thermally cut access holes shall be tested using Magnetic Particle Testing (MT) or Penetrant Testing (PT), any crack is unacceptable.	
4. Welded joints requiring soundness per Appendix 3, Table A-3.1 shall be tested by Radiographic Testing (RT) or UT. Reduction in the rate of UT is prohibited.	
5. Reduction rate for UT - Where the initial rate for UT is 100%, the NDT rate for an individual welder is permitted to be reduced to 25% provided the reject rate is 5% or less based on a minimum of 40 welds tested. For continuous welds over 3', each 12" increment shall be considered on weld.	
6. Increase rate for UT - Where the initial rate for UT is 10%, the NDT rate for an individual welder shall be increased to 100% if the reject rate is over 5% based on a minimum of 20 welds tested. Rate may be reduced if reduction rate criterion is met.	

Bolting Inspection Tasks	
Manufacturer's certifications available for fastener materials	P
Fasteners marked in accordance with ASTM requirements	O
Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	O
Proper bolting procedure selected for joint detail	O
Connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements	O
Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	O
Proper storage provided for bolts, nuts, washers and other fastener components	O
Fastener assemblies, of suitable condition, placed in all holes and washers (if required) are positioned as required.	O
Joint brought to the snug-tight condition prior to the pretensioning operation	O
Fastener component not turned by the wrench prevented from rotating	O
Fasteners are pretensioned in accordance with the RSCS Specification, progressing systematically from the most rigid point toward the free edges.	O
Document acceptance or rejection of bolted connections	P

0 - Observe these items on a random basis. Operations need not be delayed pending these inspections.

P - Perform these tasks for each welded joint or steel member

N5.7	
Anchor Bolts and other embedded items supporting structural steel, verify diameter, grade, type, length of embedded item, and the embedment depth prior to placing concrete.	
TABLE N6.1	
Inspection of Steel Elements of Composite Construction Prior to Concrete Placement	
Placement and installation of steel deck	P
Placement and installation of steel headed stud anchors	P
Document acceptance or rejection of steel elements	P

0 - Observe these items on a random basis. Operations need not be delayed pending these inspections.

P - Perform these tasks for each steel member.

In addition to the requirements below also comply w/ AISC 360-10 Chapter N

TABLE 1705.2.2 REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL			
Verification and Inspection	Continuous	Periodic	Referenced Standard (a)
1. Material verification of cold-formed steel deck			
a. Identification markings to conform to ASTM standards specified in the approved construction documents	---	X	Applicable ASTM material standards
b. Manufacturer's certified test reports	---	X	
2. Inspection of welding:			
a. Cold-formed steel deck:			
1. Floor and roof deck welds	---	X	AWS D1.3
b. Reinforcing steel:			
1. Verification of weldability of reinforcing steel other than ASTM A706	---	X	AWS D1.4 ACI 318: Section 3.5.2
2. Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.	X	X	
3. Shear reinforcement	X	---	
4. Other reinforcing steel	---	X	

(a) Where applicable, see also Section 17.05.11, Special Inspections for seismic resistance.

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04.30.2021  
Exp. 06.30.2023

**Project:**

HOLIDAY INN EXPRESS  
THRUWAY PLAZA OF  
ROCKLAND ASSOCIATES  
CLARKSTOWN (NANUET)  
ROCKLAND COUNTY, NY  
10954

**Issued for:**

BID/PERMITS 04.30.21

PROJECT NAME: L 19180 HOLIDAY INN EXPRESS & SUITES NANUET, NY  
LOCATION: 19180  
INN CODE: NYCNT  
PROJECT: 32435  
HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

Drawn by: MAQ

Checked by: JDG, DJH, LJS

Sheet Title:  
TESTING & INSPECTION  
SCHEDULES

Project No.:  
21004052.00

Sheet No.:

S002

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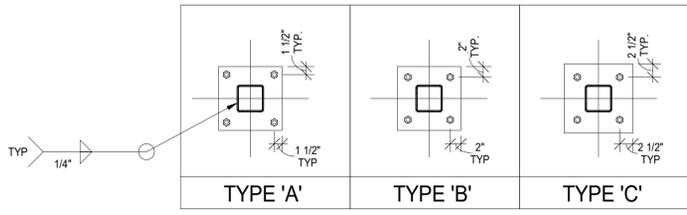
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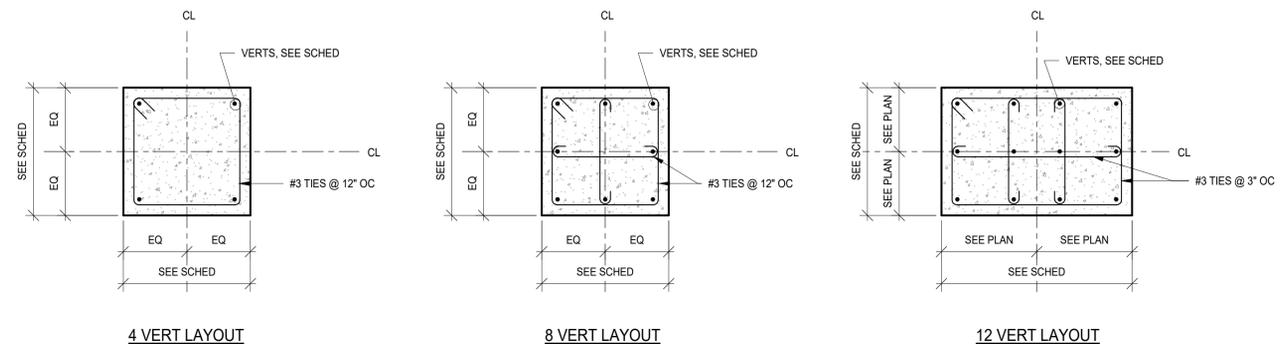
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**BASE PLATE TYPES**

3/4" = 1'-0"



**PIER LAYOUT**

3/4" = 1'-0"

**STEEL COLUMN SCHEDULE**

MARK	SIZE	BASE PL SIZE	BASE PL TYPE	ANCHOR BOLTS	BOT. OF BASE PL EL.	COMMENTS
C1	HSS 5X5X1/4	3/4"x11"x11"	A	(4)-3/4"e	99'-1 1/2"	
C2	HSS 6x6x3/8	1"x12"x12"	A	(4)-3/4"e	99'-1 1/2"	
C3	HSS 8x8x3/8	1 1/4"x14"x14"	A	(4)-1"e	99'-1 1/2"	
C4	HSS 8x8x1/2	SEE 4/S430 & 5/S430 FOR ADD'L INFO			99'-2"	
C5	HSS 10x10x1/2	1 1/2"x18"x18"	B	(4)-1"e	99'-1 1/2"	
C6	HSS 10x10x5/8	SEE 6/S430 & 7/S430 FOR ADD'L INFO			99'-2"	
C7	HSS 8x8x1/2	1 1/2"x16"x16"	C	(4)-1 1/2"e	99'-2"	1
C8	HSS 10x10x1/2	SEE 6/S430 & 7/S430 FOR ADD'L INFO			99'-2"	
C9	HSS 8x8x3/8	1"x15"x15"	B	(4)-1"e	100'-5 1/2"	2

**STL COL NOTES:**

- BASE PLATE TO BE ASTM A572 GRADE 50. ANCHOR RODS TO BE ASTM F1554 GRADE 55 W/ 8" PROJECTION & 24" EMBED. PROVIDE 1/2"x16"x16" ANCHOR PLATE W/ 6" DIA. CAST IN PIER. PROVIDE 2" GROUT.
- ANCHOR BOLTS TO HAVE 24" MINIMUM EMBEDMENT.

**CONCRETE PIER SCHEDULE**

MARK	SIZE	VERTS	#3 TIES	COMMENTS
P18	18" x 18"	(4)-#6	@12"O.C.	
P24	24" x 24"	(8)-#6	@12"O.C.	
P24-36	24" x 36"	(12)-#7	@12"O.C.	
P26	26" x 26"	(8)-#7	@12"O.C.	
P30-40	30" x 40"	(12)-#8	@6"O.C.	

**FOOTING PAD SCHEDULE**

MARK	SIZE	DEPTH	REINFORCING				REMARKS
			BOTTOM		TOP		
			LONG	TRANSV	LONG	TRANSV	
F3.0	3'-0"x3'-0"	12"	(4)-#4	(4)-#4	---	---	
F5.5	5'-0"x5'-6"	16"	(7)-#5	(7)-#5	---	---	
F6.0x10.5	6'-0"x10'-6"	18"	(7)-#6	#6@12"OC	(7)-#6	#6@12"OC	
F6.5	6'-6"x6'-6"	18"	(6)-#6	(6)-#6	---	---	
F7.0	7'-0"x7'-0"	20"	(7)-#6	(7)-#6	---	---	
F8.0	8'-0"x8'-0"	22"	(9)-#6	(9)-#6	---	---	
F8.0A	8'-0"x8'-0"	24"	(9)-#6	(9)-#6	(9)-#6	(9)-#6	
F8.5	8'-6"x8'-6"	24"	(8)-#7	(8)-#7	---	---	
F10.0x20.0	10'-0"x20'-0"	30"	(12)-#7	#7@10"OC	(12)-#7	#7@10"OC	
F10.0x24.0	10'-0"x24'-0"	30"	(13)-#7	#7@10"OC	(13)-#7	#7@10"OC	
F10.0x37.0	10'-0"x37'-0"	30"	(11)-#8	#7@10"OC	(11)-#8	#7@10"OC	
F10.5	10'-6"x10'-6"	28"	(11)-#7	(11)-#7	---	---	

**MASONRY PIER SCHEDULE**

MARK	JAMB LENGTH	REINF	COMMENTS
M1	16" LENGTH x WALL WIDTH	(2)-#5	1
M2	14" LENGTH x WALL WIDTH	(2)-#5	1

**MASONRY PIER NOTES:**

- DOWEL VERTS TO FTG AND EXTEND ONE VERT TO LINTEL BRG AND REMAINING VERTS TO TOP OF WALL. LOC ONE VERT PER CORE, CENTER VERT IN MASONRY AND GROUT CORES SOLID AT VERT LOCS.



