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ARCHITECTURE • ENGINEERING • PLANNING

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OSSINING UNION FREE
SCHOOL DISTRICT
BROOKSIDE ELEMENTARY SCHOOL
CLASSROOM ADDITION PROJECT

SED# 66-14-01-03-0-001-022

DATE	DRAWN	CHECKED
12/18/2020	RHW	RHW

SCALE 1"=30'

SHEET TITLE
**LOGISTICS
PLAN**

PROJECT NUMBER
14428.11

Drawing Name: \\clarkpatterson.local\dts\Projects\Ossining UFSD\Brookside 2 CR Add\VD Design\06 CAD\AutoCAD\STRUCT\38 BES S800.dwg

Date last acRfCsed: 1/19/2021 8:11 AM

Date last plotted: 1/19/2021 8:16 AM

Plotted By: Joe Rausch

GENERAL NOTES

1. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND CONDITIONS SHOWN ON THE DRAWINGS AND IMMEDIATELY NOTIFY THE OWNER'S REPRESENTATIVE OF ANY DISCREPANCIES PRIOR TO ORDERING OR FABRICATING MATERIALS OR OTHERWISE PROCEEDING WITH THE WORK.
 2. PROVIDE ALL LABOR, MATERIAL, EQUIPMENT AND SERVICES REQUIRED TO EXECUTE AND COMPLETE ALL ITEMS OF WORK AS SHOWN OR INDICATED ON THE DRAWINGS AND AS SPECIFIED HEREIN, INCLUDING INCIDENTAL ITEMS TO EFFECT A FINISHED AND COMPLETE JOB, EVEN THOUGH SUCH ITEMS ARE NOT SHOWN OR PARTICULARLY MENTIONED.
 3. ALL STRUCTURAL WORK, INCLUDING MATERIAL STRESSES AND METHODS OF CONSTRUCTION, SHALL BE IN COMPLIANCE WITH THE "INTERNATIONAL BUILDING CODE AS ADOPTED BY NEW YORK STATE", THE UNIFORM BUILDING CODE, OSHA AND GOVERNING AGENCIES HAVING JURISDICTION.
 4. CONTRACTOR SHALL MAINTAIN SAFE PUBLIC ACCESS TO AND FROM SITE AT ALL TIMES AND SHALL NOT BLOCK SHIPPING OR RECEIVING.
 5. CONTRACTOR SHALL USE CONSTRUCTION METHODS THAT ARE IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
 6. CONTRACTOR SHALL BE COMPLETELY RESPONSIBLE FOR ADEQUATELY SHORING EXISTING CONSTRUCTION WHILE PERFORMING NEW WORK.
 7. THE CONTRACTOR SHALL COORDINATE ALL STRUCTURAL WORK WITH THE ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS, AND WITH THE WORK OF ALL OTHER TRADES.
 8. REFER TO MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF MECHANICAL OPENINGS.
 9. ALL FLOOR, WALL AND CEILING PENETRATIONS THROUGH FIRE SEPARATION SHALL BE SEALED AGAINST SMOKE AND FIRE TRANSMISSION.
 10. MAINTAIN PREMISES AND PUBLIC PROPERTIES FREE FROM ACCUMULATIONS OF WASTE MATERIALS, DEBRIS AND RUBBISH CAUSED BY OPERATIONS.
 11. AFTER COMPLETION OF WORK, CONTRACTOR SHALL REMOVE ALL WASTE MATERIALS, RUBBISH, TOOLS, AND SURPLUS MATERIALS AND CLEAN SIGHT EXPOSED SURFACES. CONSTRUCTION SITE SHALL BE ORGANIZED AND CLEANED UP BY CONTRACTOR ON A DAILY BASIS.
 12. THE CONTRACTOR SHALL RESTORE TO ITS ORIGINAL CONDITION ALL SITE APPURTENANCES DAMAGED UNDER THIS CONTRACT AT NO ADDITIONAL COST TO THE OWNER.
 13. ALL MATERIAL SCHEDULED FOR DEMOLITION SHALL BECOME THE PROPERTY OF THE CONTRACTOR UNLESS NOTED OTHERWISE AND SHALL BE DISPOSED OF LEGALLY.
 14. ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AN SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE, UNLESS SPECIFICALLY SHOWN OTHERWISE.
