

Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners. Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.

GENERAL NOTES

- 1. ALL STRUCTURAL WORK SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND SHALL CONFORM TO THE PROJECT SPECIFICATIONS...
2. CONTRACTOR SHALL PROVIDE TEMPORARY SHORING, BRACING, AND SHEETING AND MAKE SAFE ALL FLOORS, ROOFS, WALLS, AND ADJACENT PROPERTY...

FOUNDATIONS

- 1. BUILDING FOUNDATIONS SHALL BEAR ON UNDISTURBED SOIL HAVING MINIMUM BEARING AND SHALL CAPACITY OF 2000 PSF AS SPECIFIED BY THE GEOTECHNICAL MASONRY...
2. DO NOT PLACE BACKFILL AGAINST BASEMENT WALLS UNTIL ALL FLOORS BRACING THESE ARE IN PLACE...

CONCRETE

- 1. ALL CONCRETE WORK SHALL CONFORM TO THE ACI BUILDING CODE REQUIREMENTS FOR CONCRETE (ACI 318)...
2. ALL CONCRETE COMPOSITE ON METAL DECK SHALL BE LIGHT WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS...
3. ALL OTHER CONCRETE SHALL BE NORMAL WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS...

STRUCTURAL STEEL

- 1. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING GOVERNING STANDARDS:
A. AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" AND AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
B. AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
C. AMERICAN WELDING SOCIETY (AWS D1.1) "STRUCTURAL WELDING CODE - STEEL"

Table with 4 columns: BEAM DEPTH (NOMINAL), MIN. SHEAR CAPACITY ASD (KIPS), MIN. SHEAR CAPACITY LRFD (KIPS), MIN. MOMENT CAPACITY LRFD (KIP-FT). Rows include 8", 10", 12", 14", 16", 18", 21", 24", 27", 30", 36", 40" beam depths.

- 4. MINIMUM WELD SIZE IS 1/4" FILLET UNLESS NOTED OTHERWISE.
5. ALL BEAMS EXCEPT CANTILEVER BEAMS SHALL BE FABRICATED AND INSTALLED WITH NATURAL CAMBER UP. CANTILEVER BEAMS SHALL BE FABRICATED AND INSTALLED SO THAT NATURAL CAMBER RAISES CANTILEVER END.
6. FIELD CUTTING OR BURNING OF STEEL IS PROHIBITED EXCEPT WITH THE EXPRESS WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER...

Table with 2 columns: MASONRY LINTELS, LINTEL. Rows include 4'-0" OR LESS and 4'-1" TO 7'-0" lintel sizes.

- A. 3-1/2" LEGS ARE HORIZONTAL.
B. PROVIDE ONE ANGLE FOR EACH 4' OF WALL THICKNESS.
C. PROVIDE 1/8x5/16 ANGLES FOR 8" THICK WALLS AND PARTITIONS WITH OPENINGS UP TO 6'-0".
D. PROVIDE MINIMUM 6" BEARING AT EACH END.
E. LINTELS OVER 6'-0" SHALL BE FIREPROOFED.

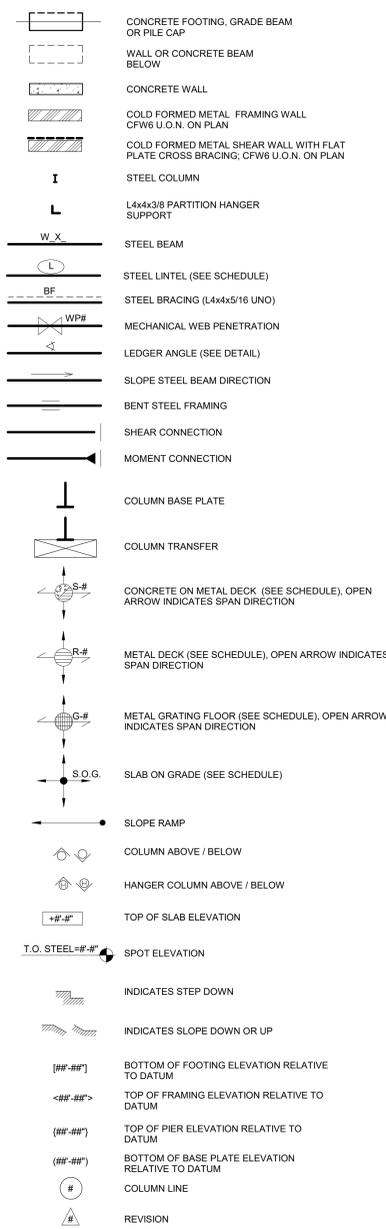
SPECIAL INSPECTIONS (BC)

- 1. INSPECTIONS REQUIRED BY THE LOCAL JURISDICTION SHALL BE PERFORMED BY A TESTING AGENCY PROVIDED BY THE OWNER FOR THE FOLLOWING ITEMS:
A. INSPECTION OF FABRICATORS (BC 1704.2.5)
B. STEEL CONSTRUCTION (BC 1705.2)
1. STRUCTURAL STEEL WELDING (AISC 360, AWS D1.1)
2. HIGH STRENGTH BOLTS (AISC 308)

STANDARD ABBREVIATIONS

Table with 2 columns: ABBREVIATION, MEANING. Includes terms like ADDL (ADDITIONAL), ADJ (ADJACENT), AVE (DESIGN TEAM OF RECORD), ALT (ALTERNATE), ANCH (ANCHOR), APPROX (APPROXIMATE/APPROXIMATELY), ARCH (ARCHITECT/ARCHITECTURAL), BLDG (BUILDING), B.F. (BRACE FRAME), BM (BEAM), B.O. (BOTTOM OF), BOT (BOTTOM), BRG (BEARING), BSMT (BASEMENT), CANT (CANTILEVER), CFS (COLD FORMED STEEL), C.I.P. (CAST IN PLACE CONCRETE), C.J. (CONTRACTION JOINT), CLG (CEILING), CLR (CLEAR), CMU (CONCRETE MASONRY UNIT), COL (COLUMN), COMP (COMPOSITE), CONC (CONCRETE), CONST (CONSTRUCTION), CONT (CONTINUOUS), COORD (COORDINATE/COORDINATION), CONTR (CONTRACTOR), COTR (CONTRACT OFFICER), CTR (CENTER), DBL (DOUBLE), DEMO (DEMOLITION/DEMOLISH), DIA (DIAMETER), DIAG (DIAGONAL), DIM (DIMENSION), D.L. (DEAD LOAD), DN (DOWN), DTL (DETAIL), DWG(S) (DRAWING(S)), DWL (DOWEL), EA (EACH), E.F. (EACH FACE), EXP (EXPANSION JOINT), ELEV (ELEVATION), ELEC. (ELECTRICAL), ELEV (ELEVATOR), EMBED. (EMBEDMENT), E.O. (EDGE OF), ENGR. (ENGINEER), E.O.R. (ENGINEER OF RECORD), EQ. (EQUAL), E.S. (EACH SIDE), E.W. (EACH WAY), EXP. (EXPANSION), EXT. (EXTERIOR), FDN. (FOUNDATION), FIN. (FINISH), FLR (FLOOR), FRM. (FRAMING), F.S. (FAR SIDE), FT. (FEET), FTG. (FITTING), GAGE (GAGE), GALV. (GALVANIZED), G.B. (GIBBET CLAM), HDR (HEADER), HGR. (HANGER), HORIZ. (HORIZONTAL), H.P. (HIGH POINT), HT. (HEIGHT), HVAC (HEATING, VENTILATION, & AIR CONDITIONING), I.D. (INSIDE DIAMETER), I.F. (INSIDE FACE), I.J. (ISOLATION JOINT), INFO (INFORMATION), INTR (INTERIOR), JOINT (JOINT), KIP (KIP), LB (POUND), L.L. (LIVE LOAD), L.L.B. (LONG LEGS BACK-TO-BACK), L.L.V. (LONG LEG VERTICAL), L.P. (LOW POINT), LGT. (LIGHT), L.W. (LONG WAY), MAS. (MASONRY), MAX. (MAXIMUM), MECH. (MECHANICAL), MECH. ELEC. PLUMB. & FIRE PROTECTION (MECHANICAL, ELECTRICAL, PLUMBING, & FIRE PROTECTION), MIN. (MINIMUM), MISC. (MISCELLANEOUS), M.O. (MEAN OPENING), N.F. (NEAR FACE), N.I.C. (NOT IN CONTRACT), NO. (NUMBER), N.S. (NEAR SIDE), N.T.S. (NOT TO SCALE), N.W. (NORMAL WEIGHT), O.C. (ON CENTER), O.D. (OUTSIDE DIAMETER), O.F. (OUTSIDE FACE), OPNG (OPENING), OPP. (OPPOSITE), PC. (PIECE), P.C. (PRECAST), PED. (PEDESTAL), PERP. (PERPENDICULAR), PL. (PLATE), PLF (POUNDS PER LINEAR FOOT), PREFAB. (PREFABRICATED), PSF (POUNDS PER SQUARE FOOT), PSI (POUNDS PER SQUARE INCH), P.T. (POST-TENSIONED), REIN. (REINFORCED/REINFORCEMENT REQUIRED), REV. (REVISION), SCHED. (SCHEDULE), SECT. (SECTION), S.I.F. (STEP IN FOOTING), S.L.B.B. (SHORT LEGS BACK-TO-BACK), SIM. (SIMILAR), S.O.G. (SLAB ON GRADE SPECIFICATION), S.O.G. (SLAB ON GRADE), SPEC. (SPECIFICATION), SQ. (SQUARE), S.S. (STAINLESS STEEL), STD. (STANDARD), STIFF. (STIFFENER), STL. (STEEL), S.W. (SHORT WAY), SYM. (SYMMETRIC), T & B (TOP & BOTTOM), TEMP. (TEMPERATURE), THK. (THICKNESS), T.O. (TOP), TR. (TRANSFER), TYP. (TYPICAL), U.N.O. (UNLESS NOTED OTHERWISE), VERT. (VERTICAL), W/ (WITH), W.P. (WORK POINT), W.W.R. (WELDED WIRE REINFORCEMENT), # (NUMBER/SIZE), Ø (DIAMETER)

LEGEND



CONCRETE BLOCK

- 1. ALL CONCRETE BLOCK WORK SHALL CONFORM TO THE NATIONAL CONCRETE MASONRY ASSOCIATION TEK MANUAL FOR THE DESIGN AND CONSTRUCTION OF CONCRETE MASONRY...
2. CONCRETE BLOCK SHALL BE OF LIGHT WEIGHT AGGREGATE AND CONFORM TO THE FOLLOWING STANDARDS: SOLID/HOLLOW BLOCK - ASTM C90.

Table with 2 columns: NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNIT (PSI), NET AREA COMPRESSIVE STRENGTH OF MASONRY ASSEMBLY, Fm (PSI) USING TYPE S MORTAR. Rows include 1900, 2800, 3750, 4800 PSI values.

- UNLESS OTHERWISE NOTED ON PLANS AND/OR ELEVATIONS, CONCRETE BLOCK UNIT STRENGTH SHALL BE 1900 PSI MIN. NOTE: CONCRETE BLOCK WITH UNIT STRENGTH HIGHER THAN 1900 PSI REQUIRE LONGER DELIVERY LEAD TIMES.
3. ALL MORTAR SHALL BE ASTM C270, TYPE S.
4. ALL GROUT FOR FILLING CELLS SHALL BE ASTM C 476 WITH MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI...
5. DIMENSIONS INDICATED ON STRUCTURAL PLANS ARE NOMINAL DIMENSIONS.

STEEL DECK

- 1. STEEL DECKING WORK SHALL CONFORM TO THE AISI NORTH AMERICAN "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
2. STEEL DECKING UNITS AND ACCESSORY ITEMS SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM A611 OR A653 WITH A MINIMUM YIELD STRENGTH OF 33,000 PSI.
3. STEEL DECKING SHALL BE SHORED BY PLANS OR BY SPAN AND LOAD CONDITIONS TO SUPPORT WET WEIGHT OF CONCRETE AND ALL CONSTRUCTION LOADS.
4. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY APPROVED METHOD TO THE DECK SPAN AND AT EDGE AND INTERIOR SUPPORTS PARALLEL TO THE DECK SPAN.

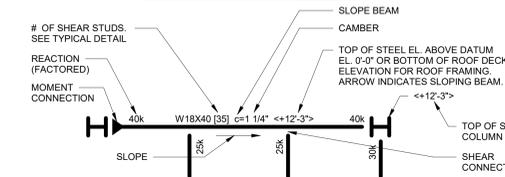
POST INSTALLED ADHESIVE AND MECHANICAL ANCHORS

- 1. POST INSTALLED ANCHORAGE SHALL BE INSTALLED PER MANUFACTURER TECHNICAL DATA TO INTACT BASE MATERIAL.
2. MANUFACTURER DATA FOR ALTERNATE ANCHORAGE PROPOSED BY CONTRACTOR SHALL BE SUBMITTED TO ENGINEER OF RECORD FOR REVIEW AND APPROVAL.
3. UNLESS OTHERWISE INDICATED, POST INSTALLED ANCHORAGE SHALL BE ADHESIVE TYPE HLT HI -HY 200 INTO CONCRETE, GROUT FILLED CMU OR STONE BASE MATERIAL OR HLT HI -HY 70 INTO BRICK MASONRY OR UNGROUTED CMU BASE MATERIAL.
4. EXISTING REINFORCING BARS IN THE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS.

DESIGN PARAMETER TABLE

Table with 2 columns: PARAMETER, VALUE. Includes governing code (2015 INTERNATIONAL BUILDING CODE), snow load (35 PSI), wind load (120 MPH), seismic design (1.0), and equivalent static force (95 KIP).

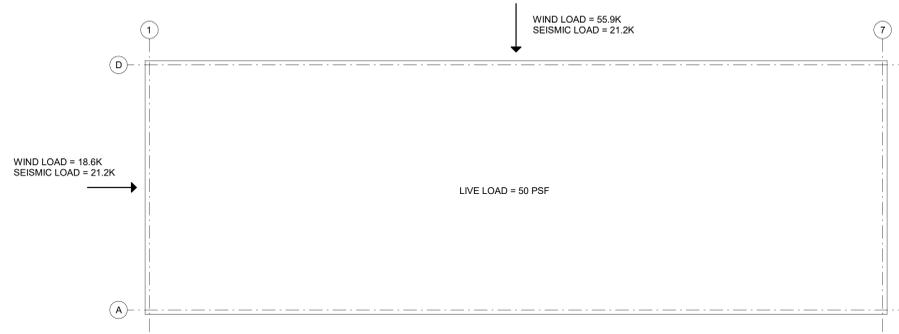
STRUCTURAL STEEL FRAMING KEY



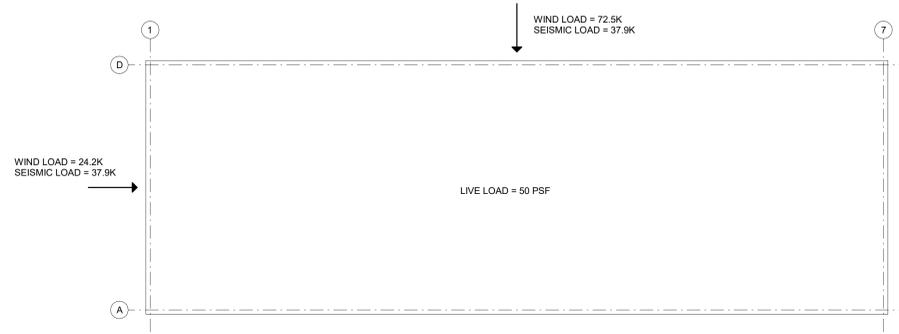
02 - Structural Steel Framing Key CONNECTION REQUIREMENTS. 3" = 1'-0"

- 01/20/19 15% DESIGN DEVELOPMENT
06/03/19 100% DESIGN DEVELOPMENT
12/13/19 90% CONSTRUCTION DOCUMENTS
04/10/20 ISSUED FOR PERMIT
06/10/20 100% CONSTRUCTION DOCUMENTS
07/10/20 GMP SET
12/10/20 FINAL GMP SET
06/10/21 BID SET

Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.