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PROJECT # 21004052.00



**Project:**  
HOLIDAY INN EXPRESS  
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10954

**Issued for:**  
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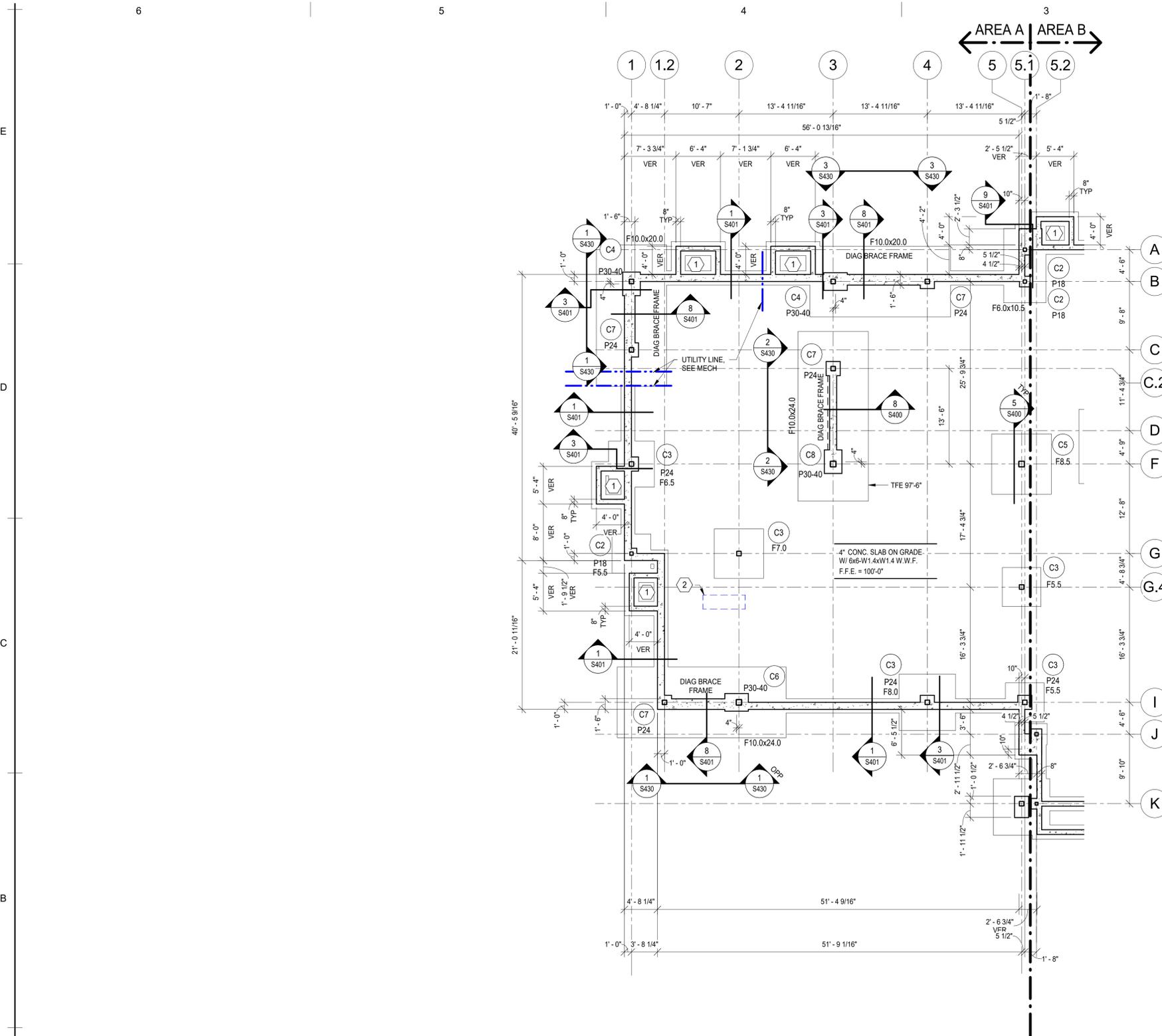
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LOCATION: 19180  
INN CODE: NYCNT  
PROJECT: 32435  
HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

Drawn by: MAQ  
Checked by: JDG, DJH, LJS

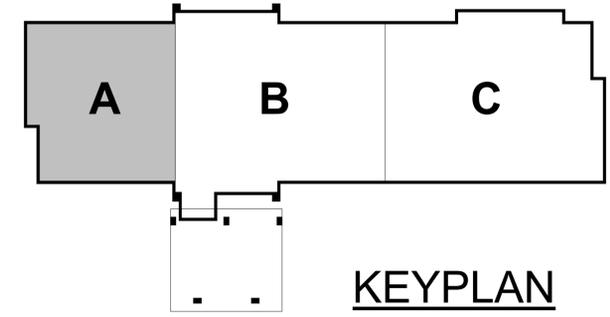
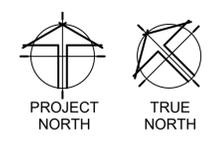
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SCHEDULES

Project No.:  
21004052.00

Sheet No.:  
**S010**



**FOUNDATION PLAN - AREA A**  
1/8" = 1'-0"



**KEYPLAN**

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**GEN'L PLAN NOTES: FOUNDATION**

- = INDICATES MECH LINE CROSSING FTG (NOT ALL SHOWN).
  - (CX) = INDICATES COL MARK. SEE SHEET S010 FOR SCHED.
  - (MX) = INDICATES MASONRY PIER. SEE SHEET S010 FOR SCHED.
  - (X) = INDICATES MASONRY WALL REINFORCING. SEE BELOW FOR REINFORCING.
1. VERIFY ALL DIMENSIONS W/ ARCH DRAWINGS.
  2. DO NOT SCALE DRAWINGS.
  3. PX INDICATES CONC PIER BELOW GRADE. SEE SHEET S010 FOR SCHED.
  4. FX INDICATES CONC FTG. SEE SHEET S010 FOR SCHED.
  5. TYP EXT TFE = 97'-6"  
TYP INT TFE = 99'-0"
  6. TYP STEPPED FTG. SEE SECTION 1/S400.
  7. TYP ANCHOR ROD DETAIL. SEE SECTION 4/S400.
  8. CONTRACTOR TO VERIFY UNDERGROUND UTILITIES LOCATIONS AND INVERT ELEVATIONS. DROP TOP OF FTG ELEVATIONS AS REQ'D TO ALLOW MECH PIPE TO PASS OVER FTG.
  9. PROVIDE PIPE SLEEVES AT ALL LOCS WHERE MECH PIPES PENETRATE WALL. VER LOCS WITH MECH. DWGS, SEE SECTION 2/S400.
  10. SEE ARCH DWGS FOR LOC OF WALLS NOT DIMENSIONED ON PLAN.
  11. REFER TO ARCH DWGS FOR BELOW GRADE WATERPROOFING DETAILS.
  12. VERIFY DEPRESSED OR RECESSED SLAB LOCS & DIMS WITH ARCHITECT.
  13. ALL EXPOSED STEEL TO BE HOT-DIPPED GALVANIZED. TOUCH UP ALL WELDS W/ ZRC PAINT.

**KEY PLAN NOTES: FOUNDATION**

1. 5" MIN CONC STOOP SLAB OVER 1.0C-22GA STL FORM DECK W/ #4 @ 12" OC EA WAY BOT. #4 TOP DWLS @ 16" OC PROVIDE 6" MIN VOID BELOW CONC SLAB. TOP OF SLAB EL VARIES. SEE ARCH. VERIFY SIZE, LOCATION AND QUANTITY WITH ARCHITECT. SEE SECTION 3/S400.
2. PROVIDE THICKENED SLAB 12" DEEP x 24" WIDE AT BASE OF STAIR W/ (2)-#5 CONT. COORD LOC & LENGTH W/ ARCH. SEE 5/S401.
3. ELEVATOR DIVIDER BEAM. VERIFY QUANTITY, LOCATION AND ELEVATION WITH ELEVATOR SUPPLIER.
4. INFILL WITH MASONRY PIER M2 AFTER ELEVATOR IS INSTALLED.

**WALL FTG WIDTH**

ALL CONT WALL FTG SIZES NOT DIM ON PLAN SHALL BE AS FOLLOWS (SEE FDN NOTES FOR DEPTH & REINF) UNLESS NOTED OR DETAILED OTHERWISE

WALL WIDTH	FTG WIDTH
8"	2'-0"
10"	2'-0"
1'-0"	2'-0"
1'-2"	2'-2"
1'-4"	2'-4"
1'-6"	2'-6"
1'-8"	2'-8"



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FAX: 309.788.5967  
PROJECT # 21004052.00



**Project:**  
HOLIDAY INN EXPRESS  
THRUWAY PLAZA OF  
ROCKLAND ASSOCIATES  
CLARKSTOWN (NANUET)  
ROCKLAND COUNTY, NY  
10954

**Issued for:**  
BID/PERMITS 04.30.21

PROJECT NAME: L 19180 HOLIDAY INN EXPRESS & SUITES NANUET, NY  
LOCATION: 19180  
INN CODE: NYCNT  
PROJECT: 32435  
HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

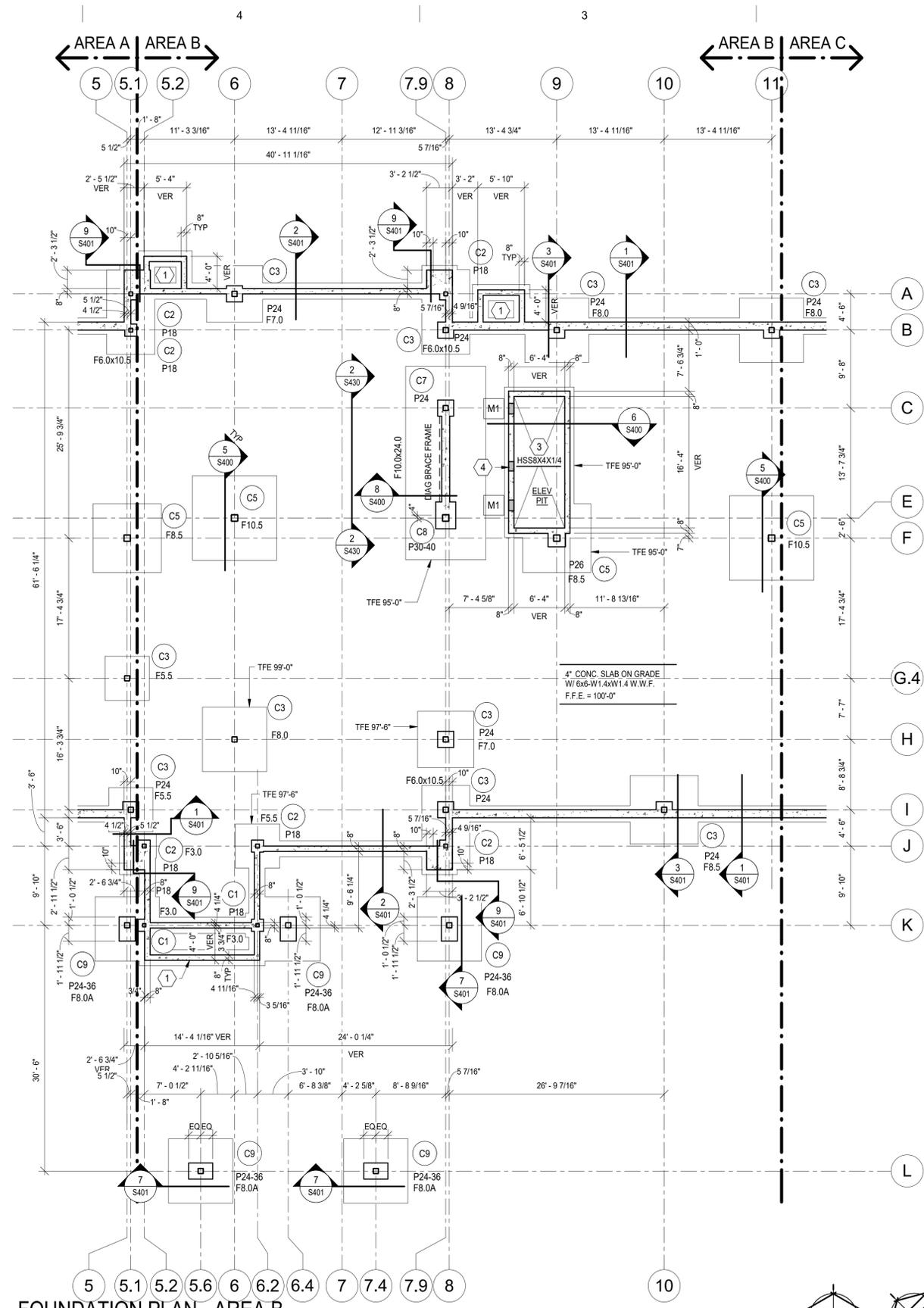
Drawn by: MAQ  
Checked by: JDG, DJH, LJS