 15. COORDINATE ALL WORK WITH GEOTECHNICAL REPORT, TYPICAL
- EXCAVATION & BACKFILL
1. EXCAVATIONS TO BE SHEETED AND BRACED, OR LAID BACK TO PREVENT SLOUGHING IN OF THE ESCAVATED AREAS PER OSHA REGULATIONS.
 2. PLACE ALL FOOTINGS ON FIRM, DRY, LEVEL, ACCEPTABLE BEARING SOIL.
 3. REMOVE AND DISPOSE OF LEGALLY FROM SITE:
 - UNACCEPTABLE BEARING SOIL
 - EXCESS EXCAVATED MATERIAL
 - ASPHALT MATERIAL
 4. BACKFILL WITHIN BUILDING - TO WITHIN 6 INCHES OF UNDERSIDE OF FLOOR SLAB SHALL BE "SUBBASE COURSE" (NYSDOT 304.03) CONSISTING OF HARD DURABLE PEBBLES, ROCK FRAGMENTS AND SOIL BINDER. IT SHALL BE FREE OF CLAY, ORGANIC MATTER, AND OTHER DELETERIOUS MATERIAL. GRADATION: 3 INCHES MAXIMUM SIZE, 25-60% PASSING THE 1/4" SIEVE, 5-40% PASSING NO. 40 SIEVE, AND NOT MORE THAN 10% PASSING NO. 200 SIEVE.
 5. UNDER SLABS ON GRADE - POROUS 6 INCH LIFT OF WASHED "CRUSHED STONES" CONSISTING OF A 50/50 MIX OF #1'S AND #2'S.
 6. BACKFILL OUTSIDE OF BUILDING - "SELECT GRANULAR FILL" (NYSDOT 203.06) CONSISTING OF SAND, FINE GRAVEL, COARSE SILT, OR SIMILAR NON-COHESIVE HARD DURABLE MATERIALS AND SOIL BINDERS WITHOUT EXCESSIVE CLAY, ORGANIC MATTER, OR FROZEN OR DELETERIOUS MATERIAL. GRADATION: 8 INCHES MAXIMUM SIZE, 0-70% PASSING THE #40 SIEVE AND 0-15% PASSING THE #200 SIEVE.
 7. FILL COMPACTION:
 - A. WITHIN BUILDING - 95% DRY DENSITY BY MODIFIED PROCTOR
 - B. OUTSIDE OF BUILDING - 95% DRY DENSITY BY MODIFIED PROCTOR
 8. FILL PLACEMENT - PLACE FILL SIMULTANEOUSLY ON EACH SIDE OF FOUNDATION WALL IN 6 INCH LIFTS. THE MAXIMUM DIFFERENCE IN ELEVATION ON EITHER SIDE OF WALL SHALL NOT EXCEED 1'-6".
- OPEN WEB STEEL JOISTS
1. MANUFACTURE AND INSTALLATION OF OPEN WEB STEEL JOISTS SHALL CONFORM TO THE "STANDARD SPECIFICATIONS" OF THE STEEL JOIST INSTITUTE.
 2. DESIGN AND INSTALLATION OF BRIDGING SHALL CONFORM TO THE STEEL JOIST INSTITUTE SPECIFICATIONS.
 3. ALL JOISTS SHALL BE CONNECTED TO SUPPORTING STEEL BY TWO 1/4" FILLET WELDS, 3" LONG, OR TWO 3/4 INCH ANCHOR BOLTS.
 4. WHERE HVAC DUCTWORK INTERSECTS DIAGONAL BRIDGING LINES, PROVIDE HORIZONTAL BRIDGING AT TOP AND BOTTOM CHORDS AS FOLLOWS:
 - A. INSTALL DIAGONAL BRIDGING AS TYPICAL DURING ERECTION, AFTER ERECTION
 - B. REMOVE DIAGONALS AND INSTALL HORIZONTAL BRIDGING.
 - C. HORIZONTAL REPLACEMENT BRIDGING SHALL BE DESIGNED AND SUPPLIED BY JOIST MANUFACTURER.
 - D. REFER TO MECHANICAL DRAWINGS FOR EXTENT AND LOCATIONS.
 - E. AT NO TIME SHALL THIS BE DONE IN TWO CONSECUTIVE BAYS w/o ENGINEER APPROVAL.
 5. PROVIDE MINIMUM CAMBER TO JOISTS IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE.
 6. PROVIDE BOTTOM CHORD EXTENSIONS TO BOTTOM OF EXTERIOR BEAMS.
 7. CONTRACTOR SHALL PROVIDE ANY AND ALL EXTRA STEEL TO FRAME AROUND ANY MECHANICAL ROOF PENETRATIONS. PROVIDE A MINIMUM OF TWO EXTRA JOISTS, SAME SIZES AS SHOWN ON DRAWINGS, FOR EACH AC UNIT LOCATION. SUBMIT PROPOSAL TO ENGINEER FOR REVIEW.
 8. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW.

