GENERAL NOTES

1. BUILDING CODE:
   THESE PLANS HAVE BEEN PREPARED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE 2018, AS ADOPTED BY NEW YORK STATE WITH AMENDMENTS.

2. SUBMITTALS
   A. THE FOLLOWING ITEMS REQUIRE SUBMITTAL OF SHOP AND ERECTION DRAWINGS, FOR REVIEW AND APPROVAL:
      a. REINFORCING STEEL FOR CAST-IN-PLACE CONCRETE
      b. STRUCTURAL STEEL
      c. COMPOSITE DRAWING OF ALL SLAB PENETRATIONS
      d. CONCRETE CONSTRUCTION AND CONTRACTION JOINTS
      e. SLAB ON GRADE JOINT LAYOUTS
   B. THE FOLLOWING ITEMS REQUIRE SUBMITTAL OF SHOP AND ERECTION DRAWINGS AND STRUCTURAL CALCULATIONS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THIS PROJECT FOR REVIEW AND APPROVAL:
      a. EXTERIOR CURTAIN WALLS, COLD FORMED STEEL FRAMING, AND STOREFRONTS.
      b. UNDERPINNING EXCAVATION SUPPORT, SHEETING, OR BENCHING WHERE SOILS REQUIRE SUCH BY VIRTUE OF OSHA REQUIREMENTS (ALL EXCAVATIONS GREATER THAN 5' REQUIRE SPECIFIC TRENCHING CONSIDERATIONS) OR SOIL CONDITIONS
      c. STEEL STAIRS, GUARDRAILS, HANDRAILS
      d. CONCRETE MIX DESIGNS
   C. THE CONTRACTOR SHALL SUBMIT TO THE EOR A SUBMITTAL SCHEDULE A MINIMUM OF 15 BUSINESS DAYS PRIOR TO THE ISSUANCE OF THE FIRST SUBMITTAL PACKAGE FOR REVIEW AND APPROVAL.
   D. THE CONTRACTOR SHALL ALLOW FOR A MINIMUM REVIEW TIME OF 10 BUSINESS DAYS ON AVERAGE BY THE EOR PLUS ANY ADDITIONAL TIME REQUIRED BY THE OTHER DESIGN PROFESSIONALS. SUBMITTAL PACKAGES SHALL BE BROKEN UP AND SCHEDULED SO THAT THE RATE OF REVIEW EXPECTED OF THE STRUCTURAL ENGINEER DOES NOT EXCEED THE FOLLOWING:

STRUCTURAL STEEL:	80 SHEETS/WEEK (1PC/SHEET)
STRUCT. PRECAST CONCRETE:	80 SHEETS/WEEK
CONCRETE REINFORCEMENT:	20 SHEETS/WEEK
WOOD WALL PANEL/FLOOR TRUSS:	1000 SHTS/WEEK (1PC/SHEET)
LGMF WALL PANEL/FLOOR TRUSS:	1000 SHTS/WEEK (1PC/SHEET)

   E. SUBMITTALS ISSUED TO THE DESIGN TEAM FOR REVIEW SHALL BEAR THE CONTRACTOR'S STAMP OF APPROVAL, CERTIFYING THAT ALL FIELD MEASUREMENTS, CONSTRUCTION CRITERIA, MATERIALS, ETC. HAVE BEEN VERIFIED AND EACH SHEET HAS BEEN REVIEWED FOR COMPLETENESS, COORDINATION BETWEEN TRADES, AND COMPLIANCE WITH THE CONTRACT DOCUMENTS. FURTHER, STRUCTURAL SHOP DRAWINGS WILL ONLY BE REVIEWED ONCE ANY REQUIRED CALCULATION PACKAGES FOR THAT WORK HAS BEEN ISSUED ALONG WITH A SIGNED AND SEALED LETTER BY THE CONTRACTOR'S ENGINEER CERTIFYING THAT THE SHOP DRAWINGS HAVE PROPERLY INCORPORATED THEIR DESIGN, IN ACCORDANCE WITH THE 2010 AISC CODE OF STANDARD PRACTICE-SECTION 3.1.2 (OPTION 3), OTHERWISE THE SUBMITTAL PACKAGE WILL BE REJECTED.

3. SPECIAL INSPECTIONS: AS PER IBC CHAPTER 17, THE FOLLOWING ITEMS ARE SUBJECT TO SPECIAL INSPECTION BY AN INDEPENDENT INSPECTION AND/OR TESTING AGENCY HIRED BY THE OWNER AND APPROVED BY THE EOR AND BUILDING OFFICIAL. OWNER/SPECIAL INSPECTOR SHALL PROVIDE SPECIAL INSPECTION REPORTS WITHIN 5 DAYS OF PERFORMING THE INSPECTION AND IMMEDIATELY NOTIFY THE ENGINEER.

A. STEEL CONSTRUCTION	(1705.2)
a. STRUCTURAL STEEL (SEE AISC360-10, CHAP. N)	(1705.2.1)
D. CONCRETE CONSTRUCTION	(1705.3)
E. MASONRY CONSTRUCTION	(1705.4)
F. SOILS	(1705.6)
G. POST-INSTALLED ANCHORS	(ACI318 17.8.2)

4. DESIGN LOADS:
   A. DEAD AND LIVE LOADS: SEE LOAD MAPS ON S-010
   B. FLOOR LIVE LOADS HAVE BEEN REDUCED IN ACCORDANCE WITH CODE FOR COLUMNS.
   C. ROOF DEAD LOAD INCLUDES 15 PSF ALLOWANCE FOR WEIGHT OF MECHANICAL EQUIPMENT.
   D. WIND LOADS:

BASIC WIND SPEED (V 3-SEC GUST):	115 MPH
WIND IMPORTANCE FACTOR (Iw):	1.0
EXPOSURE CATEGORY:	B
INTERNAL PRESSURE COEFFICIENT (Cp):	± 0.18

   E. COMPONENTS AND CLADDING (MEMBERS SUPPORTING A TRIBUTARY AREA LESS THAN 700 SQ. FT.) SHALL BE DESIGNED FOR WIND PRESSURES GIVEN IN THE COMPONENTS AND CLADDING TABLES GIVEN ON S-002.
   F. ROOF SNOW LOADS:

GROUND SNOW LOAD (Pg):	30 PSF
SNOW EXPOSURE FACTOR (Ce):	0.90
SNOW THERMAL FACTOR (Ct):	1.00
SNOW IMPORTANCE FACTOR (Is):	1.00
FLAT ROOF SNOW LOAD (Pf):	20 PSF

   SNOW DRIFT LOAD HAS BEEN CONSIDERED WHERE REQUIRED.
   G. SEISMIC LOADS:

SEISMIC IMPORTANCE FACTOR (Ie):	1.0
OCCUPANCY CATEGORY:	II
SPECTRAL RESPONSE ACCELERATIONS:	Ss= .292g S1= .061g
SITE CLASS:	C
SPECTRAL RESPONSE COEFFICIENTS:	Sds= .253g Sd1= .061g
SEISMIC DESIGN CATEGORY (SDC):	B
BASIC SEISMIC FORCE RESISTING SYSTEM:	ORDINARY REINFORCED CONCRETE SHEAR WALLS
DESIGN BASE SHEAR:	276 KIPS
RESPONSE MODIFICATION FACTOR (R):	5
ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE

5. SERVICEABILITY:
   THE NEW STRUCTURE HAS BEEN DESIGNED FOR THE FOLLOWING DEFLECTION AND DRIFT LIMITS:

LIVE LOAD DEFLECTION:	L/360
LONG TERM TOTAL LOAD DEFLECTION:	L/240
LATERAL WIND DRIFT:	H/400
LATERAL SEISMIC DRIFT:	H/50
SPANDREL BEAMS LIVE LOAD DEFLECTION:	L/600

   ALL FACADE CURTAINWALLS, STOREFRONTS, AND NEW STRUCTURAL PARTITIONS SHALL BE DESIGNED TO ACCOMMODATE THE BUILDING MOVEMENT.

