PROJECT NAME: IONA ART CENTER

PROJECT ARCHITECT: PETER GISOLFI ASSOCIATES PROJECT ADDRESS: IONA PREP PERFORMING ARTS CENTER, NEW ROCHELLE, NY
STRUCTURAL ENGINEER: DOMINICK R.PILLA ASSOCIATES PC

ISSUED: FOR REVIEW

STRUCTURAL PLANS

DATE: 02/01/2021

INDEX OF DRAWINGS

LEGEND

- NEW LOAD BEARING CMU WALL
- NEW NON-LOAD BEARING CMU WALL
- NEW CONCRETE WALL
- LIGHT GAUGE FRAMED WALL
- SPAN OF VULCRAFT 1.5VL METAL DECK WITH 3½" CONCRETE TOPPING (4½" TOTAL THICKNESS)
- PROPERTY LINE
- STEEL BEAM
- W-COLUMN
- MOMENT FRAME
- COLUMN DIRECTION

SYMBOL LEGEND

SECTION # / XX / SHT # / SHEET #	SECTION SYMBOL
DETAIL # / XX / SHT# / SHEET #	DETAIL REFERENCE SYMBOL
ELEVATION # / A / SHT # / SHEET #	ELEVATION REFERENCE SYMBOL
TITLE FOR PLAN, SECTION OR DETAIL # / XX / SHT # / IF SHOWN POINT TO SHEET WHERE SECTION OR DETAIL WAS TAKEN	PLAN, SECTION, OR DETAIL TITLE SYMBOL
XXX	ELEVATION SYMBOL
◀ ▶	BEAM MOMENT CONNECTION
↕	POST UP/POST DOWN
LT-#	PRECAST LINTEL
GB-#	CONCRETE GRADE BEAM
F-#,#	FOOTING NUMBER
MW-#	MASONRY WALL NUMBER
CB-#	CONCRETE BEAM
CP-#	CONCRETE PIER
CW-#	CONCRETE WALL NUMBER

ABBREVIATION

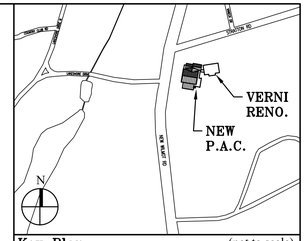
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
B	BOTTOM REINFORCEMENT
BM	BEAM
BS	BOTH SIDES
BU	BUILT UP MEMBER
C	COMPRESSION FORCE IN KIPS
CANT.	CANTILEVER
CL	CENTER LINE
CG	CENTER OF GRAVITY
COL	COLUMN
CONT	CONTINUOUS
COMP LAP	COMPRESSION REINF LAP SLICE
CP	COMPLETE PENETRATION WELD
DB	DEVELOPMENT LENGTH OF REINFORCEMENT BAR
DEL	DELTA OR CHANGE IN ELEVATION
(E)	EXISTING CONSTRUCTION
EF	EACH FACE
EL	ELEVATION
EW	EACH WAY
F	FINISHED SURFACE
FA	FOOTING ANCHOR
GB	GRADE BEAM
H	HORIZONTAL REINFORCEMENT
H	HORIZONTAL FORCE IN KIPS
J1,J2	NEW CODE FORMED STEEL JOISTS
LAP	FULL TENSION CAPACITY LAP SPLICE
LD	TENSION DEVELOPMENT LENGTH FOR REINFORCING BARS
LDC	COMPRESSION SPLICE LENGTH FOR REINFORCEMENT BARS
LLBB	LONG LEGS BACK-TO-BACK
LW	LIGHTWEIGHT CONCRETE
M	BENDING MOMENT IN FOOT-KIPS
MC	MOMENT CONNECTION SHOWN ON DRAWING
MIN	MINIMUM
(N)	NEW CONSTRUCTION
N	BEARING BOLTS THREADS INCLUDED IN SHEAR PLANE
NTS	NOT TO SCALE
OC	ON CENTER
PC	PILE CAP
PL	PLATE
PP	PARTIAL PENETRATION WELD
PL	PROPERTY LINE
SAD	SEE ARCHITECTURAL DRAWINGS/DETAILS
S1,S2	SLAB ON DECK TYPE
SC	SLIP CRITICAL BOLT
SIM	SIMILAR
SPW	SOLDIER PILE LAGGING WALL
T	TENSION FORCE IN KIPS
T	THICKNESS
T	TOP REINFORCEMENT
TBC	TO BE CONFIRMED
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOS	TOP OF STEEL
TYP	TYPICAL
UNO,UON	UNLESS OTHERWISE NOTED
M	MOMENT
V	VERTICAL BEAM END REACTION IN KIPS
VIF	VERIFY IN FIELD
WP	WORKPOINT
WWF	WELDED WIRE FABRIC
XB	CROSS BRACING

SPECIAL INSPECTIONS

•	STRUCTURAL STEEL	BC 1705.2.1
•	COLD-FORMED STEEL DECK	BC 1705.2.2
	OPEN -WEB STEEL JOISTS AND JOIST GIRDERS	BC 1705.2.3
•	COLD-FORMED STEEL TRUSSES SPANNING 60 FEET OR GREATER	BC 1705.2.4
•	CONCRETE CONSTRUCTION	BC 1705.3
	MASONRY CONSTRUCTION	BC 1705.4
	WOOD CONSTRUCTION - HIGH LOAD DIAPHRAGMS	BC 1705.5.1
	WOOD CONSTRUCTION - METAL-PLATE-CONNECTED TRUSSES	BC 1705.5.2
•	SOILS	BC 1705.6
	DEEP DRIVEN FOUNDATIONS	BC 1705.7
	CAST-IN-PLACE DEEP FOUNDATIONS	BC 1705.8
	HELICAL PILE FOUNDATIONS	BC 1705.9
	FABRICATED ITEMS	BC 1705.10
	WIND RESISTANCE	BC 1705.11
•	SEISMIC RESISTANCE	BC 1705.12
	TESTING FOR SEISMIC RESISTANCE	BC 1705.13

DESIGN LOADING-(PSF)

FLOOR LEVEL	SDL	LL	SL
ROOF	20	20	21
2	20	100	0
STAIRS	20	100	0



Key Plan (not to scale)

No.	Date	Revision/Submission
7.	6/01/2021	ISSUED FOR BID
6.	5/07/2021	RE-ISSUED FOR BUILDING PERMIT REVIEW
5.	2/01/2021	ISSUED FOR BUILDING PERMIT REVIEW
4.	10/14/2020	ISSUED FOR PLANNING BOARD REVIEW
3.	9/23/2020	RESUBMITTED FOR ZONING REVIEW
2.	8/28/2020	ISSUED FOR PRELIMINARY DOB REVIEW
1.	1/10/2020	ISSUED FOR DD ESTIMATE

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Stamp

Project Title

IONA PREPARATORY SCHOOL
ADDITION AND ALTERATION TO THE
PAUL VERNI FINE ARTS CENTER

Project Address

IONA PREPARATORY SCHOOL
255 Wilmot Road
New Rochelle, NY 10804

Drawing Title

TITLE SHEET

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-001.00

Drawn

Peter Gisolfi Associates
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P E T E R G I S O L F I A S S O C I A T E S

GENERAL NOTES

UNLESS OTHERWISE NOTED OR SHOWN ON THE STRUCTURAL DRAWINGS, THE FOLLOWING REQUIREMENTS, TOGETHER WITH THE PROJECT PLANS, SPECIFICATIONS AND GEOTECHNICAL REPORT APPLY TO THE STRUCTURES IN THIS CONTRACT.

1. CONSTRUCTION IS TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL CODES, STANDARDS, REGULATIONS AND LAWS.
2. THE STRUCTURAL DOCUMENTS SHALL BE USED IN CONJUNCTION WITH AND COORDINATED WITH THE ARCHITECTURAL, CIVIL AND MEP CONTRACT DOCUMENTS AS WELL AS ANY OTHER TRADES. IF A CONFLICT EXISTS, CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER AND OBTAIN CLARIFICATION PRIOR TO BIDDING AND PROCEEDING WITH WORK
3. THE GENERAL CONTRACTOR SHALL COORDINATE ALL CONTRACT DOCUMENTS WITH FIELD CONDITIONS, DIMENSIONS, ELEVATIONS AND PROJECT SHOP DRAWINGS PRIOR TO CONSTRUCTION. DO NOT SCALE DRAWINGS; USE ONLY PRINTED DIMENSIONS. REPORT ANY DISCREPANCIES IN WRITING TO THE ARCHITECT/ENGINEER PRIOR TO PROCEEDING WITH WORK. DO NOT CHANGE SIZE OR LOCATION OF STRUCTURAL MEMBERS WITHOUT WRITTEN INSTRUCTIONS FROM THE STRUCTURAL ENGINEER OF RECORD.
4. OPENINGS SHOWN ON STRUCTURAL DRAWINGS ARE ONLY PICTORIAL. SEE THE ARCHITECTURAL AND M.E.P. DRAWINGS FOR THE SIZE AND LOCATION OF OPENINGS IN THE STRUCTURE.
5. CONTRACTORS WHO DISCOVER DISCREPANCIES, OMISSIONS OR VARIATIONS IN THE CONTRACT DOCUMENTS DURING BIDDING SHALL IMMEDIATELY NOTIFY THE ARCHITECT. THE ARCHITECT WILL RESOLVE THE CONDITION AND ISSUE A WRITTEN CLARIFICATION.
6. THE CONTRACTOR SHALL PROTECT ADJACENT PROPERTY, HIS OWN WORK AND THE PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, AND JOBSITE SAFETY INCLUDING ALL OSHA REQUIREMENTS.
7. SEE PROJECT SPECIFICATIONS FOR TESTING. SEE THE STRUCTURAL SPECIAL INSPECTION NOTES FOR INSPECTION REQUIREMENTS.
8. DETAILS LABELED "TYPICAL" APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED, WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION. QUESTIONS REGARDING THE APPLICABILITY OF TYPICAL DETAILS SHALL BE RESOLVED BY THE ARCHITECT.
9. THE STRUCTURE IS DESIGNED TO BE STRUCTURALLY SOUND WHEN COMPLETED. PRIOR TO COMPLETION, THE CONTRACTOR IS RESPONSIBLE FOR STABILITY AND TEMPORARY BRACING, INCLUDING, BUT NOT LIMITED TO, MASONRY WALLS. THE CONTRACTOR SHALL VERIFY THAT CONSTRUCTION LOADS DO NOT EXCEED THE CAPACITY OF THE STRUCTURE AT THE TIME THE LOAD IS APPLIED. WHENEVER THE CONTRACTOR IS UNSURE OF THESE REQUIREMENTS, THE CONTRACTOR SHALL RETAIN AN ENGINEER LICENSED IN THE STATE OF THE PROJECT TO DESIGN AND INSPECT THE TEMPORARY BRACING AND STABILITY OF THE STRUCTURE.

CODES AND SPECIFICATIONS

THE DESIGN SHOWN ON THESE DRAWINGS IS BASED ON THE FOLLOWING CODES, SPECIFICATIONS AND STANDARDS:

1. "NEW YORK STATE BUILDING CODE," 2020 EDITION.
   ASCE 7-16: “MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.”
   AWS D1.1: “STRUCTURAL WELDING CODE,” 2014
   "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," ACI 318-2014.
   “BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES,” ACI 530-2013.
   “SPECIFICATIONS FOR STRUCTURAL CONCRETE," ACI 301-1999.
   "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS," AISC 360-16
   “CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES," AISC 303-16
   "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS," AISC-341-16
   “STANDARD FOR COMPOSITE STEEL FLOOR DECK-SLAB,” SDI, 2011.
   “STANDARD FOR STEEL ROOF DECK,” SDI 2010.
   ASCE 37-14: “DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION.”

DESIGN CRITERIA

1. SEE GRAVITY LOADS (DEAD LOADS AND LIVE LOADS) ON S-001.
2. STAIRS TO BE DESIGNED FOR LIVE LOAD OF 100 PSF.
3. DESIGN SNOW LOAD

FLAT ROOF SNOW LOAD	P_f	=	21 PSF
GROUND SNOW LOAD	P_g	=	30 PSF
SNOW EXPOSURE FACTOR	C_e	=	1.0
SNOW LOAD IMPORTANCE FACTOR	I_s	=	1.0
THERMAL FACTOR	C_t	=	1.0

SNOW DRIFTING PER S-602

4. DESIGN WIND LOADS

ULTIMATE WIND SPEED	V	=	116 MPH
RISK CATEGORY	II		
WIND IMPORTANCE FACTOR	I	=	1.0
EXPOSURE	B		
INTERNAL PRESSURE COEFFICIENT	GC_{PI}	=	±0.18
WIND PRESSURES			
MWFRS	P = 20 PSF		
COMPONENTS AND CLADDING	Pnet = 40 PSF		

5. DESIGN SEISMIC LOADS

RISK CATEGORY	II		
SEISMIC IMPORTANCE FACTOR	=	1.0	
SPECTRAL RESPONSE ACCELERATION	S_S	=	0.292
SPECTRAL RESPONSE ACCELERATION	S_1	=	0.061
SITE CLASS	C		
SHORT PERIOD SITE COEFFICIENT	F_a	=	1.3
LONG PERIOD SITE COEFFICIENT	F_v	=	1.5
SPECTRAL RESPONSE COEFFICIENT	S_{DS}	=	0.253
SPECTRAL RESPONSE COEFFICIENT	S_{D1}	=	0.061
SEISMIC DESIGN CATEGORY	B		

BASIC SEISMIC- FORCE-RESISTING SYSTEM:
STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE

DESIGN BASE SHEAR	V	=	31 KIPS
SEISMIC RESPONSE COEFFICIENT	C_S	=	0.073
RESPONSE MODIFICATION FACTOR	R	=	3
OVERSTRENGTH FACTOR	Ω_0	=	3
DEFLECTION AMPLIFICATION FACTOR	C_D	=	3

SHOP DRAWINGS AND OTHER SUBMITTALS

1. INCOMPLETE SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.
2. SUBMIT SPECIFIC COMPONENTS, SUCH AS WALLS, FOOTINGS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.
3. ON FIRST SUBMITTAL, CLEARLY FLAG AND CLOUD ALL DIFFERENCES FROM THE CONTRACT DOCUMENTS. ON RE-SUBMITTALS, FLAG AND CLOUD ALL CHANGES AND ADDITIONS TO PREVIOUS SUBMITTAL. ONLY CLOUDED ITEMS WILL BE REVIEWED.
4. SUBMITTAL REQUIREMENTS:

   THE FOLLOWING ITEMS REQUIRE ERECTION AND SHOP DRAWINGS FOR REVIEW:
   CONCRETE REINFORCING STEEL LAYOUT
   CONCRETE CONSTRUCTION JOINT LAYOUT
   ANCHOR ROD LAYOUT
   STRUCTURAL STEEL
   STEEL DECKING AND STUD LAYOUT
   STEEL STAIRS
   COMPOSITE DRAWINGS OF ALL SLAB PENETRATIONS