Sheet Title:  
FOUNDATION PLAN - AREA A

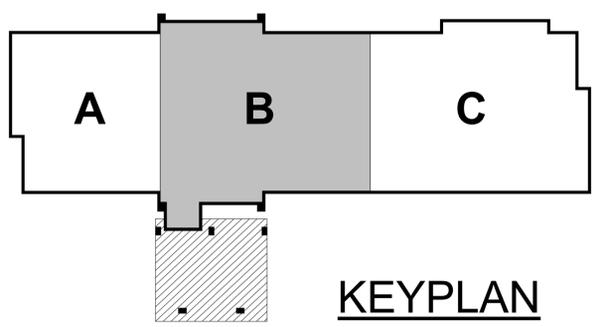
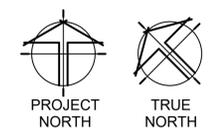
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FOUNDATION PLAN - AREA B  
1/8" = 1'-0"



KEYPLAN

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309.788.5967  
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New York Design Firm Registration #0012979

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REFERENCE SCALE IN INCHES  
0 1 2 3

GEN'L PLAN NOTES: FOUNDATION

- = INDICATES MECH LINE CROSSING FTG (NOT ALL SHOWN).
  - ⊗ = INDICATES COL MARK. SEE SHEET S010 FOR SCHED.
  - ⊗ = INDICATES MASONRY PIER. SEE SHEET S010 FOR SCHED.
  - ⊗ = INDICATES MASONRY WALL REINFORCING. SEE BELOW FOR REINFORCING.
1. VERIFY ALL DIMENSIONS W/ ARCH DRAWINGS.
  2. DO NOT SCALE DRAWINGS.
  3. PX INDICATES CONC PIER BELOW GRADE. SEE SHEET S010 FOR SCHED.
  4. FX INDICATES CONC FTG. SEE SHEET S010 FOR SCHED.
  5. TYP EXT TFE = 97'-6"  
TYP INT TFE = 99'-0"
  6. TYP STEPPED FTG, SEE SECTION 1/S400.
  7. TYP ANCHOR ROD DETAIL, SEE SECTION 4/S400.
  8. CONTRACTOR TO VERIFY UNDERGROUND UTILITIES LOCATIONS AND INVERT ELEVATIONS. DROP TOP OF FTG ELEVATIONS AS REQ'D TO ALLOW MECH PIPE TO PASS OVER FTG.
  9. PROVIDE PIPE SLEEVES AT ALL LOCS WHERE MECH PIPES PENETRATE WALL. VER LOCS WITH MECH. DWGS, SEE SECTION 2/S400.
  10. SEE ARCH DWGS FOR LOC OF WALLS NOT DIMENSIONED ON PLAN.
  11. REFER TO ARCH DWGS FOR BELOW GRADE WATERPROOFING DETAILS.
  12. VERIFY DERESSED OR RECESSED SLAB LOCS & DIMS WITH ARCHITECT.
  13. ALL EXPOSED STEEL TO BE HOT-DIPPED GALVANIZED. TOUCH UP ALL WELDS W/ ZRC PAINT.

KEY PLAN NOTES: FOUNDATION

1. 5" MIN CONC STOOP SLAB OVER 1.0C-22GA STL FORM DECK W/ #4 @ 12" OC EA WAY BOT. #4 TOP DWLS @ 15" OC PROVIDE 6" MIN VOID BELOW CONC SLAB. TOP OF SLAB EL VARIES. SEE ARCH. VERIFY SIZE, LOCATION AND QUANTITY WITH ARCHITECT. SEE SECTION 3/S400.
2. PROVIDE THICKENED SLAB 12" DEEP x 24" WIDE AT BASE OF STAIR W/ (2)#5 CONT. COORD LOC & LENGTH W/ ARCH. SEE 5/S401.
3. ELEVATOR DIVIDER BEAM. VERIFY QUANTITY, LOCATION AND ELEVATION WITH ELEVATOR SUPPLIER.
4. INFILL WITH MASONRY PIER M2 AFTER ELEVATOR IS INSTALLED.

MASONRY WALL REINFORCING:

- TYP CMU WALL REINF: UNO PROVIDE #5 VERTS @ 48" OC, DWL TO CONC FTG OR CONC FDN WALL. EXTEND VERT TO BOND BM AT TOP OF WALL. CENTER VERT IN CORE & GROUT WALL SOLID AT VERT LOCS.
1. PROVIDE #6 VERTS @ 24" OC DOWEL VERTS TO CONC WALL & EXTEND TO ROOF BOND BM. GROUT BLOCK CORES SOLID AT VERT LOCS. CENTER VERTS IN CMU.
  2. PROVIDE #6 VERTS @ 8" OC DOWEL VERTS TO CONC WALL & EXTEND TO ROOF BOND BM. GROUT BLOCK CORES SOLID AT VERT LOCS. CENTER VERTS IN CMU. PROVIDE 8" HIGH x WALL WIDTH BOND BM W/ (2)#5 CONT. @ 48" OC VERTICALLY.

WALL FTG WIDTH

ALL CONT WALL FTG SIZES NOT DIM ON PLAN SHALL BE AS FOLLOWS (SEE FDN NOTES FOR DEPTH & REINF) UNLESS NOTED OR DETAILED OTHERWISE

WALL WIDTH	FTG WIDTH
8"	2'-0"
10"	2'-0"
1'-0"	2'-0"
1'-2"	2'-2"
1'-4"	2'-4"
1'-6"	2'-6"
1'-8"	2'-8"



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ROCKLAND COUNTY, NY  
10954

Issued for:  
BID/PERMITS 04.30.21

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LOCATION: 19180 INN CODE: NYCN  
PROJECT: 32435  
HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

Drawn by: MAQ  
Checked by: JDG, DJH, LJS

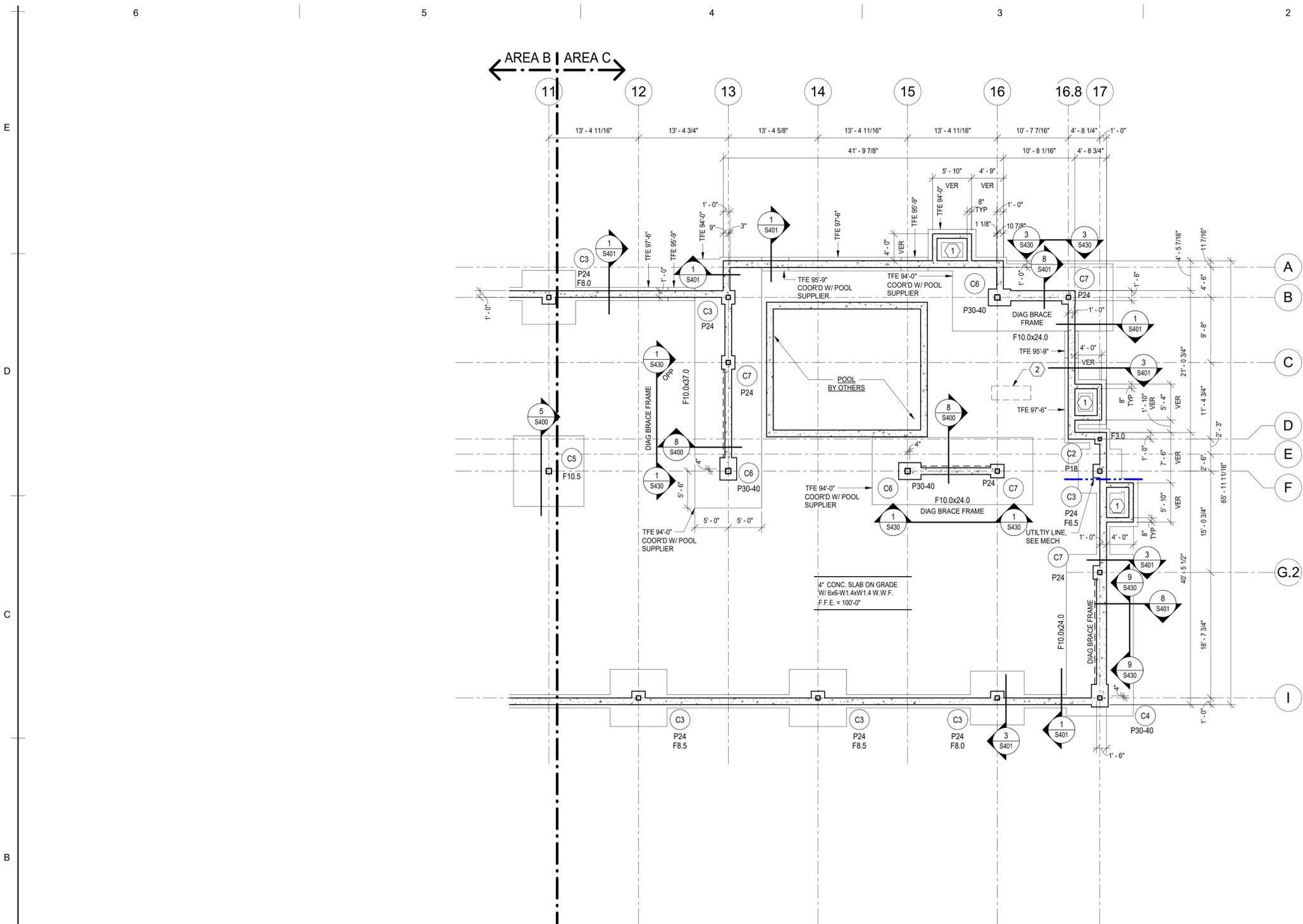
Sheet Title:  
FOUNDATION PLAN - AREA B

Project No.:  
21004052.00

Sheet No.:  
S112

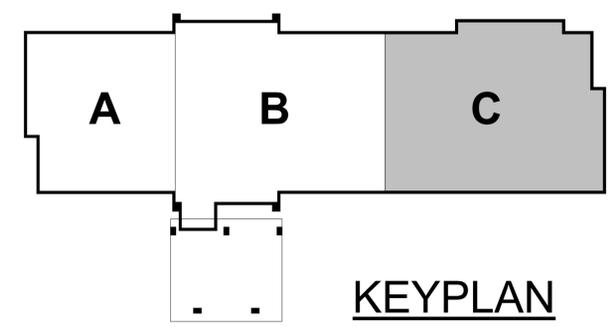
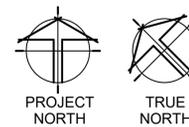
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4/28/2021 9:38:41 AM



**FOUNDATION PLAN - AREA C**

1/8" = 1'-0"



**KEYPLAN**

**WALL FTG WIDTH**

ALL CONT WALL FTG SIZES NOT DIM ON PLAN SHALL BE AS FOLLOWS (SEE FDN NOTES FOR DEPTH & REINF) UNLESS NOTED OR DETAILED OTHERWISE

WALL WIDTH	FTG WIDTH
8"	2'-0"
10"	2'-0"
1'-0"	2'-0"
1'-2"	2'-2"
1'-4"	2'-4"
1'-6"	2'-6"
1'-8"	2'-8"