6. ALL WORK SHALL BE COORDINATED WITH ARCHITECTURAL, ELECTRICAL, AND MECHANICAL DRAWINGS. CONFLICTS IN DIMENSIONS AND INTERFERENCES SHALL BE DIRECTED TO OUR OFFICE PRIOR TO CONSTRUCTION OF WORK.

7. THE CONTRACTOR SHALL CHECK THE BUILDING LOCATION WITH REGARD TO PROPERTY LINE, AND VERIFY ALL EXISTING CONDITIONS BEFORE EXCAVATION AND SHOP DRAWING PREPARATION. NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES.

8. IN CASE OF CONTRADICTION BETWEEN THE DRAWINGS, THE SPECIFICATIONS, AND THE CODES, OR IF ANY CHANGE IS REQUIRED, THE CONTRACTOR SHALL INFORM THE ENGINEER IMMEDIATELY. NO CHANGE SHALL BE MADE WITHOUT WRITTEN APPROVAL OF THE ARCHITECT.

9. ALL DIMENSIONS AND DETAILS FOR ESCALATORS AND ELEVATORS MUST BE VERIFIED WITH ESCALATOR AND ELEVATOR CERTIFIED DRAWINGS BEFORE BEGINNING CONSTRUCTION. ADJUST OPENINGS SIZES, DETAILS, AND STEEL FRAMING AS REQUIRED TO CONFORM TO THOSE DRAWINGS. CONTRACTOR SHALL SUPPLY ANY SUPPLEMENTAL STEEL FRAMING FOR SUPPORT OF THE ELEVATORS AND ESCALATORS NOT SHOWN ON THE STRUCTURAL DRAWINGS.

10. THE STABILITY OF STRUCTURE, ADJACENT STRUCTURES IMPACTED BY THE WORK, AND SITE SAFETY ARE THE CONTRACTOR'S RESPONSIBILITY UNTIL CONSTRUCTION IS COMPLETE AND THE STRUCTURE HAS REACHED ITS FINAL CONDITION. THE CONTRACTOR IS RESPONSIBLE FOR ANY TEMPORARY BRACING, ERECTION PIECES, CONSTRUCTION SUPPORTS, FALL PROTECTION, DEBRIS CATCHES, TEMPORARY SHORING, ETC. AS REQUIRED TO SAFEGUARD THE SITE THROUGHOUT THE COURSE OF CONSTRUCTION.

11. THE CONTRACTOR SHALL VERIFY THAT ANY CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN CAPACITY OF THE STRUCTURE.

12. VIBRATION EQUIPMENT MUST BE MOUNTED ON VIBRATION ISOLATORS. THE STRUCTURAL SYSTEM HAS NOT BEEN DESIGNED FOR VIBRATION EQUIPMENT.

FOUNDATION NOTES

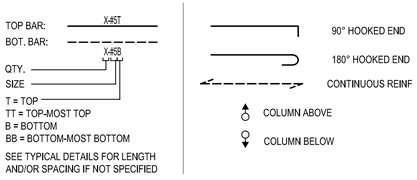
1. GENERAL
   A. FOUNDATIONS HAVE BEEN DESIGNED TO AN ALLOWABLE SOIL BEARING PRESSURE OF 4000 PSF, BASED ON A SOILS REPORT ISSUED BY CARLIN SIMPSON & ASSOCIATES DATED 1/13/2016. THIS CAPACITY SHALL BE VERIFIED BY A REGISTERED SOILS ENGINEER, SHOULD CONDITIONS VARY FROM THOSE ASSUMED, THE EOR SHALL BE NOTIFIED BEFORE CONTINUATION OF WORK.
   B. ALL FOOTINGS SHALL BE PLACED DIRECTLY ON COMPETENT NATURAL, GRANULAR SOILS OR ENGINEERED CERTIFIED COMPACTED FILL OVER COMPETENT NATURAL SOILS.
   C. ALL FILL SHALL BE PLACED IN EIGHT INCH LOOSE LIFTS (MAXIMUM) COMPACTED WITH VIBRATORY ROLLERS. FILL MATERIAL SHALL BE TESTED BY MODIFIED PROCTOR DENSITY METHOD (ASTM D1557) AND MUST QUALIFY AS SELECT, WITH LESS THAN 10% PASSING THROUGH THE NO. 200 SIEVE. SOIL SHALL BE PLACED WITH MOISTURE CONTENT AND ENERGY TO PROVIDE 92% OF MAXIMUM DRY DENSITY BELOW SLABS ON GRADE AND 95% BELOW FOOTINGS. IN PLACE DENSITY TESTS SHALL BE TAKEN FOR EACH 10,000 S.F. IN EACH LIFT. FOR ACCEPTANCE OF SOIL, AVERAGE OF DENSITY TESTS MUST EXCEED THE SPECIFIED COMPACTION, NO TESTS SHALL BE PERMITTED TO FALL BELOW 88% COMPACTION BELOW SLABS ON GRADE OR 90% COMPACTION BELOW FOOTINGS.

2. SHALLOW FOUNDATIONS
   A. ALL EXTERIOR FOOTINGS SHALL BE PLACED A MINIMUM OF 3'-6" BELOW FINAL GRADE WHEN BEARING ON SOIL.
   B. WHERE NECESSARY, FOOTING STEPS SHALL BE CONSTRUCTED AT MAXIMUM SLOPE OF 1 VERTICAL TO 2 HORIZONTAL.
   C. WHERE ROCK OUTCROPPINGS ARE ENCOUNTERED IN A BUILDING FOUNDATION BEARING ON SOIL, SUCH OUTCROPPING OR INTERFERENCE SHALL BE REMOVED TO A DEPTH 12 INCHES BELOW BOTTOM OF FOOTING AND REPLACED WITH CLEAN GRANULAR MATERIAL CONTAINING LESS THAN 15% SILT, COMPACTED TO 95% MAXIMUM DENSITY PER MODIFIED PROCTOR METHOD. MAINTAIN A MINIMUM COVER OF 4'-0" TO BOTTOM OF CONCRETE.
   D. WHERE SOLID UNFRACTURED ROCK IS ENCOUNTERED FOR A WALL LENGTH OF AT LEAST 25 FEET, WALLS MAY BE PLACED WITHOUT FOOTINGS BY TRENCHING 6 INCHES INTO THE ROCK AND PINNING THE WALL TO ROCK WITH DOWELS TO MATCH VERTICAL REINFORCING, GROUTED INTO ROCK, EXTENDING 24 BAR DIAMETERS INTO ROCK. NO FROST PROVISIONS ARE REQUIRED FOR THIS DETAIL. PROVIDE CONTROL JOINT IN WALL AT ANY TRANSITION BETWEEN ROCK BEARING AND SOIL BEARING CONDITIONS.
   E. EXCAVATIONS SHALL BE DEWATERED TO ALLOW INSTALLATION OF FOOTINGS IN DRY ATMOSPHERE.
   F. DIFFERENTIAL BACKFILL AGAINST FOUNDATION WALLS SHALL NOT EXCEED FOUR FEET UNTIL TOP BRACING SLAB OR FRAMEWORK HAS BEEN IN PLACE FOR A MINIMUM OF THREE DAYS. CANTILEVERED RETAINING WALLS MAY BE BACKFILLED AFTER 14 DAYS FROM CONCRETE PLACEMENT, BUT IN NO CASE SHALL DIFFERENTIAL OF BACKFILL, ON OPPOSITE SIDES OF THE WALL, EXCEED THE FINAL DESIGN DIFFERENTIAL.
   G. ALL BOTTOM OF FOOTING ELEVATIONS ARE SUBJECT TO CHANGE UPON INSPECTION OF SOIL CONDITION. ELEVATION OF ADJACENT FOOTING BOTTOMS SHALL NOT EXCEED A MAXIMUM SLOPE OF:
      1.1.1. 1H:1V FOR COHESIVE SOILS WITH AN UNCONFINED COMPRESSIVE STRENGTH GREATER THAN 0.5 TSF
      2.1.2. 1 1/2H:1V FOR COHESIVE SOILS WITH AN UNCONFINED COMPRESSIVE STRENGTH OF 0.5TSF OR LESS.
   H. THE CONTRACTOR SHALL NOTIFY THE ENGINEER WHERE BOTTOM OF FOOTING ELEVATION IS CHANGED AND OBTAIN REVISED DESIGN OF THE FOUNDATION AND RETAINING WALLS AS REQUIRED.