   THE FOLLOWING ITEMS REQUIRE ERECTION AND SHOP DRAWINGS AND STRUCTURAL CALCULATIONS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT FOR VIEW
   STRUCTURAL STEEL CONNECTIONS
   CONCRETE DESIGN MIXES
   COLD FORMED STEEL FRAMING
   COLD FORMED STEEL TRUSSES (PERMANENT AND TEMPORARY BRACING)

5. THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL REVIEW AND APPROVE SUBMITTALS AND SHALL SIGN AND DATE EACH DRAWING PRIOR TO SUBMITTING THEM TO THE ARCHITECT/ENGINEER. THIS APPROVAL IS TO CONFIRM THAT THE SUBMITTAL IS COMPLETE, COMPLIES WITH THE SUBMITTAL REQUIREMENTS AND IS COORDINATED WITH FIELD DIMENSIONS, OTHER TRADES, ERECTION SEQUENCING AND CONSTRUCTABILITY.
6. STRUCTURAL ENGINEER'S REVIEW OF DELEGATED ENGINEER SUBMITTALS IS LIMITED TO VERIFYING THAT THE SPECIFIED STRUCTURAL SUBMITTAL HAS BEEN FURNISHED, SIGNED AND SEALED BY THE DELEGATED ENGINEER AND THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE. THE DELEGATED ENGINEER IS SOLELY RESPONSIBLE FOR HIS/HER DESIGN, INCLUDING BUT NOT LIMITED TO THE ACCURACY OF HIS/HER CALCULATIONS AND COMPLIANCE WITH THE APPLICABLE CODES AND STANDARDS.
7. REVIT/CAD FILES OF STRUCTURAL DRAWINGS MAY BE USED AS AN AID IN PREPARING SHOP DRAWINGS ONLY UPON THE CONTRACTOR SIGNING AN AGREEMENT. WHEN CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE MADE AVAILABLE, IT IS UNDER THE FOLLOWING CONDITIONS:

   ALL INFORMATION CONTAINED IN THE CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT/ENGINEER AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT OR THE COMPLETION OF THE PROJECT BY OTHERS. CAD FILES AND COPIES OF THE STRUCTURAL DRAWINGS REMAIN THE PROPERTY OF DOMINICK R. PILLA ASSOCIATES AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE;

   REVIT/CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS ARE NOT CONTRACT DOCUMENTS. IN THE EVENT OF A CONFLICT, THE STRUCTURAL DRAWINGS SHALL GOVERN;

   THE USE OF REVIT/CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZES AND QUANTITIES OF MATERIALS AS REQUIRED FOR THE PREPARATION OF COMPLETE AND ACCURATE SHOP DRAWINGS;

   THE CONTRACTOR SHALL REVISE ALL REFERENCES TO CONTRACT DOCUMENT SHEET NUMBERS AND SECTION MARKS AND SHALL REMOVE INFORMATION THAT IS NOT REQUIRED FOR THEIR WORK FROM THE CAD FILES OR COPIES OF THE STRUCTURAL DRAWINGS, INCLUDING THE TITLE BLOCK; AND

   DIMENSIONS IN THE REVIT/CAD FILES MAY NOT BE PRECISE AND, IN SOME CASES, HAVE BEEN INTENTIONALLY ALTERED FOR PRESENTATION PURPOSES. DO NOT SCALE DIMENSIONS ELECTRONICALLY OR OTHERWISE.

SHALLOW FOUNDATIONS

1. FOUNDATION DESIGN IS BASED ON THE 'REPORT ON SUBSURFACE SOIL AND FOUNDATION INVESTIGATION' PREPARED BY CARLIN SIMPSON & ASSOCIATES CONSULTING GEOTECHNICAL AND ENVIRONMENTAL ENGINEERS. SEE REPORT FOR ADDITIONAL INFORMATION.
2. FOUNDATIONS PLACED ON UNDISTURBED SCHIST, COMPLETELY WEATHERED, AT ELEVATIONS INDICATED IS DESIGNED FOR AN ALLOWABLE NET SOIL BEARING PRESSURE OF 6,000 PSF.
3. THE CONTRACTOR SHALL NOTIFY THE ENGINEER WHERE BOTTOM OF FOOTING ELEVATION IS CHANGED AND OBTAIN REVISED DESIGN OF THE FOUNDATION AND RETAINING WALLS AS REQUIRED.
4. ALL FILL REQUIRED BELOW ANY PORTION OF THE STRUCTURE SHALL BE COMPACTED IN 9" LIFTS TO AT LEAST 98% OF THE MAXIMUM DRY DENSITY PER ASTM D-1557. REMOVE UNSUITABLE FILL AND REPLACE WITH CONTROLLED FILL AS REQUIRED FOR SOUND PLACEMENT OF FOUNDATIONS.
5. SOIL - SUPPORTED FOOTING SHALL BE FOUNDED UPON UNDISTURBED NATURAL SUBGRADE WITH A MINIMUM BEARING CAPACITY AS NOTED AND AS FIELD VERIFIED AND APPROVED BY A REGISTERED SOIL ENGINEER. THE BOTTOM OF THE FOOTING ELEVATIONS AND BEARING CAPACITIES AS SHOWN ON THE DRAWINGS ARE ESTIMATED AND WILL REQUIRE VERIFICATION. FINAL, EXACT ELEVATIONS AND BEARING CAPACITIES SHALL BE FIELD DETERMINED.
6. ELEVATION OF ADJACENT FOOTINGS SHALL VARY ON A SLOPE NOT STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL.
7. CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, U.O.N.

EXCAVATION, BACKFILL AND DEWATERING

1. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS. DO NOT EXCAVATE WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOIL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROPERLY PROTECTED AGAINST SETTLEMENT.
2. DO NOT BACKFILL AGAINST WALLS UNTIL 7 DAYS AFTER THE WALLS ARE BRACED BY THE STRUCTURE OR ARE TEMPORARILY BRACED. DO NOT BACKFILL CANTILEVERED RETAINING WALLS UNTIL CONCRETE IS 7 DAYS OLD. DO NOT BACKFILL UNTIL AFTER COMPLETION AND INSPECTION OF ANY WATERPROOFING.
3. EXCAVATIONS SHALL BE DEWATERED TO ALLOW INSTALLATION OF FOOTINGS IN DRY ATMOSPHERE.
4. THE CONTRACTOR IS RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER IN A MANNER THAT DOES NOT INCONVENIENCE OR DAMAGE THE WORK.

SLABS ON GRADE

1. SLAB ON GRADE TO BE INSTALLED AT ELEVATIONS SHOWN ON PLAN. EXISTING FILL TO BE REMOVED AND REPLACED WITH COMPACTED FILL AS PER REQUIREMENTS OF GEOTECHNICAL REPORT.
2. FOR INTERIOR SLABS, PLACE 10 MIL POLYETHYLENE SHEETING BETWEEN SOIL AND BOTTOM OF SLAB. DO NOT USE ANY SHEETING BELOW EXTERIOR CONCRETE SLABS.
3. USE FIBER REINFORCED NORMAL WEIGHT CONCRETE FOR CONCRETE SLAB. USE 4 LB/YD^3 TUF-STRAND SF 1 MACRO SYNTHETIC FIBER BY EUCLID CHEMICAL OR APPROVED EQUAL. AIR CONTENT SHALL BE 6%±1.5%. MAXIMUM AGGREGATE SIZE SHALL BE ¾". USE TYPE II CEMENT AND FLY ASH. THE DESIGN MIX SHALL HAVE THE LOWEST SLUMP THAT CAN BE PLACED AND FINISHED, BUT NOT GREATER THAN 5 INCHES.
4. SLABS-ON-GRADE SHALL BE REINFORCED CONCRETE STRUCTURAL SLAB AS NOTED ON PLANS. SLAB SHALL BE FINISHED IN ACCORDANCE WITH ACI STANDARD 302.1R FOR CLASS 2 FLOORS. TYPE II CEMENT AND 1” COURSE AGGREGATE (SIZE NO. 57) SHALL BE USED.
5. FOLLOW RECOMMENDATIONS OF ACI 302.1R.
6. SEE THE ARCHITECTURAL DRAWINGS FOR SLAB ON GRADE DEPRESSIONS AND OTHER REQUIREMENTS.

REINFORCED CONCRETE

1. COMPLY WITH ACI 301 AND 318.
2. ALL CAST-IN-PLACE CONCRETE SHALL BE AIR-ENTRAINED CONTROLLED CONCRETE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'C) AT 28 DAYS AS FOLLOWS:

   CAST IN PLACE WALLS 4,000 PSI
   CAST IN PLACE BEAMS 4,000 PSI
   COLUMNS/PIERS 4,000 PSI
   SLABS-ON-GRADE 4,000 PSI
   SUSPENDED SLABS 4,000 PSI

3. USE NORMAL WEIGHT CONCRETE FOR ALL STRUCTURAL MEMBERS. U.O.N.
4. CONCRETE REINFORCEMENT SHALL BE ASTM A615, GRADE 60 DEFORMED REINFORCING STEEL. LAP BOTTOM STEEL OVER SUPPORTS AND TOP STEEL AT MIDSPAN (U.O.N.). HOOK DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS, U.O.N.
5. WHERE SPECIFIED, PROVIDE PLAIN, COLD-DRAWN ELECTRICALLY-WELDED WIRE REINFORCEMENT (WWF) CONFORMING TO ASTM A185. SUPPLY IN FLAT SHEETS ONLY. LAP SPLICE SHALL BE ONE CROSS WIRE SPACING PLUS TWO INCHES.
6. FOLLOW ACI 117-10 “SPECIFICATION FOR TOLERANCES OF CONCRETE CONSTRUCTION AND MATERIALS” FOR REQUIRED TOLERANCES.

10. UTILITIES SHALL NOT BE PLACED IN SLABS, BEAMS OR COLUMNS BUT MAY PASS THROUGH SLABS AND WALLS INDIVIDUALLY, UON. SEE TYPICAL DETAILS.

11. PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI 318, SECTION 6.4. PROVIDE KEYWAYS AND ADEQUATE DOWELS. SUBMIT DRAWINGS SHOWING LOCATION OF CONSTRUCTION JOINTS AND DIRECTION OF POUR FOR REVIEW.

10. PROVIDE 3/4" CHAMFER FOR ALL EXPOSED CORNERS. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL CONCRETE FINISH REQUIREMENTS.

CONCRETE FIELD TESTING:

1. TESTING: OWNER WILL ENGAGE A QUALIFIED TESTING AGENCY TO PERFORM FIELD TESTS AND PREPARE TEST REPORTS.
2. CONCRETE TESTS: TESTING OF COMPOSITE SAMPLES OF FRESH CONCRETE OBTAINED ACCORDING TO ASTM C172 AND SECTION BC 1903 OF 2015 INTERNATIONAL BUILDING CODE CODE SHALL BE PERFORMED ACCORDING TO THE FOLLOWING REQUIREMENTS:

   a. TESTING FREQUENCY: OBTAIN ONE COMPOSITE SAMPLE FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE LESS THAN 25 CU. YD., PLUS ONE SET FOR EACH ADDITIONAL 50 CU. YD. OR FRACTION THEREOF.
   b. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE STRENGTH TEST OF EACH CONCRETE MIXURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.
   c. WATER CONTENT AND SLUMP: VERIFY WATER CONTENT IN ACCORDANCE WITH AASHTO T-318 “STANDARD METHOD OF TESTING FOR WATER CONTENT USING MICROWAVE OVEN DRYING.” TEST SLUMP IN ACCORDANCE WITH ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.
   d. AIR CONTENT: ASTM C231, PRESSURE METHOD, FOR NORMAL-WEIGHT CONCRETE; ASTM C173, VOLUMETRIC METHOD, FOR LIGHTWEIGHT CONCRETE; ONE TEST FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE.
   e. CONCRETE TEMPERATURE: ASTM C1064; ONE TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEG F AND BELOW AND WHEN 80 DEG F AND ABOVE, AND ONE TEST FOR EACH COMPOSITE SAMPLE.
   f. COMPRESSION TEST SPECIMENS: ASTM C31.
   i. CAST AND LABORATORY CURE ALL TEST CYLINDER SPECIMENS.
   ii. WHEN REQUIRED, CAST AND FIELD CURE TWO SETS OF TWO STANDARD CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.
   g. COMPRESSIVE-STRENGTH TESTS: ASTM C39 AND SECTION BC 1905.6.2 OF THE NYC BUILDING CODE; TEST FIRST SET OF TWO LABORATORY-CURED SPECIMENS AT 7 DAYS FOR INFORMATION, SECOND SET OF TWO LABORATORY-CURED SPECIMENS AT 28 DAYS FOR ACCEPTANCE AND THIRD SET OF TWO SPECIMENS AT 56 DAYS IF NECESSARY.
   i. TEST ONE SET OF FIELD-CURED SPECIMENS AT 7 DAYS AND ONE SET OF TWO SPECIMENS AT 28 DAYS.
   ii. A COMPRESSIVE-STRENGTH TEST SHALL BE THE AVERAGE COMPRESSIVE STRENGTH FROM A SET OF TWO SPECIMENS OBTAINED FROM SAME COMPOSITE SAMPLE AND TESTED AT AGE INDICATED.

STRUCTURAL STEEL

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", WITH COMMENTARY, AND ALL OSHA REQUIREMENTS.
2. STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS, UNLESS OTHERWISE NOTED ON THE CONTRACT DOCUMENTS:

   ROLLED W SHAPES: ASTM A992, GRADE 50.
   ROLLED M, S, C, MC AND L SHAPES: ASTM A36, FY=36 KSI.
   PLATES AND BARS: ASTM A36, FY=36 KSI, UON.
   PLATES FOR MOMENT CONNECTIONS: ASTM A572, GR. 50.
   STEEL PIPE: ASTM A53, TYPE E OR S, GRADE B, FY=35 KSI.
   HOLLOW STRUCTURAL SECTIONS:
   ROUND SECTIONS: ASTM A500, GRADE C, FY=46 KSI.
   SQUARE AND RECTANGULAR SECTIONS: ASTM A500, GRADE C, FY=50 KSI.