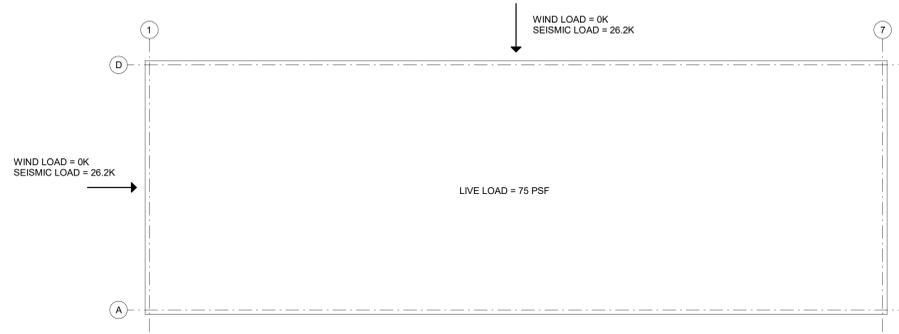


INN SECOND FLOOR LOADING

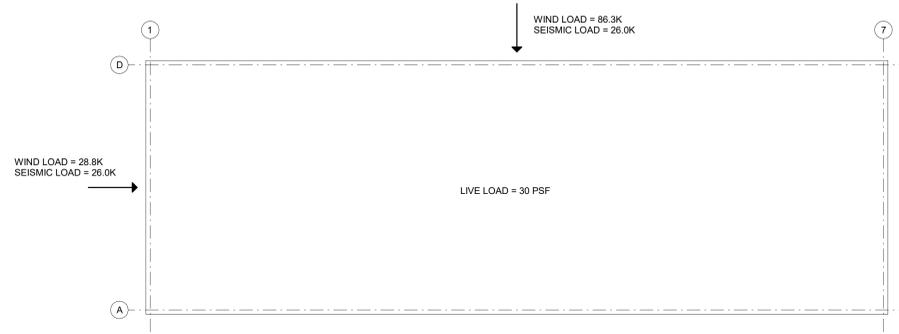


INN THIRD FLOOR LOADING

NOTES:
 1. WIND LOADING IS SHOWN FOR MWFRS
 2. REFER TO S-001 FOR ALL OTHER GENERAL NOTES AND DESIGN LOAD PARAMETERS



INN MECHANICAL MEZZANINE FLOOR LOADING



INN ROOF FLOOR LOADING

INN LOADING
 N.T.S.

VASSAR COLLEGE INN & INSTITUTE
 18738
 COLLEGE AVENUE POUGHKEEPSIE, NY

CONTRACTOR _____

CONSULTANT _____

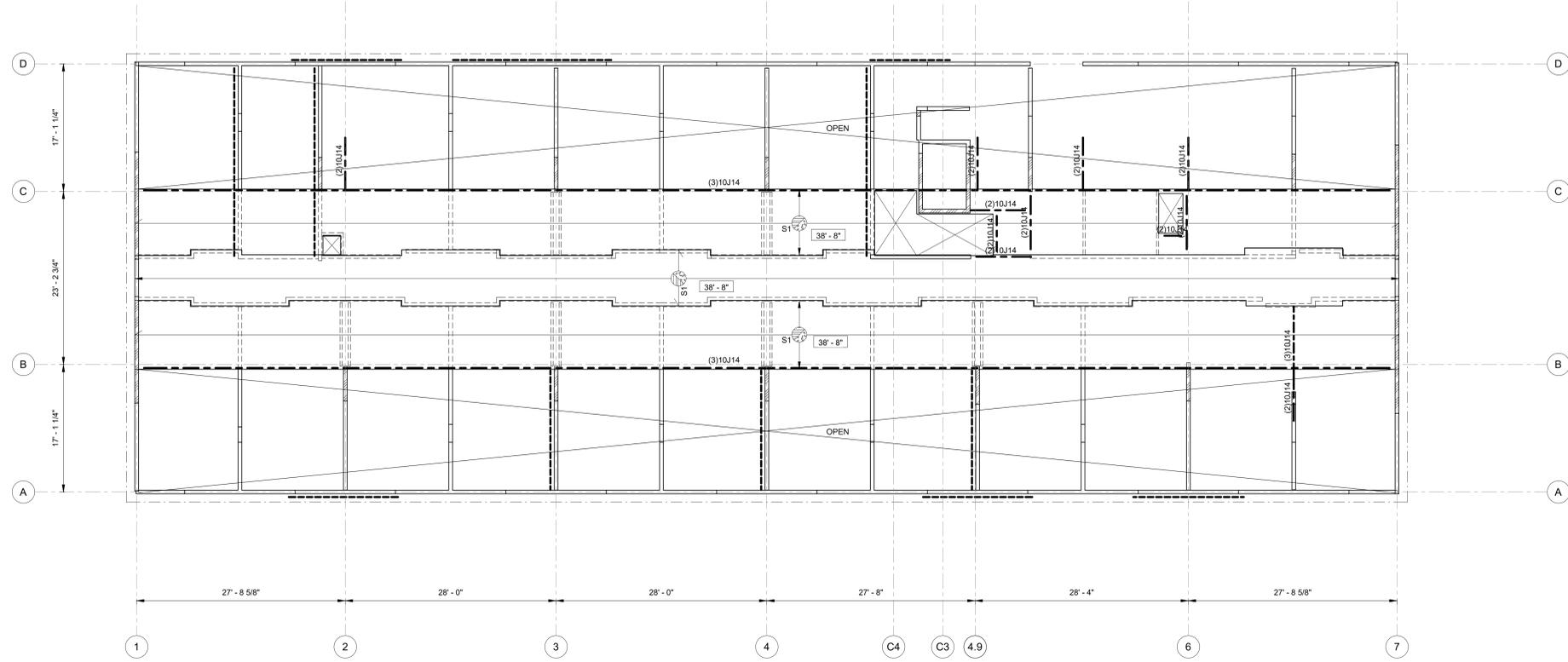
ISSUE DATES
 01/23/2019 75% DESIGN DEVELOPMENT
 09/03/2019 100% DESIGN DEVELOPMENT
 12/13/2019 90% CONSTRUCTION DOCUMENTS
 04/15/2020 ISSUED FOR PERMIT
 09/01/2020 ISSUED FOR CONSTRUCTION
 07/01/2020 GMP SET
 12/18/2020 FINAL GMP SET
 09/21/2021 BID SET

Frederick Fisher and Partners
 12248 Santa Monica Blvd, Los Angeles, CA 90025 | (310) 801-6680 | info@frederickfisher.com
 350 West 28th St, Suite 1802, New York, NY 10001

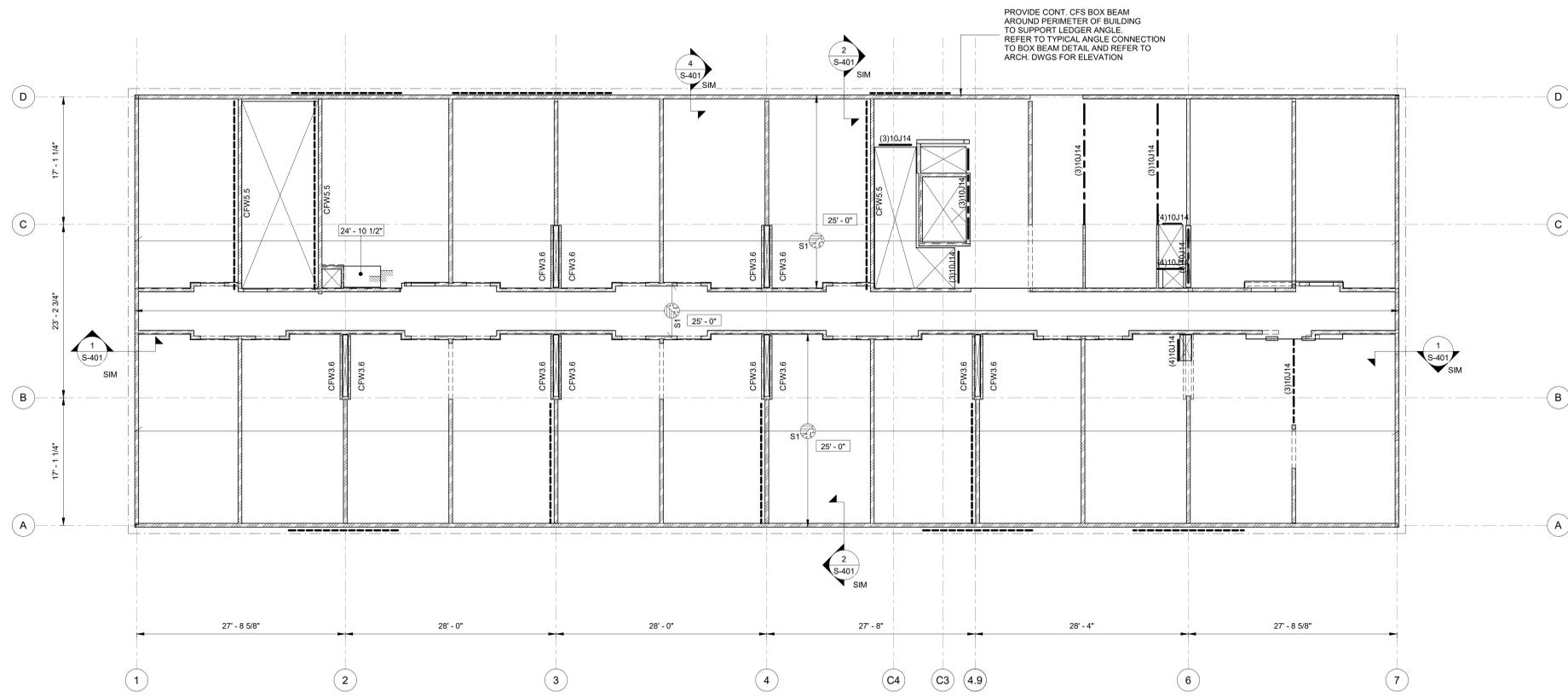
DESIGN
 LOADING
 SCALE: 1/16" = 1'-0"

S-002

Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.



2 MECHANICAL LEVEL FRAMING PLAN
S-102 1/8" = 1'-0"



1 LEVEL 3 FRAMING PLAN
S-102 1/8" = 1'-0"

NOTES

1. TOP OF SLAB ELEVATIONS NOTED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR FINISH FLOOR ELEVATION AND DATUM INFORMATION.
2. TOP OF STEEL ELEVATION IS $-0.6 \frac{1}{4}$ " FROM TOP OF SLAB NOTED THUS $-X'X \frac{1}{4}$ " ON PLAN.
3. REFER TO S-001 FOR GENERAL NOTES.
4. REFER TO S-500 SERIES DRAWINGS FOR TYPICAL DETAILS.
5. COORDINATE ALL SLAB OPENINGS, SLEEVES, SLOPES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
6. COORDINATE ALL DIMENSIONS, LOCATIONS AND WALL OPENING WITH ARCHITECT, CIVIL, MEP, AND ALL OTHER PRIME CONTRACTORS.
7. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF MASONRY SHELF ELEVATIONS AND CONFIGURATION.
8. LIVE LOAD AT GROUND AND 2ND FLOOR LEVEL IS A MINIMUM OF 100 PSF. ROOF LIVE LOAD IS A MINIMUM OF 20 PSF.

VASSAR COLLEGE INN & INSTITUTE
18738
COLLEGE AVENUE POUGHKEEPSIE, NY

CONTRACTOR

CONSULTANT

ISSUE DATES

01/23/2019	15% DESIGN DEVELOPMENT
04/23/2019	100% DESIGN DEVELOPMENT
12/13/2019	90% CONSTRUCTION DOCUMENTS
04/15/2020	ISSUED FOR PERMIT
06/01/2020	ISSUED FOR CONSTRUCTION
07/01/2020	GMP SET
12/18/2020	FINAL GMP SET
06/21/2021	BD SET

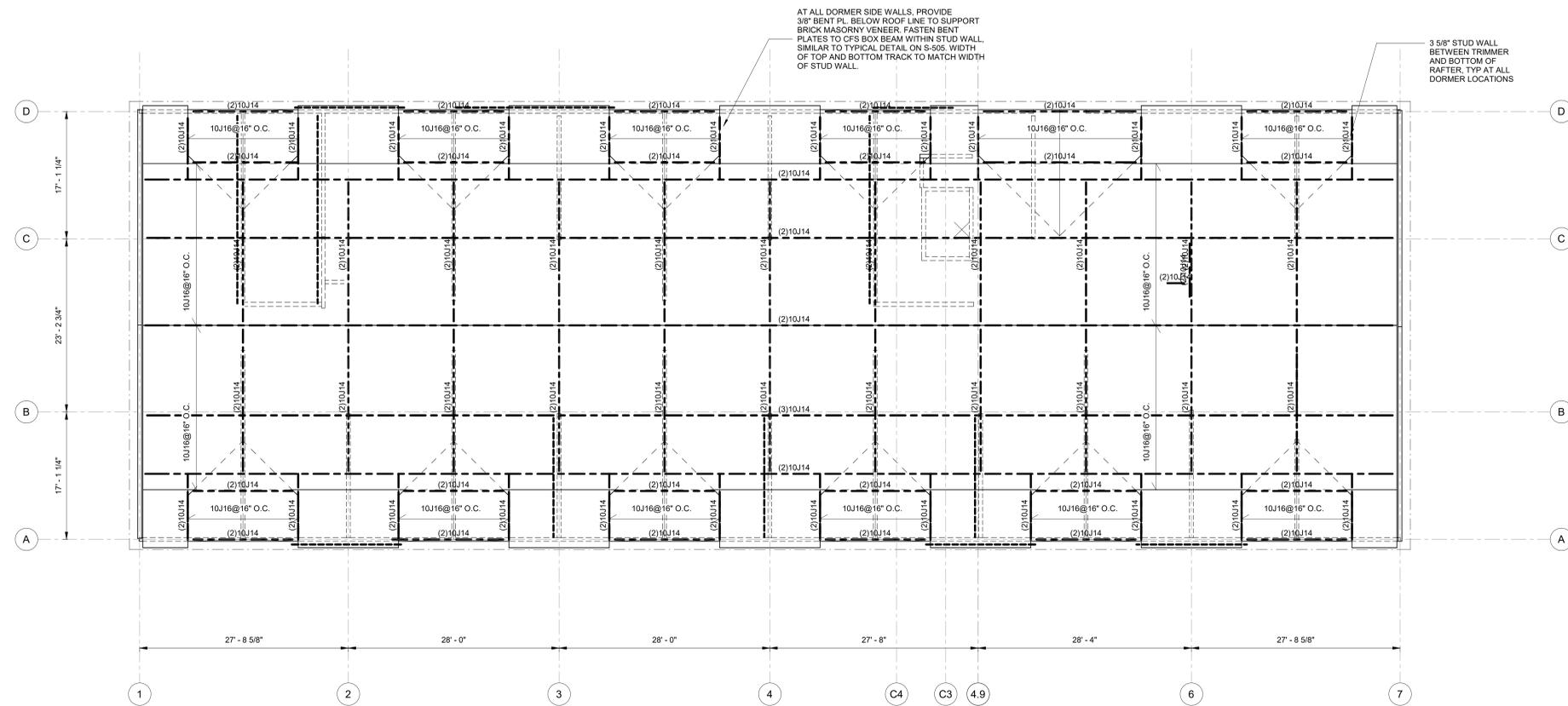
Frederick Fisher and Partners
12248 Santa Monica Blvd, Los Angeles, CA 90025 | (310) 501-6688 | ffp@frederickfisher.com
350 West 20th St, Suite 3502, New York, NY 10011

INN - THIRD FLOOR AND MECH FRAMING PLANS
SCALE: 1/8" = 1'-0"

S-102



Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.