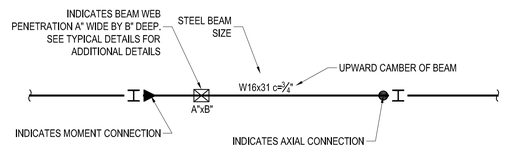
ABBREVIATIONS AND SYMBOLS

AB	ANCHOR BOLT
ADDL	ADDITIONAL
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
ALT	ALTERNATE
ARCH	ARCHITECTURAL
ASD	ALLOWABLE STRESS DESIGN (SERVICE LEVEL LOADS)
BLDG	BUILDING
BM	BEAM
B/	BOTTOM OF
BS	BOTH SIDES
BTWN	BETWEEN
C	CAMBER
CLR	CLEAR
CL	CENTERLINE
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONST	CONSTRUCTION
CONT	CONTINUOUS
DIA OR Ø	DIAMETER
DIM	DIMENSION
db	DIAMETER OF BAR
-do-	DITTO
DWG	DRAWING
DWL(S)	DOWEL(S)
EA	EACH
EE	EACH END
EF	EACH FACE
EL	ELEVATION
EOD	EDGE OF DECK
EOS	EDGE OF SLAB
EOR	ENGINEER OF RECORD
EQ	EQUAL
EQUIV	EQUIVALENT
EW	EACH WAY
E-W	EAST-WEST
EXIST	EXISTING
EJ	EXPANSION JOINT
FD	FULL DEPTH
FDN	FOUNDATION
FIN	FINISH
FLR	FLOOR
FLG	FLANGE
FP	FIRE PROTECTION
FS	FAR SIDE
FT OR '	FEET OR FOOT
FTG	FOOTING
Fy	YIELD STRESS
GA	GAUGE
GALV	GALVANIZED
GR	GRADE
HDG	HOT DIPPED GALVANIZED
HORIZ	HORIZONTAL
HP	HIGH POINT
HT	HEIGHT
HVAC	HEATING, VENTILATION, AIR CONDITIONING
ID	INSIDE DIAMETER
IF	INSIDE FACE
IN OR "	INCHES
J	JOIST
KSI	KIPS PER SQUARE INCHES
KSF	KIPS PER SQUARE FOOT
LG	LONG
LGMF	LIGHTGAGE MANUFACTURER
LONG	LONGITUDINAL
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LW	LONG WAY
LWC	LIGHT WEIGHT CONCRETE
LP	LOW POINT
LRFD	LOAD RESISTANCE FACTOR DESIGN (ULTIMATE LOADS)
MAX	MAXIMUM
MIN	MINIMUM
MD	METAL DECK
MECH	MECHANICAL
MEP	MECHANICAL, ELECTRICAL, & PLUMBING
MISC	MISCELLANEOUS
NS	NEAR SIDE
N-S	NORTH-SOUTH
NTS	NOT TO SCALE
NIC	NOT IN CONTRACT
NWC	NORMAL WEIGHT CONCRETE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OPNG	OPENING
OPP	OPPOSITE
PL	PLATE
PSI	POUNDS PER SQUARE INCH
PSF	POUNDS PER SQUARE FEET
R	RADIUS
REINF	REINFORCING
REQD	REQUIRED
RTU	ROOF TOP UNIT
SCJ	SAW CUT JOINT
SCHED	SCHEDULE
SIM	SIMILAR
SOG	SLAB ON GRADE
SOMD	SLAB ON METAL DECK
SST or SS	STAINLESS STEEL
STD	STANDARD
STIF	STIFFENER
STRUCT	STRUCTURE
SYMM	SYMMETRICAL
SW	SHORT WAY
T	TOP
T&B	TOP & BOTTOM
THK OR THKNS	THICK OR THICKNESS
THRU	THROUGH
T/	TOP OF
T&S	TEMPERATURE & SHRINKAGE
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
VAR	VARIES
VERT	VERTICAL
VIF	VERIFY IN FIELD
W/	WITH
WP	WORK POINT
WWF	WELDED WIRE FABRIC

CONCRETE COLUMN & SLAB LEGEND



STEEL BEAM LEGEND



DRAWING LIST:

- ● ISSUED - NEW SHEET
- ◒ ISSUED - REVISION MADE
- ○ ISSUED - NO REVISION MADE
- ⊗ SHEET REMOVED

ISSUED FOR: ISSUE FOR BID	DATE: 04/23/2021	SHT.	DWG. NO.	DRAWING TITLE
	●	1	S-001	GENERAL NOTES I
	●	2	S-002	GENERAL NOTES II
	●	3	S-010	LOAD MAPS
	●	4	S-101	FIRST FLOOR PLAN
	●	5	S-102	MEZZANINE FLOOR PLAN
	●	6	S-103	SECOND FLOOR PLAN
	●	7	S-104	THIRD AND FOURTH FLOOR PLAN
	●	8	S-105	FIFTH FLOOR PLAN
	●	9	S-106	ROOF FLOOR PLAN
	●	10	S-107	BULKHEAD FLOOR PLAN
	●	11	S-110	SUPERSTRUCTURE PART PLANS
	●	12	S-120	FOUNDATION MAT PART PLANS
	●	13	S-200	CONCRETE COLUMN SCHEDULE
	●	14	S-210	SHEAR WALL ELEVATION I
	●	15	S-211	SHEAR WALL ELEVATION II
	●	16	S-212	SHEAR WALL ELEVATION III
	●	17	S-300	TYP.CONCRETE FOUNDATION DETAILS I
	●	18	S-301	TYP.CONCRETE FOUNDATION DETAILS II
	●	19	S-302	TYP.CONCRETE FOUNDATION DETAILS III
	●	20	S-310	TYP.CONCRETE DETAILS I
	●	21	S-311	TYP.CONCRETE DETAILS II
	●	22	S-312	TYP.CONCRETE DETAILS III
	●	23	S-313	TYP.CONCRETE DETAILS IV
	●	24	S-314	TYP.CONCRETE DETAILS V
	●	25	S-315	TYP.CONCRETE DETAILS VI
	●	26	S-330	MASONRY DETAILS
	●	27	S-400	FOUNDATION DETAILS I
	●	28	S-401	FOUNDATION DETAILS II
	●	29	S-410	SUPERSTRUCTURE CONCRETE DETAILS I
	●	30	S-420	SUPERSTRUCTURE STEEL DETAILS I