3. ALL STRUCTURAL STEEL CONNECTIONS BOLTS SHALL BE ASTM A325 OR ASTM A490, UNLESS OTHERWISE NOTED, AND SHALL COMPLY WITH "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS," INCLUDING COMMENTARY. ALL JOINT TYPE SHALL BE 'PT' (PRETENSIONED).
4. BOLT SIZE SHALL BE 3/4” DIAMETER MINIMUM, UNLESS OTHERWISE NOTED.
5. A MINIMUM OF TWO (2) - 3/4” DIAMETER A325 BOLTS SHALL BE PROVIDED AT EACH CONNECTIONS.
6. SHOP DRAWINGS SHALL BE COORDINATED WITH STAIR DETAILS. IF HANGER RODS ARE USED, PROVIDE STIFFENER PLATE, 3/8” THICK MINIMUM, ALONGSIDE HANGER LOCATION.
7. SHEAR AND BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR IN ACCORDANCE WITH AISC FOR THE FORCES AND/OR REACTIONS SHOWN, THE FABRICATOR SHALL SUBMIT CALCULATIONS DEMONSTRATING THAT THE SELECTED SHEAR AND BRACING CONNECTIONS WILL ACHIEVE THE FORCES AND/OR REACTIONS INDICATED, OR AS REQUIRED BY THE CODES.
8. ANCHOR RODS SHALL BE ASTM F1554 GRADE 55 WITH WELDABILITY SUPPLEMENTARY REQUIREMENT S1, HOOKED OR ANCHOR RODS SHALL BE A449, TYPE 1, THREADED WITH NUTS AND WASHERS EACH END.
9. WHERE CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION.
10. PROVIDE HOLES IN ALL STEEL TO PREVENT ANY ACCUMULATION OF WATER. HOLES SHALL B\NOT EXCEED 1” DIAMETER.
11. CUT, DRILL, OR PUNCH HOLES PERPENDICULAR TO METAL SURFACES. REAM HOLES THAT MUST BE ENLARGED TO ADMIT BOLTS AS PERMITTED BY ARCHITECT. DO NOT ENLARGE UNFAIR HOLES BY BURNING OR USING DRIFT PINS.
12. DO NOT SPLICE STRUCTURAL STEEL MEMBERS EXCEPT WHERE INDICATED ON THE DRAWINGS.
13. UNLESS NOTED OTHERWISE, PROVIDE A 1/4” CAP PLATE CONTINUOUSLY WELDED AT THE ENDS OF EXTERIOR EXPOSED HOLLOW STRUCTURAL SHAPES.
14. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MISCELLANEOUS STEEL NOT SHOWN ON THE STRUCTURAL DRAWINGS.
15. REFER TO ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR PAINTING AND FIREPROOFING OF STRUCTURAL STEEL. DO NOT PAINT STEEL SURFACES IN CONTACT WITH CONCRETE OR FIREPROOFING

WELDING

1. ALL SHOP AND FIELD WELDING SHALL CONFORM TO THE AWS D1.1. STRUCTURAL WELDING CODE.
2. WELDING ELECTRODES SHALL CONFORM TO E70XX.
3. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
4. ALL WELDERS SHALL BE LICENSED AND CERTIFIED TO AWS STANDARDS OR THOSE REQUIRED BY APPLICABLE BUILDING CODES.
5. ALL WELD SHALL BE VISUALLY INSPECTED. ALL GROOVE WELDS SHALL RECEIVE RADIOGRAPHIC OR ULTRASONIC TESTING. MAGNETIC PARTICLE TEST 20 PERCENT OF ALL FILLET WELDS.
6. WELDING SHALL PROGRESS IN AMANNER THAT BALANCES THE STRESSES IN THE MEMBER, IN ACCORDANCE WITH AWS.
7. FOLLOW PREHEAT REQUIREMENTS FOR BASE METAL PER AWS GUIDELINES.

METAL DECK AND SHEAR STUDS

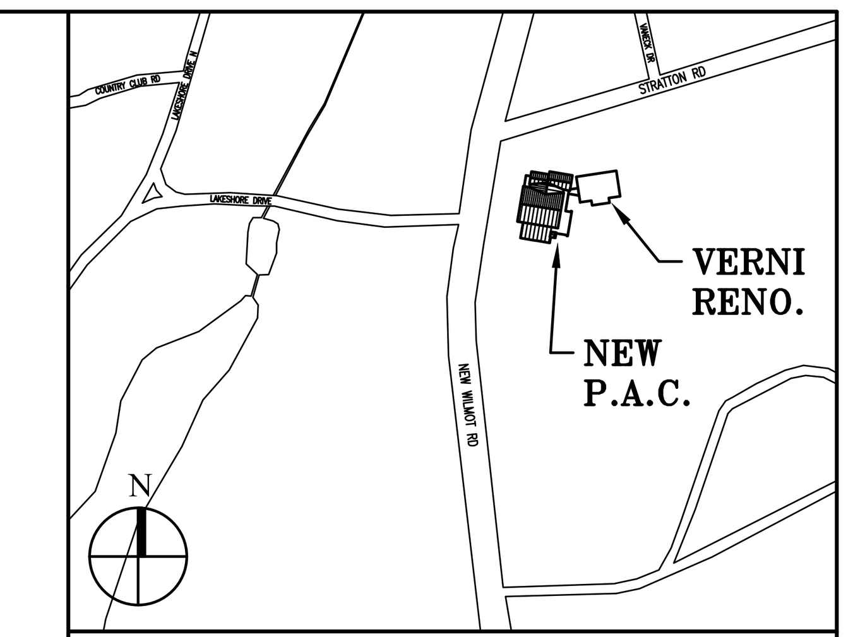
1. DECK UNITS SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH CURRENT “DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS,” PUBLISHED BY THE STEEL DECK INSTITUTE.
2. ALL FLOOR DECK SECTIONS SHALL BE 3" DEEP, 20 GAGE, GALVANIZED COMPOSITE METAL DECK WITH A MINIMUM I = 0.993 IN4/FT.
3. ALL ROOF DECK SHALL BE 1 1/2" TYPE 'B' WIDE RIB ROOF DECK, 22 GAGE, GALVANIZED WITH MINIMUM I = 0.17 IN4/FT.
4. ALL DECKS SHALL BE CONTINUOUS OVER A MINIMUM OF TWO SPANS.
5. SHEAR CONNECTORS SHALL BE HEADED STUDS TYPE, ASTM A 108, GRADE 1015 OR 1020 COLD FINISHED CARBON STEEL, SIZE AND SPACING AS SPECIFIED ON THE DRAWINGS.
6. NO ELECTRICAL OR ANY OTHER ACCESS HOLES SHALL BE ALLOWED IN THE FLOOR SLAB.
7. PROVIDE CELLULAR, FLUTED AND FORM UNITS, OPENINGS, SLEEVES, ETC. AS INDICATED ON THE STRUCTURAL, ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS.
8. PROVIDE METAL DECK REINFORCEMENT AT ALL UNFRAMED OPENINGS, SLEEVES AND COLUMN CUTOUTS AS SHOWN ON PLANS OR AS INDICATED IN THE SPECIFICATIONS.
9. ALL METAL DECK UNITS SHALL BE FASTENED TO THE STEEL FRAME BY 5/8” DIAMETER PUDDLE WELD WITH AQN AVERAGE WELD SPACING OF AT LEAST 12” ON CENTER. SIDE LAPS ARE TO BE WELDED AT A MAXIMUM SPACING OF 36” AN CENTER.
10. FURNISH, INSTALL AND WELD IN POSITION, CLOSURES AND OTHER MISCELLANEOUS ITEMS AS REQUIRED TO CLOSE OPENINGS BETWEEN FLOOR UNITS AND COLUMNS, BEAMS AND GIRDERS AND AREAS WHERE DECK CHANGES SPAN DIRECTION. CLOSURE SHALL NOT BE LESS THAN 16 GAGE SHEET METAL.
11. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF DECK PANELS INCLUDING DETAILS OF SPECIAL CONDITIONS. ALL METAL DECK SHALL EXTEND OVER THREE OR MORE SPANS IF POSSIBLE.

COLD FORMED WALL STUDS, FLOOR JOISTS, AND TRUSSES

1. EXTERIOR WALL MEMBERS SHALL BE DESIGNED USING THE FOLLOWING MINIMUM PARAMETERS:
   a. MAXIMUM ALLOWABLE DEFLECTION (HORIZONTAL) NOT TO EXCEED 1/600 OF SPAN.
   b. MAXIMUM STUD SPACING IS 16” O.C.
   c. MINIMUM STUD THICKNESS IS 18 GAGE.
2. FLOOR JOISTS SHALL BE DESIGNED USING THE FOLLOWING MINIMUM PARAMETERS:
   a. LIVE LOAD DEFLECTION SHALL BE LESS THAN 1/480 OF SPAN.
   b. MAXIMUM TOTAL LOAD DEFLECTION SHALL BE LESS THAN 1/240 OF SPAN.
   c. MAXIMUM JOIST SPACING IS 24” O.C.
   d. MINIMUM STUD THICKNESS IS 18 GAGE.
3. ALL BRIDGING, BRACING, BLOCKING, STRAPPING, WEB REINFORCEMENT, ETC., MUST BE IN PLACE PRIOR TO LOADING THE FLOOR.
4. SUBMIT SHOP DRAWINGS SHOWING MATERIALS, DETAILS, CONNECTIONS, BRIDGING AND OTHER ACCESSORIES REQUIRED FOR PROPER INSTALLATION. SUBMITTAL SHALL INCLUDE CALCULATIONS FOR ALL MEMBERS AND CONNECTIONS. DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT.
5. ALL STEEL STUDS SHALL BE COLD FORMED ASTM 446, GRADE D (Fy = 50 KSI). ALL STUDS SHALL BE GALVANIZED.
6. STUD SIZES CALLED FOR ON PLANS ARE BASED ON STUDS MANUFACTURED BY MARINO/WARE. STUDS BY OTHER MANUFACTURERS THAT MEET THE DESIGN PARAMETERS MAY BE USED.
7. TRUSS DESIGN DRAWINGS SHALL CONFORM TO THE REQUIREMENTS OF SECTION I1 OF AISI S202.

SPECIAL STRUCTURAL INSPECTIONS PLAN

1. SPECIAL INSPECTIONS SHALL BE PERFORMED BY A SPECIAL INSPECTOR AS DEFINED BY THE NYS BUILDING CODE IN ACCORDANCE WITH CHAPTER 17 OF THE CODE.
2. SEE S-001 FOR LIST OF REQUIRED SPECIAL INSPECTIONS.



Key Plan (not to scale)

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Stamp

Project Title
IONA PREPARATORY SCHOOL
ADDITION AND ALTERATION TO THE
PAUL VERNI FINE ARTS CENTER

Project Address
IONA PREPARATORY SCHOOL
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Drawing Title
GENERAL NOTES

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-002.00

Drawn

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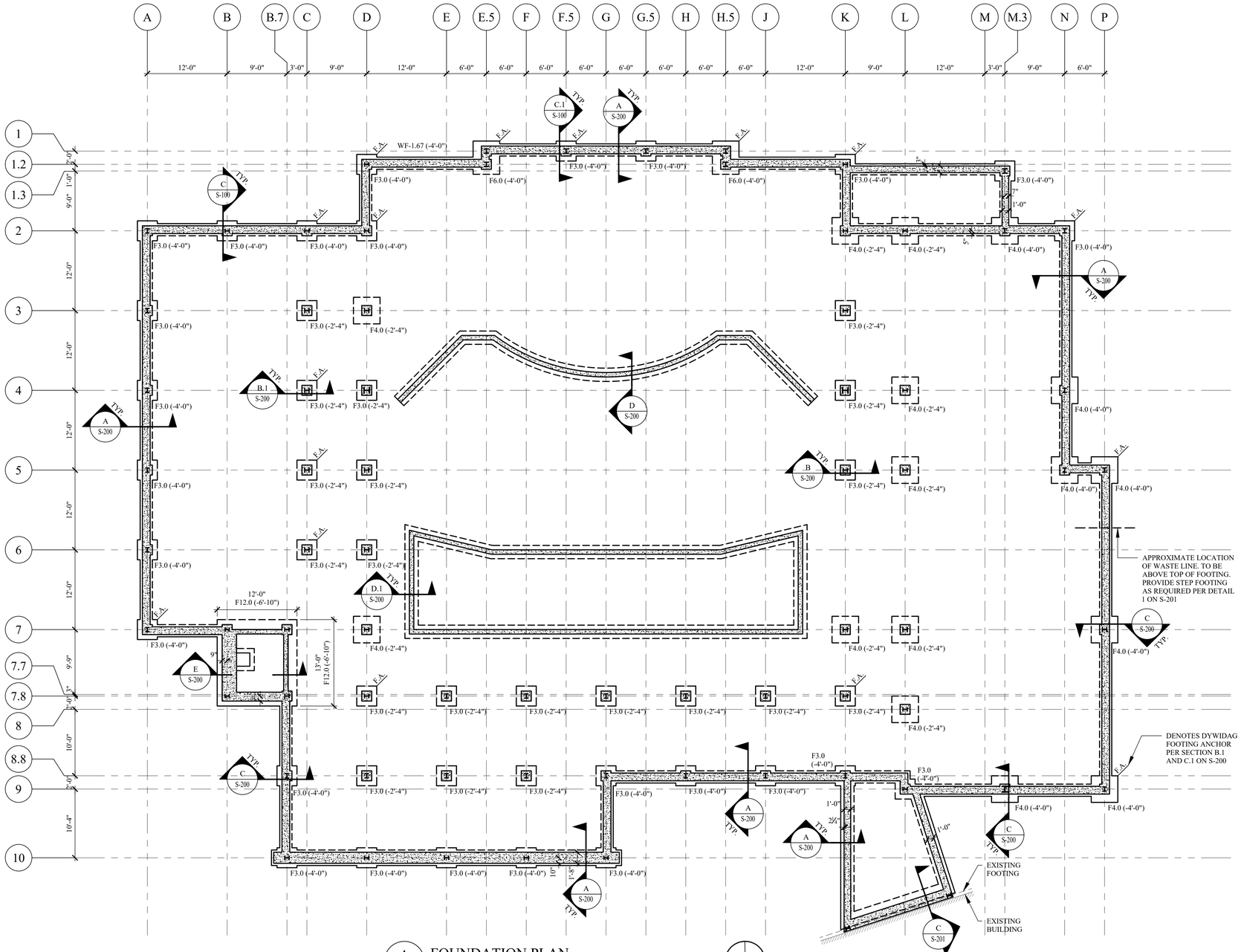
P E T E R G I S O L F I A S S O C I A T E S

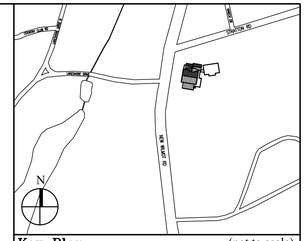


A FOUNDATION PLAN
SCALE: 1/8"=1'-0"

NOTES:
1. SEE S-200 FOR TYPICAL FOUNDATION AND CONCRETE DETAILS
2. TOP OF SLAB ON GRADE ELEVATION IS 0'-0"
3. VALUE IN (X'-XX") BEHIND FOOTING ID INDICATED DEPTH OF FOOTING BELOW FIRST FLOOR SLAB ON GRADE.
4. FOOTINGS TO BEAR ON SCHIST BEDROCK AS IDENTIFIED IN 'REPORT ON SUBSURFACE SOIL AND FOUNDATION INVESTIGATION' PREPARED BY 'CARLIN SIMPSON & ASSOCIATES' AND DATED AUGUST 7, 2020. DEPTH OF SCHIST BEDROCK VARIES. ELEVATION OF FOOTINGS LISTED IS MINIMUM ELEVATION OF FOOTING BELOW GRADE. BEARING MATERIAL ON SITE TO BE VERIFIED BY SPECIAL INSPECTOR IN FIELD. NOTIFY ENGINEER OF RECORD IF ADDITIONAL EXCAVATION BEYOND EXCAVATION SPECIFIED ON PLAN IS REQUIRED TO PROVIDE REQUIRED SUBSTRATE FOR FOOTING BEARING.