1 ROOF FRAMING PLAN
S-103 1/8" = 1'-0"



NOTES

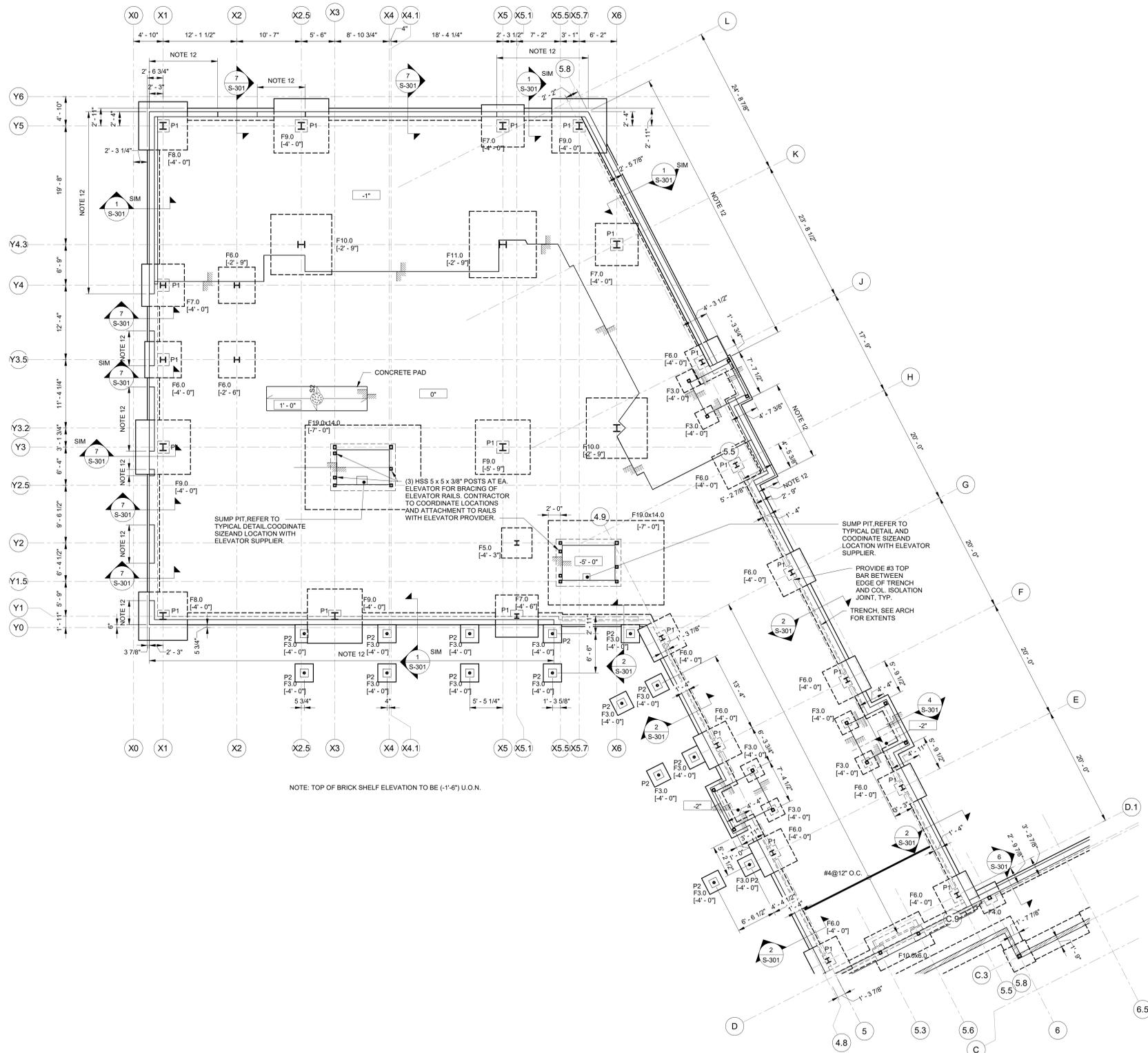
1. TOP OF SLAB ELEVATIONS NOTED ON PLAN, REFER TO ARCHITECTURAL DRAWINGS FOR FINISH FLOOR ELEVATION AND DATUM INFORMATION.
2. TOP OF STEEL ELEVATION IS $-0'-6 \frac{1}{4}>$ FROM TOP OF SLAB NOTED THUS $-0'-X'>$ ON PLAN.
3. REFER TO S-001 FOR GENERAL NOTES.
4. REFER TO S-500 SERIES DRAWINGS FOR TYPICAL DETAILS.
5. COORDINATE ALL SLAB OPENINGS, SLEEVES, SLOPES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
6. COORDINATE ALL DIMENSIONS, LOCATIONS AND WALL OPENINGS WITH ARCHITECT, CIVIL, MEP, AND ALL OTHER PRIME CONTRACTORS.
7. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF MASONRY SHELF ELEVATIONS AND CONFIGURATION.
8. LIVE LOAD AT GROUND AND 2ND FLOOR LEVEL IS A MINIMUM OF 100 PSF. ROOF LIVE LOAD IS A MINIMUM OF 20 PSF.

CONTRACTOR _____
CONSULTANT _____

ISSUE DATES

01/23/2019	75% DESIGN DEVELOPMENT
04/03/2019	100% DESIGN DEVELOPMENT
12/13/2019	90% CONSTRUCTION DOCUMENTS
04/15/2020	ISSUED FOR PERMIT
06/01/2020	ISSUED FOR CONSTRUCTION
07/01/2020	GMP SET
12/18/2020	FINAL GMP SET
06/21/2021	BD SET

Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.



NOTES

1. TOP OF SLAB ELEVATION IS NOTED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF FINISH FLOOR ELEVATION AND DATUM INFORMATION.
2. BOTTOM OF FOOTING ELEVATIONS IS [-4'-0"] BELOW GRADE AT EXTERIOR AND [-2'-0"] BELOW TOP OF SLAB AT INTERIOR UNLESS NOTED THUS [X'-Y"].
3. TOP OF BRICK SHELF ELEVATION IS [-1'-0"] BELOW TOP OF SLAB.
4. SEE S-001 FOR GENERAL NOTES.
5. SEE S-500 SERIES FOR TYPICAL DETAILS.
6. SEE S-601 FOR COLUMN SCHEDULES.
7. COORDINATE ALL DIMENSIONS WITH ARCHITECT, CIVIL, MEP AND OTHER PRIME CONTRACTORS.
8. COORDINATE ALL SLAB OPENINGS, SLOPES, SLEEVES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, MEP AND OTHER PRIME CONTRACTORS.
9. FXX INDICATES FOOTING DESIGNATION. SEE S-601 FOR SCHEDULE.
10. WALL FOOTINGS TO BE W1, U.O.N. ON PLAN.
11. LIVE LOAD AT GROUND AND 2ND FLOOR LEVEL IS A MINIMUM OF 100 PSF. ROOF LIVE LOAD IS A MINIMUM OF 20 PSF.
12. APPROXIMATE EXTENT OF BRICK SHELF. COORDINATE WITH ARCHITECTURE.

CONTRACTOR

CONSULTANT

ISSUE DATES
01/23/2019 15% DESIGN DEVELOPMENT
06/03/2019 100% DESIGN DEVELOPMENT
12/13/2019 90% CONSTRUCTION DOCUMENTS
04/15/2020 ISSUED FOR PERMIT
06/01/2020 ISSUED FOR CONSTRUCTION
07/01/2020 GMP SET
12/18/2020 FINAL GMP SET
06/21/2021 BID SET

Frederick Fisher and Partners
12248 Santa Monica Blvd, Los Angeles, CA 90025 | (310) 580-6688 | info@frederickfisher.com
350 West 28th St, Suite 1850, New York, NY 10001

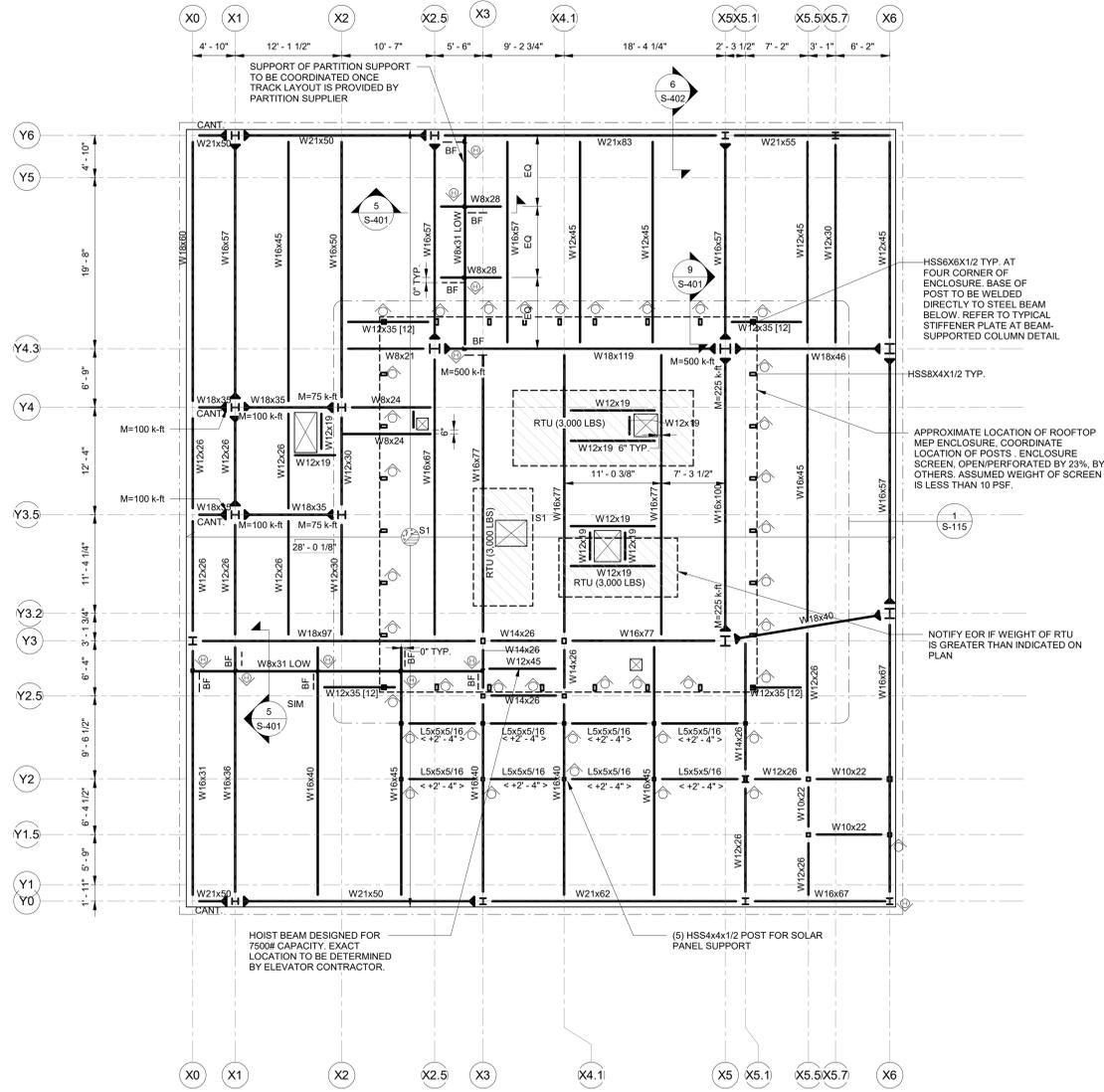
INSTITUTE - FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

S-111

VASSAR COLLEGE INN & INSTITUTE
18738
COLLEGE AVENUE POUGHKEEPSIE, NY



Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.



NOTES

1. TOP OF SLAB ELEVATIONS NOTED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR FINISH FLOOR ELEVATION AND DATUM INFORMATION.
2. TOP OF STEEL ELEVATION IS $-0'-6 \frac{1}{4}"$ FROM TOP OF SLAB NOTED THUS $-X'-X"$ ON PLAN.
3. REFER TO S-001 FOR GENERAL NOTES.
4. REFER TO S-500 SERIES DRAWINGS FOR TYPICAL DETAILS.
5. COORDINATE ALL SLAB OPENINGS, SLEEVES, SLOPES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
6. COORDINATE ALL DIMENSIONS, LOCATIONS AND WALL OPENING WITH ARCHITECT, CIVIL, MEP, AND ALL OTHER PRIME CONTRACTORS.
7. REFER TO ARCHITECTURAL DRAWINGS FOR TOP OF MASONRY SHELF ELEVATIONS AND CONFIGURATION.
8. LIVE LOAD AT GROUND AND 2ND FLOOR LEVEL IS A MINIMUM OF 100 PSF. ROOF LIVE LOAD IS A MINIMUM OF 20 PSF.

CONTRACTOR

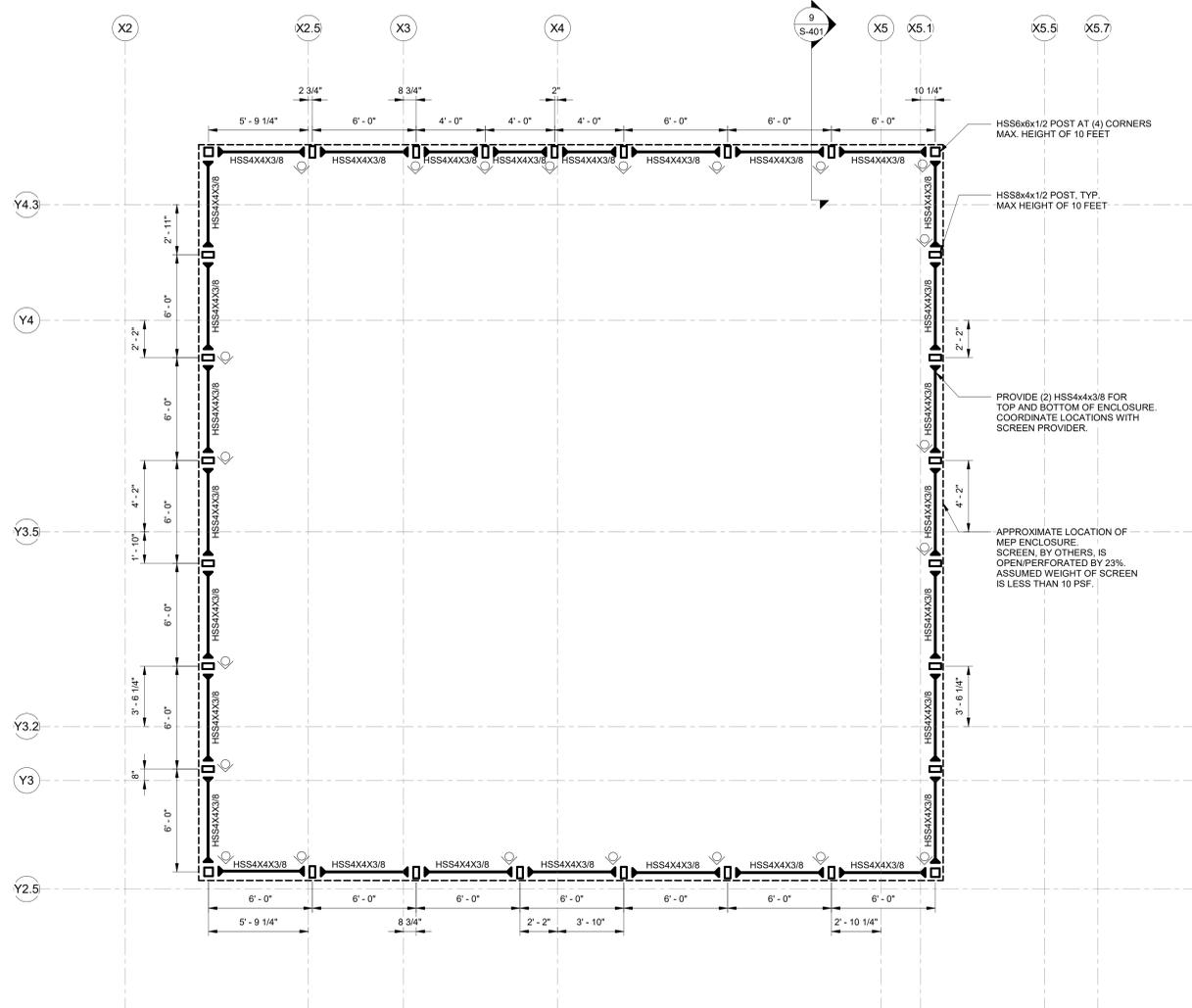
CONSULTANT

ISSUE DATES

05/23/2019	75% DESIGN DEVELOPMENT
08/03/2019	100% DESIGN DEVELOPMENT
12/13/2019	90% CONSTRUCTION DOCUMENTS
04/15/2020	ISSUED FOR PERMIT
05/01/2020	ISSUED FOR CONSTRUCTION
07/01/2020	GMP SET
12/18/2020	FINAL GMP SET
09/21/2021	BD SET



Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.



1 INSTITUTE - MECHANICAL ENCLOSURE SCREEN FRAMING PLAN
S-115 1/4" = 1'-0"

NOTES

1. REFER TO ARCH FOR TOP OF STEEL ELEVATIONS.
2. REFER TO S-001 FOR GENERAL NOTES.
3. REFER TO S-000 SERIES DRAWINGS FOR TYPICAL DETAILS.
4. COORDINATE ALL SLAB OPENINGS, SLEEVES, SLOPES, DEPRESSIONS, EDGE DIMENSIONS AND CURBS WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.
5. COORDINATE ALL DIMENSIONS, LOCATIONS AND WALL OPENING WITH ARCHITECT, CIVIL, M.E.P. AND ALL OTHER PRIME CONTRACTORS.