**GEN'L PLAN NOTES: FOUNDATION**

- = INDICATES MECH LINE CROSSING FTG (NOT ALL SHOWN).
  - ⊗ = INDICATES COL MARK. SEE SHEET S010 FOR SCHED.
  - ⊗ MX = INDICATES MASONRY PIER. SEE SHEET S010 FOR SCHED.
  - ⊗ X = INDICATES MASONRY WALL REINFORCING. SEE BELOW FOR REINFORCING.
1. VERIFY ALL DIMENSIONS W/ ARCH DRAWINGS.
  2. DO NOT SCALE DRAWINGS.
  3. PX INDICATES CONC PIER BELOW GRADE. SEE SHEET S010 FOR SCHED.
  4. FX INDICATES CONC FTG. SEE SHEET S010 FOR SCHED.
  5. TYP EXT TFE = 97'-6"  
TYP INT TFE = 99'-0"
  6. TYP STEPPED FTG, SEE SECTION 1/S400.
  7. TYP ANCHOR ROD DETAIL, SEE SECTION 4/S400.
  8. CONTRACTOR TO VERIFY UNDERGROUND UTILITIES LOCATIONS AND INVERT ELEVATIONS. DROP TOP OF FTG ELEVATIONS AS REQ'D TO ALLOW MECH PIPE TO PASS OVER FTG.
  9. PROVIDE PIPE SLEEVES AT ALL LACS WHERE MECH PIPES PENETRATE WALL. VER LACS WITH MECH. DWGS, SEE SECTION 2/S400.
  10. SEE ARCH DWGS FOR LOC OF WALLS NOT DIMENSIONED ON PLAN.
  11. REFER TO ARCH DWGS FOR BELOW GRADE WATERPROOFING DETAILS.
  12. VERIFY DEPRESSED OR RECESSED SLAB LACS & DIMS WITH ARCHITECT.
  13. ALL EXPOSED STEEL TO BE HOT-DIPPED GALVANIZED. TOUCH UP ALL WELDS W/ ZRC PAINT.

**KEY PLAN NOTES: FOUNDATION**

1. 5" MIN CONC STOOP SLAB OVER 1.0C-22GA STL FORM DECK W/ #4 @ 12" OC EA WAY BOT. #4 TOP DWLS @ 16" OC PROVIDE 6" MIN VOID BELOW CONC SLAB. TOP OF SLAB EL VARIES. SEE ARCH. VERIFY SIZE, LOCATION AND QUANTITY WITH ARCHITECT. SEE SECTION 3/S400.
2. PROVIDE THICKENED SLAB 12" DEEP x 24" WIDE AT BASE OF STAIR W/ (2)-#5 CONT. COORD LOC & LENGTH W/ ARCH. SEE 5/S401.
3. ELEVATOR DIVIDER BEAM. VERIFY QUANTITY, LOCATION AND ELEVATION WITH ELEVATOR SUPPLIER.
4. INFILL WITH MASONRY PIER M2 AFTER ELEVATOR IS INSTALLED.

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ROCKLAND COUNTY, NY  
10954

**Issued for:**

BID/PERMITS 04.30.21

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LOCATION: 19180  
INN CODE: NYCNT  
PROJECT: 32435  
HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

Drawn by: MAQ  
Checked by: JDG, DJH, LJS

Sheet Title:  
FOUNDATION PLAN - AREA C

Project No.:  
21004052.00

Sheet No.:  
**S113**

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 LOCATION: 19180 INN CODE: NYCNT  
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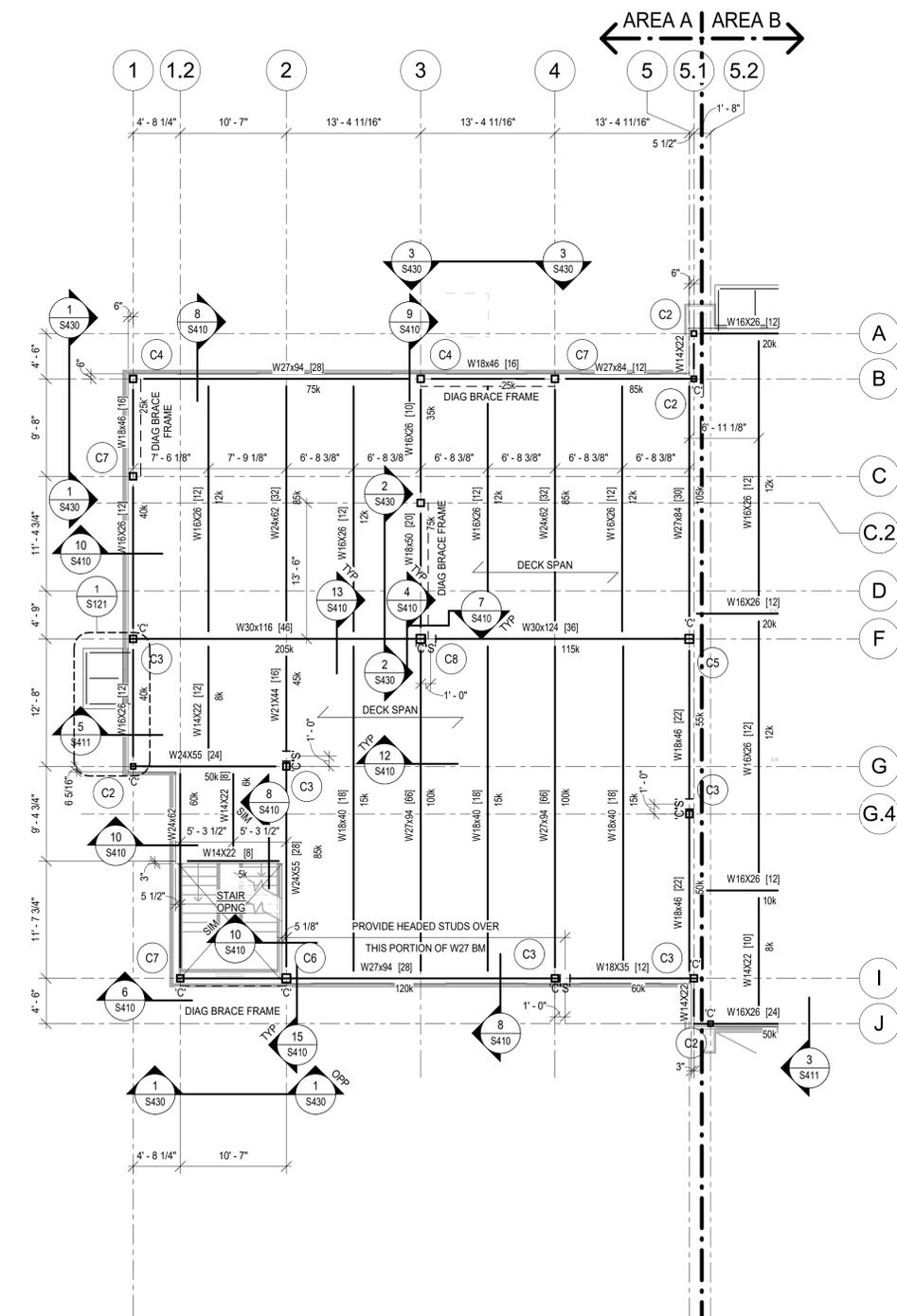
Drawn by: MAQ  
 Checked by: JDG, DJH, LJS

Sheet Title:  
 SECOND FLOOR FRAMING PLAN - AREA A

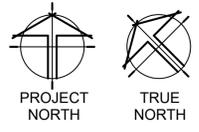
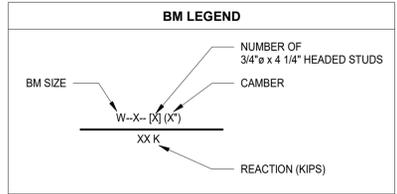
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Sheet No.:  
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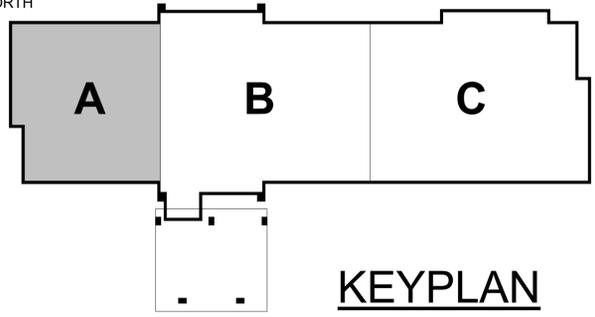
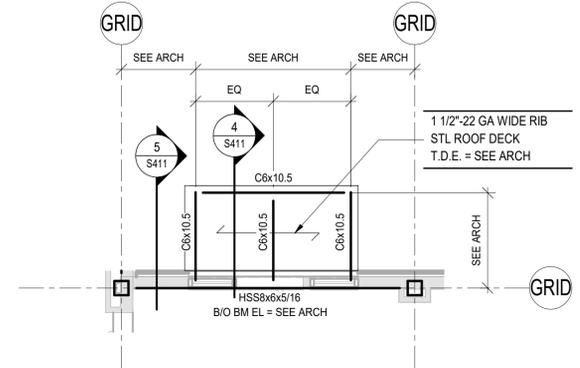
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SECOND FLOOR FRAMING PLAN - AREA A  
 1/8" = 1'-0"