FOOTING SCHEDULE

FOOTING ID	SIZE			BOTTOM REINFORCEMENT		TOP REINFORCEMENT		REMARKS
	WIDTH	LENGTH	HEIGHT	LONGITUDINAL	TRANSVERSE	LONGITUDINAL	TRANSVERSE	
F3.0	3'-0"	3'-0"	1'-4"	4-#5	4-#5	4-#5	4-#5	DYWIDAG ANCHOR AS REQUIRED PER PLAN
F4.0	4'-0"	4'-0"	1'-4"	5-#5	5-#5	5-#5	5-#5	DYWIDAG ANCHOR AS REQUIRED PER PLAN
F6.0	4'-0"	6'-0"	1'-4"	5-#5	7-#5	5-#5	7-#5	DYWIDAG ANCHOR AS REQUIRED PER PLAN
F12.0	12'-0"	13'-0"	1'-4"	13-#5	14-#5	13-#5	14-#5	



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Drawing Title
FOUNDATION PLAN

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-100.00

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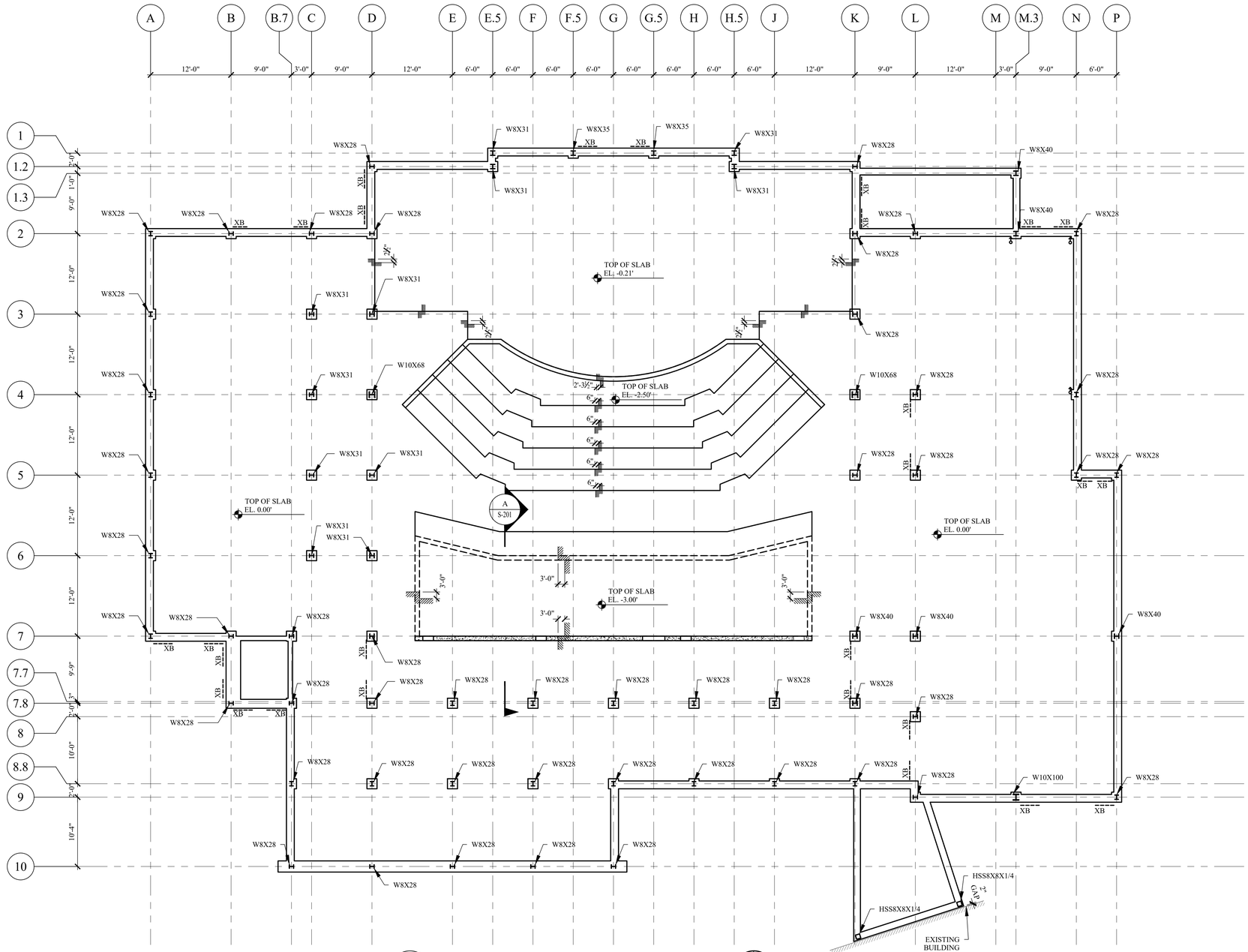
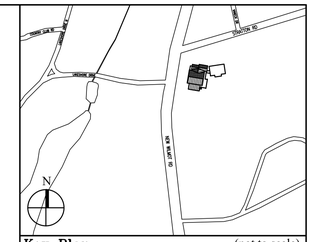


A
-
FIRST FLOOR PLAN
SCALE: 1/8"=1'-0"

N

NOTES:
1. SEE S-200, S-201, S-202 FOR TYPICAL FOUNDATION AND CONCRETE DETAILS
2. SEE DETAILS 1 AND 2 ON S-200 FOR TYPICAL SLAB ON GRADE DETAILS
3. TOP OF SLAB ON GRADE ELEVATION IS 0'-0" U.O.N.



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Drawing Title
FIRST FLOOR PLAN

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-101.00

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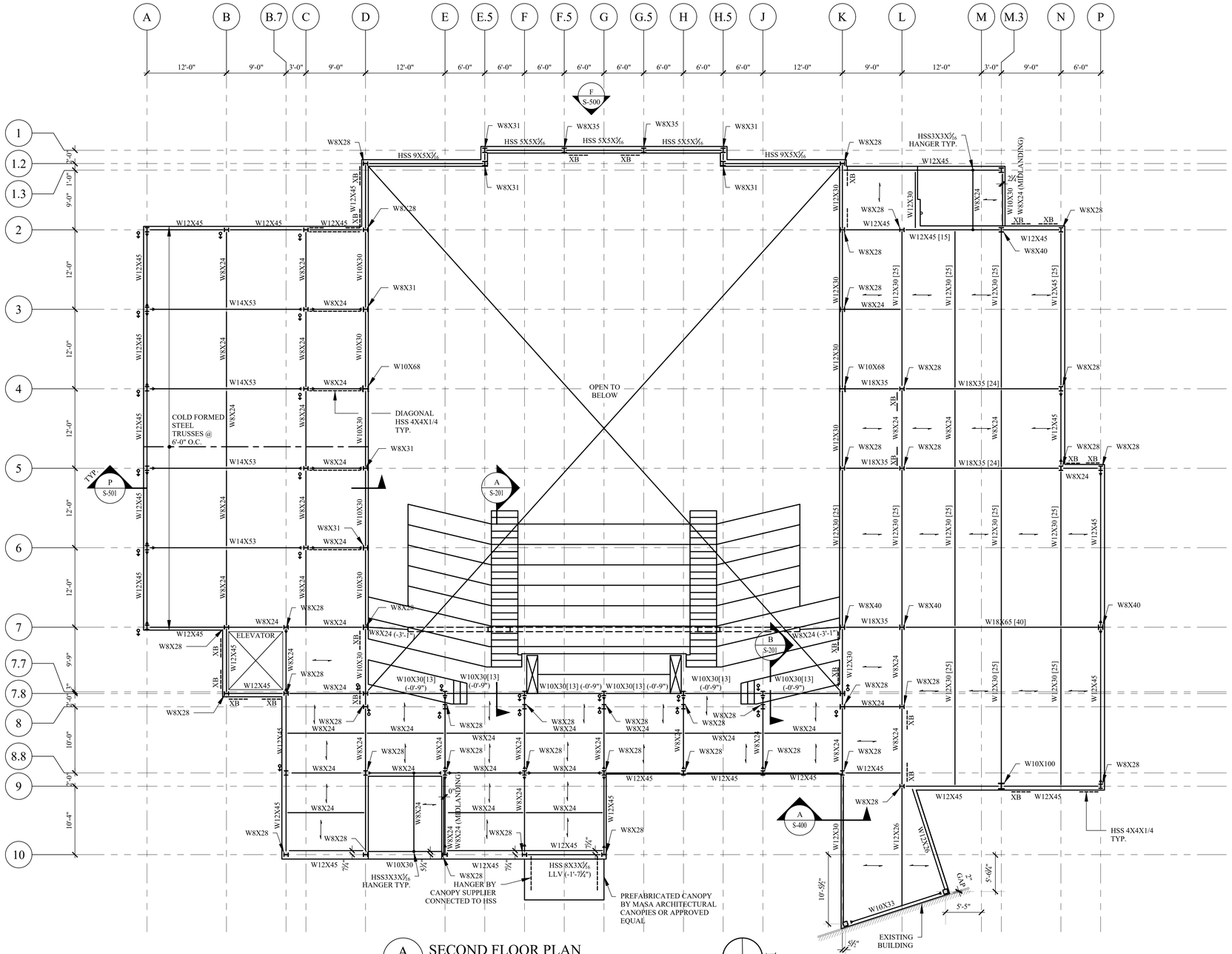
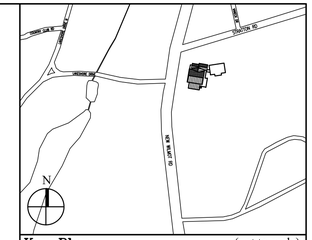


A SECOND FLOOR PLAN

SCALE: 1/8"=1'-0"

NOTES:

1. SEE S-200, S-201, S-202 FOR TYPICAL CONCRETE DETAILS
2. SEE S-300 FOR TYPICAL STEEL DETAILS.
3. SEE S-400 FOR TYPICAL LIGHT GAGE STEEL FRAMING DETAILS.
4. TOP OF STEEL ELEVATION IS 12'-3½". VALUE (X'-XX") BEHIND BEAM ID IS ELEVATION RELATIVE TO TOP OF STEEL ELEVATION ON THIS FLOOR.
5. VALUE IN [ ] BEHIND STEEL BEAM ID INDICATED QUANTITY OF ¾" HEADED STUD ANCHORS.
6. SEE S-500 AND S-501 FOR BRACED FRAME ELEVATIONS
7. SEE S-600 AND S-601 FOR BEAM REACTIONS FOR CONNECTION DESIGN BY STEEL FABRICATOR
8. SEE S-600 FOR DRIFT SURCHARGE



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Drawing Title

SECOND FLOOR PLAN

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-102.00

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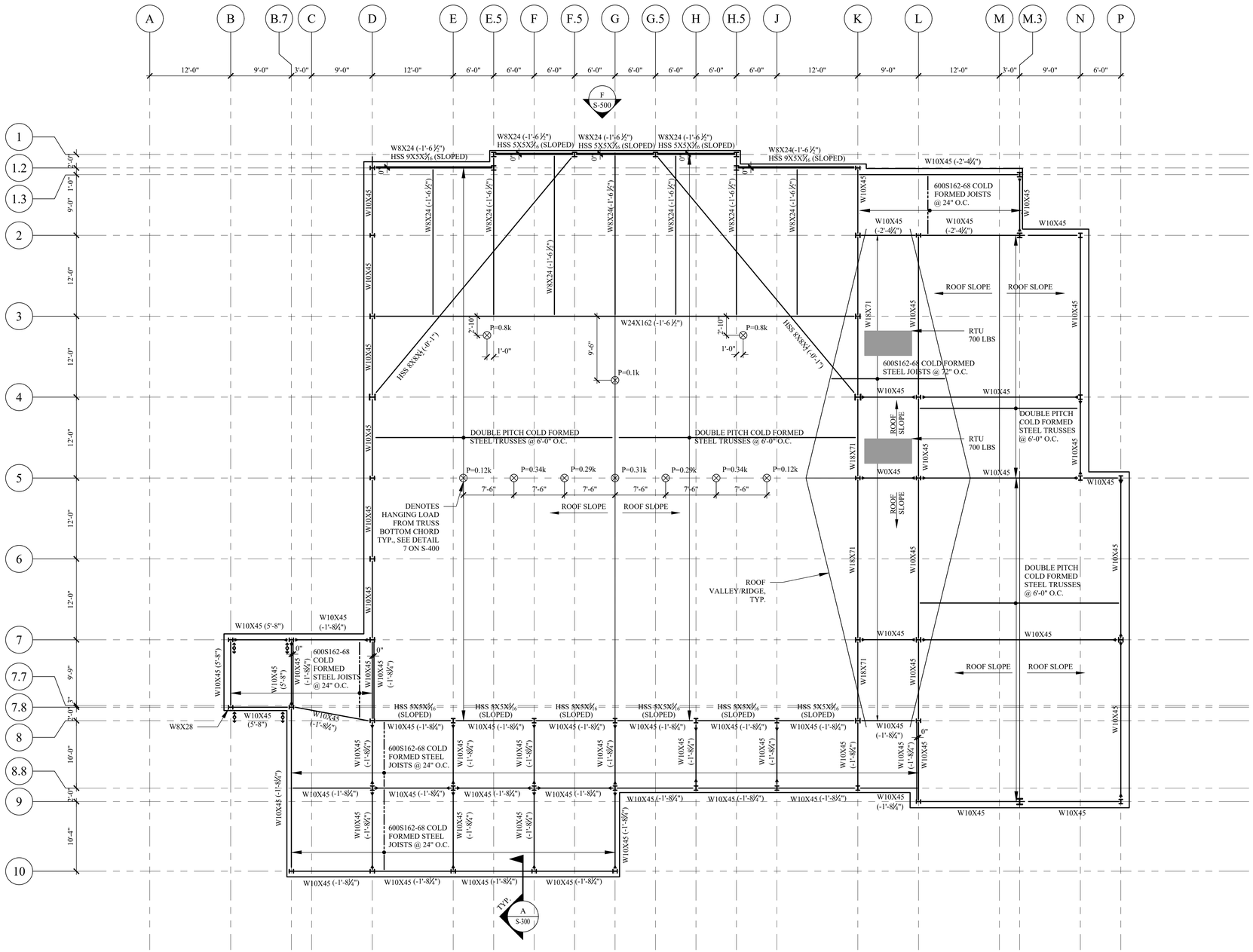


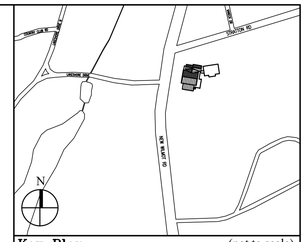
A ROOF PLAN

SCALE: 1/8"=1'-0"