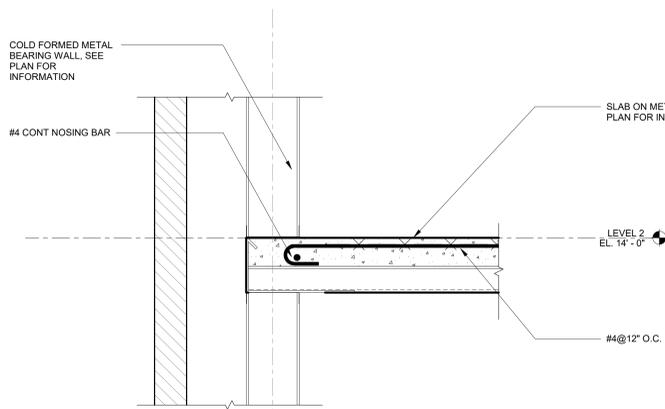
CONTRACTOR

CONSULTANT

ISSUE DATES

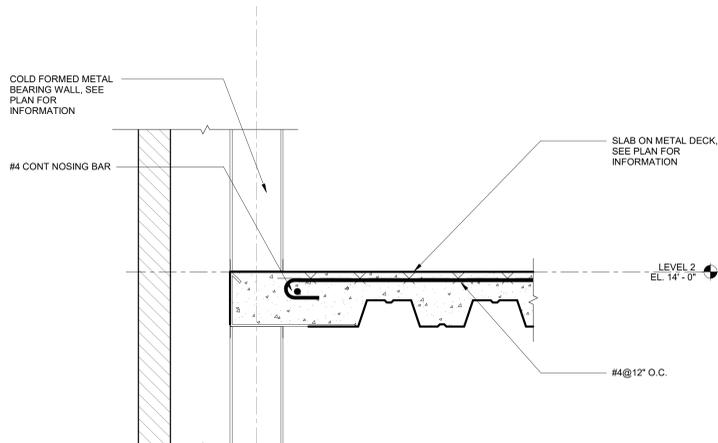
01/23/2019	75% DESIGN DEVELOPMENT
04/03/2019	100% DESIGN DEVELOPMENT
12/13/2019	90% CONSTRUCTION DOCUMENTS
04/15/2020	ISSUED FOR PERMIT
06/01/2020	ISSUED FOR CONSTRUCTION
07/01/2020	GMP SET
12/18/2020	FINAL GMP SET
06/21/2021	BID SET





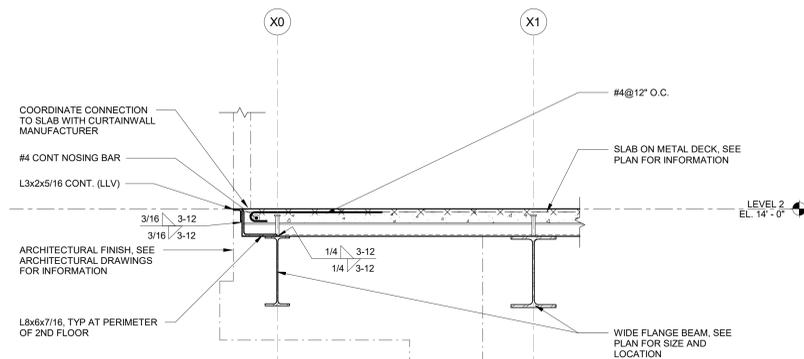
1 SECTION
S-401 1 1/2" = 1'-0"

NOTE: AT SIM CONDITION ON PLAN, PROVIDE 1'-2" HIGH CONCRETE CURB WITH (2) #6 BARS CONTINUOUS AND #6 DOWELS INTO CONCRETE SLAB ON METAL DECK. ATTACHED 3/8" THICK BENT PLATE (10"x10"), WITH 3/8" STIFFENERS AT 4'-0" O.C., TO CURB WITH CONCRETE WEDGE INSERT



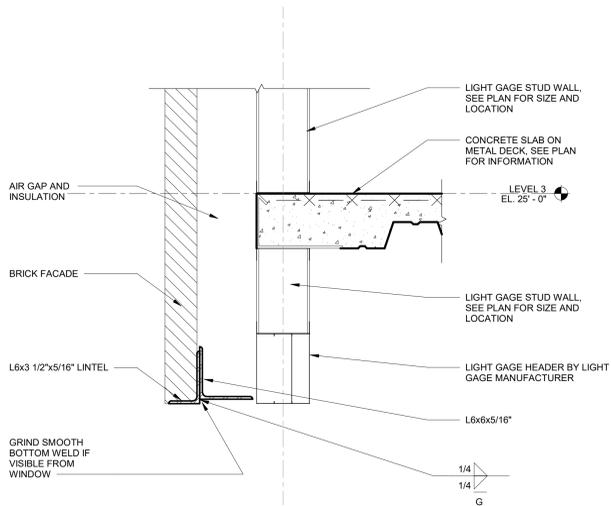
2 SECTION
S-401 1 1/2" = 1'-0"

NOTE: AT SIM CONDITION ON PLAN, PROVIDE 1'-2" HIGH CONCRETE CURB WITH (2) #6 BARS CONTINUOUS AND #6 DOWELS INTO CONCRETE SLAB ON METAL DECK. ATTACHED 3/8" THICK BENT PLATE (10"x10"), WITH 3/8" STIFFENERS AT 4'-0" O.C., TO CURB WITH CONCRETE WEDGE INSERT



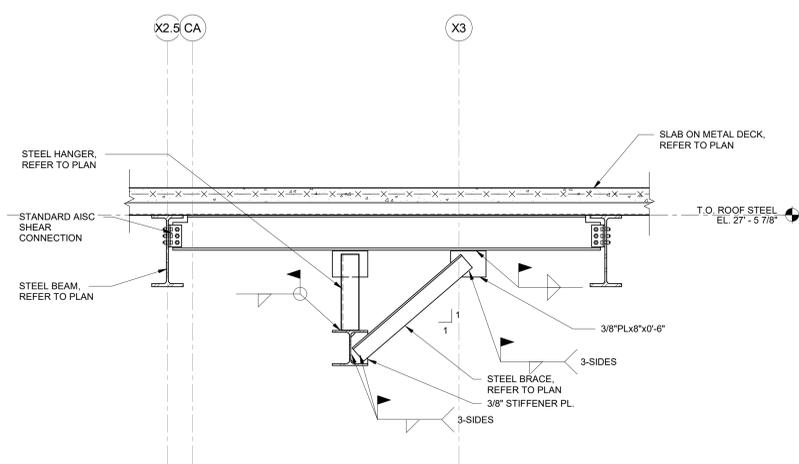
3 SECTION
S-401 3/4" = 1'-0"

COORDINATE CONNECTION TO SLAB WITH CURTAINWALL MANUFACTURER
#4 CONT NOSING BAR
L3x2x5/16 CONT. (LLV)
ARCHITECTURAL FINISH, SEE ARCHITECTURAL DRAWINGS FOR INFORMATION
L6x6x7/16, TYP AT PERIMETER OF 2ND FLOOR
WIDE FLANGE BEAM, SEE PLAN FOR SIZE AND LOCATION

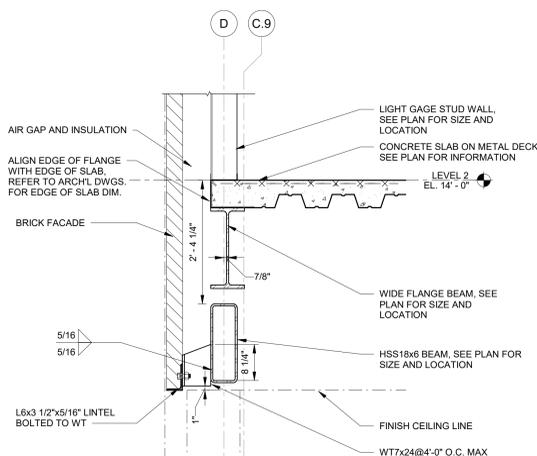


4 SECTION
S-401 1 1/2" = 1'-0"

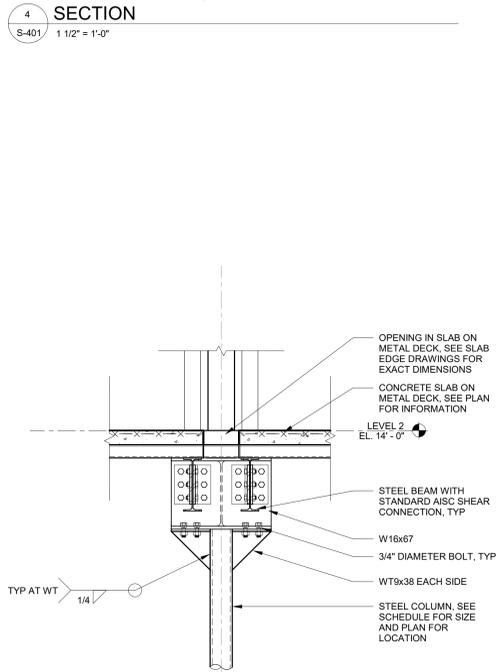
NOTE: AT SIM CONDITION ON PLAN, PROVIDE 1'-2" HIGH CONCRETE CURB WITH (2) #6 BARS CONTINUOUS AND #6 DOWELS INTO CONCRETE SLAB ON METAL DECK. ATTACHED 3/8" THICK BENT PLATE (10"x10"), WITH 3/8" STIFFENERS AT 4'-0" O.C., TO CURB WITH CONCRETE WEDGE INSERT



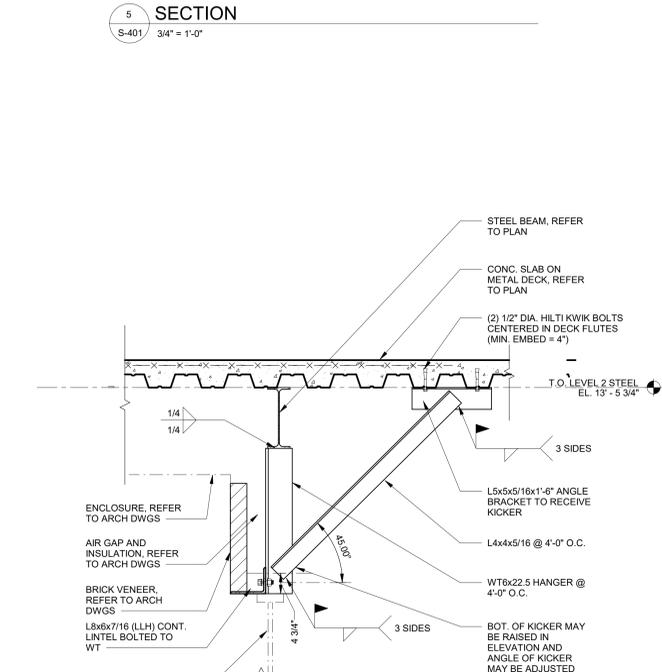
5 SECTION
S-401 3/4" = 1'-0"



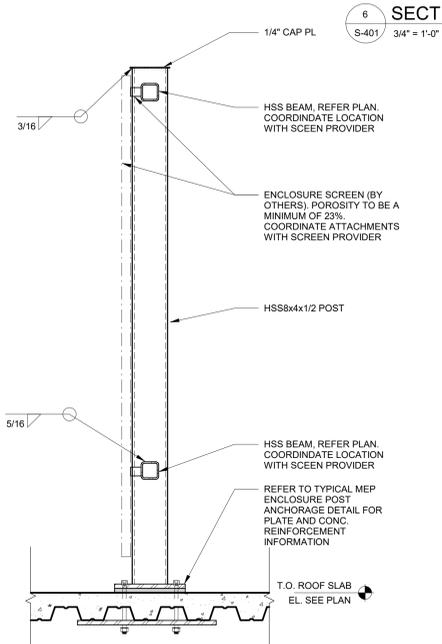
6 SECTION
S-401 3/4" = 1'-0"



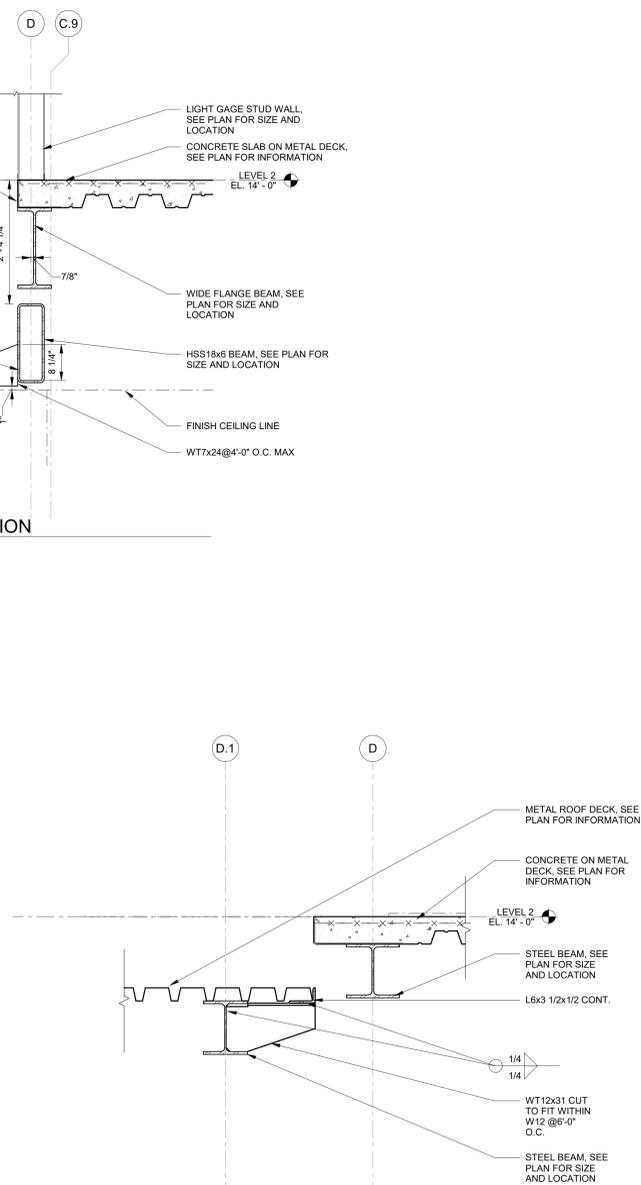
7 SECTION
S-401 3/4" = 1'-0"



8 SECTION
S-401 3/4" = 1'-0"

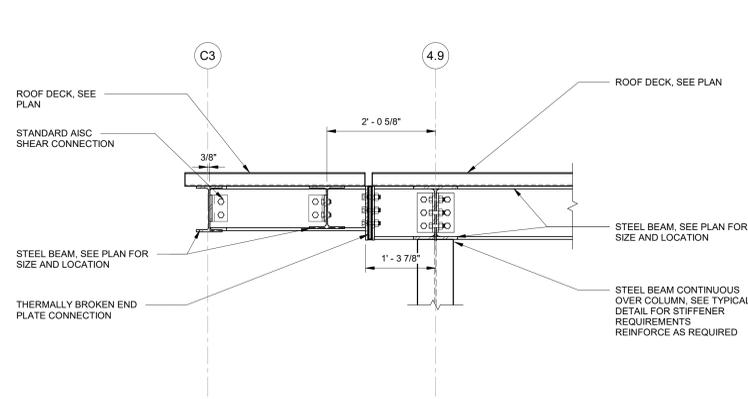


9 SECTION
S-401 3/4" = 1'-0"

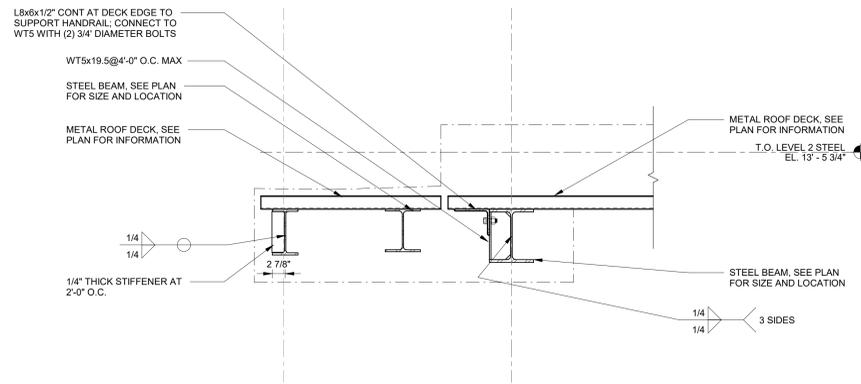


10 SECTION
S-401 3/4" = 1'-0"

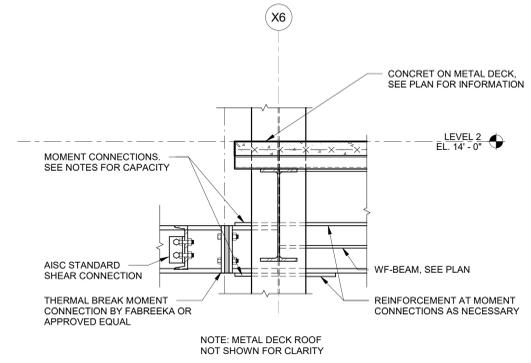
Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.