1 TYPICAL CANOPY FRAMING PLAN  
 S121 1/4" = 1'-0"



KEYPLAN

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6 5 4 3 2 1

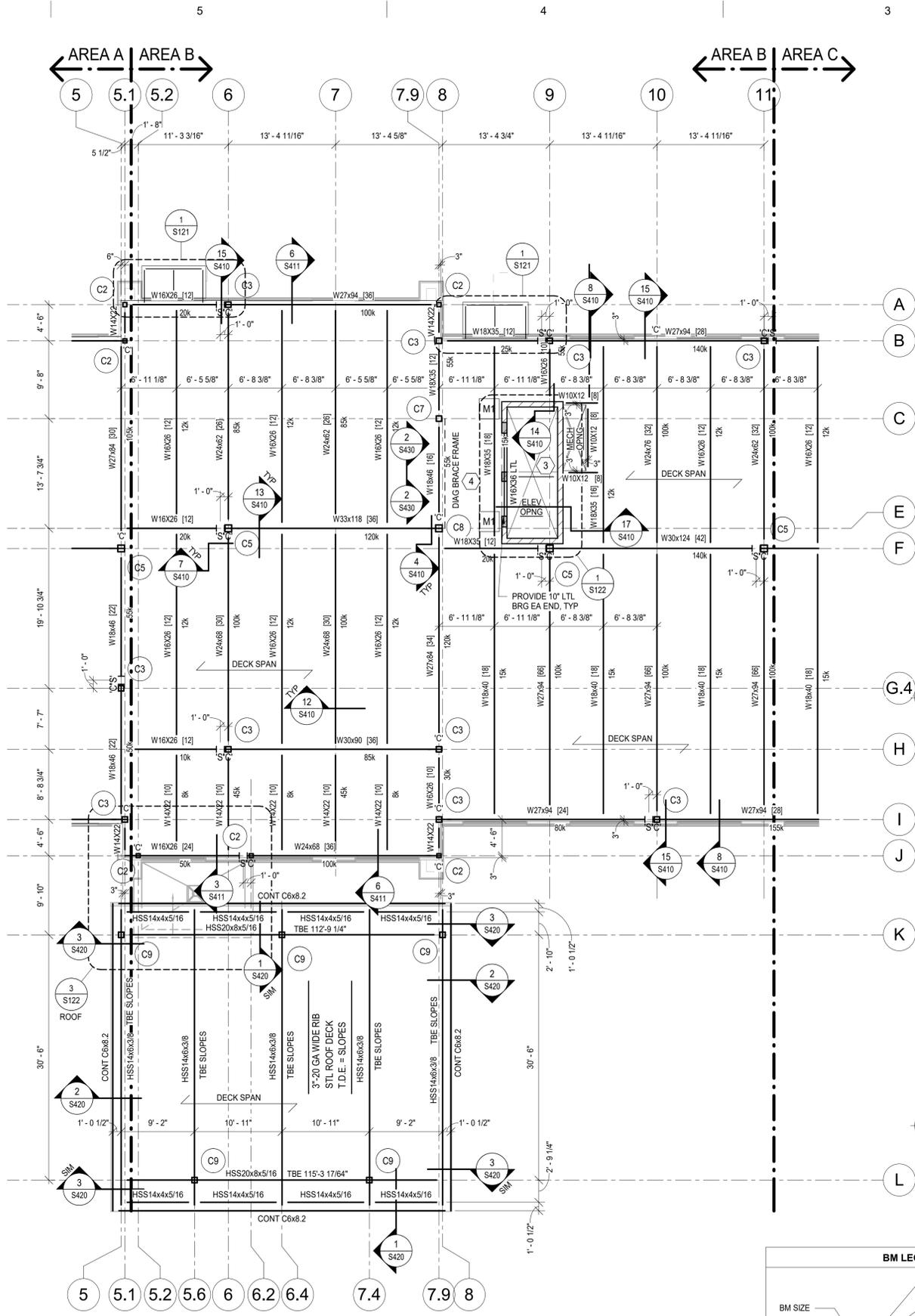
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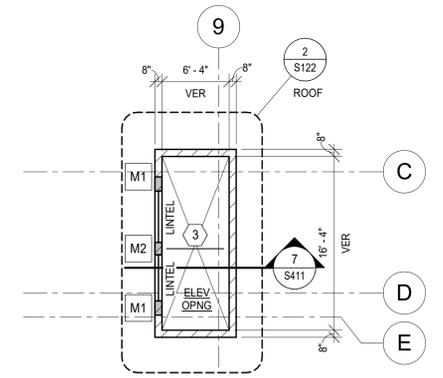


SECOND FLOOR FRAMING PLAN - AREA B  
1/8" = 1'-0"

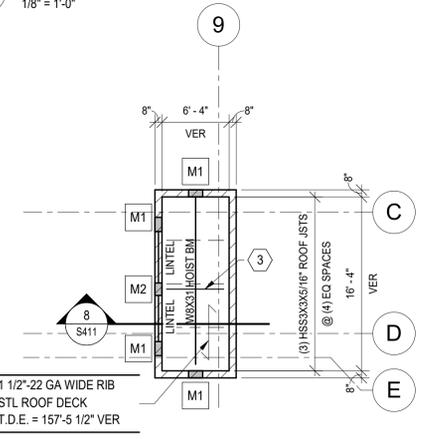
**MASONRY LINTELS (TYP. UNO):**  
1. PROVIDE W8x10 W/ 1/4" STL PL (PL WIDTH TO BE TOTAL WALL WIDTH MINUS 1") 4'-0" TO 7'-0" MAX SPAN.

**MASONRY WALL REINFORCING:**  
• TYP CMU WALL REINF: UNO PROVIDE #5 VERTS @ 48" OC, DWL TO CONC FTG OR CONC FDM WALL. EXTEND VERT TO BOND BM AT TOP OF WALL. CENTER VERT IN CORE & GROUT WALL SOLID AT VERT LOCS.

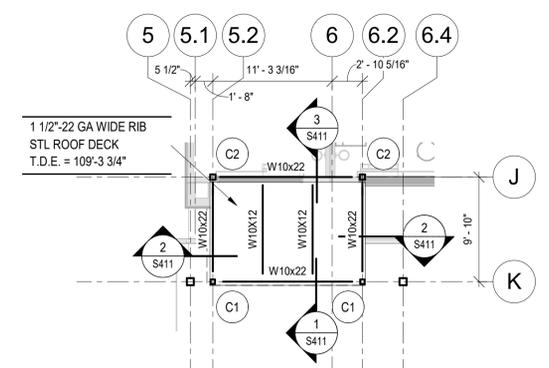
- 1 PROVIDE #6 VERTS @ 24" OC DOWEL VERTS TO CONC WALL & EXTEND TO ROOF BOND BM. GROUT BLOCK CORES SOLID AT VERT LOCS. CENTER VERTS IN CMU.
- 2 PROVIDE #6 VERTS @ 8" OC DOWEL VERTS TO CONC WALL & EXTEND TO ROOF BOND BM. GROUT BLOCK CORES SOLID AT VERT LOCS. CENTER VERTS IN CMU. PROVIDE 8" HIGH X WALL WIDTH BOND BM W/ (2) #5 CONT. @ 48" OC VERTICALLY.



3RD, 4TH & 5TH FLOOR FRAMING PLAN AT ELEV.  
1/8" = 1'-0"



ELEVATOR ROOF FRAMING PLAN  
1/8" = 1'-0"



PARTIAL ROOF FRAMING PLAN  
1/8" = 1'-0"

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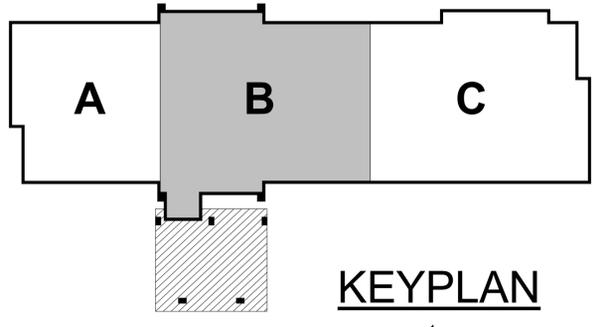
REFERENCE SCALE IN INCHES  
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**GEN'L PLAN NOTES: FLOOR FRAMING**

- (CX) = INDICATES COL MARK. SEE SHEET S010 FOR SCHED.
- 1. VERIFY ALL DIMENSIONS WITH ARCH DRAWINGS.
- 2. DO NOT SCALE DRAWINGS.
- 3. 4 1/4" TOTAL COMPOSITE LIGHTWEIGHT CONC FLOOR SLAB W/ #4 @ 12" OC EA WAY, OVER 1 1/2" 20 GA COMPOSITE STL FLOOR DECK OR APPROVED EQUAL (4 1/4" CONC SLAB + 1 1/2" DECK = 5 3/4" TOTAL SLAB THICKNESS). TSE = 113'-0 1/8"
- 4. PREFABBED MODULAR WOOD FRAMED UNITS TO BEAR DIRECTLY ON COMPOSITE SLAB. MODULAR CONSTRUCTION BY OTHERS.
- 5. STEEL FRAME AND MASONRY WALLS TO BE LATERALLY SUPPORTED UNTIL ALL INTEGRITY CONNECTIONS ARE IN PLACE.
- 6. COORDINATE ALL PENETRATIONS THROUGH SLAB WITH MODULAR DRAWINGS BY OTHERS AND ARCHITECTURAL DRAWINGS.
- 7. FOR PERMISSABLE CONST JT LOCS IN COMPOSITE SLABS, SEE 2/S410.
- 8. FOR OPGS IN COMPOSITE SLABS, SEE 1/S410. FOR OPGS LARGER THAN 24", PROVIDE W10X12 FRAME AROUND OPG. VER SIZE, LOC AND QTY WITH ARCH & MECH.
- 9. SEE ARCH DWGS FOR LOC OF WALLS NOT DIMENSIONED ON PLAN.
- 10. FOR SCHEDULES SEE SHEET S010.
- 11. SEE FOUNDATION PLANS FOR VERT WALL REINFORCEMENT.
- 12. SEE FOUNDATION PLANS FOR MASONRY PIERS NOT SHOWN.
- 13. ALL REACTIONS SHOWN ON PLAN ARE UNFACTORED.
- 14. REFER TO APPROVED ELEVATOR SHOP DRAWINGS FOR HOIST BEAM AND TIE OFF BEAM REQUIREMENTS. PROVIDE POCKETS IN MASONRY WALL ACCORDINGLY.
- 15. ALL EXPOSED STEEL TO BE HOT-DIPPED GALVANIZED. TOUCH UP ALL WELDS W/ ZRC PAINT.

**KEY PLAN NOTES: FLOOR FRAMING**

- 1 PROVIDE BRG PL 1/2"x7"x12" W/ (2)-1/2" x 4" LG. HEADED STUDS.
- 2 10" STEEL STUD JOISTS @ 24" OC MAX BY STUD SUPPLIER. DESIGN STUDS FOR 81 PSF SL & 85 PSF DL.
- 3 HSS 8X4X1/4 ELEVATOR DIVIDER BEAM. VERIFY QUANTITY, LOCATION AND ELEVATION WITH ELEVATOR SUPPLIER.
- 4 INFILL WITH MASONRY PIER M2 AFTER ELEVATOR IS INSTALLED.



KEYPLAN



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**HOTEL:** HOLIDAY INN EXPRESS & SUITES NANUET