N

NOTES:

1. SEE S-300 FOR TYPICAL STEEL DETAILS.
2. TOP OF STEEL ELEVATION IS 25'-0½". VALUE (X'-XX") BEHIND BEAM ID IS ELEVATION RELATIVE TO TYPICAL TOP OF STEEL ELEVATION.
3. VULCRAFT 1.5B22 METAL DECK TO SPAN BETWEEN ROOF JOISTS AND TRUSSES.
4. SEE S-500 AND S-501 FOR BRACED FRAME ELEVATIONS
5. SEE S-600 AND S-601 FOR BEAM REACTIONS FOR CONNECTION DESIGN BY STEEL FABRICATOR
6. DESIGN TRUSS FOR UPLIFT PRESSURE OF 19 PSF DUE TO WIND. DESIGN OF CFS IS BY DELEGATED ENGINEER PER NOTES ON S-002.
7. SEE S-601 FOR DRIFT SURCHARGE



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Drawing Title

ROOF PLAN

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-103.00

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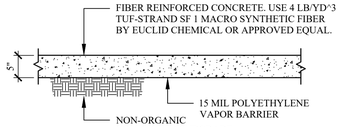
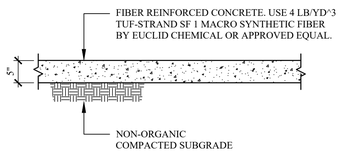


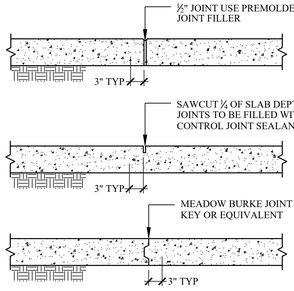
INTERIOR SLAB



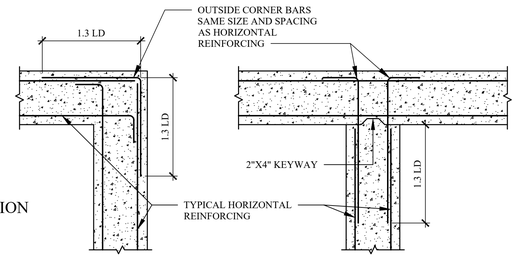
EXTERIOR SLAB

NOTES:
-PROVIDE CONTROL JOINTS PER DETAIL 2 AT 16'-0" O.C. MAX. COORDINATE JOINT LOCATIONS WITH ARCHITECT.

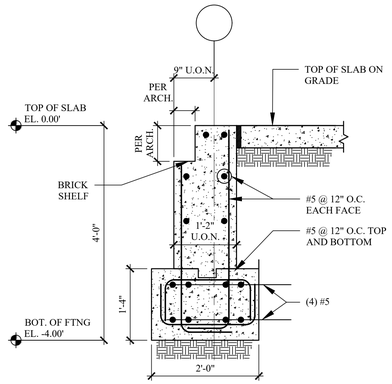
-SEE ARCHITECTURAL DRAWINGS FOR SLOPING REQUIRMENTS AT EXTERIOR SLABS



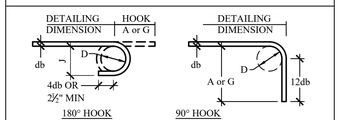
NOTES
* PROVIDE SEALANT WHERE REQUIRED BY ARCHITECT. SAWCUT AS SOON AS POSSIBLE AFTER CONCRETE HARDENS. SAW CUTTING MUST BE COMPLETE WITHIN 8 HRS AFTER CASTING.



CORNERS INTERSECTIONS



STANDARD HOOK DETAILS



RECOMMENDED END HOOKS, ALL GRADES

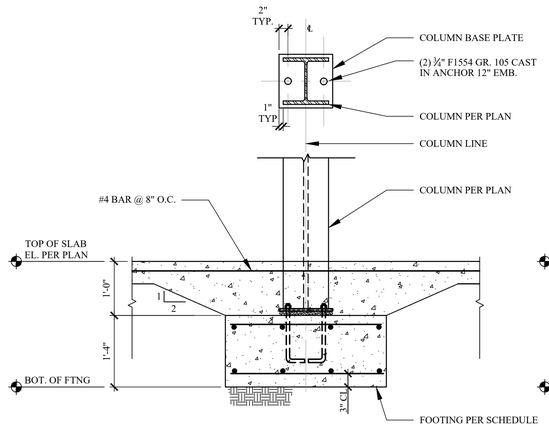
BAR SIZE	D	180° HOOKS		90° HOOKS
		A or G	J	A or G
#3	2½"	5"	3"	6"
#4	3"	6"	4"	8"
#5	3¾"	7"	5"	10"
#6	4½"	8"	6"	1'-0"
#7	5¼"	10"	7"	1'-2"
#8	6"	11"	8"	1'-4"
#9	9½"	1'-3"	11¼"	1'-7"
#10	10¼"	1'-5"	1'-1¼"	1'-10"
#11	12"	1'-7"	1'-2¼"	2'-0"

1 - DETAIL-1
TYPICAL SLAB ON GRADE DETAIL
SCALE: 3/4" = 1'-0"

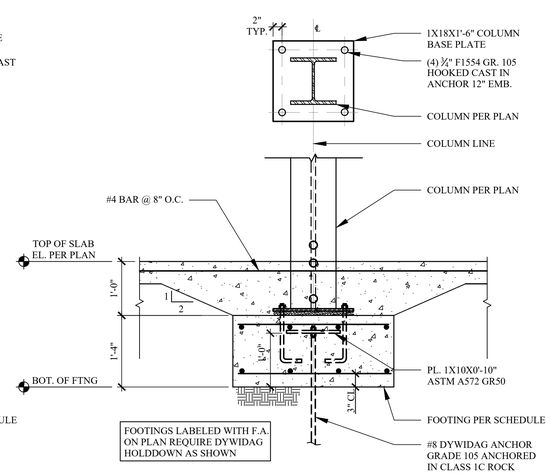
2 - DETAIL-2
SLAB ON GRADE CONTROL JOINTS
SCALE: 3/4" = 1'-0"

3 - DETAIL-3
TYPICAL HORIZONTAL REINFORCING
SCALE: 3/4" = 1'-0"

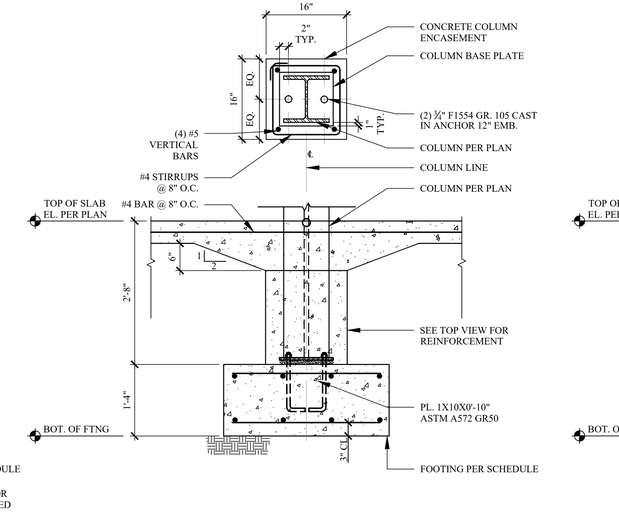
A S-100 SECTION A
TYPICAL WALL FOOTING DETAIL
SCALE: 3/4" = 1'-0"



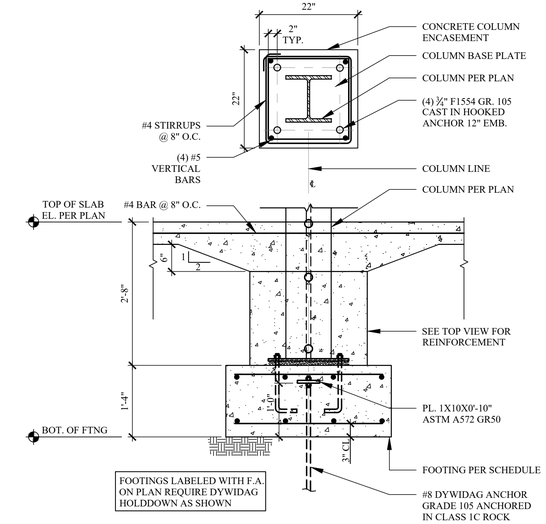
B S-100 TYPICAL COLUMN TO FOOTING DETAIL
COLUMN CENTERED ON FOOTING
SCALE: 3/4" = 1'-0"



B.1 S-100 TYPICAL COLUMN TO FOOTING DETAIL
COLUMN CENTERED ON FOOTING
SCALE: 3/4" = 1'-0"



C S-100 TYPICAL COLUMN TO FOOTING DETAIL
COLUMN CENTERED ON FOOTING
SCALE: 3/4" = 1'-0"



C.1 S-100 TYPICAL COLUMN TO FOOTING DETAIL
COLUMN CENTERED ON FOOTING
SCALE: 3/4" = 1'-0"

MIN. TENSION LAP SPLICE LENGTH SCHEDULE

f'c=4,000 psi fy=60,000 psi

CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN DB, CLEAR COVER NOT LESS THAN db, AND BEAM STIRRUPS OR COLUMN TIES THROUGHOUT LD NOT LESS THAN CODE MINIMUM

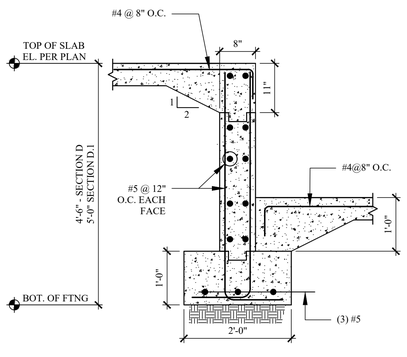
OR

CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN 2DB AND CLEAR COVER NOT LESS THAN db

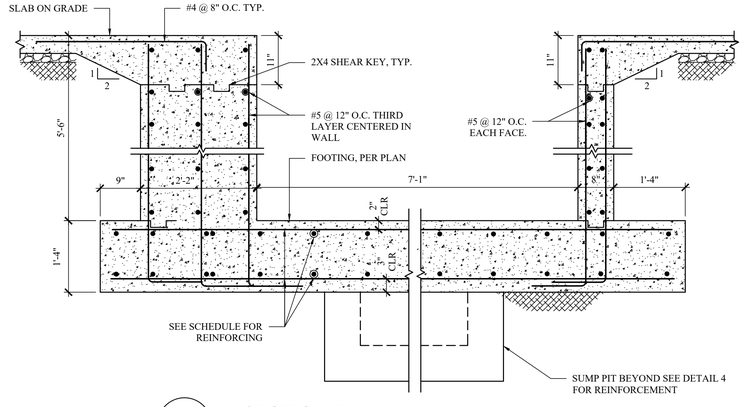
Bar size	dc	ld	ld (top bar)	1.3*ld	1.3*ld(top)
#4	0.500	19	25	25	32
#5	0.625	24	31	31	40
#6	0.750	28	37	37	48
#7	0.875	42	54	54	70
#8	1.000	47	62	62	80
#9	1.128	54	70	70	90
#10	1.270	60	78	78	102
#11	1.410	67	87	87	113
Other cases					
Bar size	dc	ld	ld (top bar)	1.3*ld	1.3*ld(top)
#4	0.500	28	37	37	48
#5	0.625	36	46	46	60
#6	0.750	43	55	55	72
#7	0.875	62	81	81	105
#8	1.000	71	92	92	120
#9	1.128	80	104	104	136
#10	1.270	90	117	117	153
#11	1.410	100	130	130	170

COMPRESSION DEVELOPENT LENGTH: 20 BAR DIAMETERS

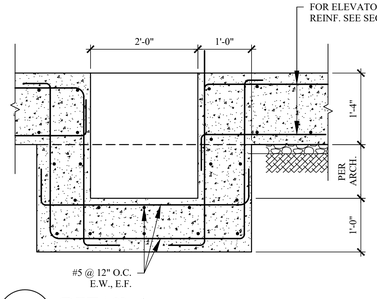
COMPRESSION SPLICE LENGTH: 30 BAR DIAMTETERS



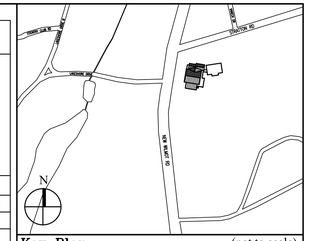
D S-100 SECTION D
SCALE: 3/4" = 1'-0"



E - SECTION E
ELEVATOR PIT
SCALE: 3/4"=1'-0"



4 - DETAIL 4
ELEVATOR SUMP PIT
SCALE: 3/4" = 1'-0"



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ROOFING CONSULTANT
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VALHALLA, NY 10595
914-948-3450