ISSUE FOR BID
APR 23, 2021

Rev. #	Revision Description	Date:
1	ISSUE FOR BID	04/23/2021

Project Description:
PROPOSED MIXED USE BUILDING:
WESTMORELAND LOFTS
136-158 WESTMORELAND AVE.
WHITE PLAINS, NY 10606

Owner/Developer:
136-158 WESTMORELAND, LLC
1485 5TH AVENUE, 24F
NEW YORK, NY 10035

Papp Architects
architecture | planning | interiors
188 East Post Road, White Plains, NY 10601
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JMC Site Development Consultants
Civil Engineer
120 Bedford Road
Armonk, NY 10504
914 273-5225

McLaren Engineering Group
Structural Engineer
131 West 35th Street, 4th Floor
New York, NY 10001
212 324-6300

Khachaturian Engineering Associates
Mechanical/Electrical/Plumbing Engineers
186 Wood Avenue South, First Floor
Iselin, NJ 08830
732 635-0044

Sheet Title:
GENERAL NOTES I

Seal & Signature

Date:	01-27-2021
Scale:	AS NOTED
Job#:	161162.00
Sheet Title:	S-001

CAST-IN-PLACE CONCRETE

1. GENERAL
   - A. ALL CONCRETE WORK SHALL CONFORM TO REQUIREMENTS OF THE A.C.I. BUILDING CODE REQUIREMENT FOR STRUCTURAL CONCRETE (318-14 ULTIMATE STRENGTH DESIGN).
   - B. 28 DAY MINIMUM COMPRESSIVE STRENGTH AND RELATED PROPERTIES FOR CONCRETE SHALL BE AS FOLLOWS:

	F'C	MAX W/C RATIO	MAX DENSITY
FOOTINGS	5000PSI	0.40	NWC (145 PCF)
SLAB ON GRADE	5000PSI	0.40	NWC (145 PCF)
WALLS	5000PSI	0.40	NWC (145 PCF)
BEAMS & SLABS	5000PSI	0.40	NWC (145 PCF)
COLUMNS	5000PSI	0.40	NWC (145 PCF)

   - C. CONCRETE COVERING OF REINFORCING STEEL (INCLUDING TIES AND STIRRUPS) SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS:
     - 3/4" SLABS AND WALLS WITH INTERIOR EXPOSURE
     - 1-1/2"SLABS AND WALLS WITH EXTERIOR EXPOSURE FOR #5 OR SMALLER, 2" OTHERWISE
     - 1-1/2"BEAMS AND COLUMNS WITH INTERIOR EXPOSURE
     - 2" BEAMS AND COLUMNS WITH EXTERIOR EXPOSURE
     - 2" FOUNDATION WALL, FOOTING & GRADE BEAM FACES NOT CAST AGAINST EARTH
     - 3" CONCRETE CAST AGAINST EARTH
   - D. ALL CONCRETE, INCLUDING FOUNDATIONS, EXPOSED TO WEATHER AND/OR OUTSIDE THE BUILDING ENVELOPE SHALL BE AIR ENTRAINED, 6%±1.5% BY VOLUME FOR 3/4" COARSE AGGREGATE, AND 7.5%±1.5% BY VOLUME FOR 3/8" LIGHT WEIGHT AGGREGATE. AIR ENTRAINING ADMIXTURE TO COMPLY WITH ASTM C260.
   - E. PROVIDE GRACE CONCRETE PRODUCTS DCI INHIBITOR (OR EQUIVALENT) THAT MEETS THE REQUIREMENTS OF ASTM C1582 AND ASTM C494 TYPE C AT A RATE OF 4 GAL PER CU YD OF CONCRETE AT THE FOLLOWING LOCATIONS:
     1. SLAB SUPPORTING THE LOADING DOCK AND VEHICULAR ACCESS
     2. EXTERIOR GRADE LEVEL SLABS AND STAIRS
     3. SLAB SUPPORTING MEZZANINE PARKING LEVEL
   - F. ALL PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE II.
   - G. ALL NORMAL WEIGHT AND LIGHT WEIGHT CONCRETE AGGREGATE SHALL CONFORM TO ASTM C33 AND ASTM C330 RESPECTIVELY.
   - H. MAXIMUM CONCRETE SLUMP SHALL BE 4" FOR CONCRETE NOT RECEIVING HIGH-RANGE WATER REDUCING ADMIXTURES.
   - I. ALL BARS MARKED CONTINUOUS SHALL BE LAPPED AT SPLICES AND CORNERS IN ACCORDANCE WITH THE SCHEDULE SHOWN ON THESE DRAWINGS, EXCEPT AS OTHERWISE SHOWN OR REQUIRED.
   - J. WELDING OF REINFORCEMENT IS PROHIBITED U.O.N.
   - K. ALL REINFORCING BARS SHALL BE OF NEW BILLET STEEL CONFORMING TO ASTM A615, WITH THE FOLLOWING GRADE.
     #3 THROUGH #11 - GRADE 60 (FY = 60,000 PSI)
   - L. ALL HEADED STEEL STUD SHEAR REINFORCING (STUDRAILS) SHALL COMPLY WITH ASTM A1044.
   - M. PROVIDE EPOXY COATED REINFORCING STEEL (ASTM A775) WHERE INDICATED AND AT THE FOLLOWING LOCATIONS:
     1. SLAB SUPPORTING THE LOADING DOCK AND VEHICULAR ACCESS
     2. EXTERIOR GRADE LEVEL SLABS AND STAIRS
     3. SLAB SUPPORTING MEZZANINE PARKING LEVEL
   - N. VERTICAL CONSTRUCTION JOINTS USING APPROVED BULKHEADS MAY BE MADE WITHIN THE MIDDLE THIRD OF BEAM, WALL, OR SLAB SPANS WHERE STOP IN CONCRETE WORK IS NECESSARY. A PLAN SHOWING PROPOSED JOINTS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED ONLY AS SHOWN ON DRAWINGS. CONSTRUCTION JOINTS SHALL CONFORM TO ACI 318, SECTION 6.4. ALL REINFORCING STEEL SHALL BE CONTINUOUS THROUGH JOINTS U.O.N. FOR ALL CONSTRUCTION JOINTS BELOW WATER TABLE, PROVIDE WATERSTOPS.
   - O. VERTICAL JOINTS SHALL NOT BE PLACED IN CONCRETE SHEAR WALLS UNLESS SPECIFICALLY APPROVED IN WRITING BY THE ENGINEER.
   - P. ALL HORIZONTAL JOINTS IN CONCRETE POURS (WHERE SHOWN ON STRUCTURAL DRAWINGS OR EXPLICITLY APPROVED BY THE ENGINEER IN WRITING) SHALL BE RAKED TO ¼" AMPLITUDE WHILE CONCRETE IS FRESH.
   - Q. ALL CONCRETE SHALL BE MIXED, TRANSPORTED AND PLACED IN ACCORDANCE WITH ACI STANDARDS 318 AND 304.
   - R. ALL REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE TO ACI 315.
   - S. ALL WELDED WIRE MESH SHALL CONFORM TO ASTM A1064.
   - R. SYNTHETIC FIBER REINFORCEMENT SHALL BE OF MACRO SYNTHETIC "COARSE" FIBERS MADE FROM VIRGIN POLYOLEFIN, BY STRUX 90/40 BY GCP APPLIED TECHNOLOGIES (OR APPROVED EQUIVALENT), AT A MINIMUM DOSAGE RATE AS SPECIFIED ON THE DRAWINGS.
   - S. TEST CYLINDERS SHALL BE TAKEN FROM THE MIXER IN ACCORDANCE WITH ASTM C172 AND THE PROJECT SPECIFICATIONS.
   - T. STONE AGGREGATE USED IN CONCRETE MIX SHALL BE FREE OF MATERIALS WITH HARMFUL REACTIVITY TO ALKALI IN CEMENT.
   - U. THE MAXIMUM WATER SOLUBLE CHLORIDE ION (CL-) CONTENT IN CONCRETE FROM ALL INGREDIENTS SHALL BE LESS THAN 0.06% OF WEIGHT OF CEMENT, PER ASTM C1218.