**Drawn by:** MAQ  
**Checked by:** JDG, DJH, LJS

**Sheet Title:**  
SECOND FLOOR FRAMING PLAN - AREA B

**Project No.:**  
21004052.00

**Sheet No.:**  
S122

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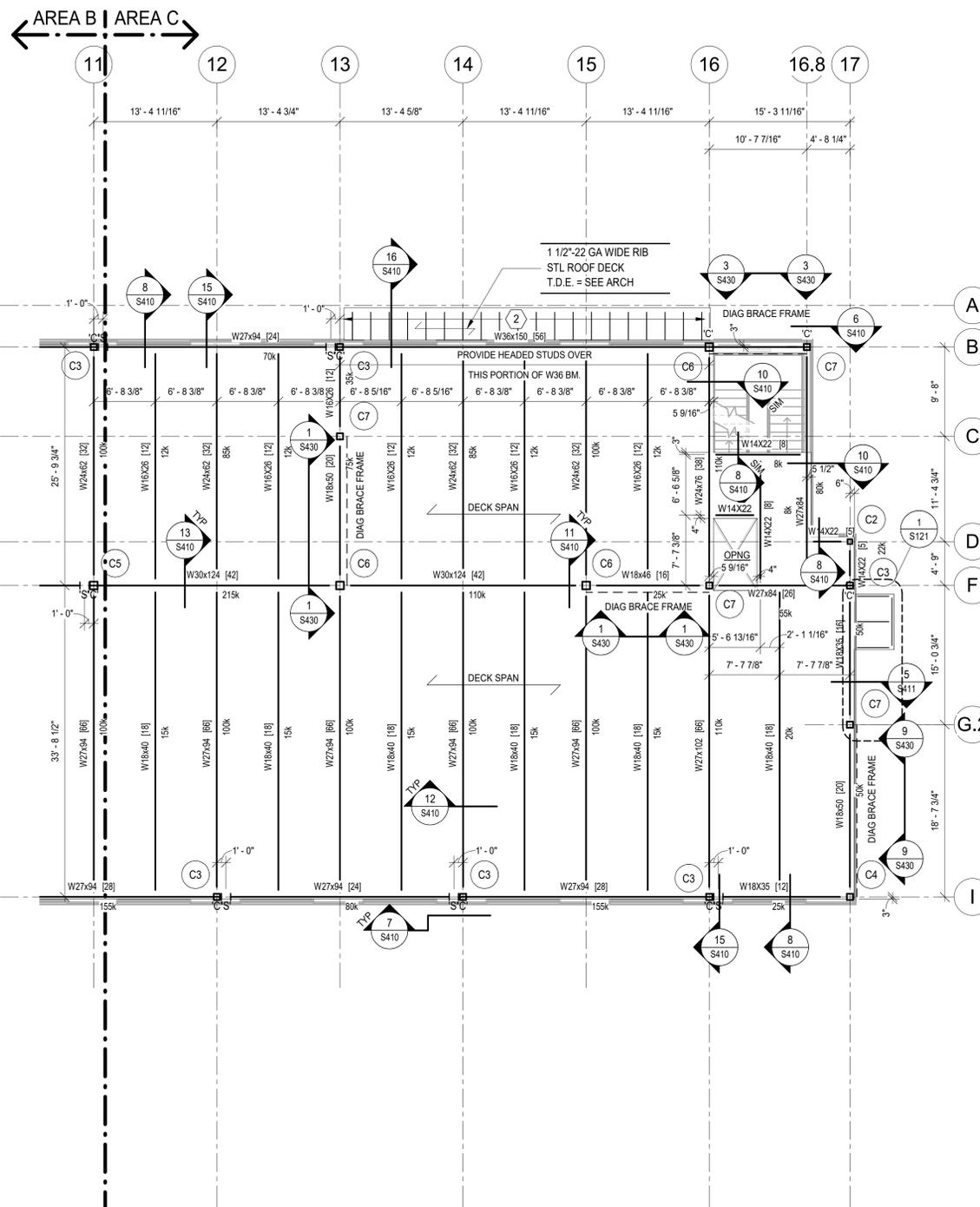
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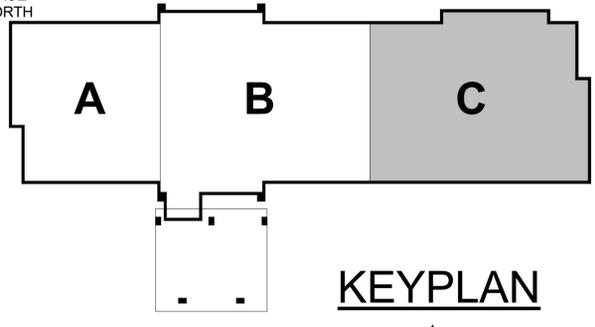
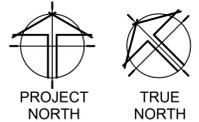
C

B

A



**SECOND FLOOR FRAMING PLAN - AREA C**  
1/8" = 1'-0"



**KEYPLAN**

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PROJECT # 21004052.00

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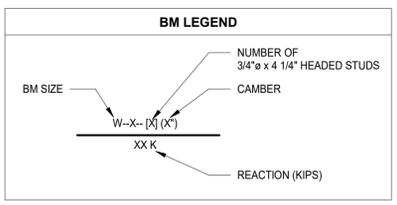
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- GEN'L PLAN NOTES: FLOOR FRAMING**
- (CX) = INDICATES COL MARK. SEE SHEET S010 FOR SCHED.
  - 1. VERIFY ALL DIMENSIONS WITH ARCH DRAWINGS.
  - 2. DO NOT SCALE DRAWINGS.
  - 3. 4 1/4" TOTAL COMPOSITE LIGHTWEIGHT CONC FLOOR SLAB W/ #4 @ 12" OC EA WAY, OVER 1 1/2"-20 GA COMPOSITE STL FLOOR DECK OR APPROVED EQUAL (4 1/4" CONC SLAB + 1 1/2" DECK = 5 3/4" TOTAL SLAB THICKNESS). TSE = 113'-0 1/8"
  - 4. PREFABBED MODULAR WOOD FRAMED UNITS TO BEAR DIRECTLY ON COMPOSITE SLAB. MODULAR CONSTRUCTION BY OTHERS.
  - 5. STEEL FRAME AND MASONRY WALLS TO BE LATERALLY SUPPORTED UNTIL ALL INTEGRITY CONNECTIONS ARE IN PLACE.
  - 6. COORDINATE ALL PENETRATIONS THROUGH SLAB WITH MODULAR DRAWINGS BY OTHERS AND ARCHITECTURAL DRAWINGS.
  - 7. FOR PERMISSABLE CONST JT LOCS IN COMPOSITE SLABS, SEE 2/S410.
  - 8. FOR OPGS IN COMPOSITE SLABS, SEE 1/S410. FOR OPGS LARGER THAN 24", PROVIDE W10X12 FRAME AROUND OPG. VER SIZE, LOC AND QTY WITH ARCH & MECH.
  - 9. SEE ARCH DWGS FOR LOC OF WALLS NOT DIMENSIONED ON PLAN.
  - 10. FOR SCHEDULES SEE SHEET S010.
  - 11. SEE FOUNDATION PLANS FOR VERT WALL REINFORCEMENT.
  - 12. SEE FOUNDATION PLANS FOR MASONRY PIERS NOT SHOWN.
  - 13. ALL REACTIONS SHOWN ON PLAN ARE UNFACTORED.
  - 14. REFER TO APPROVED ELEVATOR SHOP DRAWINGS FOR HOIST BEAM AND TIE OFF BEAM REQUIREMENTS. PROVIDE POCKETS IN MASONRY WALL ACCORDINGLY.
  - 15. ALL EXPOSED STEEL TO BE HOT-DIPPED GALVANIZED. TOUCH UP ALL WELDS W/ZRC PAINT.

- KEY PLAN NOTES: FLOOR FRAMING**
- 1. PROVIDE BRG PL 1/2"x7"x12" W/ (2)-1/2"x4"x4" LG. HEADED STUDS.
  - 2. 10" STEEL STUD JOISTS @ 24" OC MAX BY STUD SUPPLIER. DESIGN STUDS FOR 81 PSF SL & 85 PSF DL.
  - 3. HSS 8X4X1/4 ELEVATOR DIVIDER BEAM. VERIFY QUANTITY, LOCATION AND ELEVATION WITH ELEVATOR SUPPLIER.
  - 4. INFILL WITH MASONRY PIER M2 AFTER ELEVATOR IS INSTALLED.



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**Project:**

HOLIDAY INN EXPRESS  
THRUWAY PLAZA OF  
ROCKLAND ASSOCIATES  
CLARKSTOWN (NANUET)  
ROCKLAND COUNTY, NY  
10954

**Issued for:**

BID/PERMITS 04.30.21

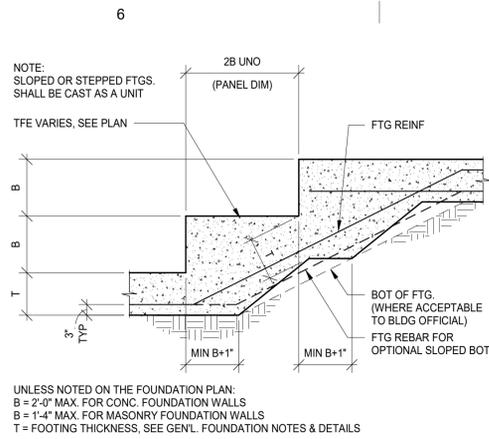
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LOCATION: 19180  
INN CODE: NYCNT  
PROJECT: 32435  
HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

Drawn by: MAQ  
Checked by: JDG, DJH, LJS

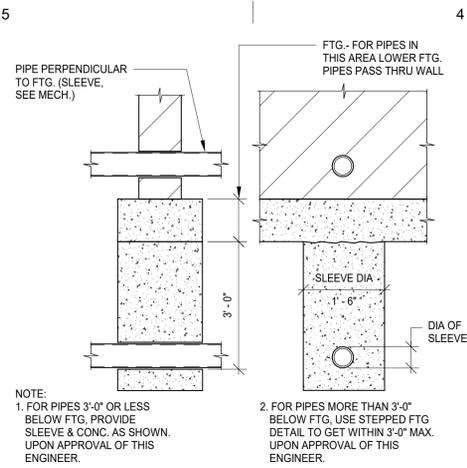
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PLAN - AREA C

Project No.:  
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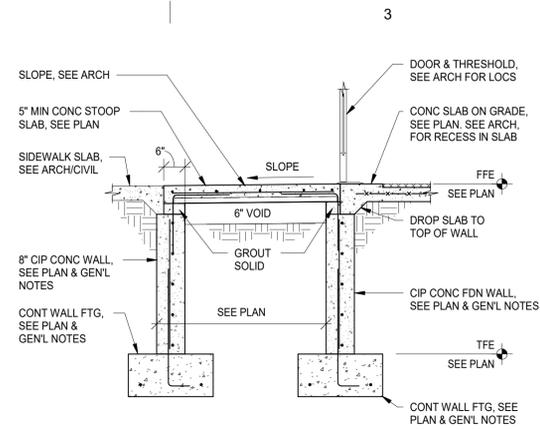
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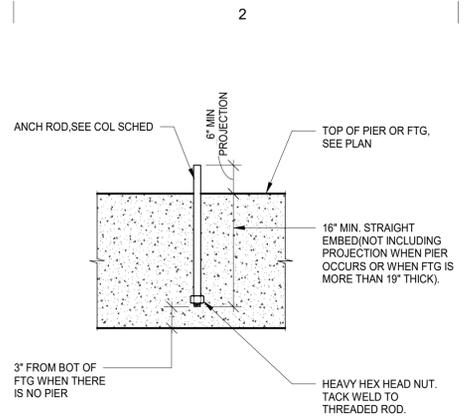
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S400 1/2" = 1'-0"



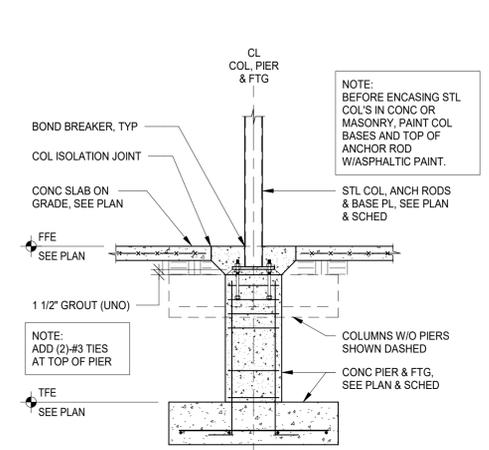
**2** FOUNDATION PIPE SLEEVE DETAIL  
S400 1/2" = 1'-0"



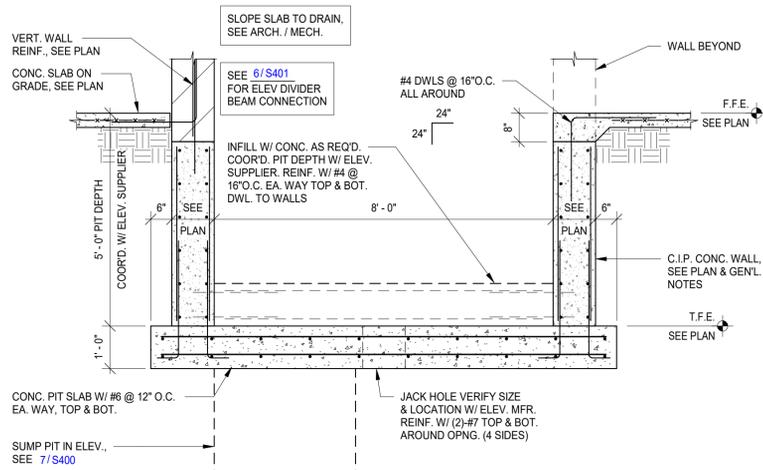
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S400 1/2" = 1'-0"