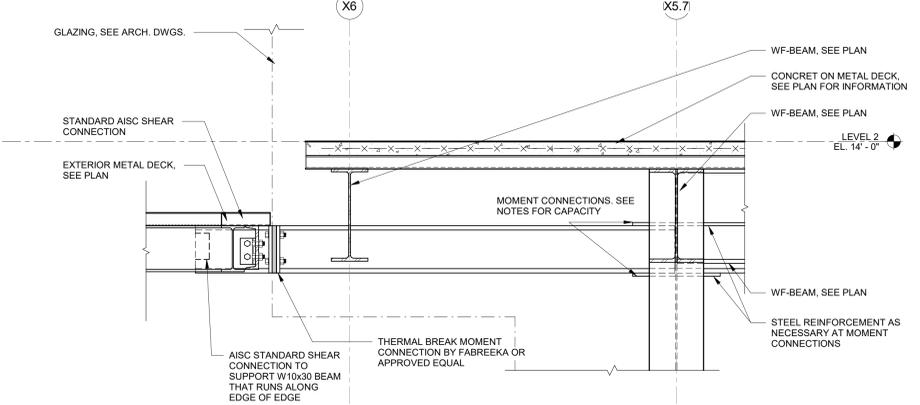
1 SECTION
S-402 3/4" = 1'-0"



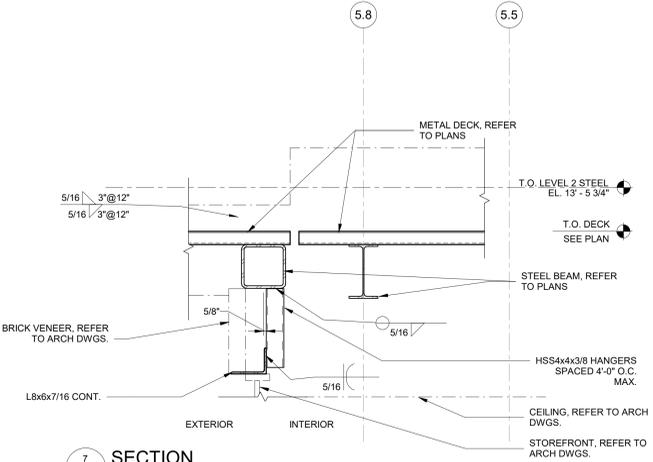
2 SECTION
S-402 3/4" = 1'-0"



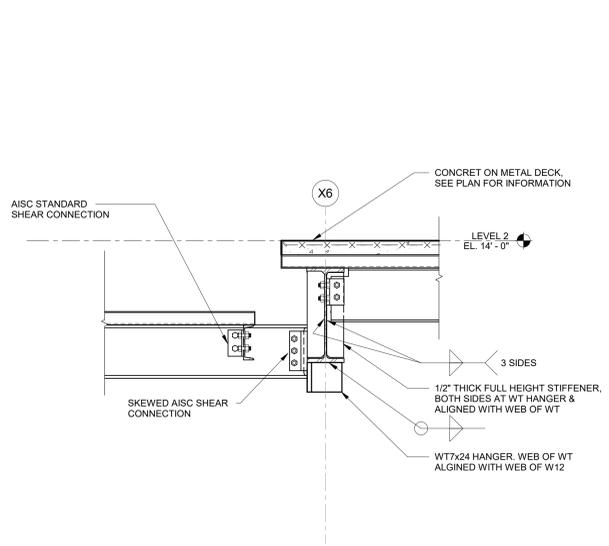
3 SECTION
S-402 3/4" = 1'-0"



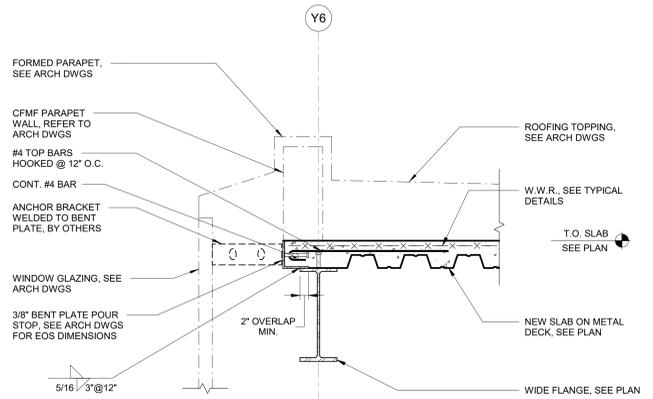
4 SECTION
S-402 3/4" = 1'-0"



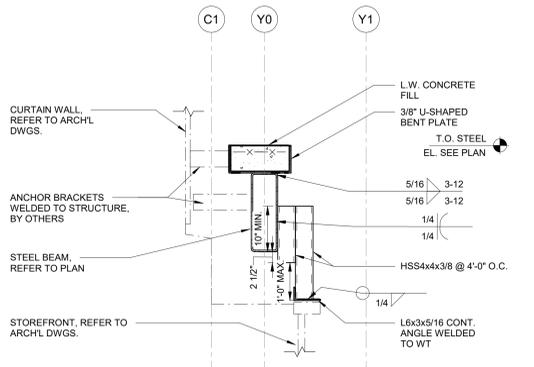
7 SECTION
S-402 3/4" = 1'-0"



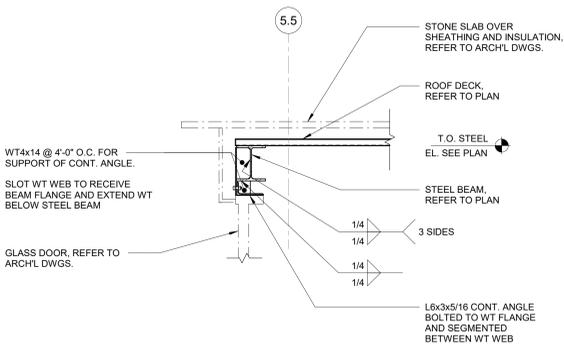
5 SECTION
S-402 3/4" = 1'-0"



6 SECTION
S-402 3/4" = 1'-0"



8 SECTION
S-402 3/4" = 1'-0"



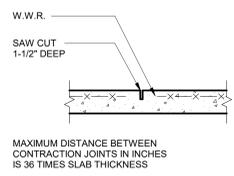
9 SECTION
S-402 3/4" = 1'-0"

CONTRACTOR

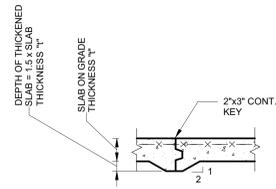
CONSULTANT

ISSUE DATES
02/20/19 75% DESIGN DEVELOPMENT
08/20/19 100% DESIGN DEVELOPMENT
12/10/19 90% CONSTRUCTION DOCUMENTS
04/15/2020 ISSUED FOR PERMIT
06/15/2020 ISSUED FOR CONSTRUCTION
07/03/2020 GMP SET
12/18/2020 FINAL GMP SET
06/21/2021 BID SET

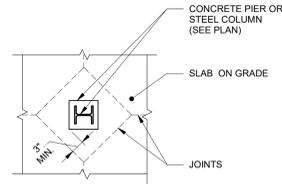
Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.



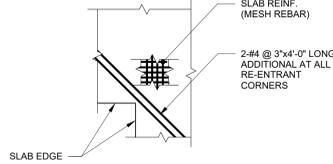
SAWED CONTRACTION JOINT



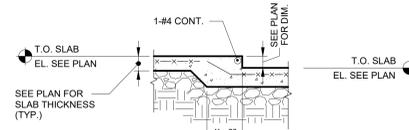
CONSTRUCTION JOINT



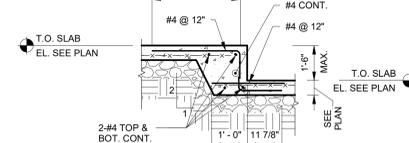
ISOLATION JOINT AT COLUMN



ADDITIONAL REINFORCEMENT AT ALL RE-ENTRANT CORNERS



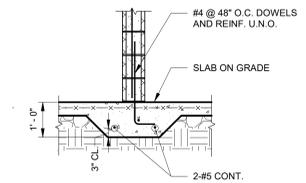
STEP LESS THAN OR EQUAL TO SLAB THICKNESS



STEP GREATER THAN SLAB THICKNESS

TYPICAL STEP IN SLAB ON GRADE

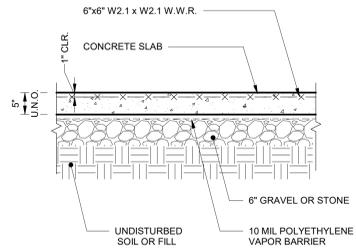
N.T.S.



NOTE: SEE TYPICAL SLAB ON GRADE DETAIL FOR ADDITIONAL INFORMATION

TYPICAL SUPPORT FOR MASONRY PARTITIONS AT SLAB ON GRADE

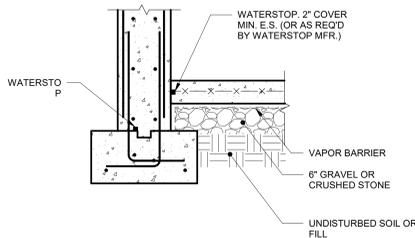
N.T.S.



SLAB ON GRADE

TYPICAL SLAB ON GRADE

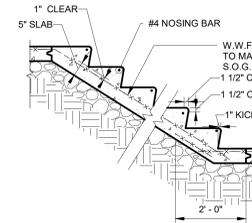
N.T.S.



SLAB-ON-GRADE & WALL INTERFACE

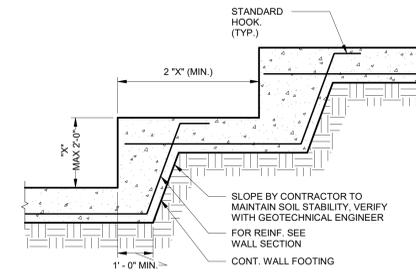
NOTES:

1. SLAB ON GRADE SHALL BE PLACED IN ALTERNATING STRIPS WHERE EACH SINGLE STRIP DOES NOT EXCEED 36 TIMES SLAB THICKNESS WIDTH IN INCHES. ALTERNATIVELY, LARGE BLOCK PLACEMENTS WITH INTERIOR CONTRACTION JOINTS ARE ACCEPTABLE IF THE CONTRACTION JOINTS ARE MADE IN BOTH DIRECTIONS AT SPECIFIED INTERVALS IN A TIMELY MANNER.
2. SEE ARCH. DWGS FOR SAWED CONTRACTION JOINT LOCATIONS. JOINTS TO BE LOCATED AT A MAXIMUM SPACING OF 10'-0". JOINTS SHALL BE SAWED NO LATER THAN 24 HOURS AFTER CONCRETE IS PLACED.
3. GRAVEL OR CRUSHED STONE BASE SHALL BE COMPACTED TO 95% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT.



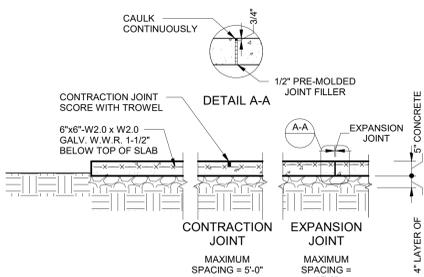
TYPICAL STAIR ON GRADE

N.T.S.



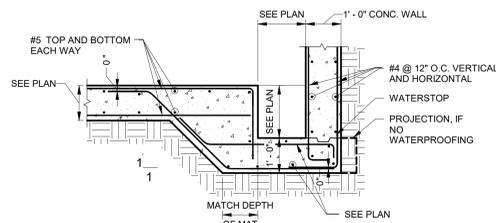
TYPICAL STEPPED WALL FOOTING

N.T.S.



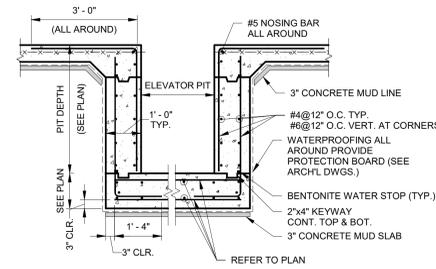
TYPICAL EXTERIOR PAVING

N.T.S.



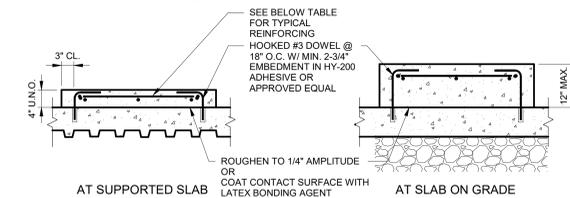
TYPICAL ELEVATOR SUMP PIT

N.T.S.



TYPICAL ELEVATOR PIT (EXTERIOR WATERPROOFING)

N.T.S.



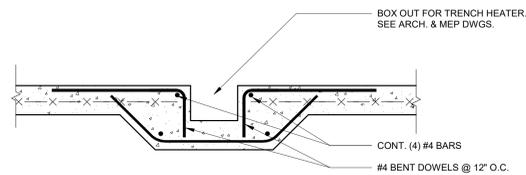
EQUIPMENT PAD TEMPERATURE AND SHRINKAGE REINFORCING	
PAD THICKNESS	REINFORCING
4'-5"	#3 @ 12" O.C. EACH WAY
6'-9"	#4 @ 12" O.C. EACH WAY
10'-12"	#5 @ 12" O.C. EACH WAY

NOTES:

1. FOR SIZE AND LOCATION SEE ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. CONCRETE FOR PADS SHALL BE NORMAL WEIGHT WITH $f_c = 4,000$ PSI.

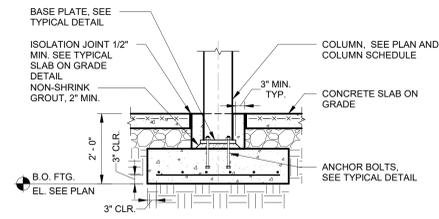
TYPICAL EQUIPMENT PAD

N.T.S.



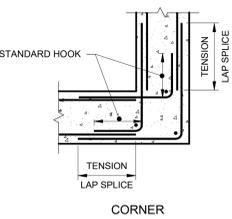
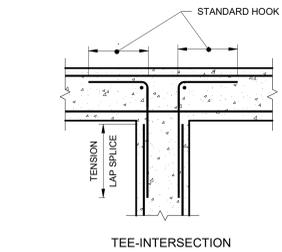
TYPICAL TRENCH DRAIN DETAIL

N.T.S.



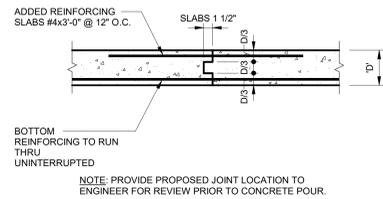
- NOTES:**
1. FOOTINGS SHALL BEAR ON FIRM UNDISTURBED SOIL WITH A MINIMUM BEARING CAPACITY AS SPECIFIED IN THE GENERAL NOTES.
 2. FOR SIZE, DEPTH AND REINFORCING SEE FOOTING SCHEDULE.

TYPICAL STEEL COLUMN ON CONCRETE FOOTING
N.T.S.

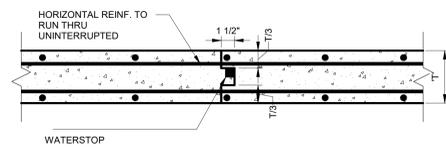


NOTE: FOR TENSION LAP SPLICE LENGTH AND DEVELOPMENT LENGTH SEE TABLE.

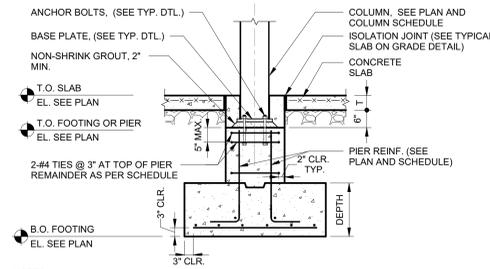
TYPICAL HORIZONTAL REINFORCEMENT AT CORNERS & JUNCTIONS OF WALLS AND BEAMS
N.T.S.



TYPICAL CONSTRUCTION JOINT IN FRAMED SLAB OR BEAM
N.T.S.

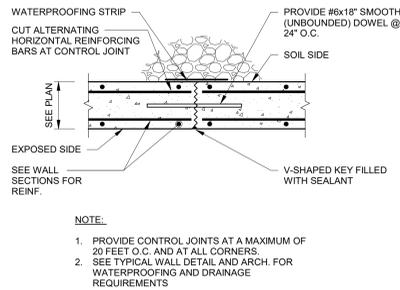


TYPICAL CONSTRUCTION JOINT IN CONCRETE WALL
N.T.S.



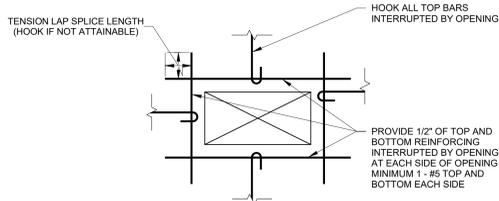
- NOTES:**
1. PIER SIZE TO BE 3\"/>

TYPICAL STEEL COLUMN/PIER AND FOOTING
N.T.S.