2. CONCRETE FOR FOUNDATIONS
   - A. ALL VERTICAL SURFACES OF CONCRETE SHALL BE FORMED FOR WALLS, FOOTINGS, AND GRADE BEAMS.
   - B. CONTRACTOR SHALL PROVIDE A MINIMUM AREA OF STEEL REINFORCEMENT EQUAL TO .0018 TIMES THE GROSS CONCRETE AREA IN CONCRETE SLABS AND FOOTINGS.

3. CONCRETE SUPERSTRUCTURE
   - A. WHEREVER POSSIBLE, SPLICES OF MILD STEEL SHALL BE MADE IN A COMPRESSION AREA. NO MORE THAN 50% OF BARS (ALTERNATED) SHALL BE SPLICED IN A TENSION AREA.
   - B. OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL. SLEEVES, BOXES, AND OTHER OPENINGS SHALL NOT BE PERMITTED IN BEAMS, OR TWO WAY SLABS UNLESS SHOWN ON A DRAWING SUBMITTED TO AND APPROVED BY THE STRUCTURAL ENGINEER.
   - C. ALL PLUMBING SLOTS SHALL BE FILLED WITH CONCRETE TO THE SAME DEPTH AS THE FLOOR SLAB AFTER PIPING HAS BEEN INSTALLED.
   - D. PIPES OR CONDUITS PLACED IN SLABS SHALL NOT HAVE AN OUTSIDE DIAMETER LARGER THAN 1/3 THE SLAB THICKNESS AND SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTER.
   - E. NO UNDERFLOOR DUCTS SHALL BE PLACED IN SLABS WITHOUT PRIOR APPROVAL BY THE STRUCTURAL ENGINEER OR AS DETAILED AND DIMENSIONED ON DRAWINGS.
   - F. ALL BEAMS, SPANDRELS AND SLABS ARE TO BE PLACED MONOLITHICALLY WITH SUPPORTS UNLESS OTHERWISE SHOWN.
   - G. ALL EMBEDDED STEEL SHALL BE ASTM A36 OR A572. ALUMINUM INSERTS ARE NOT PERMITTED.
   - H. WHERE MASONRY ABUTS CONCRETE WALLS, PROVIDE DOVETAIL SLOTS AND MASONRY ANCHORS.
   - I. SLABS SHALL BE FINISHED BY WOOD TROWEL, FOLLOWED BY TWO STEEL TROWELING OPERATIONS, EXCEPT AS OTHERWISE SPECIFIED.
   - J. FOR CONCRETE SLABS, CONTRACTOR SHALL INCLUDE IN HIS BID SUFFICIENT QUANTITY OF CONCRETE SO THAT A LEVEL SLAB IS OBTAINED AFTER DEFLECTION OF SLAB, BEAMS, AND GIRDERS. NO CLAIMS FOR ADDITIONAL CONCRETE WILL BE ENTERTAINED. CONTRACTOR SHALL CONSIDER THE EFFECTS OF CAMBER OR SHORING, AS APPROPRIATE.

CONCRETE MASONRY UNITS (CMU)

1. ALL MASONRY WORK SHALL CONFORM TO THE REQUIREMENTS OF TMS 602-2013 SPECIFICATION FOR MASONRY STRUCTURES.
2. ALL CONCRETE MASONRY UNITS SHALL BE HOLLOW LOAD BEARING UNITS CONFORMING TO ASTM C90, GRADE N-TYPE I WITH MINIMUM COMPRESSIVE STRENGTH OF UNITS = 2000 PSI ON NET AREA, WITH ASSUMED DESIGN COMPRESSIVE STRENGTH, F'M=2000 PSI. UNITS MAY BE FABRICATED EITHER WITH NORMAL WEIGHT AGGREGATE (C33) OR LIGHTWEIGHT AGGREGATE (C331).
3. ALL UNITS SHALL BE PLACED IN RUNNING BOND.
4. MORTAR SHALL BE TYPE M OR S. MORTAR SHALL MEET ASTM C270.
5. GROUT SHALL COMPLY WITH ASTM C476. SLUMP SHALL BE 8 TO 11 INCHES, STRENGTH SHALL BE EQUAL TO 3000 PSI.
6. STORE ALL UNITS OFF GROUND TO PREVENT CONTAMINATION. COVER MATERIALS TO PROTECT FROM THE ELEMENTS.
7. NO AIR-ENTRAINING ADMIXTURES OR ANTIFREEZE COMPOUNDS, SUCH AS CALCIUM CHLORIDE SHALL BE ADDED TO MORTAR.
8. ALL WALLS OR PILASTERS SUPPORTING STEEL AT BEARING PLATES SHALL BE GROUTED SOLID FOR FOUR COURSES IN DEPTH FOR A WIDTH OF 32".
9. DO NOT BACKFILL AGAINST FOUNDATION WALLS UNTIL MORTAR HAS ATTAINED MAXIMUM STRENGTH. WHERE BACKFILL IS PLACED AGAINST FOUNDATION WALLS BEFORE FLOOR CONSTRUCTION IS IN PLACE, PROVIDE TEMPORARY BRACING.
10. THE FIRST BLOCK COURSE ON FOOTING SHALL BE FILLED SOLID WITH CONCRETE, UNLESS OTHERWISE NOTED ON DRAWINGS.
11. VERTICAL CONTROL JOINTS SHALL BE PLACED SUCH THAT THE RATIO OF JOINT SPACING (S) DIVIDED BY WALL HEIGHT (H) DOES NOT EXCEED 1.5. IN NO CASE SHALL SPACING EXCEED 25 FT. CONTROL JOINTS SHALL BE CONSTRUCTED USING SASH BLOCKS AND DUR-O-WAL PREFORMED REGULAR RAPID CONTROL JOINT (OR EQUAL OF EXTRUDED RUBBER.) VERTICAL JOINTS SHALL BE LOCATED AS FOLLOWS:
    - A. CHANGES IN WALL HEIGHT OR THICKNESS.
    - B. AT CONSTRUCTION JOINTS IN FOUNDATION, IN ROOF, AND IN FLOORS.
    - C. AT CHASES AND RECESSES FOR PIPING, COLUMNS, FIXTURES, ETC.
    - D. AT ABUTMENT OF WALL AND COLUMNS.
    - E. WITHIN S/2 IF CORNERS OF WALLS OR INTERSECTIONS.
    - F. NO CLOSER THAN 2'-0" TO EDGE OF ANY OPENING IN WALL.
12. CMU WALLS SHALL BE REINFORCED WITH 3/16" DIA. TRUSS TYPE LADDER REINFORCING ASTM A82 WIRE, HOT DIPPED GALVANIZED, AT 16" ON CENTER (VERTICALLY), AND AT THE FIRST AND SECOND BED JOINTS ABOVE AND BELOW WALL OPENINGS.
13. ALL MASONRY WALLS SHALL BE ADEQUATELY BRACED DURING CONSTRUCTION TO RESIST WIND LOADS OF 25 PSF. NOTE THAT FLOOR AND ROOF DIAPHRAGMS WILL PROVIDE ULTIMATE STABILITY FOR WALLS. MASONRY WALLS SHALL NOT BE BUILT HIGHER THAN 10 TIMES THEIR THICKNESS WITHOUT BRACING.
14. ALL CMU CORES WITH VERTICAL REINFORCEMENT MUST BE FULLY GROUTED.
15. LINTELS (UNLESS OTHERWISE NOTED ON THE PLANS)
    - A. STEEL LINTELS ALONG EXTERIOR FACE OF BUILDING SHALL BE HOT DIP GALVANIZED.
    - B. STEEL LINTELS SHALL BE REQUIRED AT OPENINGS IN MASONRY WALLS. SEE TYPICAL MASONRY DETAILS FOR FURTHER INFORMATION.