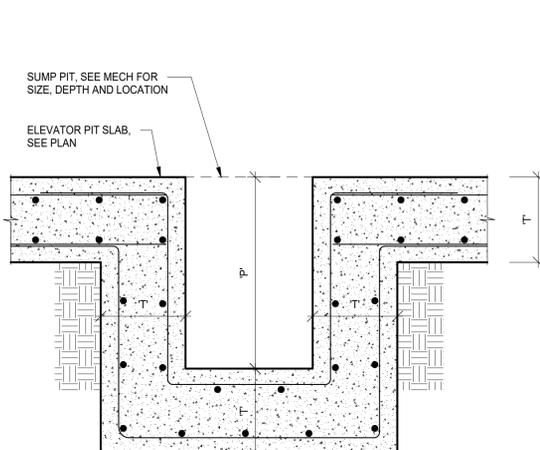
**4** TYPICAL ANCHOR ROD DETAIL  
S400 1/2" = 1'-0"



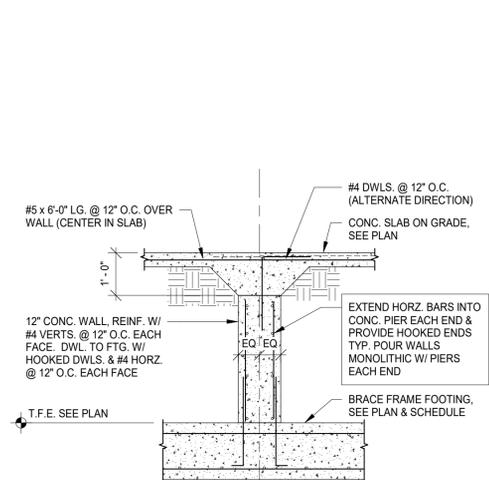
**5** TYPICAL INTERIOR COLUMN & PIER  
S400 1/2" = 1'-0"



**6** TYPICAL ELEV. PIT DETAIL  
S400 1/2" = 1'-0"



**7** ELEVATOR SUMP PIT  
S400 3/4" = 1'-0"



**8** SECTION - AT BRACE FRAME  
S400 1/2" = 1'-0"

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04.30.2021  
Exp. 06.30.2023

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ROCKLAND COUNTY, NY  
10954

Issued for:

BID/PERMITS 04.30.21

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LOCATION: 19180  
INN CODE: NYCNT  
PROJECT: 32435  
HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

Drawn by: MAQ

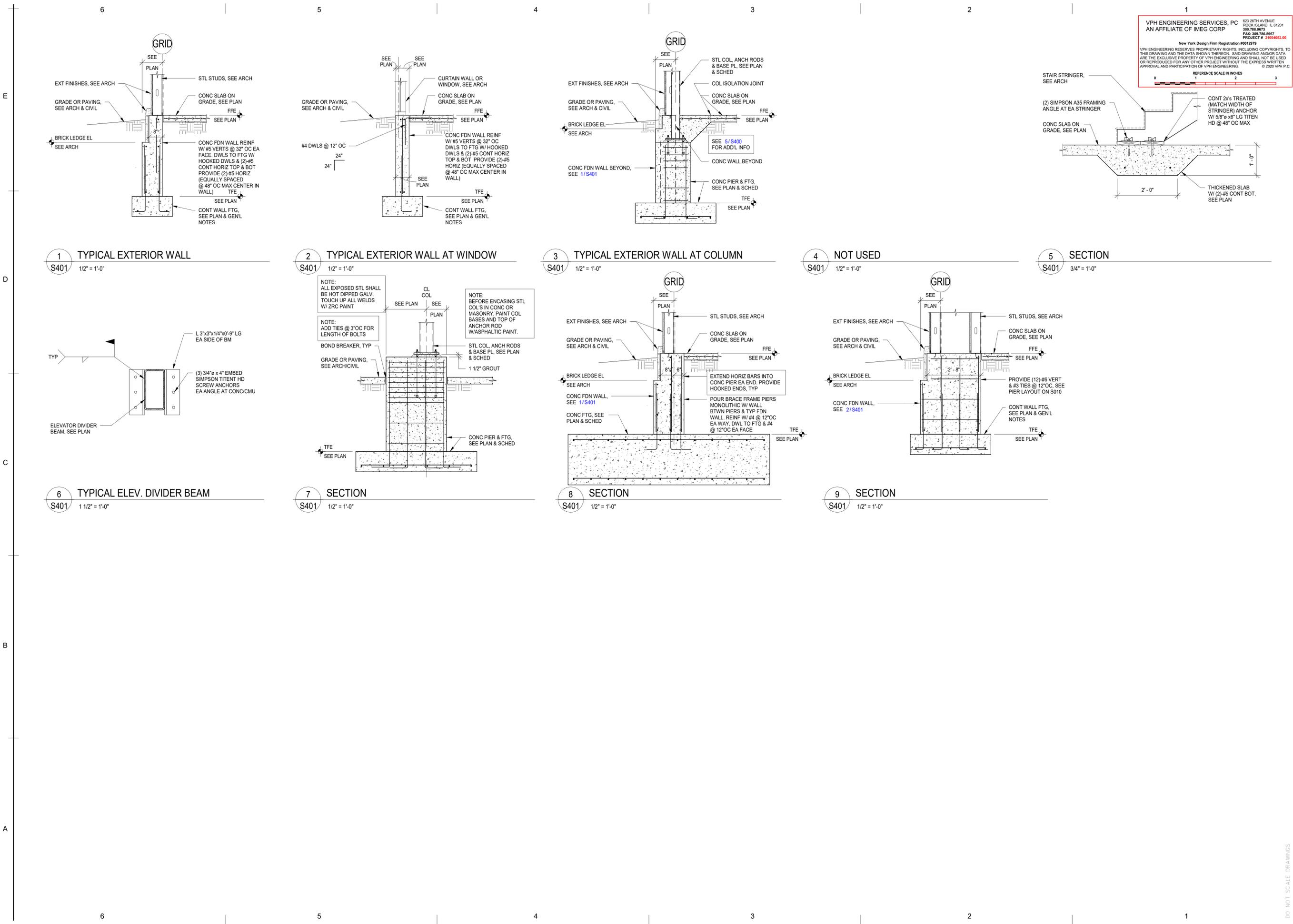
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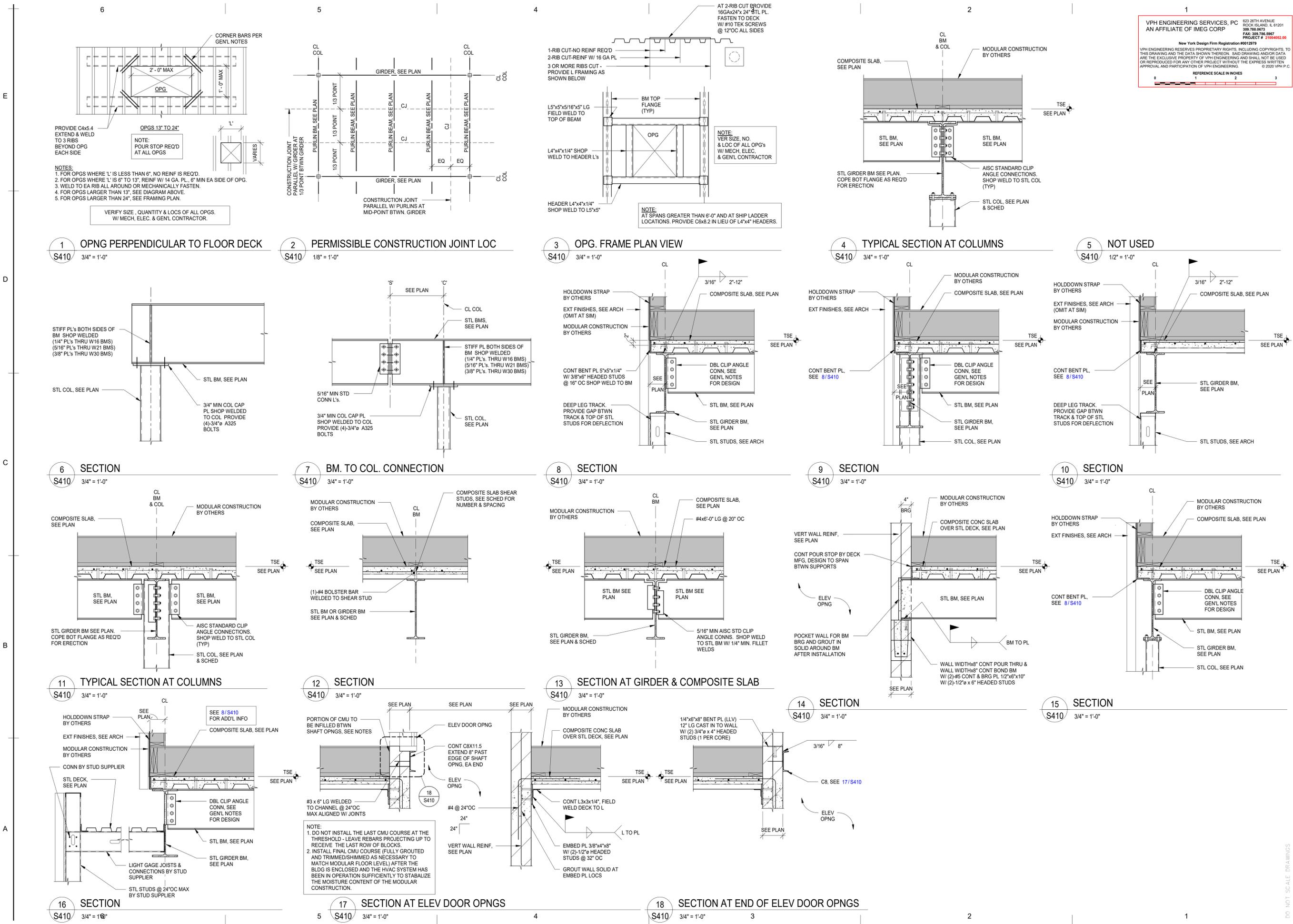
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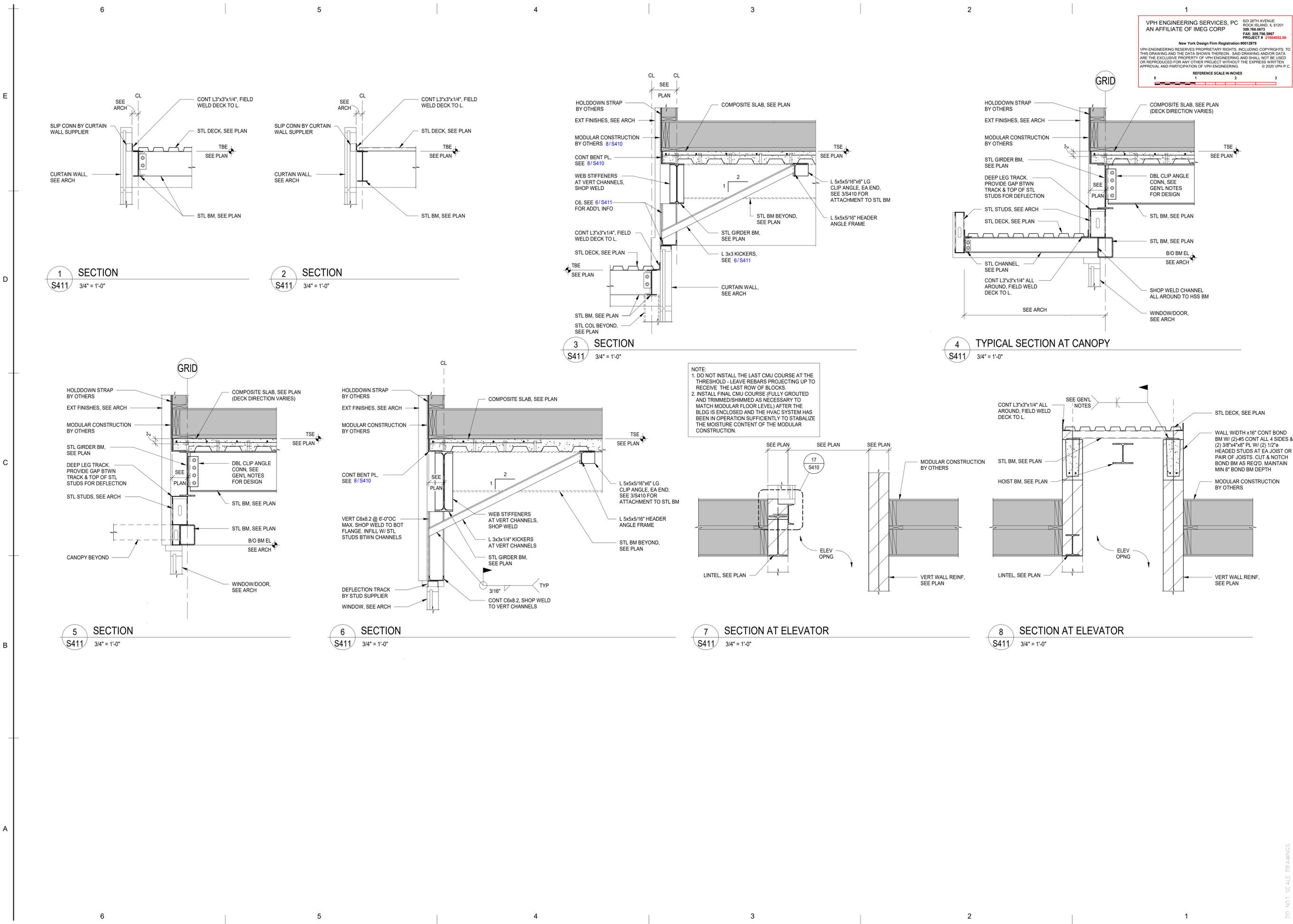
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 PROJECT: 32435  
 HOTEL: HOLIDAY INN EXPRESS & SUITES NANUET