CONCRETE NOTES: GENERAL

1. ALL CONCRETE WORK, CONSTRUCTION AND REINFORCING DETAILS SHALL CONFORM TO THE "BUILDING CODE OF NEW YORK STATE" AND "THE SPECIFICATIONS OF THE AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS" (ACI-318).
2. ALL CONCRETE SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. SEE SPECIFICATIONS FOR MIX DESIGN REQUIREMENTS.

LOCATION	WATER RATIO	SLUMP (±1")	MAX. AGG.	% AIR (±1)
FOUNDATION	.50	3.5"	1"	6
SLAB ON GRADE	.45	3.5"	3/4"	4
3. CONTRACTOR SHALL SUBMIT MIX DESIGNS PROPORTIONED BY A LICENSED TESTING LABORATORY.
4. PROVIDE MINIMUM OF FOUR (4) CYLINDERS PER EACH FIFTY (50) YARDS OR FRACTION THEREOF POURED IN ONE DAY. BREAK ONE AT 7 DAYS AND TWO AT 28 DAYS.
5. ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH "ACI MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES" (ACI-315).
6. REINFORCING STEEL SHALL CONFORM TO ASTM A-615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.
7. LAP SPLICES AND EMBEDMENT LENGTHS SHALL CONFORM TO ACI 318 - CHAPTER 12.
8. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCING WHERE WALLS OR BEAMS MEET AT CORNERS OR INTERSECT. THIS ALSO INCLUDES INTERSECTIONS OF CONCRETE WITH MASONRY WORK.
9. PROVIDE SHOP DRAWINGS FOR REINFORCING INCLUDING ALL NECESSARY ACCESSORIES TO HOLD REINFORCING SECURELY IN PLACE.
10. CLEAR COVER CONCRETE PROTECTION FOR REINFORCING STEEL SHALL BE:
 - 3'- CONCRETE CAST AGAINST EARTH.
 - 2'- FORMED SURFACES IN CONTACT WITH SOIL OR EXPOSED TO WEATHER.
 - 1'- FORMED SURFACES NOT IN CONTACT WITH SOIL OR EXPOSED TO WEATHER.3/4" - SUPPORTED FLOOR SLABS.