Stamp

Project Title
IONA PREPARATORY SCHOOL
ADDITION AND ALTERATION TO THE PAUL VERNI FINE ARTS CENTER

Project Address
IONA PREPARATORY SCHOOL
255 Wilmot Road
New Rochelle, NY 10804

Drawing Title
CONCRETE DETAILS - 1

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-200.00

Drawn

Peter Gisolfi Associates
Architects Landscape Architects, LLP
566 Warburton Avenue
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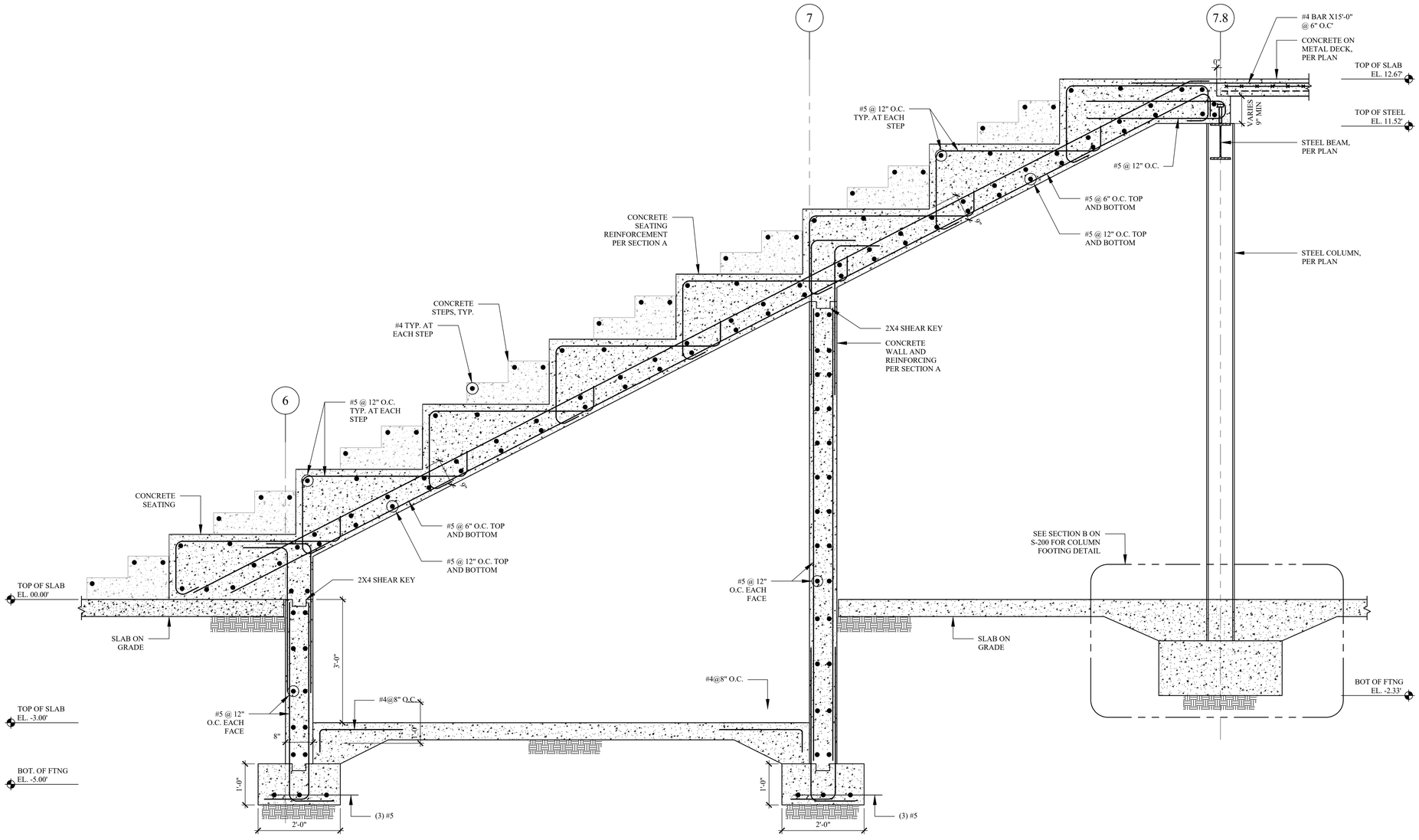
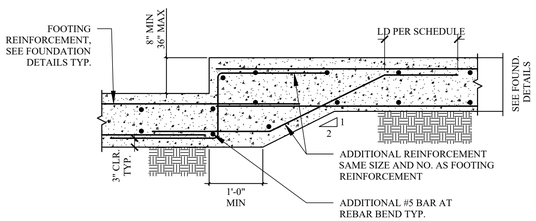
914 478 3677

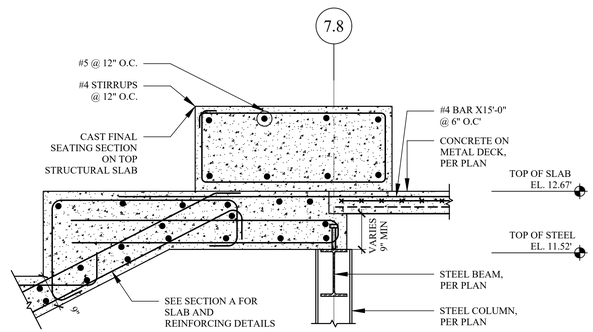
PETER GISOLFI ASSOCIATES



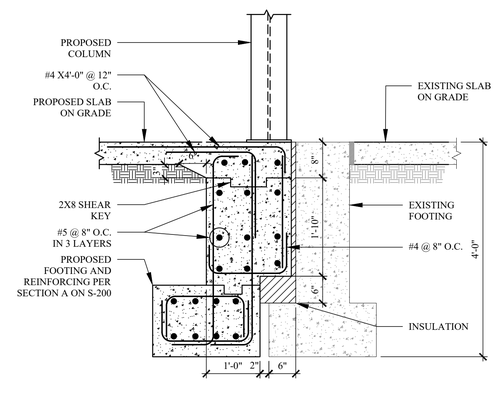
A SECTION A
S-101,S-102 SCALE: 3/4" = 1'-0"



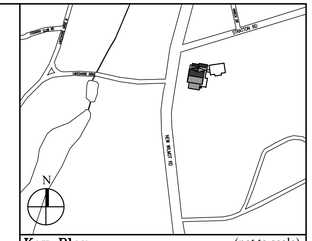
1 DETAIL
- TYPICAL STEP FOOTING DETAIL
SCALE: 3/4" = 1'-0"



B SECTION B
S-102 SCALE: 3/4" = 1'-0"



C SECTION C
S-100 FOOTING AT EXISTING BUILDING
SCALE: 3/4" = 1'-0"



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Drawing Title
CONCRETE DETAILS - 2

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-201.00

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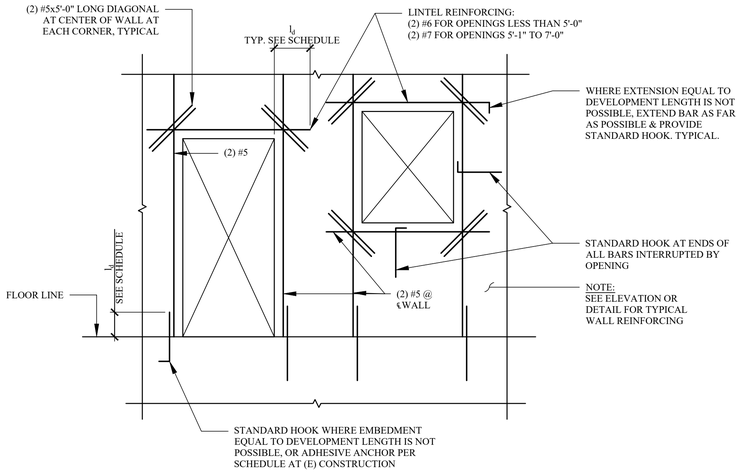
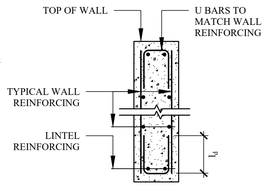
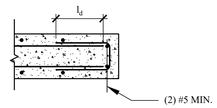


ELEVATION



LINTEL AND TOP OF WALL

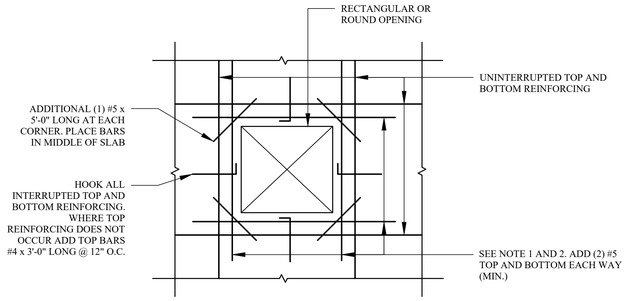


PLAN - JAMB OR END WALL

NOTES:

1. FOR WALLS WITH DOUBLE LAYERS OR REINFORCING, JAMB BARS AND CORNER BARS SHALL BE PLACE AT EACH FACE RATHER THAN AT WALL ℄
2. EXTEND JAMB BARS FLOOR TO FLOOR, OR FLOOR TO ROOF.

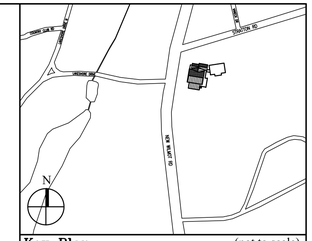
1 TYPICAL MINIMUM REINFORCING AT CONCRETE WALL OPENINGS



NOTES:

1. PLACE 50% OF TOP AND BOTTOM INTERRUPTED BARS AT EACH SIDE OF THE OPENING EACH WAY
2. BOTTOM BARS SHALL BE FULL SPAN LENGTH.

2 ADDITIONAL REINFORCING AT SLAB OPENINGS



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Project Title

IONA PREPARATORY SCHOOL
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Project Address

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New Rochelle, NY 10804

Drawing Title

CONCRETE DETAILS - 3

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-202.00

Drawn

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566 Warburton Avenue
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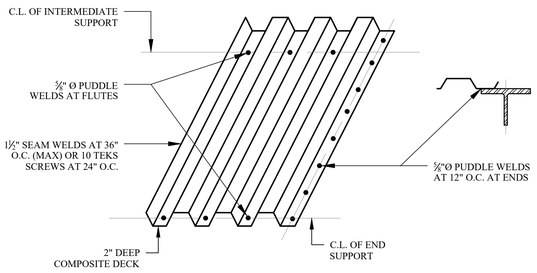
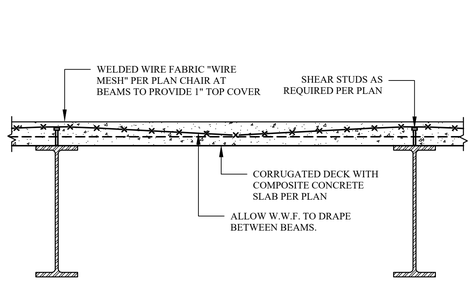
914 478 3677

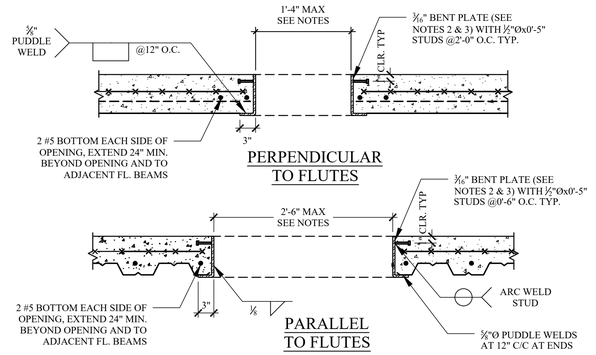
P E T E R G I S O L F I A S S O C I A T E S



1 - COMPOSITE DECK ATTACHMENT



2 - TYPICAL FLOOR BEAM AND DECK



1. SEE OTHER DETAILS FOR ADDITIONAL REQUIREMENTS AT LARGE OPENINGS IN SLABS.

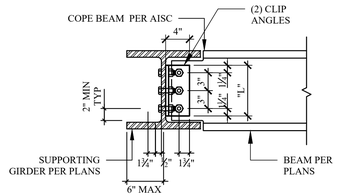
2. BEND ⅜" PLATE TO RADIUS AROUND CIRCULAR OPENINGS

3 - COMPOSITE SLAB OPENING

OPENING LESS THAN 1'-4" PERPENDICULAR AND 2'-6" PARALLEL TO RIBS

4 - NOT USED

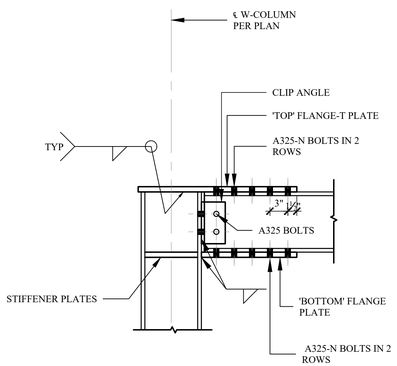
SUGGESTED CONNECTION. PROVIDE SHOP DRAWINGS AND CALCULATIONS AS NOTED ON S-002



5 - BEAM TO BEAM ANGLE CONNECTION

SCALE: 1"=1'-0"

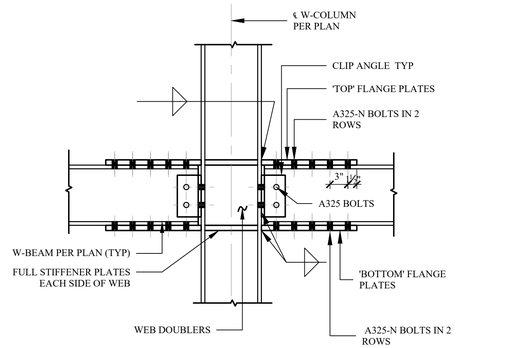
SUGGESTED CONNECTION. PROVIDE SHOP DRAWINGS AND CALCULATIONS AS NOTED ON S-002



6 - CONNECTION AT TOP SIDE COLUMN

SCALE: 1" = 1'-0"

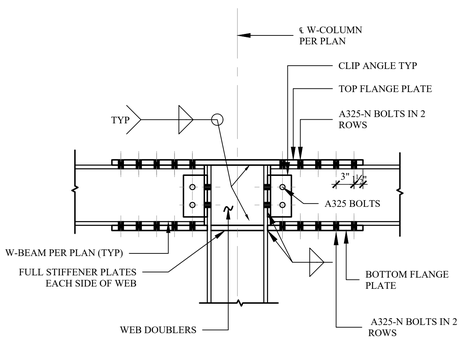
SUGGESTED CONNECTION. PROVIDE SHOP DRAWINGS AND CALCULATIONS AS NOTED ON S-002



7 - CONNECTION AT INTERMEDIATE JOINT

SCALE: 1" = 1'-0"

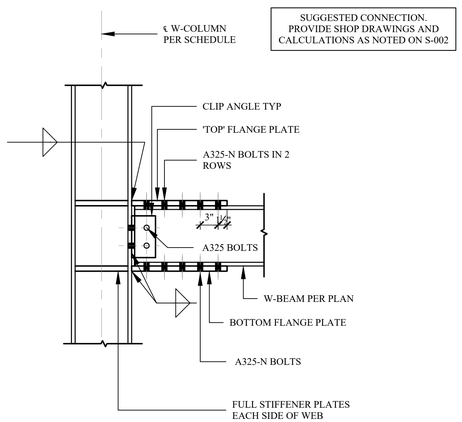
SUGGESTED CONNECTION. PROVIDE SHOP DRAWINGS AND CALCULATIONS AS NOTED ON S-002



8 - CONNECTION AT TOP MIDDLE COLUMN

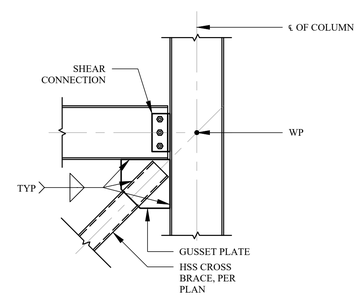
SCALE: 1" = 1'-0"

SUGGESTED CONNECTION. PROVIDE SHOP DRAWINGS AND CALCULATIONS AS NOTED ON S-002



9 - CONNECTION AT INTERMEDIATE JOINT

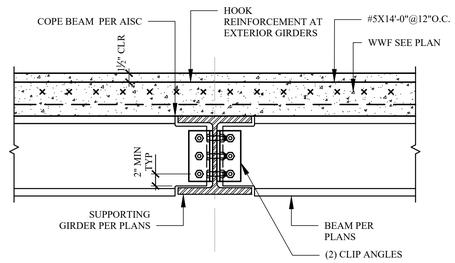
SCALE: 1" = 1'-0"



10 - DETAIL

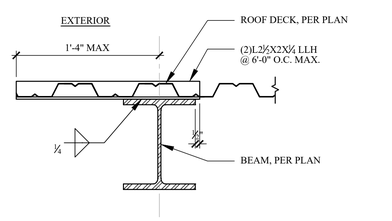
TYPICAL CROSS BRACE DETAIL
SCALE: 3/4" = 1'-0"