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1 SECTION  
S411 3/4" = 1'-0"

2 SECTION  
S411 3/4" = 1'-0"

3 SECTION  
S411 3/4" = 1'-0"

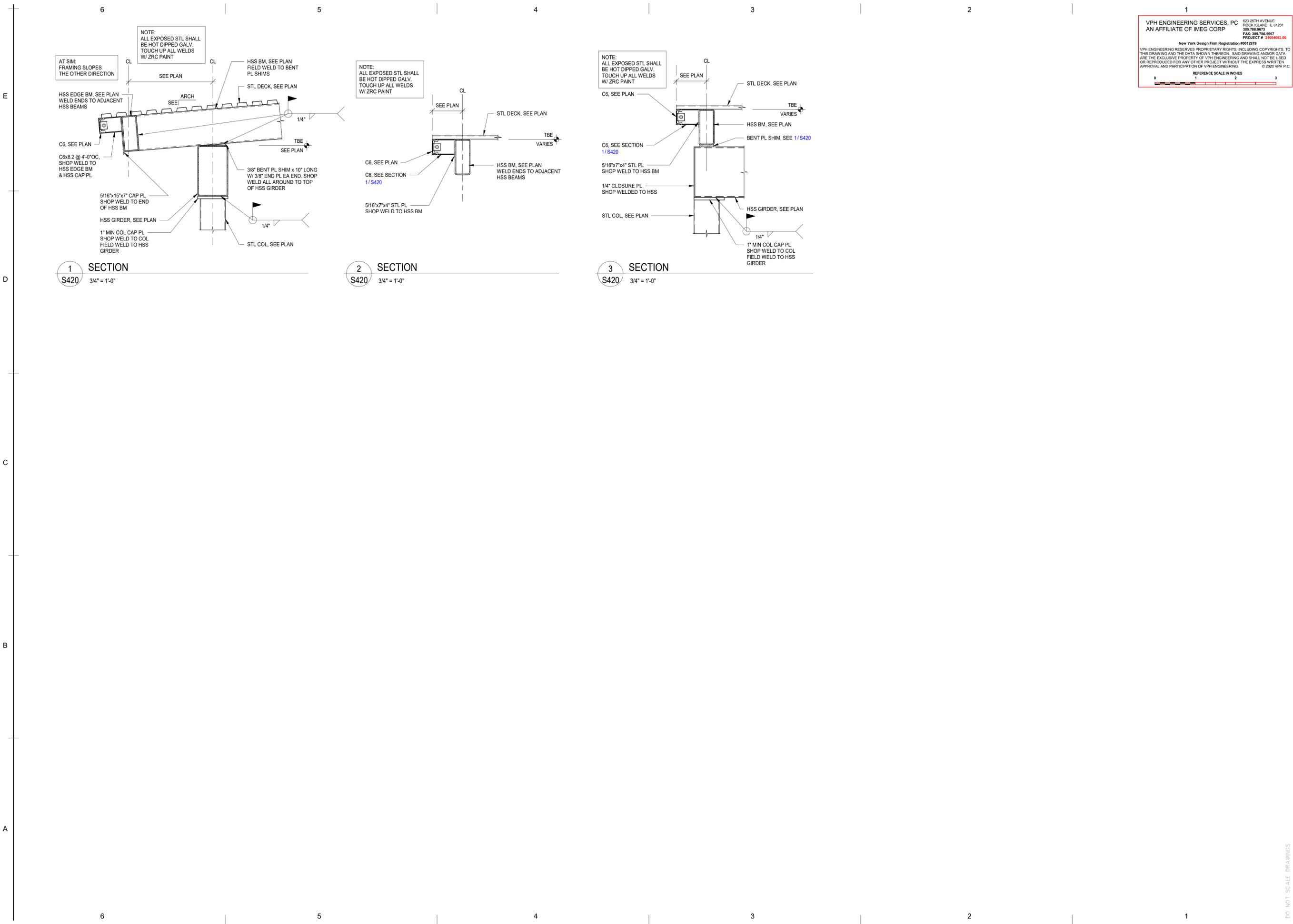
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S411 3/4" = 1'-0"

5 SECTION  
S411 3/4" = 1'-0"

6 SECTION  
S411 3/4" = 1'-0"

7 SECTION AT ELEVATOR  
S411 3/4" = 1'-0"

8 SECTION AT ELEVATOR  
S411 3/4" = 1'-0"



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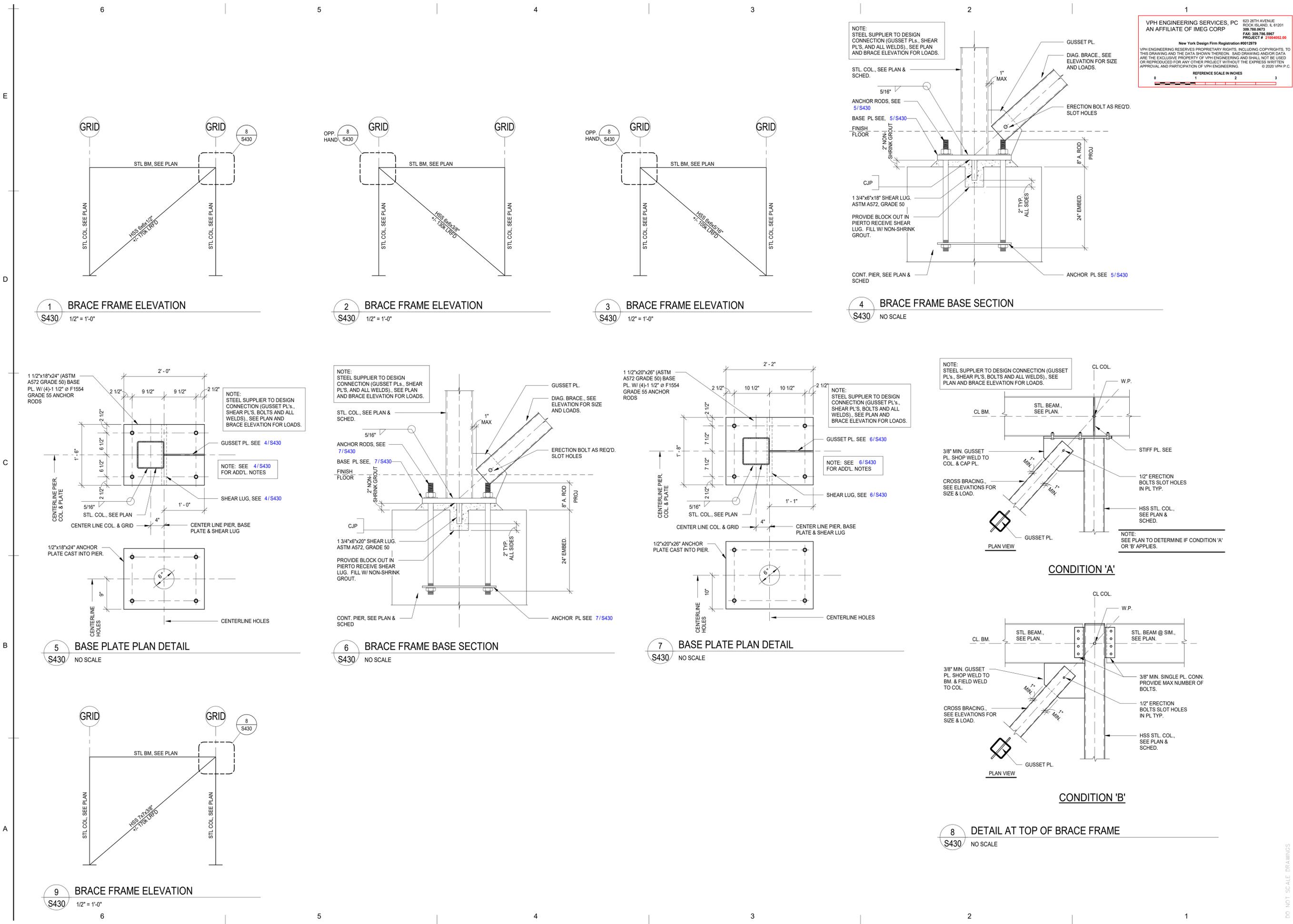
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 SECTIONS - CANOPY

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**PROJECT:** 32435  
**HOTEL:** HOLIDAY INN EXPRESS & SUITES NANUET

**Drawn by:** MAQ  
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**Sheet Title:**  
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