FOUNDATIONS

1. MAXIMUM ALLOWABLE SOIL BEARING PRESSURE IS 2500 PSF. SEE GEOTECHNICAL EVALUATION BY QUALITY GEO ENGINEERING, P.C., DATED 5/4/2020. BEARING PRESSURE SHALL BE FIELD VERIFIED AND FOUNDATIONS LOWERED TO ACCEPTABLE BEARING SOIL IF POOR SOIL IS ENCOUNTERED.
2. ALL FOUNDATION EXCAVATIONS ARE SUBJECT TO APPROVAL BY THE OWNER'S REPRESENTATIVE BEFORE ANY CONCRETE IS PLACED.
3. ALL FORMS AND REINFORCING STEEL IN PLACE SHALL BE APPROVED BY THE OWNER'S REPRESENTATIVE BEFORE ANY CONCRETE IS PLACED.
4. NO FOUNDATION SHALL BE PLACED IN WATER OR ON FROZEN GROUND.
5. IN GENERAL, EXTERIOR CONSTRUCTION SHALL BE CARRIED DOWN A MINIMUM OF 4'-0" BELOW FINISHED EXTERIOR GRADE.
6. CENTERLINE OF FOOTINGS, WALLS, GRADE BEAMS, COLUMNS, AND BEAMS SHALL COINCIDE, UNLESS OTHERWISE NOTED.
7. REFER TO ARCHITECTURAL DRAWINGS FOR FOUNDATION DRAINAGE.
8. ALL CONCRETE USED ABOVE GRADE SHALL HAVE AN AIR ENTRAINING AGENT.
9. RUB ALL SIGHT EXPOSED CONCRETE AFTER FORMS HAVE BEEN REMOVED.
10. EXPOSED CONCRETE PIER CORNERS SHALL BE CHAMFERED 3/4".
11. ALL GROUT FOR BASE PLATES SHALL BE NON-SHRINK WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS.
12. ANCHOR BOLTS - ASTM F1554, Fy=55 KSI, 3/4" DIAMETER U.N.O.
13. ISOLATION JOINT - ASPHALT IMPREGNATED FILLER STRIP CONFORMING TO ASTM D-944.
14. ALL STEEL COLUMNS BELOW GRADE SHALL BE TWICE COATED WITH A BITUMINOUS COATING.
15. CONTRACTOR SHALL VERIFY ALL DIMENSIONS ON THE JOB BEFORE COMMENCING WORK. REFER TO ARCHITECTURAL DRAWINGS FOR ANY DIMENSIONS AND DETAILS NOT SHOWN. REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR LOCATION AND DIMENSIONS OF ANY OPENING, SLEEVES, INSERTS, SLAB DEPRESSIONS, ETC.
16. EPOXY ANCHORS SHALL BE HIT HY 150 INJECTION ADHESIVE ANCHORS AS MANUFACTURED BY HILTI, INC., TULSA OK (800-879-8000).
17. PERIMETER INSULATION SHALL BE 1 1/2" TYPE SM STYROFOAM AS MANUFACTURED BY DOW CHEMICAL CO.

SLABS ON GRADE

1. ALL SLABS ON GRADE SHALL BE PLACED OVER A 15 MIL VAPOR BARRIER, TAPE ALL SEAMS AND PROVIDE FLASHING/BOOTS AROUND PIPE PENETRATIONS.
2. UNDER SLABS ON GRADE, PROVIDE POROUS 6" LIFT OF WASHED "CRUSHED STONES" CONSISTING OF A 50/50 MIX OF #1'S AND #2'S. POROUS FILL SHALL BE COMPACTED TO 95% DENSITY.
3. SLAB ON GRADE REINFORCEMENT SHALL BE 6x6-W2.1xW2.1WWF, UNLESS NOTED OTHERWISE.
4. MEMBRANE CURING COMPOUND SHALL BE USED ON ALL TOP SURFACES AND SHALL CONFORM TO ASTM C-309.
5. CONTROL JOINTS: JOINTS SHALL BE SPACED NO FARTHER THAN 15'-0" O.C. JOINTS SHALL TYPICALLY RUN BETWEEN COLUMNS AND TERMINATE AT A COLUMN ISOLATION POUR. THE LENGTH OF ANY INDIVIDUAL JOINTED AREA SHALL NOT EXCEED 1.5 TIMES ITS WIDTH.
6. CONSTRUCTION/COLD JOINTS:
7. CONCRETE SURFACE SHALL BE HARD STEEL TROWEL FINISH.
8. FOR FLOOR FINISH, FLOOR DRAINS, SLAB DEPRESSIONS, AND WATERPROOFING DETAILS SEE ARCHITECTURAL DRAWINGS.