POST INSTALLED ANCHORAGES:

1. POST-INSTALLED ANCHORS SHALL BE INSTALLED INTO SOUND CONCRETE AND MASONRY AND INSPECTED IN ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS, THE BUILDING CODE AND ACI 318, SECTION D.9.
2. POST-INSTALLED ANCHORS MUST BE INSTALLED BY AN ACI/CRSI CERTIFIED INSTALLER (OR APPROVED EQUAL CERTIFICATION), THE EOR MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
3. POST-INSTALLED ANCHORS SHALL MEET THE TESTING REQUIREMENTS OF ACI 355.2 FOR MECHANICAL ANCHORS AND 355.4 FOR ADHESIVE ANCHORS AND SHALL BE ICC CERTIFIED.
4. ALL ADHESIVE ANCHOR INSTALLATIONS SHALL USE A PISTON PLUG TO ENSURE COMPLETE FILLING OF DRILL HOLE.

STRUCTURAL STEEL:

1. GENERAL
   - A. STEEL CONSTRUCTION SHALL CONFORM TO AISC "STEEL CONSTRUCTION MANUAL", 15TH EDITION, AND SHALL BE FABRICATED AND INSTALLED IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AS ADOPTED MARCH 18, 2005.
   - B. MATERIALS FOR STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS, U.O.N.:

SHAPE	ASTM	MIN YIELD STRENGTH
WIDE FLANGE SHAPE	A992	50KSI
CHANNELS, ANGLES, M, S-SHAPES	A36	36KSI
PLATE, BAR, AND MISC. STEEL	A36	36KSI
RECTANGULAR HOLLOW STRUCTURAL SECTIONS	A1085	50KSI OR
	A500, GR. C	50KSI
ROUND HOLLOW STRUCTURAL SECTIONS	A1085	50KSI OR
	A500, GR. C	46KSI
STEEL PIPE (TYPE E OR S)	A53, GR. B	35KSI

   - C. ALL BOLTED CONNECTIONS SHALL BE MADE USING ASTM F3125 GR. A325/A490 (CONVENTIONAL) BOLTS, OR F1852/F2280 (TWIST OFF TYPE TENSION-CONTROL) BOLTS, 3/4" MIN. DIAMETER. ALL LATERAL SYSTEM (BRACING AND MOMENT FRAME) BOLTED CONNECTIONS SHALL BE PRETENSIONED FOR STANDARD BOLT HOLES AND SLIP-CRITICAL FOR OVERSIZED OR SLOTTED BOLT HOLES (IN THE DIRECTION OF THE LOAD). ALL BOLTED CONNECTIONS SHALL BE INSTALLED IN ACCORDANCE WITH THE AISC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, UON.
   - D. BOLTED CONNECTIONS DESIGNATED AS PRETENSIONED OR SLIP CRITICAL SHALL BE PRETENSIONED AND INSPECTED AS PER THE "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
   - E. ANCHOR RODS SHALL BE OF ASTM F1554 GRADE 55 "WELDABLE" AS PER THE 'S1' SUPPLEMENT.
   - F. SHOP AND ERECTION DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. NO FABRICATION OF STEEL SHALL COMMENCE WITHOUT APPROVED SHOP DRAWINGS. SHOP DRAWINGS ARE PREPARED AND USED BY THE CONTRACTOR AS INSTRUMENTS TO SEQUENCE HIS WORK AND TO FACILITATE FABRICATION AND ERECTION. REVIEW OF SHOP DRAWINGS SHALL BE FOR GENERAL DETAIL AND ARRANGEMENT ONLY. CONTRACTOR SHALL BEAR FULL RESPONSIBILITY FOR DIMENSIONS, PROPER FIT AND DETAILED DESIGN OF CONNECTIONS. THEIR APPROVAL BY THE STRUCTURAL ENGINEER IS NOT TO BE CONSTRUED AS A WAIVER OF CONSTRUCTION CONTRACT REQUIREMENTS OR RESPONSIBILITIES, UNLESS THE CONTRACTOR HAS BEEN GRANTED A DEVIATION IN WRITING.
   - G. ALL SHEAR CONNECTIONS SHALL BE DESIGNED AND DETAILED FOR SERVICE LEVEL (ASD) VALUES INDICATED ON PLANS. IF NO VALUES ARE GIVEN ON PLANS, SEE TYPICAL CONNECTION DETAILS FOR DESIGN FORCES. ALL BEAM WEB SHEAR CONNECTIONS SHALL BE DESIGNED AND DETAILED SO THAT THE DEPTH OF THE CONNECTION PLATES OR ANGLES ARE NOT LESS THAN ONE-HALF OF THE BEAMS "T" DIMENSION. NO CONNECTION SHALL BE DESIGNED AND DETAILED TO SUPPORT LESS THAN 6 KIPS SERVICE LEVEL REACTION. WHERE POSSIBLE, THE CONTRACTOR SHALL SELECT A SIMPLE SHEAR CONNECTION FROM THE TABLES IN PART 10 OF THE AISC STEEL CONSTRUCTION MANUAL "15TH EDITION-ASD".
   - H. THE FORCES INDICATED ON THE PLANS/ELEVATIONS ARE THE MEMBER DESIGN FORCES. LATERAL MOMENT FRAME CONNECTIONS SHALL BE DESIGNED AND DETAILED FOR THE GREATER OF THE FORCES SHOWN ON PLANS/ELEVATIONS OR AS REQUIRED BY AISC 341 FOR THE SEISMIC RESPONSE MODIFICATION FACTOR (R) AND BASIC SEISMIC FORCE RESISTING SYSTEM SHOWN IN THE GENERAL NOTES. THE CONTRACTOR SHALL PROVIDE ANY REINFORCEMENT NECESSARY (STIFFENER PLATES, DOUBLER PLATES, ETC.) REQUIRED TO CONSTRUCT THESE CONNECTIONS.
   - I. DURING ERECTION, APPROVED TEMPORARY BRACING SHALL BE INSTALLED AS REQUIRED TO PREVENT DISTORTION OR DAMAGE TO THE FRAMEWORK DUE TO ERECTION FORCES.
   - J. STEEL SHOP DRAWINGS SHALL BE COORDINATED WITH STAIR DETAILS.
   - K. PROVIDE FITTED WELDED STIFFENER PLATES 1/4" THICK MIN. IN SUPPORT BEAMS ALONGSIDE HANGER LOCATIONS, AT SUPPORT POSTS, AND AT STAIR STRINGER BEAM BEARING POINTS.
   - L. ALL EXTERIOR HSS TUBES SHALL HAVE ¼" THICK STEEL CAP PLATES WELDED ALL AROUND AT ENDS.
   - M. ALL INTERIOR STRUCTURAL STEEL SHALL BE CLEANED TO SSPC-SP3 AND SHALL RECEIVE THE FOLLOWING SHOP PRIMER, EXCEPT WHERE FIELD WELDING OR SLIP CRITICAL BOLTING IS TO BE DONE, WHERE STEEL IS TO RECEIVE SPRAY APPLIED FIREPROOFING, OR WHERE SPECIFIED TO BE GALVANIZED:
     - a. BUILDING INTERIOR STEEL: TNEMEC 10-99 PRIMER AT 2.5 MILS DFT, OR APPROVED EQUAL.
     - b. BUILDING PERIMETER STEEL: TNEMEC 394 PERIMEPRIME AT 3.0 MILS DFT, OR APPROVED EQUAL.
     - c. ALL WELDS AND BARE SPOTS SHALL RECEIVE TOUCHUP PAINT.
   - N. ALL EXTERIOR OR EXPOSED STRUCTURAL STEEL, MISCELLANEOUS COMPONENTS, AND HARDWARE SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123, EXCEPT THE FOLLOWING LOCATIONS: WHERE FIELD WELDING OR SLIP CRITICAL BOLTING IS TO BE DONE AND ON A490 BOLTS. ALL WELDS AND BARE SPOTS SHALL RECEIVE ZRC COLD GAVANIZING COMPOUND (OR APPROVED EQUIVALENT), WITH SURFACE PREPARATION AND APPLICATION IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS. ALL A490 BOLTS SHALL RECEIVE ZINC RICH/ALUMINUM COATING IN ACCORDANCE WITH ASTM F2833 GR 1 (MAGNI 565 OR APPROVED EQUIVALENT).
   - O. ALL STEEL PAINTING REQUIREMENTS SHALL BE COORDINATED WITH THE PROJECT SPECIFICATIONS AND ARCHITECTURAL DRAWINGS. PLEASE NOTIFY THE EOR FOR ANY DISCREPANCIES BETWEEN THE CONTRACT DOCUMENTS FOR STEEL PAINTING REQUIREMENTS FOR DIRECTION PRIOR TO STEEL FABRICATION.