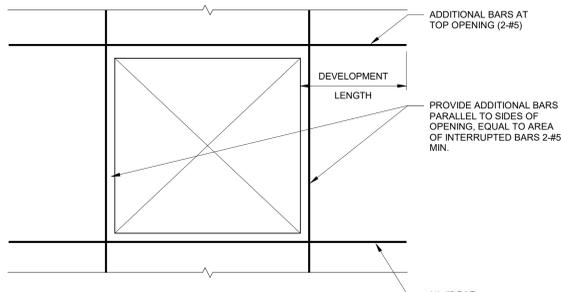


- NOTE:**
1. PROVIDE CONTROL JOINTS AT A MAXIMUM OF 20 FEET O.C. AND AT ALL CORNERS.
 2. SEE TYPICAL WALL DETAIL AND ARCH. FOR WATERPROOFING AND DRAINAGE REQUIREMENTS.

TYPICAL CONTROL JOINT IN CONCRETE WALL
N.T.S.

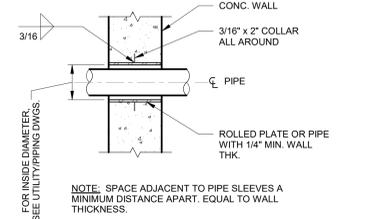


TYPICAL ADDITIONAL REINFORCEMENT AT OPENING IN FRAMED SLAB
N.T.S.



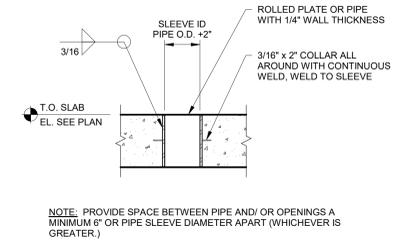
- NOTES:**
1. HOOK ALL BARS INTERRUPTED BY OPENING.
 2. HORIZONTAL BARS TO EXTEND DEVELOPMENT LENGTH BEYOND OPENING, VERTICAL BARS TO BE FULL STORY HEIGHT.

TYPICAL OPENING IN CONCRETE WALL
N.T.S.



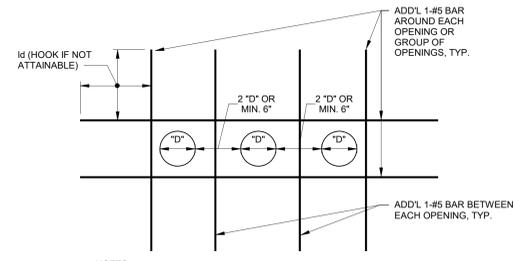
NOTE: SPACE ADJACENT TO PIPE SLEEVES A MINIMUM DISTANCE APART, EQUAL TO WALL THICKNESS.

TYPICAL PIPE SLEEVE IN CONCRETE WALL
N.T.S.



NOTE: PROVIDE SPACE BETWEEN PIPE AND/OR OPENINGS A MINIMUM 6\"/>

TYPICAL PIPE SLEEVE IN CONCRETE SLAB
N.T.S.



- NOTES:**
1. LIMIT 3 PENETRATIONS IN A ROW IN ANY DIRECTION.
 2. COORDINATE WITH TYPICAL DETAIL FOR FORMED CONCRETE SLAB PIPE SLEEVE.
 3. SHOULD PENETRATIONS BE CUT AFTER CONCRETE IS POURED. CONTRACTOR TO SUBMIT PLAN SHOWING ALL PROPOSED CORE DRILLING LOCATIONS TO E.O.R. FOR APPROVAL. CONTRACTOR TO USE NDE METHODS TO LOCATE REBAR PRIOR TO CUTTING SLAB.

TYPICAL SLAB PENETRATION WITH SLEEVE DIAMETER LESS THAN 6\"/>

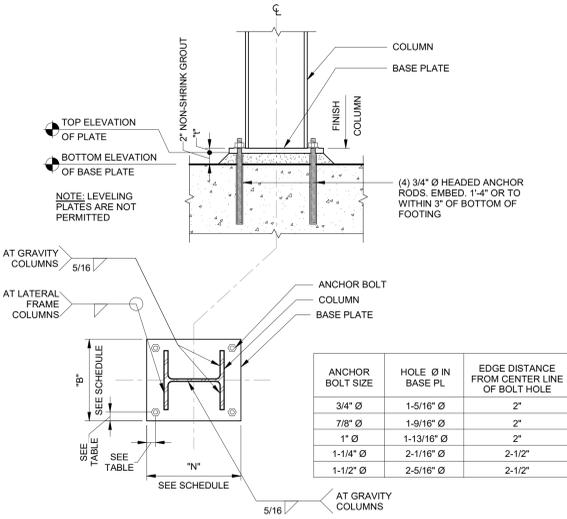
BAR SIZE	DEFORMED BAR TENSION DEVELOPMENT LENGTH (Ld)							
	FOR NORMAL WEIGHT STONE CONCRETE & UNCOATED BARS							
	3000 PSI CONCRETE		4000 PSI CONCRETE		5000 PSI CONCRETE		6000 PSI CONCRETE	
	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	17	25	15	22	13	20	12	18
#4	22	33	19	29	17	26	16	24
#5	28	42	24	36	22	32	20	30
#6	33	50	29	43	26	39	24	35
#7	48	72	42	63	38	56	34	51
#8	55	83	48	72	43	64	39	59
#9	62	93	54	81	48	72	44	66
#10	70	105	61	91	54	81	50	74
#11	78	116	67	101	60	90	55	82

- DEFORMED TENSION BAR NOTES:**
1. FOR HORIZONTAL REINFORCEMENT WITH 12 INCH OR MORE FRESH CONCRETE CAST BELOW IT, TENSION DEVELOPMENT LENGTH/ TENSION LAP SPLICE LENGTH SHALL BE 1.3X THE VALUES GIVEN.
 2. FOR EPOXY-COATED BARS:
 - A. WHERE CONCRETE COVER IS LESS THAN 3X BAR DIAMETER, OR CLEAR SPACING IS LESS THAN 6X BAR DIAMETER, TENSION DEVELOPMENT LENGTH/ TENSION LAP SPLICE LENGTH SHALL BE 1.5X THE VALUES GIVEN.
 - B. WHERE CONCRETE COVER IS EQUAL TO OR GREATER THAN 3X BAR DIAMETER AND CLEAR SPACING IS GREATER THAN 6X BAR DIAMETER, TENSION DEVELOPMENT LENGTH/ TENSION LAP SPLICE LENGTH SHALL BE 1.2X THE VALUES GIVEN.
- CASE 1: CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN DB, CLEAR COVER NOT LESS THAN DB, AND STIRRUPS OR TIES THROUGHOUT LD NOT LESS THAN THE CODE MINIMUM OR CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN 2DB AND CLEAR COVER NOT LESS THAN DB.
CASE 2: OTHER CASES

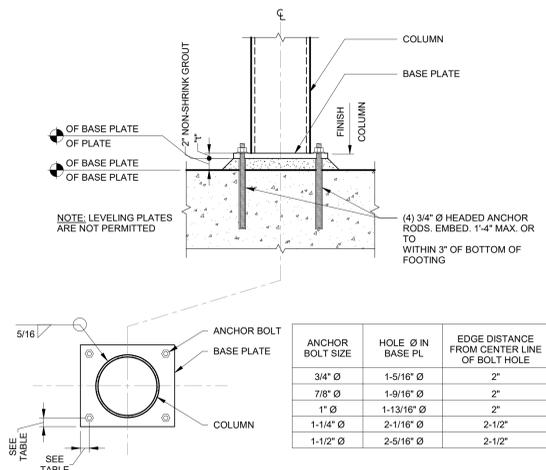
BAR SIZE	DEFORMED BAR TENSION LAP SPLICE - CLASS B							
	FOR NORMAL WEIGHT STONE CONCRETE & UNCOATED BARS							
	3000 PSI CONCRETE		4000 PSI CONCRETE		5000 PSI CONCRETE		6000 PSI CONCRETE	
	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	22	33	19	28	17	25	16	23
#4	29	43	25	37	23	34	21	31
#5	36	54	31	47	28	42	26	38
#6	43	65	37	56	34	50	31	46
#7	63	94	54	81	49	73	45	67
#8	72	107	62	93	56	83	51	76
#9	81	121	70	105	63	94	57	86
#10	91	136	79	118	71	106	64	96
#11	101	151	87	131	78	117	71	107

BAR SIZE	DEFORMED BAR COMPRESSION DEVELOPMENT LENGTH (Ldc)			
	FOR NORMAL WEIGHT STONE CONCRETE & UNCOATED BARS			
	3000 PSI CONCRETE	4000 PSI CONCRETE	5000 PSI CONCRETE	6000 PSI CONCRETE
#3	9	8	8	8
#4	11	10	9	9
#5	14	12	12	12
#6	17	15	14	14
#7	20	17	16	16
#8	22	19	18	18
#9	25	22	21	21
#10	28	25	23	23
#11	31	27	26	26

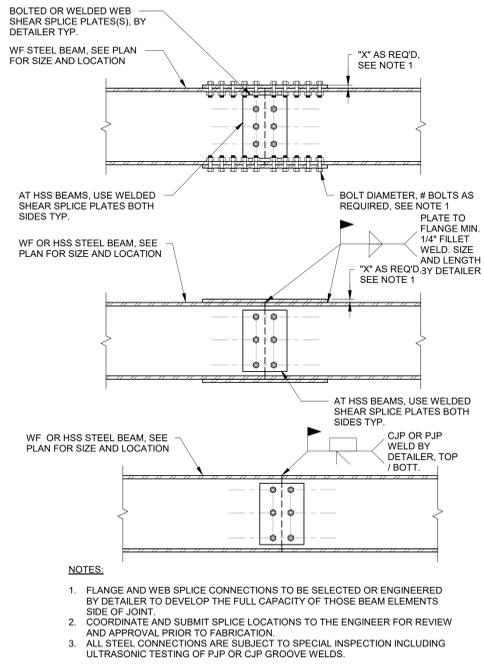
BAR SIZE	DEFORMED BAR COMPRESSION LAP SPLICE			
	FOR NORMAL WEIGHT STONE CONCRETE & UNCOATED BARS			
	3000 PSI CONCRETE	4000 PSI CONCRETE	5000 PSI CONCRETE	6000 PSI CONCRETE
#3	12	12	12	12
#4	15	15	15	15
#5	19	19	19	19
#6	23	23	23	23
#7	27	27	27	27
#8	30	30	30	30
#9	34	34	34	34
#10	39	39	39	39
#11	43	43	43	43



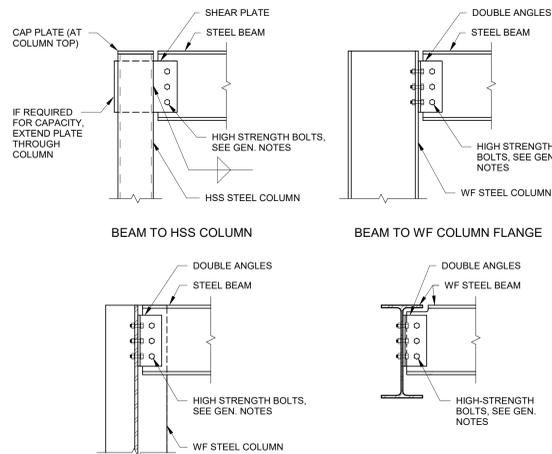
TYPICAL COLUMN BASE PLATE AT WIDE FLANGE COLUMN
N.T.S.



TYPICAL COLUMN BASE PLATE AT PIPE OR TUBE COLUMN
N.T.S.

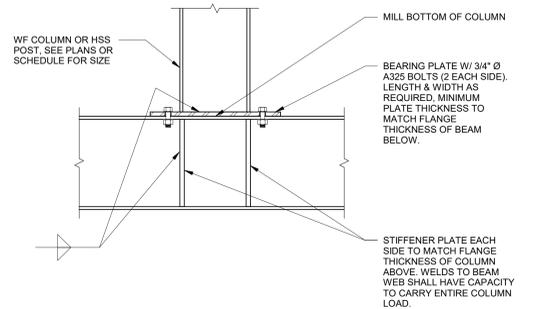


TYPICAL STEEL BEAM SPlice
N.T.S.

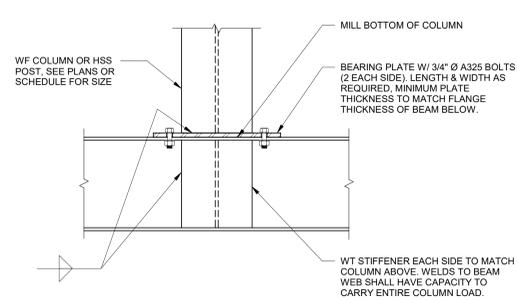


- NOTES:**
- CONNECTION DETAILS SHOWN ABOVE ARE SCHEMATIC ONLY. THE CONTRACTOR MAY SUBMIT ALTERNATE DETAILS FROM THOSE SHOWN ABOVE, BUT IN ANY CASE THE CONTRACTOR IS RESPONSIBLE FOR PRODUCING STEEL SHOP DRAWINGS IN ACCORDANCE WITH THE PROJECT GENERAL NOTES AND AISC GUIDELINES. CALCULATIONS SHALL BE SUBMITTED AS REQUIRED IN THE GENERAL NOTES AND ELSEWHERE IN THE CONTRACT DOCUMENTS.
 - THE CONTRACTOR SHALL DETAIL THE PROJECT CONNECTIONS FOR THE LOADS AS INDICATED IN THE CONTRACT DOCUMENTS. THE SCHEMATIC DETAILS ABOVE ARE NOT SUGGESTIVE OF SPECIFIC CAPACITIES. THE NUMBER AND SIZE OF BOLTS, SIZE AND LENGTH OF WELDS, AND SIZE OF STEEL PIECES MUST BE DETERMINED PER AISC GUIDELINES AND THE CONTRACT DOCUMENTS.
 - PROVIDE FULL DEPTH CONNECTIONS AT ALL PERIMETER BEAM CONNECTIONS AND AT BEAM CONNECTIONS TO PERIMETER BEAMS, UNLESS OTHERWISE NOTED.

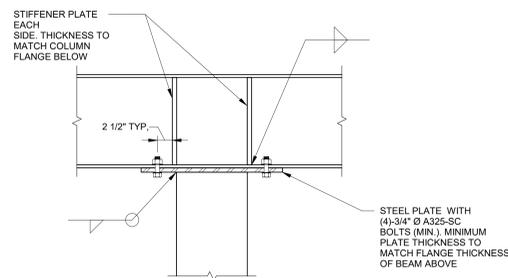
TYPICAL SUGGESTED SHEAR CONNECTIONS AT GRAVITY LOADS ONLY
N.T.S.



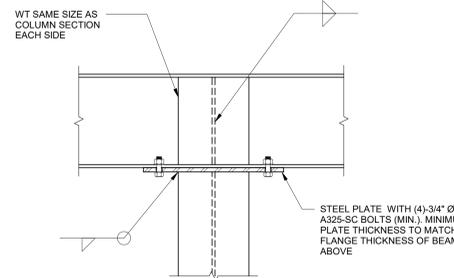
TYPICAL STIFFENER PLATE AT BEAM-SUPPORTED COLUMN (PARALLEL WEBS)
N.T.S.



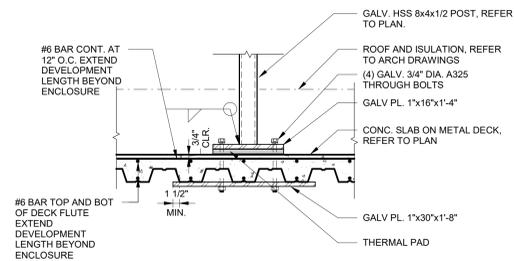
TYPICAL STIFFENER PLATE AT BEAM-SUPPORTED COLUMN (PERPENDICULAR WEBS)
N.T.S.



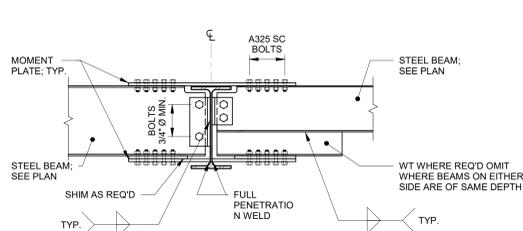
TYPICAL BEAM CONTINUOUS OVER COLUMN (PARALLEL WEBS)
N.T.S.



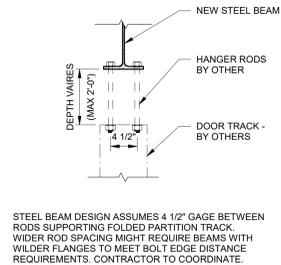
TYPICAL BEAM CONTINUOUS OVER COLUMN (PERPENDICULAR WEBS)
N.T.S.



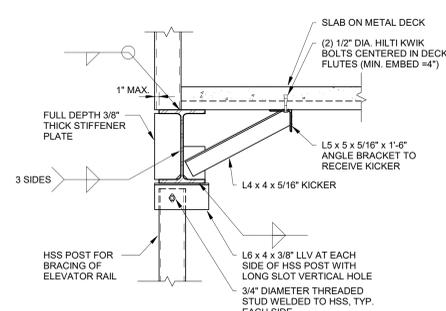
TYPICAL MEP ENCLOSURE POST ANCHORAGE
N.T.S.