MASONRY NOTES

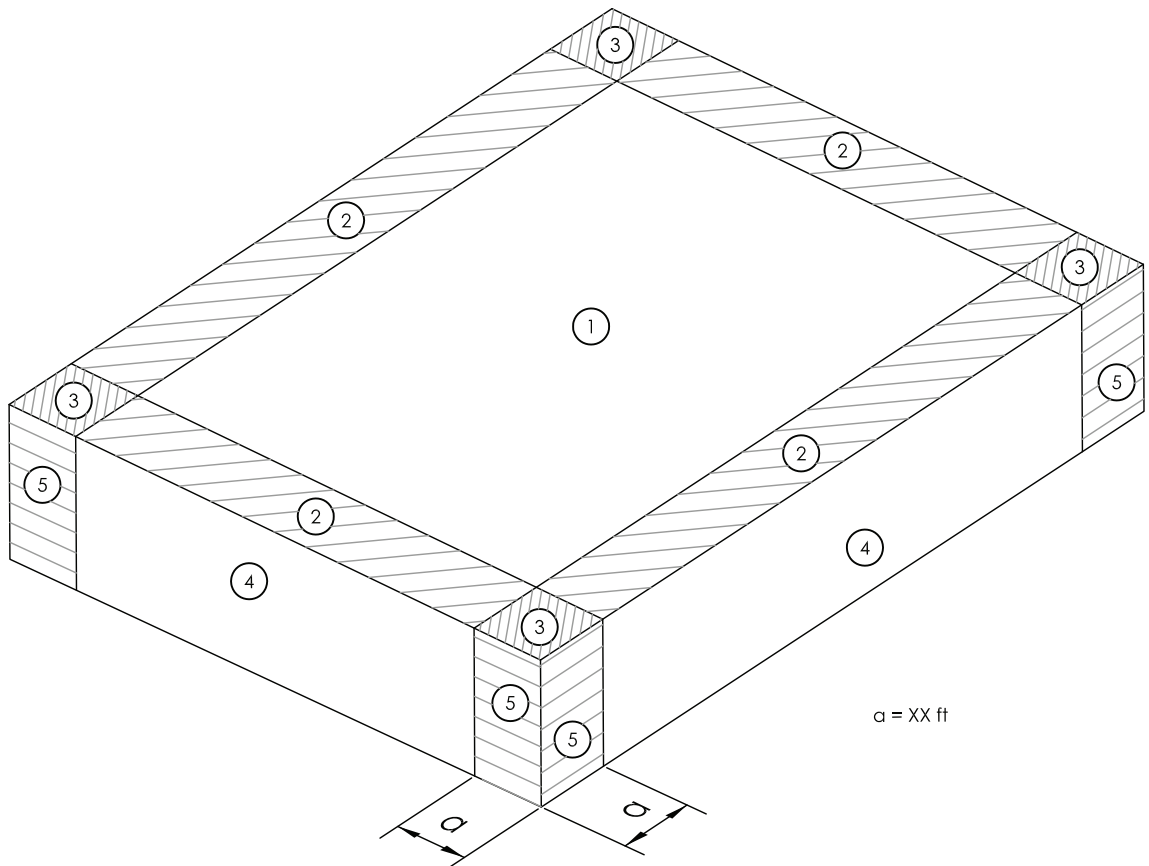
1. MASONRY CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES" (ACI-ASCE 530 AND 530.1).
2. ALL CONCRETE BLOCK SHALL CONFORM TO ASTM-C90 GRADE N-I. TWO CORE BLOCK F'M = 1,900 PSI
3. ALL GROUT SHALL BE NON-SHRINK, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. AS MEASURED IN ACCORDANCE WITH ASTM C-109. GROUT FOR FILINGS CELLS SHALL CONFORM TO ASTM C-476. SUBMIT MIX DESIGN FOR GROUT.
4. ALL MORTAR SHALL BE PORTLAND CEMENT, SAND, AND HYDRATED LIME TO CONFORM TO ASTM C270, TYPE S OR TYPE N. NO BAG MIXES ARE ALLOWED. MINIMUM AVERAGE COMPRESSIVE STRENGTH OF MORTAR SHALL BE:
 - TYPE S - 1,800 PSI - MASONRY FOUNDATION WALLS & PIERS
 - TYPE N - 750 PSI - BRICK WORK, BLOCK WORK, BRICK & BLOCK COMPOSITE.
5. CONCRETE MASONRY UNITS SHALL BE LAID IN RUNNING BOND, UNLESS OTHERWISE NOTED. HORIZONTAL AND VERTICAL JOINTS SHALL HAVE A UNIFORM WIDTH OF 3/8 INCHES. SEE ARCHITECTURAL DRAWINGS FOR OTHER BOND PATTERNS OF VENEER BLOCK.
6. MAINTAIN A MINIMUM OF 1/2" CLEARANCE BETWEEN REINFORCING BARS AND MASONRY. GROUT SHALL BE PLACED IN LIFTS NOT TO EXCEED 4'-0" IN HEIGHT.