SUGGESTED CONNECTION. PROVIDE SHOP DRAWINGS AND CALCULATIONS AS NOTED ON S-002



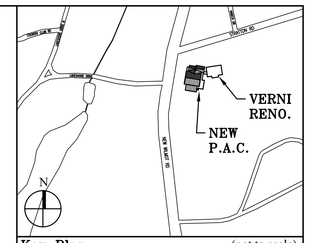
11 - TYP. FLOOR REINFORCEMENT OVER GIRDER

SCALE 1"=1'-0"



A - SECTION - A

SECTION AT ROOF OVERHANG
SCALE 1 1/2"=1'-0"



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STEEL DETAILS

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-300.00

Drawn

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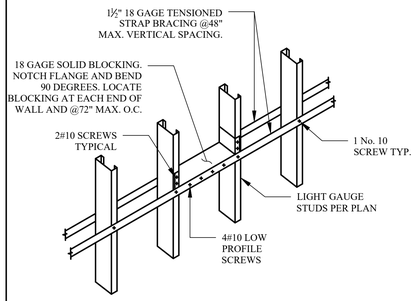
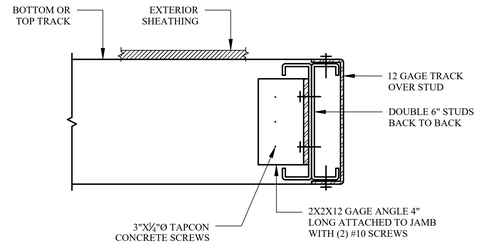
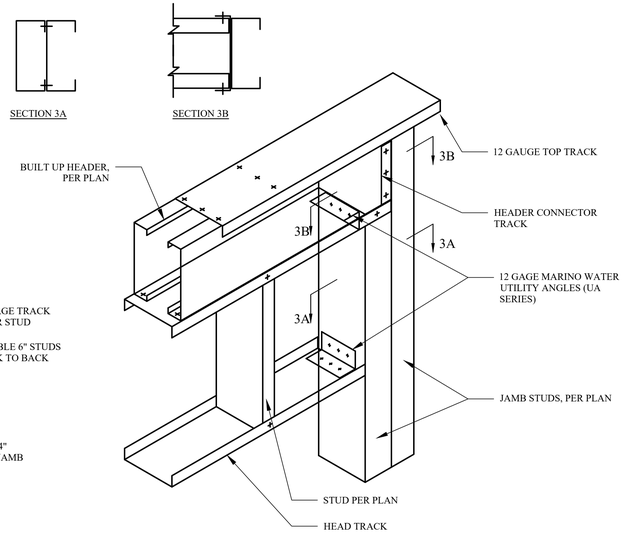


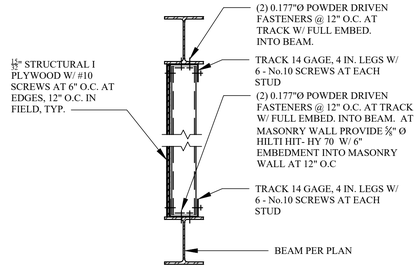
1 - TYPICAL STUD BRACING DETAIL
NOT TO SCALE



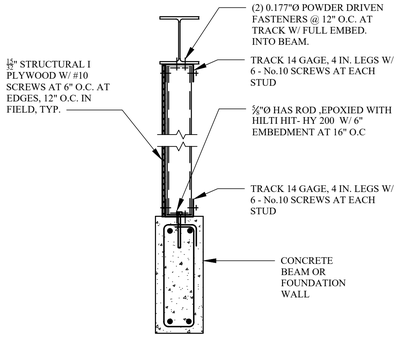
2 - JAM AT BOTTOM
PLAN DETAIL
NOT TO SCALE



3 - TYPICAL LTGA WALL OPENING DETAIL
NOT TO SCALE

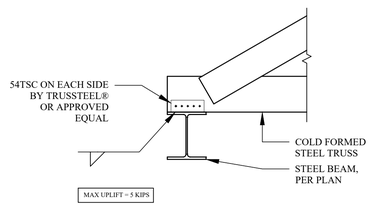


4 - TYPICAL LTGA WALL ON TOP OF STEEL BEAM
NOT TO SCALE

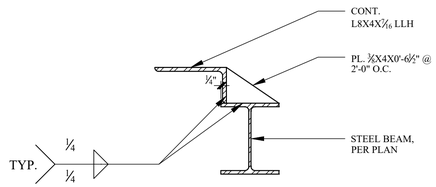


SHOWN ON TOP OF CONCRETE BEAM, ON TOP OF WALL SIMILAR

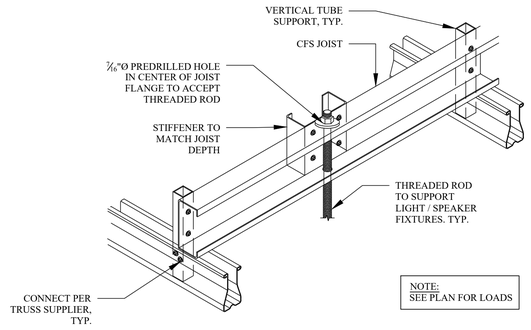
5 - TYP. LTGA WALL ON CONCRETE
NOT TO SCALE



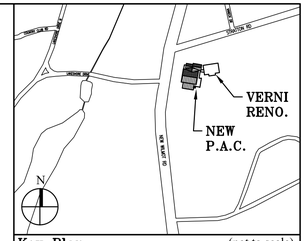
6 - TYP. TRUSS UPLIFT ATTACHMENT
NOT TO SCALE



A - SECTION A
SCALE: 1"=1'-0"



7 - HANGING SUPPORT DETAIL
SCALE: N.T.S.



Key Plan (not to scale)

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914-948-3450

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Project Title
IONA PREPARATORY SCHOOL
ADDITION AND ALTERATION TO THE PAUL VERNI FINE ARTS CENTER

Project Address
IONA PREPARATORY SCHOOL
255 Wilmot Road
New Rochelle, NY 10804

Drawing Title
COLD FORMED STEEL DETAILS

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-400.00

Drawn

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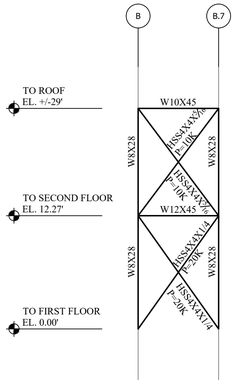
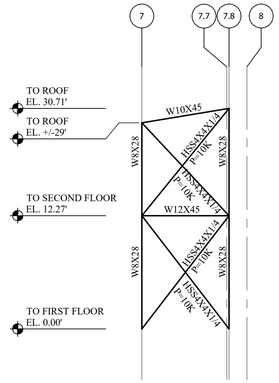
914 478 3677

PETER GISOLFI ASSOCIATES



A GRID B-B.7 ON 7.8 ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
1. ALL REACTIONS ARE SERVICE LOADING



B GRID 7-7.8 ON B ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
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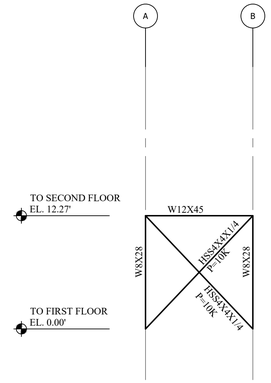

C GRID A-B ON 7 ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
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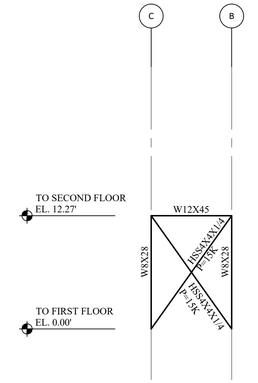

D GRID C-D ON 2 ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
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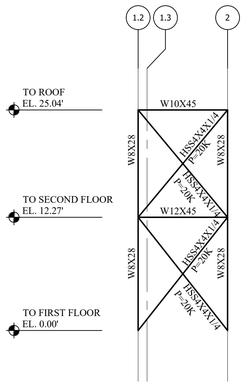

E GRID 1.2-2 ON D ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
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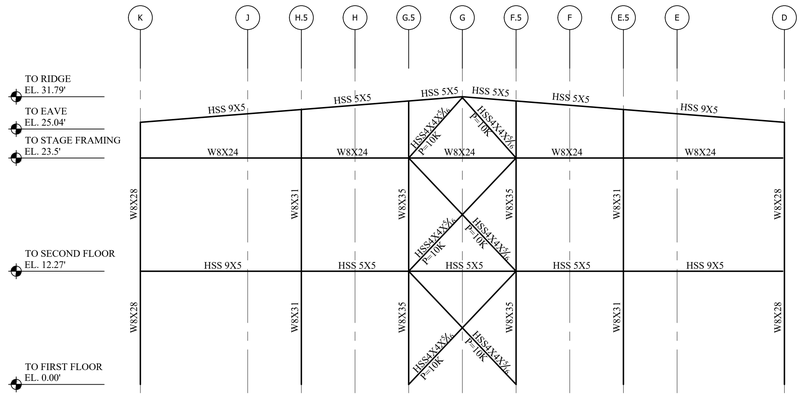

F GRID F.5-G.5 ON 1 ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
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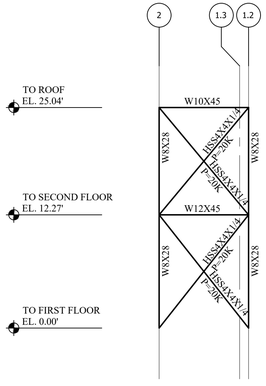

G GRID 1.2-2 ON K ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
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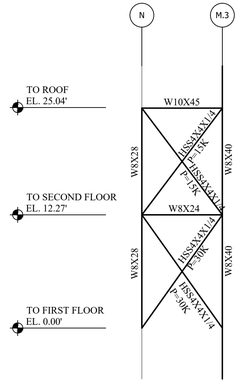

H GRID M.3-N ON 2 ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
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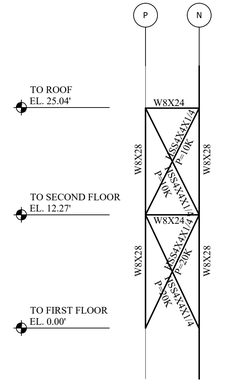

J GRID N-P ON 5 ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
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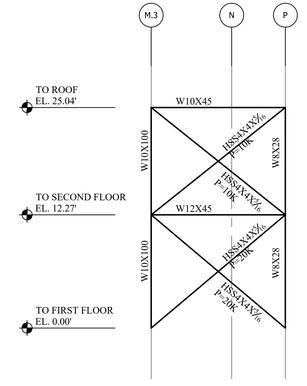
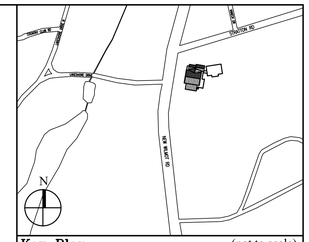


K GRID M.3-P ON 9 ELEVATION
SCALE: 1/8"=1'-0"

NOTES:
1. ALL REACTIONS ARE SERVICE LOADING



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Project Title
IONA PREPARATORY SCHOOL
ADDITION AND ALTERATION TO THE PAUL VERNI FINE ARTS CENTER

Project Address
IONA PREPARATORY SCHOOL
255 Wilmot Road
New Rochelle, NY 10804

Drawing Title
FRAMING ELEVATIONS - 1

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-500.00

Drawn

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566 Warburton Avenue
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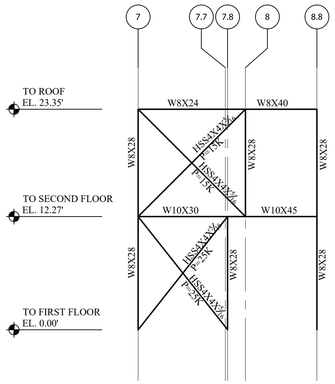
914 478 3677

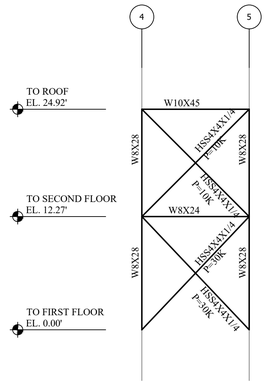
PETER GISOLFI ASSOCIATES



L GRID 7-7.8 ON D ELEVATION

SCALE: 1/8"=1'-0"

NOTES:
1. ALL REACTIONS ARE SERVICE LOADING



M GRID 4-5 ON L ELEVATION

SCALE: 1/8"=1'-0"

NOTES:
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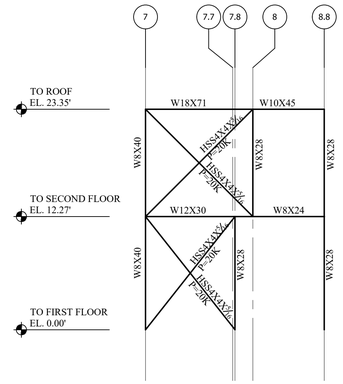

N GRID 7-7.8 ON K ELEVATION

SCALE: 1/8"=1'-0"

NOTES:
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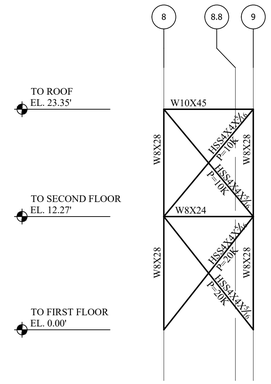

O GRID 8-9 ON L ELEVATION

SCALE: 1/8"=1'-0"

NOTES:
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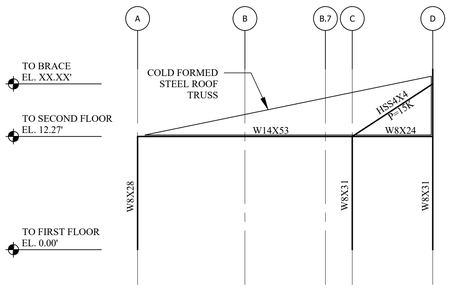

P SECTION

SCALE: 1/8"=1'-0"

NOTES:
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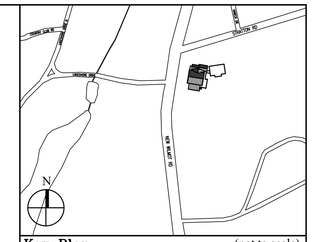

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Project Title

IONA PREPARATORY SCHOOL
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Project Address

IONA PREPARATORY SCHOOL
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Drawing Title