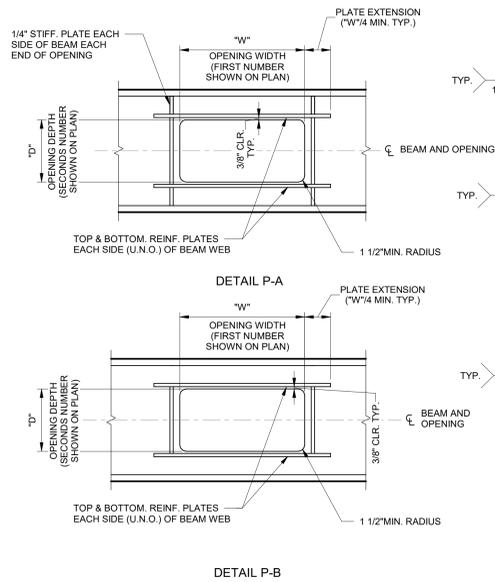
TYPICAL BEAM-TO-BEAM MOMENT CONNECTION
N.T.S.



TYPICAL DETAIL OF HANGER AT PARTITION WALL
N.T.S.



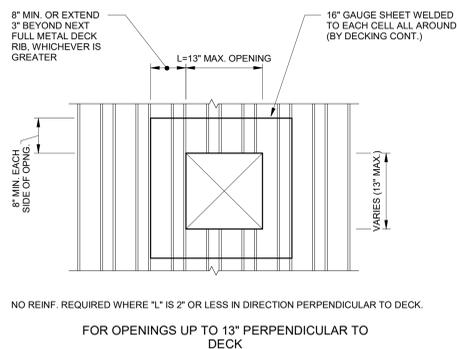
TYPICAL DETAIL OF ATTACHMENT OF HSS POST REQUIRED FOR ELEVATOR RAIL SUPPORT
N.T.S.



- NOTES:**
1. SEE PLAN FOR OPENING SIZE AND LOCATION
 2. ALL OPENING TO BE CENTERED AT BEAM MID-DEPTH.
 3. AN ADDITIONAL 2 STUDS PER FOOT (IN ADDITION TO STUDS SHOWN ON PLAN) ARE REQUIRED AT EACH END OF OPENING FOR 3'-0".
 4. COORDINATE ALL BEAM PENETRATION WITH HVAC, PLUMBING AND ELECTRICAL CONTRACTORS.

TYPICAL BEAM WEB PENETRATION

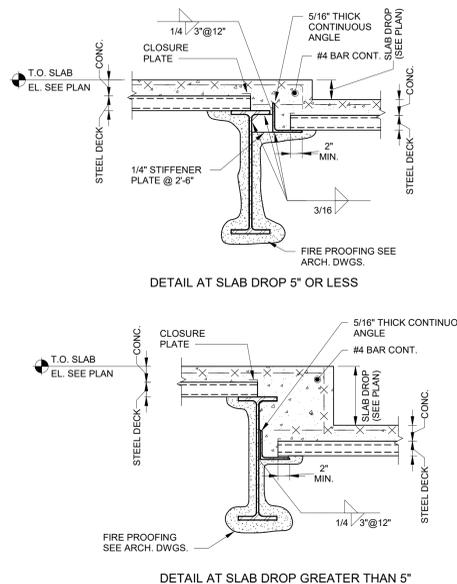
N.T.S.



- NOTES:**
1. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR OPENING SIZE AND LOCATION.
 2. WHERE POSSIBLE, EXTEND DECK CONTINUOUSLY OVER OPENING. REINFORCE AND CUT DECK WHEN OPENING IS REQUIRED.

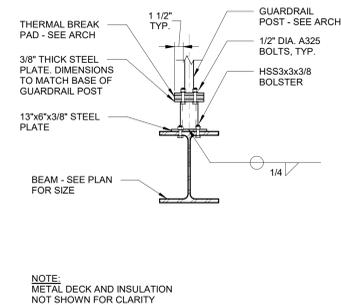
TYPICAL REINFORCEMENT FOR UNFRAMED OPENINGS IN ROOF DECK

N.T.S.



TYPICAL DECK SUPPORT AT COLUMNS (SMALL MEP PENETRATIONS)

N.T.S.



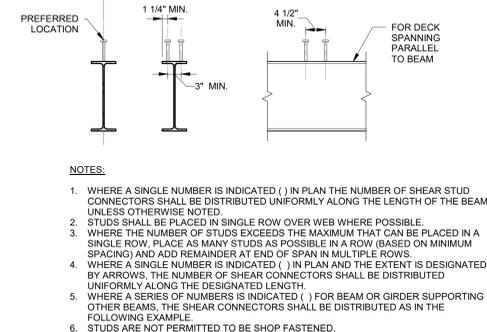
NOTE: METAL DECK AND INSULATION NOT SHOWN FOR CLARITY

TYPICAL ROOF GUARDRAIL POST BASE CONNECTION

N.T.S.

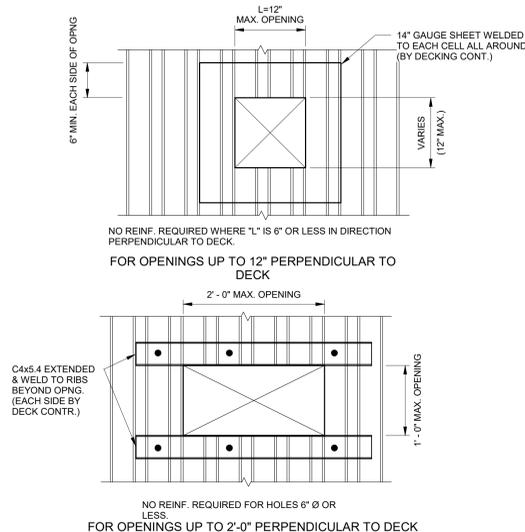
TYPICAL COLUMN FLANGE MOMENT CONNECTION (R=3)

N.T.S.



TYPICAL SHEAR STUD CONNECTORS AT COMPOSITE BEAMS

N.T.S.



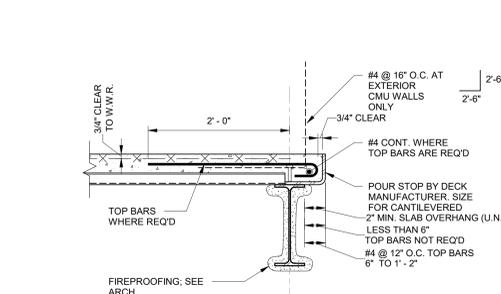
- NOTES:**
1. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR OPENING SIZE AND LOCATION.
 2. WHERE POSSIBLE, EXTEND DECK CONTINUOUSLY OVER OPENING REINFORCE, BLOCK/BOX OUT CONCRETE, AND CUT DECK WHEN OPENING IS REQUIRED.

TYPICAL REINFORCEMENT FOR UNFRAMED OPENINGS IN COMPOSITE FLOOR DECK

N.T.S.

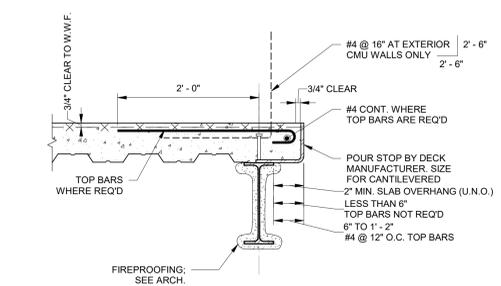
TYPICAL COLUMN WEB MOMENT CONNECTION (R=3)

N.T.S.



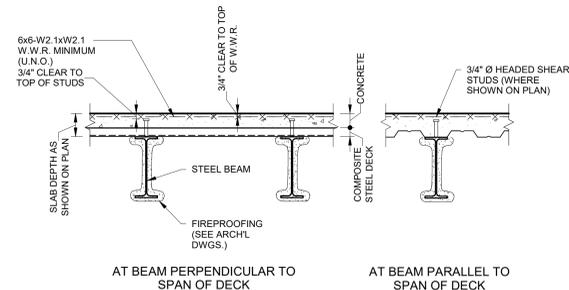
TYPICAL COMPOSITE FLOOR DECK PERPENDICULAR EDGE CONDITION

N.T.S.



TYPICAL COMPOSITE FLOOR DECK PARALLEL EDGE CONDITION

N.T.S.



TYPICAL COMPOSITE BEAM & SLAB CONSTRUCTION

N.T.S.

TYPICAL STEP IN SLAB ON METAL DECK

N.T.S.

CONTRACTOR

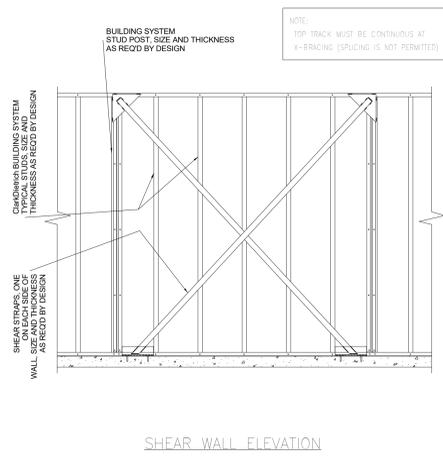
CONSULTANT

ISSUE DATES

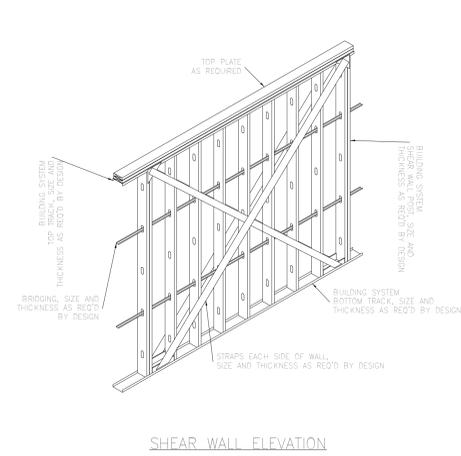
- 05/23/2019 75% DESIGN DEVELOPMENT
- 08/23/2019 100% DESIGN DEVELOPMENT
- 12/13/2019 90% CONSTRUCTION DOCUMENTS
- 04/15/2020 ISSUED FOR PERMIT
- 06/01/2020 ISSUED FOR CONSTRUCTION
- 07/01/2020 GMP SET
- 12/18/2020 FINAL GMP SET
- 09/10/2021 BID SET

TYPICAL DETAILS
SCALE: As Indicated

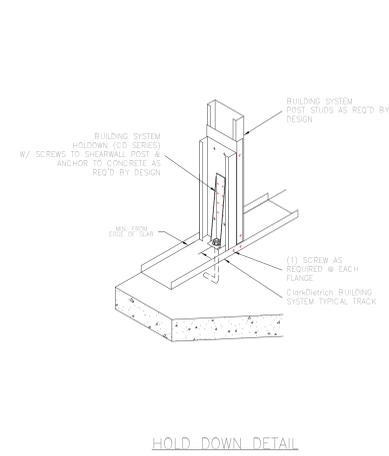
Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners, Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.



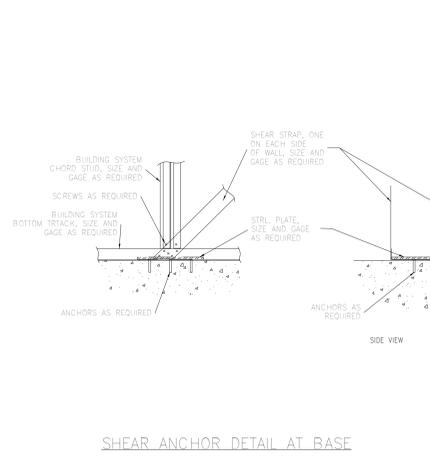
SHEAR WALL ELEVATION



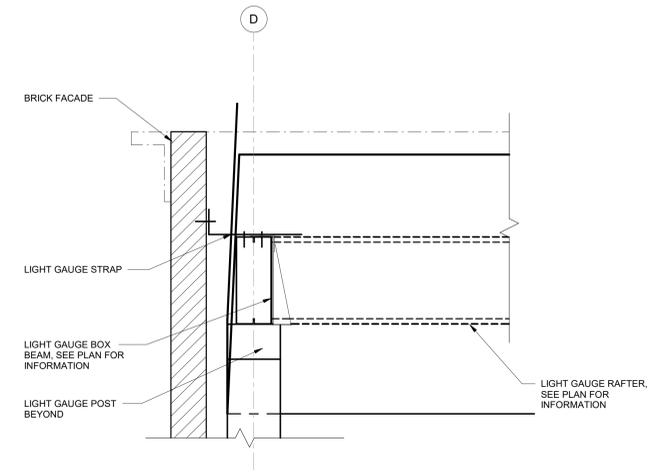
SHEAR WALL ELEVATION



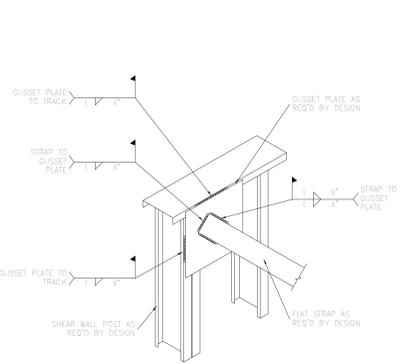
HOLD DOWN DETAIL



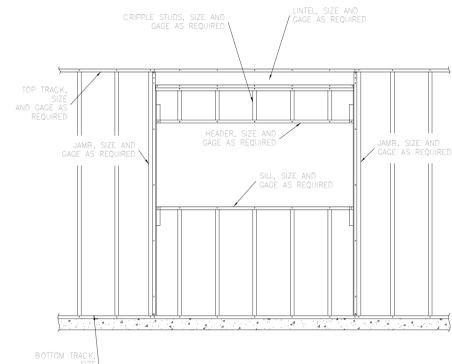
SHEAR ANCHOR DETAIL AT BASE



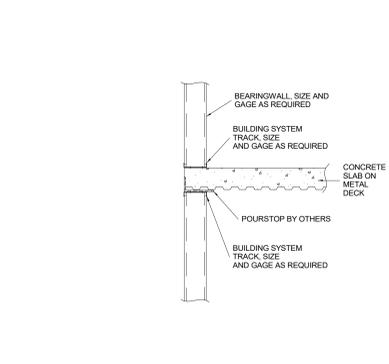
TYPICAL ROOF JOIST AT DORMER
N.T.S.



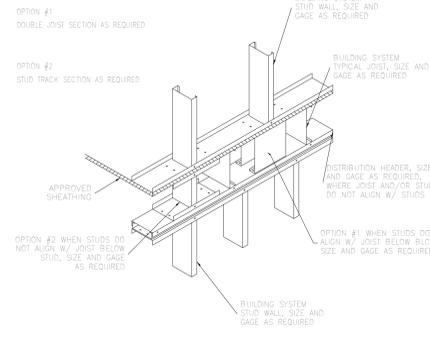
GUSSET PLATES



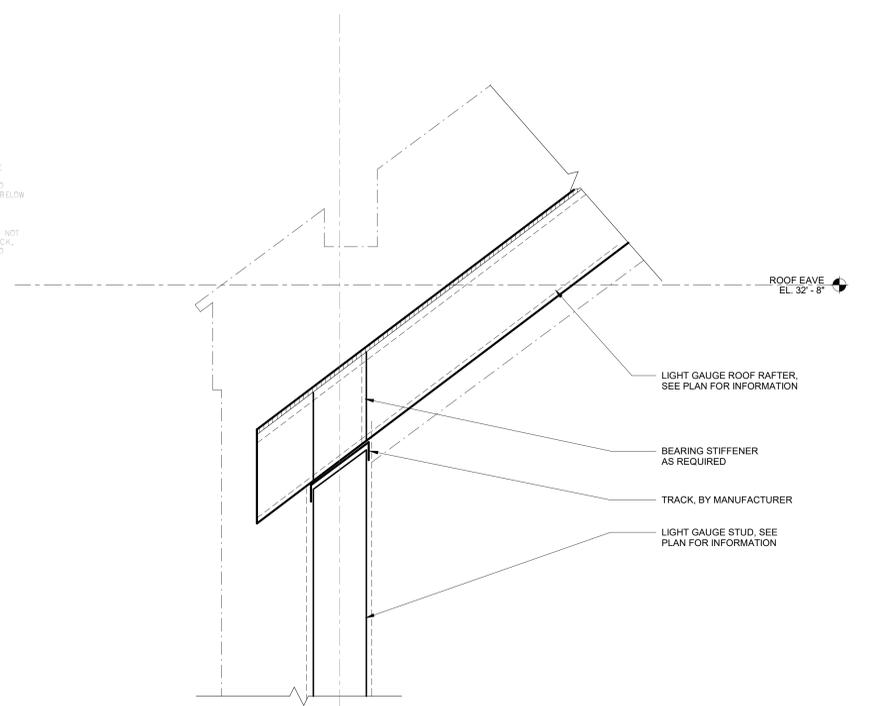
LOAD BEARING WINDOW OPENING ELEVATION



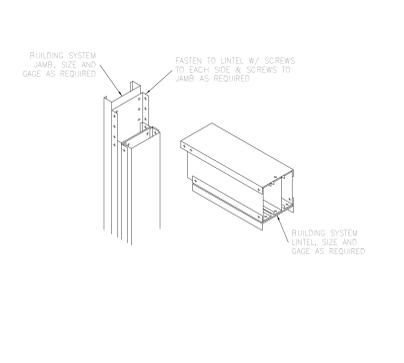
LOAD BEARING EXTERIOR



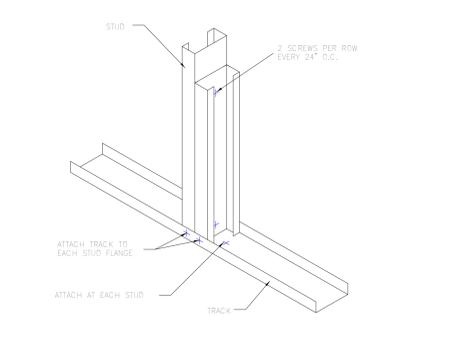
LOAD DISTRIBUTION HEADER



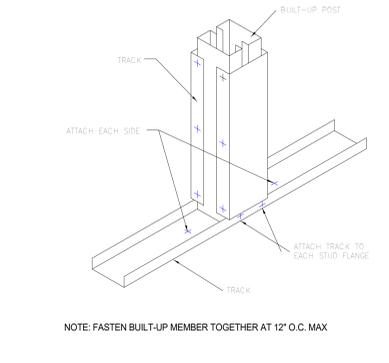
TYPICAL ROOF JOIST SUPPORT ON BEARING WALL
N.T.S.