7. TRUSS TYPE REINFORCEMENT (9 GAGE) SHALL BE INSTALLED IN BED JOINTS 16" APART VERTICALLY, U.O.N. IN REGULAR TWO CORE BLOCK. HORIZONTAL TRUSS REINFORCEMENT SHALL BE CONTINUOUS AND LAPPED AT LEAST SIX INCHES AT SPLICES. REINFORCEMENT SHALL BE SPLICED AT ALL CORNERS AND INTERSECTIONS.
8. GROUT ALL CELLS OF MASONRY UNITS FOR THE FIRST TWO COURSES ABOVE ALL FOUNDATION WALLS AND SLABS. PLACE GROUT IN ALL REINFORCED CELLS.
9. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. REINFORCING BARS MARKED "CONTINUOUS" SHALL BE LAPPED 36 BAR DIAMETERS. LAP SPLICES SHALL BE 48 BAR DIAMETER. EMBEDMENT LENGTHS SHALL BE 36 BAR DIAMETERS.
10. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCING WHERE HORIZONTAL REINFORCING MEETS AT A CORNER OR INTERSECTION.
11. PROVIDE LOOSE LINTELS OVER ALL OPENINGS IN EXTERIOR AND INTERIOR NON-BEARING WALLS AS LISTED BELOW, EXCEPT WHERE OTHERWISE INDICATED ON DRAWINGS:
 - 4'-11" OR LESS: L 5 X 3 1/2 X 5/16
 - 5'-0" TO 7'-0": L 6 X 3 1/2 X 5/16
 - 7'-1" TO 8'-0": WT 9 X 25
 - A. 3 1/2 LEGS ARE HORIZONTAL
 - B. PROVIDE ONE L FOR EACH 4" OF NOMINAL WALL THICKNESS.
 - C. MINIMUM BEARING TO BE 8" AT EACH END.
 - D. ANGLE LINTELS FOR MASONRY OPENINGS GREATER THAN 5'-0" SHALL BE BOLTED TOGETHER AT ENDS AND AT MID-SPAN UNLESS OTHERWISE NOTED.
12. SUBMIT REINFORCING SHOP DRAWINGS.