FRAMING ELEVATIONS - 2

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-501.00

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566 Warburton Avenue
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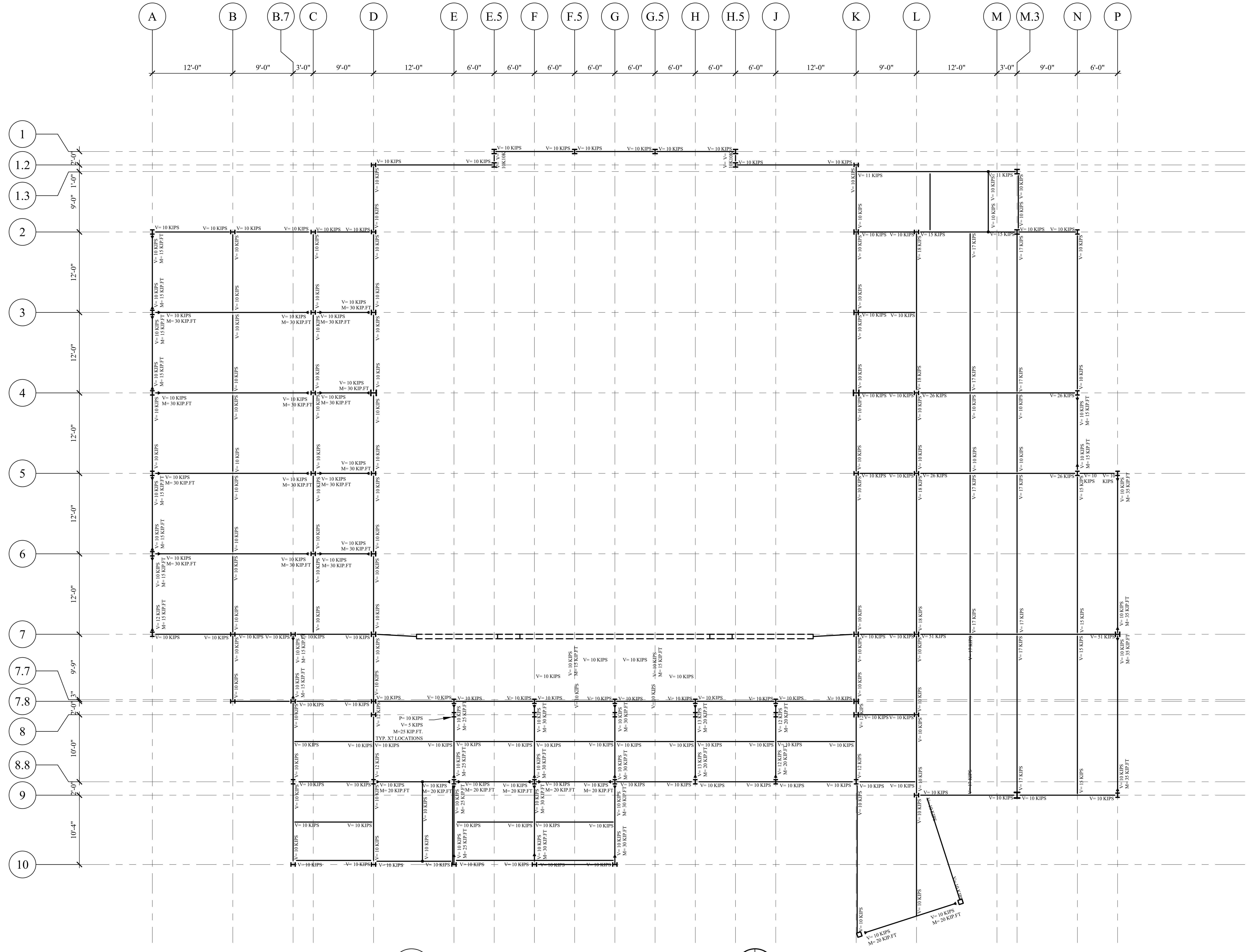
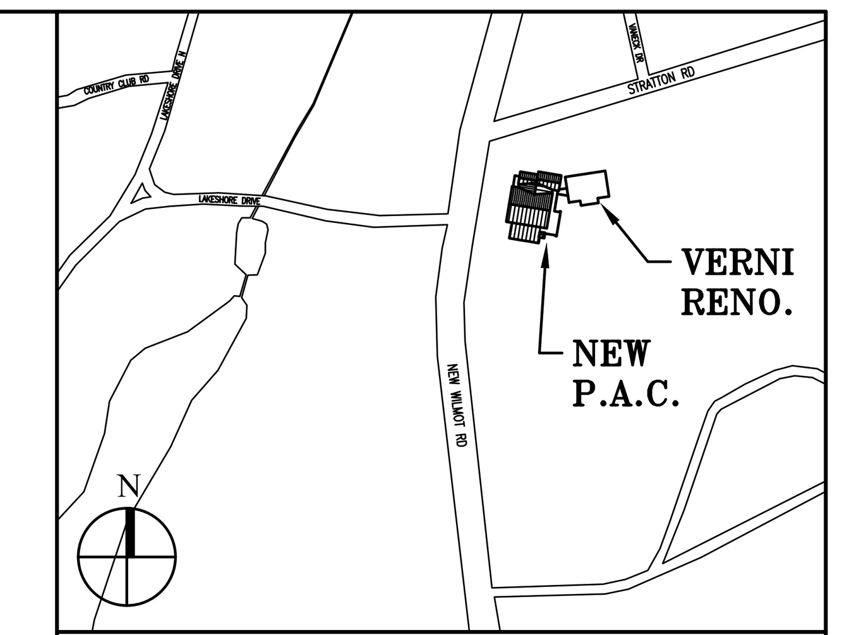
P E T E R G I S O L F I A S S O C I A T E S



SECOND FLOOR BEAM REACTION PLAN

SCALE: 1/8"=1'-0"

NOTES:
1. ALL REACTIONS ARE SERVICE LOADS



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Drawing Title

SECOND FLOOR BEAM REACTION PLAN

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-600.00

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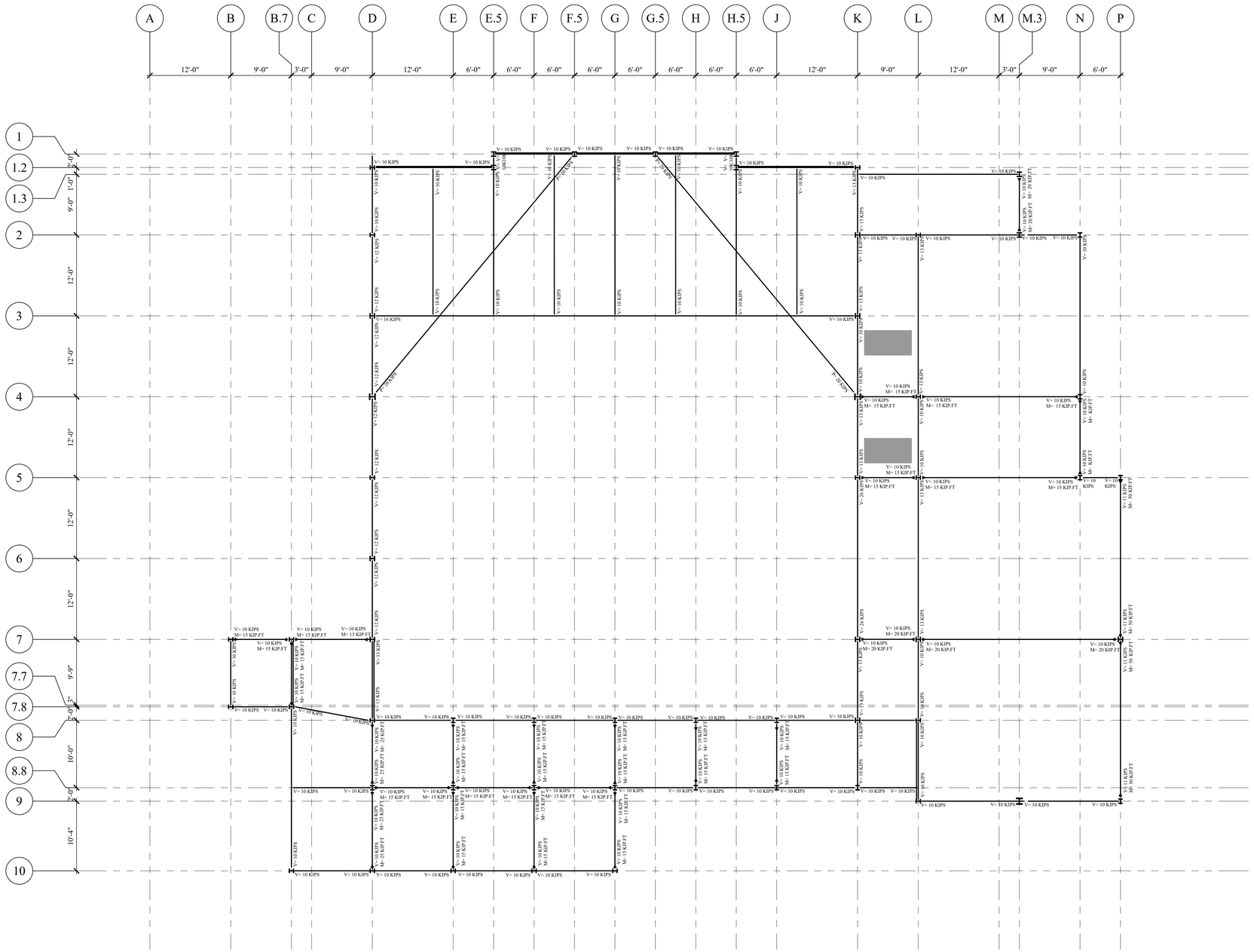

A ROOF BEAM REACTION PLAN

SCALE: 1/8"=1'-0"

NOTES:
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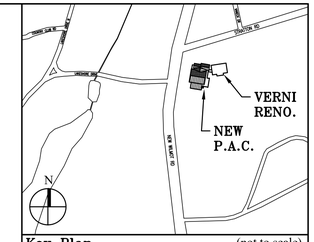

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Drawing Title

ROOF BEAM REACTION PLAN

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-601.00

Drawn

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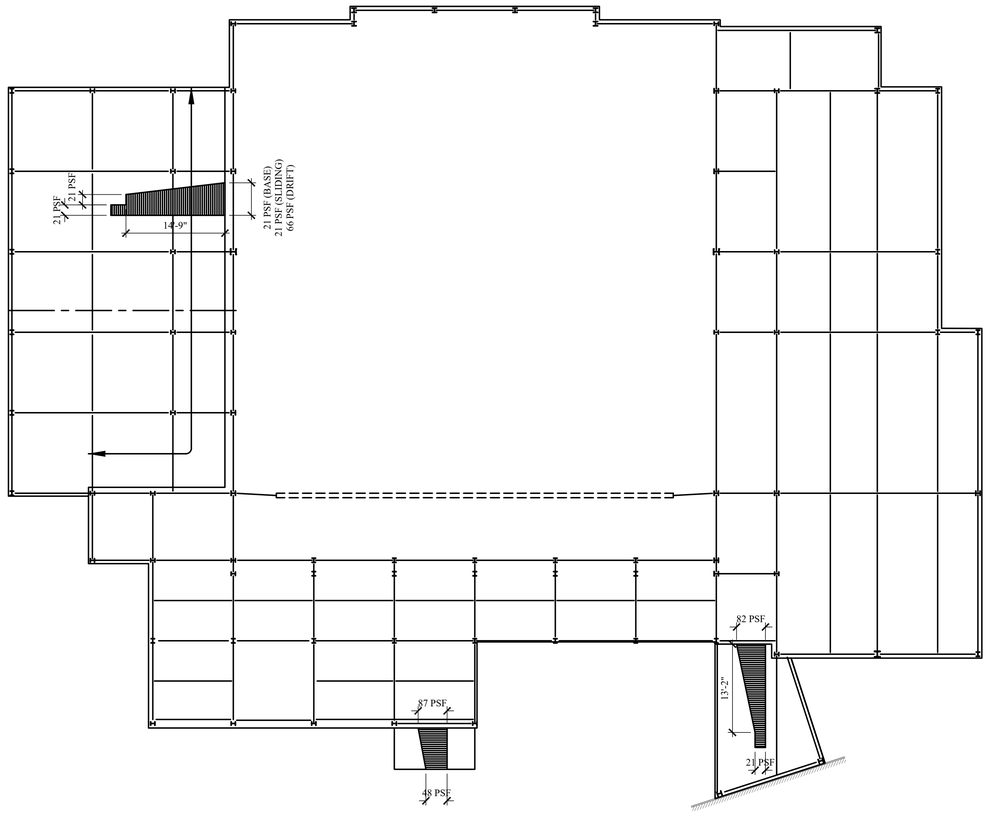
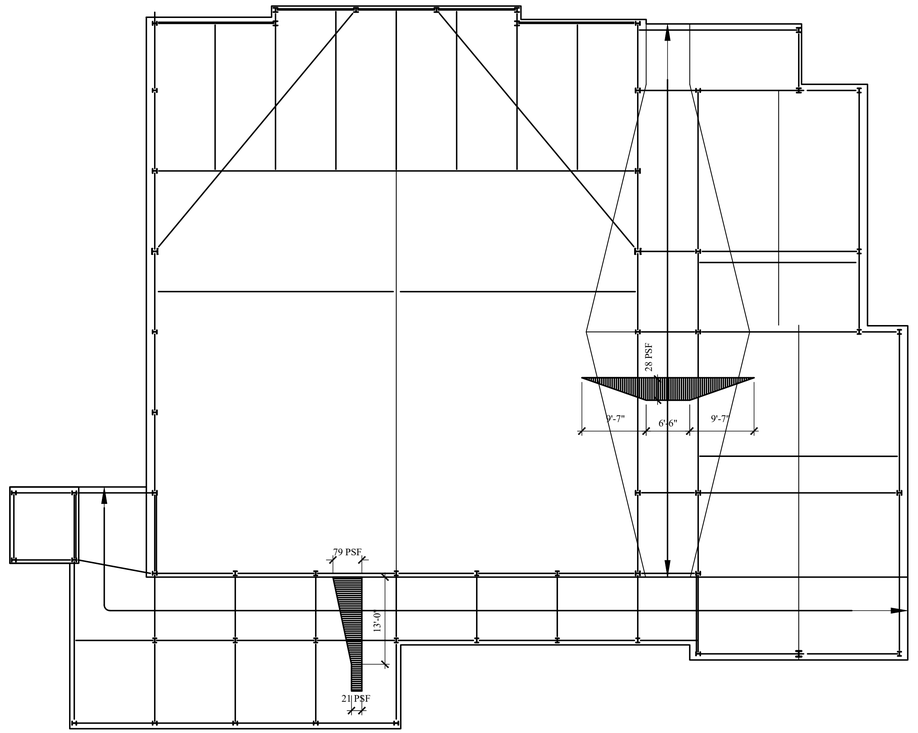
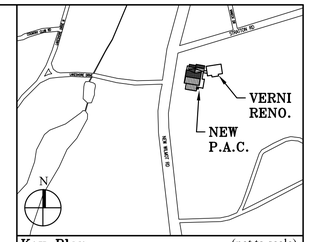


A SECOND FLOOR SNOW DRIFT DIAGRAM
SCALE: 3/32"=1'-0"



B ROOF SNOW DRIFT DIAGRAM
SCALE: 3/32"=1'-0"



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Drawing Title

SNOW DRIFT DIAGRAM

Scale	Job No.	Date	Drawing No.
	1618	04/03/2019	S-602.00

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