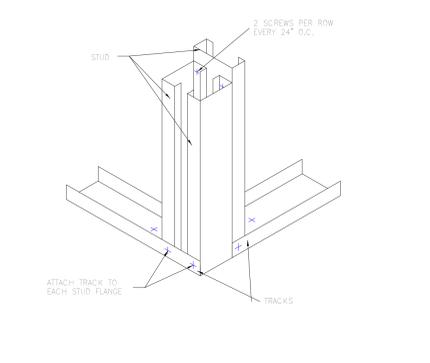
LOAD BEARING BOX HEADER LINTEL



BACK-TO-BACK STUDS

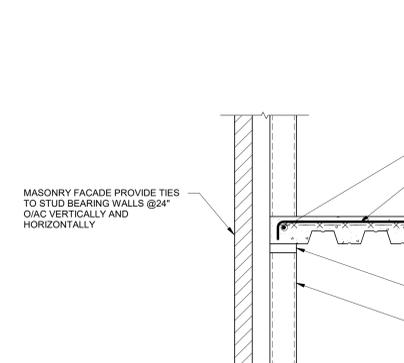


BUILT-UP POST

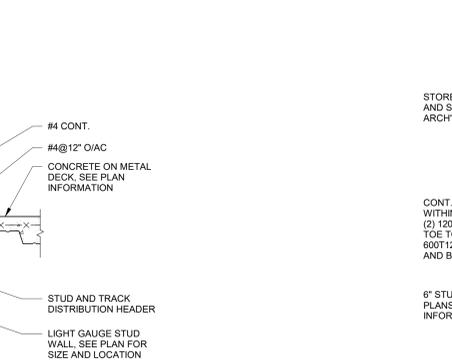


THREE-STUD CORNER

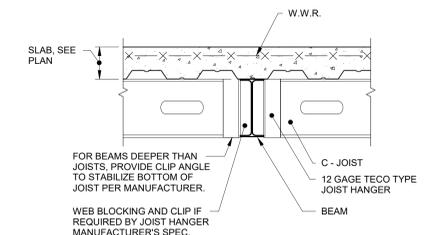
NOTE: FASTEN BUILT-UP MEMBER TOGETHER AT 12" O.C. MAX



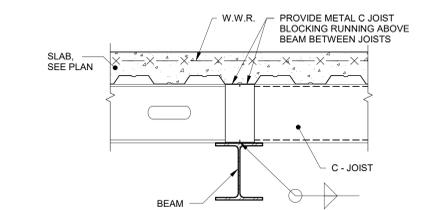
TYPICAL DETAIL EXTERIOR BEARING WALL AT INN
N.T.S.



TYPICAL BENT PL. CONNECTION TO BOX BEAM (TYPE L)
N.T.S.



WHERE BEAM IS ANY DEPTH



WHERE BEAM IS ANY DEPTH

TYPICAL C JOISTS SUPPORTED ON STEEL BEAM
N.T.S.

Frederick Fisher & Partners reserves its common law copyright and other property rights in these plans. These plans and drawings are not to be reproduced in any form or manner whatsoever without first obtaining the express written permission and consent of Frederick Fisher & Partners. Architects, nor are they to be assigned to any third party without obtaining said written permission and consent.

LEVEL 2																			LEVEL 2						
14' - 0"																			14' - 0"						
LEVEL 1																			LEVEL 1						
0"																			0"						
Column Locations	B.8.4	B.8.6	B.8.6.5	C-4	C(6'-7 5/8")-4.8(2'-6")	C(2'-7 3/4")-4.8(8')	C(2'-7 3/4")-4.8(4'-9")	C-6	C.2-2.5	C.3-6	C.3-6.5	C.3-7	D-2.5	D-4	D-4.8	D-5.3	D-5.6	D-6	D-7	D.1-4.9	D.1-5.5	E-4.9	E-5.5	F-4.9	F-5.5

LEVEL 2					LEVEL 2
14' - 0"					14' - 0"
LEVEL 1					LEVEL 1
0"					0"
Column Locations	G-4.9	G-5.5	H-5.5	J-5.8	

ROOF INSTITUTE																									ROOF INSTITUTE
28' - 0 1/8"																									28' - 0 1/8"
LEVEL 2																									LEVEL 2
14' - 0"																									14' - 0"
LEVEL 1																									LEVEL 1
0"																									0"
Column Locations	X0-Y3	X1-Y0	X1-Y1	X1-Y3	X1-Y3.5	X1-Y4	X1-Y5	X1-Y6	X2-Y3.5	X2-Y4	X2.5-Y4.3	X2.5-Y5	X2.5-Y6	X3-Y0	X3-Y1	X3-Y2.5	X3-Y2.5(1'-3 1/2")	X3-Y3	X3-Y3(1'-0 1/4")	X4-Y2.5(2'-9")	X4.1-Y2.5	X4.1-Y3	X5-Y3	X5-Y4.3	X5-Y5

ROOF INSTITUTE															ROOF INSTITUTE
28' - 0 1/8"															28' - 0 1/8"
LEVEL 2															LEVEL 2
14' - 0"															14' - 0"
LEVEL 1															LEVEL 1
0"															0"
Column Locations	X5-Y6	X5.1-Y0	X5.1-Y1	X5.1-Y2	X5.5-Y1.5	X5.5-Y1.5(11 3/8")	X5.5-Y2	X5.5-Y2(1'-5")	X5.7-Y5	X5.7-Y6	X6-Y0	X6-Y1.5	X6-Y2	X6-Y3.2	X6-Y4.3

LEVEL 2															LEVEL 2
14' - 0"															14' - 0"
LEVEL 1															LEVEL 1
0"															0"
Column Locations	C1-CB	C1-CC	C1-CD	C2-CA	C2-CB	C2-CC	C2-CD	C3-E	C3-CE	C3-CF	C4-E	C4-CE	C4-CF		

BASE PLATE SCHEDULE						
LOCATION	COLUMN SECTION	BP TYPE	LEVEL	GROUT THICKNESS	BOTTOM OF PLATE	REMARKS
B.8.4	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
B.8.6	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
B.8.6.5	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
C1-C3	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C1-CA	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C1-CB	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C1-CC	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C1-CD	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C2-CA	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C2-CB	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C2-CC	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C2-CD	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C3-CE	HSS4.000x0.313	BP3	LEVEL 1	2"	-10"	
C3-CF	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C3-E	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C4-CE	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C4-CF	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C4-E	HSS4.000x0.313	BP2	LEVEL 1	2"	-10"	
C.4	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
C-6	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
C.2-2.5	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
C.3-6	HSS6X6X1/2	BP2	LEVEL 1	2"	-10"	
C.3-6.5	HSS6X6X1/2	BP2	LEVEL 1	2"	-10"	
C.3-7	HSS6X6X1/2	BP2	LEVEL 1	2"	-10"	
D-2.5	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
D-4	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
D-4.8	HSS6X6X1/2	BP2	LEVEL 1	2"	-10"	
D-5.3	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
D-5.6	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
D-6	HSS6X6X1/2	BP2	LEVEL 1	2"	-10"	
D-7	HSS6X6X1/2	BP2	LEVEL 1	2"	-10"	
D.1-4.9	W8x48	BP3	LEVEL 1	2"	-10"	
D.1-5.5	W8x48	BP3	LEVEL 1	2"	-10"	
E-4.9	W8x48	BP3	LEVEL 1	2"	-10"	
E-5.5	W8x48	BP3	LEVEL 1	2"	-10"	
F-4.9	W8x48	BP3	LEVEL 1	2"	-10"	
F-5.5	W8x48	BP3	LEVEL 1	2"	-10"	
G-4.9	W8x48	BP3	LEVEL 1	2"	-10"	
G-5.5	W8x48	BP3	LEVEL 1	2"	-10"	
H-5.5	W8x48	BP3	LEVEL 1	2"	-10"	
J-5.8	W8x48	BP3	LEVEL 1	2"	-10"	
X1-Y1	W12x138	BP1	LEVEL 1	2"	-10"	
X1-Y3	W12x120	BP1	LEVEL 1	2"	-10"	
X1-Y3.5	W10x60	BP3	LEVEL 1	2"	-10"	
X1-Y4	W10x60	BP3	LEVEL 1	2"	-10"	
X1-Y5	W12x120	BP1	LEVEL 1	2"	-10"	
X2-Y3	W10x60	BP3	LEVEL 1	2"	-10"	
X2-Y4	W10x60	BP3	LEVEL 1	2"	-11' - 1"	
X2.5-Y4.3	W12x120	BP1	LEVEL 1	2"	-10"	
X2.5-Y5	W12x120	BP1	LEVEL 1	2"	-10"	
X3-Y1	W12x120	BP1	LEVEL 1	2"	-10"	
X3-Y2.5	HSS5X5X3/8	BP4	LEVEL 1	2"	2"	
X3-Y2.5(1'-3 1/2")	HSS5X5X3/8	BP4	LEVEL 1	2"	2"	
X3-Y3	HSS7X5X1/2	BP4	LEVEL 1	2"	-10"	
X3-Y3(1'-0 1/4")	HSS5X5X3/8	BP4	LEVEL 1	2"	2"	
X4-Y2.5(2'-9")	HSS5X5X3/8	BP4	LEVEL 1	2"	2"	
X4.1-Y2.5	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
X4.1-Y3	HSS7X5X1/2	BP4	LEVEL 1	2"	-10"	
X5-Y3	W12x120	BP1	LEVEL 1	2"	-10"	
X5-Y4.3	W12x152	BP1	LEVEL 1	2"	-10"	
X5-Y5	W8x48	BP3	LEVEL 1	2"	-10"	
X5.1-Y1	W8x48	BP3	LEVEL 1	2"	-10"	
X5.1-Y2	W8x48	BP3	LEVEL 1	2"	-10"	
X5.5-Y1.5	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
X5.5-Y1.5(11 3/8")	HSS5X5X3/8	BP4	LEVEL 1	2"	2"	
X5.5-Y2	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
X5.5-Y2(1'-5")	HSS5X5X3/8	BP4	LEVEL 1	2"	2"	
X5.7-Y5	W12x120	BP1	LEVEL 1	2"	-10"	
X6-4.9(1'-3 3/4")	HSS5X5X3/8	BP4	LEVEL 1	2"	2"	
X6-Y1.5	HSS5X5X1/2	BP2	LEVEL 1	2"	-10"	
X6-Y2	HSS6X6X1/2	BP2	LEVEL 1	2"	-10"	
X6-Y3.2	W12x120	BP1	LEVEL 1	2"	-10"	
X6-Y4.3	W12x120	BP1	LEVEL 1	2"	-10"	

- SEE PLAN FOR COLUMN ORIENTATION.
- THE FOLLOWING INDICATES TRANSFER GIRDER BELOW COLUMN
- SEE BASE PLATE SCHEDULE FOR BASE PLATE SIZES.
- LOADS INDICATED AT FLOOR LEVELS ARE ULTIMATE LOADS FOR THE CONTROLLING LOAD COMBINATIONS IN KIPS.
- PX INDICATES CONCRETE PIER. SEE TYPICAL DETAIL FOR SIZE AND REINFORCING

BEAM WEB PENETRATION SCHEDULE							
MARK	SIZE			REINFORCING	THICKNESS "t"	PENETRATION CL OFFSET	REMARKS
	WIDTH	DEPTH	DIAMETER				
WP-1			1"			+1-1/2"	

COLD FORMED WALL SCHEDULE				
MARK	WIDTH	STUD	SPACING	REMARKS
CFW3.6	3 5/8"	3 5/8 J 16	16" O.C.	14 GAUGE AT FIRST FLOOR
CFW3.6x2	8 1/4"	(2) 3 5/8 J 16	16" O.C.	14 GAUGE AT FIRST FLOOR
CFW5.5	5 1/2"	5 1/2 J 16	16" O.C.	14 GAUGE AT FIRST FLOOR
CFW6	6"	6 J 16	16" O.C.	14 GAUGE AT FIRST FLOOR

SLAB/DECK SCHEDULE		
MARK	TOTAL DEPTH	COMPOSITION/REINFORCEMENT
R1.5	1 1/2"	3" DEEP 16 GAGE ROOF DECK
S1	6 1/4"	3 1/4" LW CONCRETE ON 3" DEEP 18 GAGE COMPOSITE DECK. REINFORCE WITH #66 - W1.4KW1.4
S2	1' - 0"	SEE TYPICAL DETAILS FOR REINFORCEMENT
SOG	5"	SEE TYPICAL DETAILS FOR REINFORCEMENT

FOOTING SCHEDULE								
MARK	DIMENSIONAL INFORMATION			BOTTOM REINFORCEMENT		TOP REINFORCEMENT		REMARKS
	LENGTH	WIDTH	THICKNESS	LONG WAY	SHORT WAY	LONG WAY	SHORT WAY	
F3.0	3' - 0"	3' - 0"	1' - 0"	(4) #6	(4) #6			
F4.0	4' - 0"	4' - 0"	1' - 2"	(5) #6	(5) #6			
F5.0	5' - 0"	5' - 0"	1' - 3"	(6) #6	(6) #6			
F6.0	6' - 0"	6' - 0"	1' - 6"	(7) #6	(7) #6			
F7.0	7' - 0"	7' - 0"	1' - 6"	(7) #6	(7) #6			
F8.0	8' - 0"	8' - 0"	1' - 6"	(7) #6	(7) #6			
F9.0	9' - 0"	9' - 0"	1' - 9"	(8) #6	(8) #6			
F10.0	10' - 0"	10' - 0"	1' - 9"	(8) #6	(8) #6			
F10.0x6.0	6' - 0"	10' - 0"	1' - 2"	(7) #6	(11) #6			
F11.0	11' - 0"	11' - 0"	1' - 9"	(10) #6	(10) #6			
F19.0x14.0	14' - 0"	19' - 0"	2' - 0"	(14) #7	(20) #7	(8) #6	(12) #6	

PIER SCHEDULE							
MARK	SIZE		TYPE	VERTICAL BARS	CLOSED TIES	DOWEL EMBEDMENT	REMARKS
	WIDTH	LENGTH					
P1	2' - 0"	2' - 0"		(8) #6	#4 @ 6"		
P2	1' - 4"	1' - 4"		(8) #6	#4 @ 6"		

WALL FOOTING SCHEDULE					
MARK	WIDTH	DEPTH	BOTTOM REINFORCEMENT		REMARKS
			LONG WAY	SHORT WAY	
WF1		1' - 0"			
WF2	3' - 6"	1' - 0"	(5) #6		
WF3	3' - 0"	1' - 0"	(4) #6		

BASE PLATE TYPE SCHEDULE							
MARK	SIZE		THICKNESS	GRADE	BOLTS	LAYOUT	REMARKS
	WIDTH	LENGTH					
BP1	20"	20"	1 1/2"	A36	(4) 3/4" DIA. BOLTS	S503	
BP2	12"	12"	1 1/2"	A36	(4) 3/4" DIA. BOLTS	S503	
BP3	16"	16"	1"	A36	(4) 3/4" DIA. BOLTS	S503	
BP4	12"	16"	1"	A36	(4) 3/4" DIA. BOLTS	S503	