STRUCTURAL STEEL NOTES

1. STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS". STRUCTURAL STEEL SHALL CONFORM TO ASTM A572 GRADE 50. TUBE SHAPES SHALL CONFORM TO ASTM A500 GRADE B. FRAMING FOR BRICK PANELS, CHANNELS, AND OTHER MISCELLANEOUS METALS SHALL CONFORM TO ASTM A36.
 2. STEEL CONNECTIONS ARE SHOWN SCHEMATICALLY. FABRICATOR IS RESPONSIBLE FOR DESIGN AND DETAILING OF CONNECTIONS.
 3. EACH BEAM CONNECTION SHALL BE DESIGNED FOR ONE HALF OF THE TOTAL LOAD SHOWN IN THE AISC TABLES FOR THE RESPECTIVE SPAN UNLESS OTHERWISE NOTED. COMPOSITE BEAMS CONNECTION SHALL BE DESIGNED FOR THREE FOURTHS OF THE TOTAL LOAD. WHERE POSSIBLE, EACH BEAM CONNECTION SHALL BE OF THE TWO SIDED WEB ANGLE TYPE AS PER AISC SPECIFICATION. UNLESS OTHERWISE NOTED ON THE DRAWINGS, MINIMUM CONNECTION SHALL BE TWO (2) BOLTS. ALL BEAM AND GIRDER CONNECTIONS SHALL BE BOLTED CONNECTIONS USING ASTM A325X BOLTS, 3/4" DIAMETER OR WELDED CONNECTIONS.
 4. ALL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE DRAWINGS SHALL BE EITHER WELDED CONNECTIONS, OR BOLTED CONNECTIONS USING ASTM A325X BOLTS.
 5. UNLESS SPECIFICALLY DETAILED OTHERWISE, SPLICES SHALL BE DESIGNED TO DEVELOP THE FULL CAPACITY OF THE MEMBER AT THE POINT OF THE SPLICE.
 6. CUTS, HOLES, COPEs, ETC., REQUIRED FOR WORK OF OTHER TRADES SHALL BE SHOWN ON SHOP DRAWINGS AND MADE IN THE SHOP. FIELD CUTTING OR BURNING WILL NOT BE PERMITTED.
 7. ALL WELDING BOTH SHOP AND FIELD, SHALL BE PERFORMED BY CERTIFIED WELDERS IN ACCORDANCE WITH AWS SPECIFICATIONS. WELDING ELECTRODES SHALL CONFORM TO ASTM A233, E70-XX. MINIMUM WELD SIZE SHALL BE 1/4 INCHES (FILLET) UNLESS OTHERWISE NOTED. WELDED CONNECTIONS SHALL BE DESIGNED TO BE STRESSED TO LESS THAN 50% OF THEIR ALLOWABLE CAPACITIES.
 8. STRUCTURAL STEEL SHALL RECEIVE A SHOP COAT OF RUST INHIBITING PAINT EXCEPT AS FOLLOWS:
 - A. CONTACT MILLED BEARING SURFACES
 - B. WITHIN TWO INCHES OF FIELD WELDS.
 - C. TOP FLANGES OF COMPOSITE BEAMS.
 9. AFTER ERECTION, ALL DAMAGED AREAS IN THE SHOP COAT SHALL BE TOUCHED UP WITH THE SAME PAINT USED FOR THE SHOP COAT.
 10. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW.
- METAL ROOF DECK
1. METAL ROOF DECK SHALL BE 1 1/2 INCH X 22 GAUGE, WIDE RIB TYPE B, CLASS I, FACTORY MUTUAL APPROVED.
 2. DECK SHALL SPAN A MINIMUM OF THREE SPANS.
 3. DECK SHALL BE WELDED TO SUPPORTING FRAME WORK ACCORDING TO MANUFACTURER'S REQUIREMENTS. ANCHORING SHALL RESIST UPLIFT OF 30 PSF. BEARING SHALL BE THREE INCHES MINIMUM.
 4. PROVIDE SUPPORT FOR METAL ROOF DECK AT 6'-0" ON CENTER MAXIMUM.
 5. FINISH COATING FOR METAL ROOF DECK SHALL BE GALVANIZED IN ORDER TO BE COMPATIBLE WITH FIREPROOFING REQUIREMENTS.
 6. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW.
 7. SEE TYPICAL DETAIL FOR WELD PATTERN.

DESIGN CRITERIA

1. DESIGN LOADS AND INFORMATION:
 - A. FLOOR LIVE LOAD.....
 1. OFFICE 60 PSF
 2. STORAGE/FILES 120 PSF
 - B. ROOF LIVE LOAD..... N/A
 - C. SNOW LOAD.....
 1. GROUND SNOW LOAD [Pg] = 40 PSF
 2. FLAT ROOF SNOW LOAD [P_f] = 30 PSF
 3. SNOW EXPOSURE FACTOR [C_e] = 1.0
 4. SNOW IMPORTANCE FACTOR [I_s] (CATEGORY III): 1.1
 5. THERMAL FACTOR [C_t] = 1.0
 - D. WIND LOAD.....
 1. BASIC WIND SPEED (3-SECOND GUST) = 90 MPH
 2. WIND IMPORTANCE FACTOR [I_w] (CATEGORY III): = 1.00
 3. WIND EXPOSURE: B
 4. INTERNAL PRESSURE COEFFICIENT [GC_{pi}] = ±0.18
 5. COMPONENTS AND CLADDING = 30 PSF
 6. NET UPLIFT (ROOF) = 15 PSF
 - E. SEISMIC.....
 1. SITE CLASS C
 2. SPECTRAL RESPONSE COEFFICIENTS
S_{DS} = 0.253g
S₀₁ = 0.061g
 3. SEISMIC USE GROUP III [I_e] (IMPORTANCE FACTOR) = 1.25)
 4. SEISMIC DESIGN CATEGORY: B
 5. BASIC SEISMIC FORCE RESISTING SYSTEM: A-9
 6. DESIGN BASE SHEAR = 64KIPS
 7. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD
 - F. FOUNDATION
 1. BEARING PRESSURE 2500 PSF
 2. MODULUS OF SUBGRADE 200 PCI
 3. DEPTH 48" BELOW FINISHED GRADE
 4. GEOTECHNICAL EVALUATION IS PART OF THE CONTRACT DOCUMENTS. ALL RECOMMENDATIONS SHALL BE STRICTLY ADHERED TO.



COMPONENTS AND CLADDING PRESSURES					
ZONE	COMPONENT TRIBUTARY AREA				
	10 SQ. FT.	20 SQ. FT.	50 SQ. FT.	100 SQ. FT.	500 SQ. FT.
①	16.8 psf	16.3 psf	15.8 psf	15.3 psf	15.3 psf
②	28.1 psf	25.1 psf	21.2 psf	18.2 psf	18.2 psf
③	42.3 psf	35.1 psf	25.4 psf	18.2 psf	18.2 psf
④	18.2 psf	17.4 psf	16.4 psf	15.6 psf	13.9 psf
⑤	22.4 psf	21.0 psf	19.0 psf	17.4 psf	13.9 psf

COMPONENTS AND CLADDING DIAGRAM

1. PRESSURES ARE ASSUMED TO ACT IN EITHER DIRECTION.
2. I_w = 1.15

(*** THIS TABLE BASED ON 90 MPH, 30' HEIGHT, EXPOSURE B, I_w = 1.00, ROOF SLOPE 0 - 5 DEGREES! REVIEW WITH PROJECT CONDITIONS PRIOR TO USING. ***)



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