2. WELDING
   - A. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH STANDARDS OF THE AMERICAN WELDING SOCIETY. ELECTRODES MUST MEET E70XX SERIES REQUIREMENTS, LOW HYDROGEN, WITH MINIMUM TENSILE STRESS OF 70,000 PSI. ELECTRODES SHALL BE PRODUCED AND STORED IN ACCORDANCE WITH AWS D1.1 SECTIONS 5.3.
   - B. STEEL ERECTOR SHALL PROVIDE A FIRE WATCH DURING ALL FIELD WELDING OPERATIONS.
   - C. ALL WELDERS ARE TO BE LICENSED AND CERTIFIED TO AWS STANDARDS OR THOSE REQUIRED BY APPLICABLE BUILDING CODE.
   - D. ALL WELDS SHALL BE VISUALLY INSPECTED. ALL COMPLETE JOINT PENETRATION GROOVE WELDS SHALL RECEIVE RADIOGRAPHIC OR ULTRASONIC TESTING. MAGNETIC PARTICLE TEST 20% OF ALL MULTI-PASS FILLET WELDS.
   - E. REPORTS OF EACH TEST SHALL BE GIVEN TO THE STRUCTURAL ENGINEER. NO FAILED WELD SHALL BE PERMITTED TO REMAIN IN SERVICE. IT IS THE RESPONSIBILITY OF THE TESTING LABORATORY TO PROVIDE TIMELY NOTICE OF FAILED TESTS TO THE CONTRACTOR.
   - F. WELDING SHALL PROGRESS IN A MANNER WHICH BALANCES THE STRESSES IN THE MEMBERS, IN ACCORDANCE WITH AWS.
   - G. PREHEATING REQUIREMENTS FOR BASE METAL SHALL FOLLOW AWS GUIDELINES

3. STAIRS
   - A. STAIRS SHALL BE STEEL, PAN TYPE CONCRETE TREAD, OR PRECAST CONCRETE TREAD ON STEEL RISERS, DESIGNED FOR 100 PSF LIVE LOAD.
   - B. DESIGN OF STAIRS AND LANDINGS SHALL BE THE RESPONSIBILITY OF THE FABRICATOR AND SHALL CONFORM TO OSHA SAFETY IMPROVEMENTS. SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW.
   - C. SURROUNDING STRUCTURAL FRAMING HAS BEEN DESIGNED TO CARRY STAIR LOADS BASED ON THE LOADING ASSUMPTIONS AND LOCAL REINFORCEMENT AS PER THE STRUCTURAL STEEL NOTES. ATTACHMENT POINTS OUTSIDE OF THESE REQUIREMENTS WILL REQUIRE BRACING BY THE STAIR CONTRACTOR TO RESOLVE ECCENTRIC FORCES ON THE BUILDING STRUCTURE WITH APPROVAL OF THE ARCHITECT AND ENGINEER.

4. STEEL DECK AND SHEAR STUDS
   - A. ROOF DECK SHALL BE GALVANIZED AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A653 COATING CLASS G90. UNITS SHALL BE WELDED TO JOISTS OR BEAMS WITH 5/8" PUDDLE WELDS IN A 36/4 PATTERN, U.O.N. SIDE LAPS ARE TO BE WELDED AT A MAXIMUM SPACING OF 36" ON CENTER, U.O.N. DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 2 SPANS.
   - B. FLOOR DECK SHALL BE GALVANIZED AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A653 COATING CLASS G60. UNITS SHALL BE WELDED TO JOISTS OR BEAMS WITH 5/8" PUDDLE WELDS A 36/4 PATTERN. SIDE LAPS ARE TO BE WELDED AT A MAXIMUM SPACING OF 36" ON CENTER, U.O.N. DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 2 SPANS.
   - C. AS AN ALTERNATE TO PUDDLE WELDING OF STEEL DECKING, HILTI X-HSN OR X-ENP 19 POWDER ACTUATED FASTENERS (PAFS) WITH EQUIVALENT OR GREATER CAPACITY TO SPECIFIED ATTACHMENT CAN BE INSTALLED, IF PAFS ARE USED, USE HILTI S-SLC SIDELAP CONNECTORS. THE CONTRACTOR SHALL SUBMIT ALTERNATIVE FASTENING PATTERN TO THE ENGINEER OF RECORD FOR APPROVAL PRIOR TO INSTALLATION.
   - D. PAFS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. CONTRACTOR SHALL BE CERTIFIED AND TRAINED BY THE MANUFACTURER'S REPRESENTATIVE FOR PROPER USE AND INSTALLATION OF PAFS.
   - E. SHEAR CONNECTORS SHALL BE HEADED STUD TYPE, ASTM A108 GRADE 1015 OR 1020 COLD FINISHED CARBON STEEL. PROVIDE STUDS OF THE SIZE AND SPACING AS SPECIFIED ON DRAWINGS OR A MAXIMUM SPACING OF 1'-0" O.C. EVEN IF THIS EXCEEDS THE NUMBER CALLED FOR ON THE DRAWINGS.
   - F. THE DECK SHALL MEET THE FOLLOWING MINIMUM DESIGN PROPERTIES:

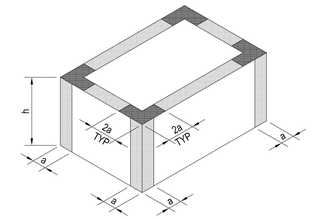
MINIMUM STEEL DECK PROPERTIES SCHEDULE

DECK TYPE	GAUGE	Ip	Sp	In	Sn	Fy
	(in)	(in^4/FT)	(in^3/FT)	(in^4/FT)	(in^3/FT)	ksi
ROOF - 1.5B	22	0.155	0.186	0.183	0.192	33
	20	0.201	0.234	0.222	0.247	33
	18	0.289	0.318	0.295	0.327	33
	16	0.373	0.408	0.373	0.411	33

COMPONENT AND CLADDING WIND LOADS

POSITIVE WALL LOADS (PSF)

HEIGHT ABOVE GROUND, Z	ZONE 4 & 5		
	AREA (FT^2) ≤20	AREA (FT^2) =100	AREA (FT^2) >500
15	22.8	19.7	16.7
20	22.8	19.7	16.7
30	22.8	19.7	16.7
40	24.3	21	17.8
50	25.6	22.1	18.6
60	26.7	23.1	19.4
70	27.7	23.9	20.0



INTERIOR ZONES ROOFS-ZONE 1/WALLS-ZONE 4

END ZONES ROOFS-ZONE 2/WALLS-ZONE 5

CORNER ZONES ROOFS-ZONE 3

NOTES:
1) a INDICATES 10% OF LEAST HORIZONTAL DIMENSION, BUT NOT LESS THAN 3FT.
2) h INDICATES MEAN ROOF HEIGHT, IN FEET.
3) WHERE ROOF PARAPETS ARE PROVIDED THAT ARE >= 3'-0", ZONE 3 PRESSURES SHALL BE REDUCED TO ZONE 2 PRESSURES.
4) + AND - INDICATE PRESSURE ACTING TOWARD AND AWAY FROM THE SURFACE, RESPECTIVELY.
5) WHERE PROVIDED AREAS OR BUILDING HEIGHTS FALL BETWEEN PROVIDED VALUES, INTERPOLATE.

NEGATIVE WALL LOADS (PSF)

ZONE 4			ZONE 5		
AREA (FT^2) ≤20	AREA (FT^2) =100	AREA (FT^2) >500	AREA (FT^2) ≤20	AREA (FT^2) =100	AREA (FT^2) >500
-27.7	-25.2	-22.6	-50.9	-40.6	-30.3

ROOF LOADS (PSF)

ZONE 1			ZONE 2			ZONE 3		
AREA (FT^2) ≤10	AREA (FT^2) =100	AREA (FT^2) >500	AREA (FT^2) ≤10	AREA (FT^2) =100	AREA (FT^2) >500	AREA (FT^2) ≤10	AREA (FT^2) =100	AREA (FT^2) >500
-38.3	-33.1	-27.7	-60.6	-53.2	-45.7	-82.7	-73.2	-63.7

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1	ISSUE FOR BID	04/23/2021

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136-158 WESTMORELAND AVE.
WHITE PLAINS, NY 10606

Owner/Developer:
136-158 WESTMORELAND, LLC
1485 5TH AVENUE, 24F
NEW YORK, NY 10035

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McLaren Engineering Group
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Khachaturian Engineering Associates
Mechanical/Electrical/Plumbing Engineers
186 Wood Avenue South, First Floor
Iselin, NJ 08830
732 635–0044

Sheet Title:
GENERAL NOTES II

Seal & Signature

Date: 01-27-2021
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Sheet Title: S-002

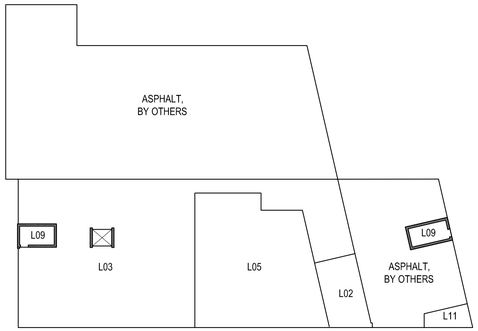
NORTH

DEAD AND LIVE LOAD SCHEDULE (PSF)

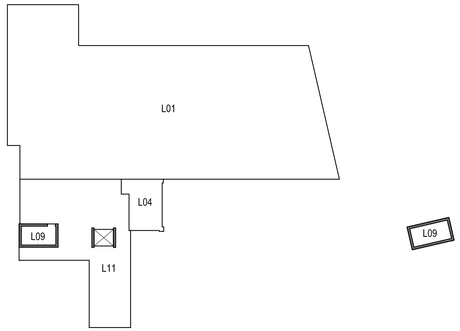
USE	MARK / AREA	L01 PARKING	L02 LOADING AREA	L03 LOBBY, RETAIL	L04 STORAGE	L05 INDUSTRIAL	L06 PUBLIC AREA / AMENITY	L07 RESIDENTIAL / RESIDENTIAL CORRIDORS	L08 BALCONY	L09 STAIRS	L10 EXTERIOR AMENITY AREA	L11 MECHANICAL	L12 ROOF
SUPERPOSED DEAD LOADS (SDL)	PARTITIONS	-	-	-	-	-	-	15	-	-	-	-	-
	CEILING LOADS (HUNG & MEP)	5	-	-	10	-	10	-	5	-	5	10	10
	FLOOR FINISHES	-	20	5	5	15	5	5	15	30	15	5	-
	ROOFING & INSULATION	-	-	-	-	-	-	-	5	-	5	-	5
	TOTAL SDL	5	20	5	15	15	15	20	25	30	25	15	15
	LIVE LOAD	40	250 (NOTE 3)	150	125	250	100	40	60	100	100	150	30
	FLAT ROOF SNOW LOAD	20	-	-	-	-	-	-	20	-	20	-	20

NOTES:

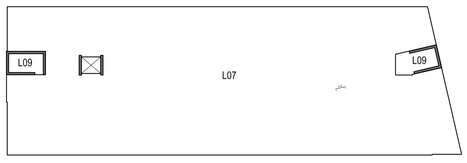
1. SUPERIMPOSED DEAD LOADS SHOWN DO NOT INCLUDE SELF-WEIGHT OF STRUCTURE.
2. LIVE LOADS SHOWN ARE UNREDUCED. LIVE LOAD MAY BE REDUCED AS ALLOWED BY BUILDING CODE.
3. LOADING AREA HAS BEEN DESIGNED FOR MAX TRUCK AXLE LOAD OF 10k (5k WHEEL LOAD).



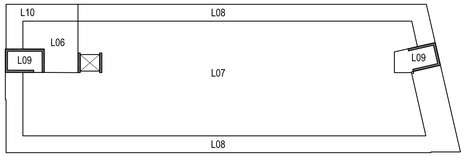
A S-010 FIRST FLOOR LOAD MAP
1/32" = 1'-0"



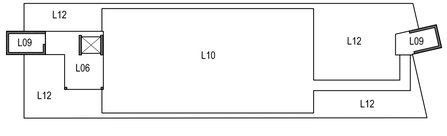
B S-010 MEZZANINE FLOOR LOAD MAP
1/32" = 1'-0"



C S-010 2ND TO 4TH FLOOR LOAD MAP
1/32" = 1'-0"



D S-010 FIFTH FLOOR LOAD MAP
1/32" = 1'-0"



E S-010 ROOF FLOOR LOAD MAP
1/32" = 1'-0"

F S-010 BULKHEAD FLOOR LOAD MAP
1/32" = 1'-0"

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New York, NY 10001
212 324-6300

Khachaturian Engineering Associates
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732 635-0044

Sheet Title:
LOAD MAPS

Seal & Signature

Date: 01-27-2021
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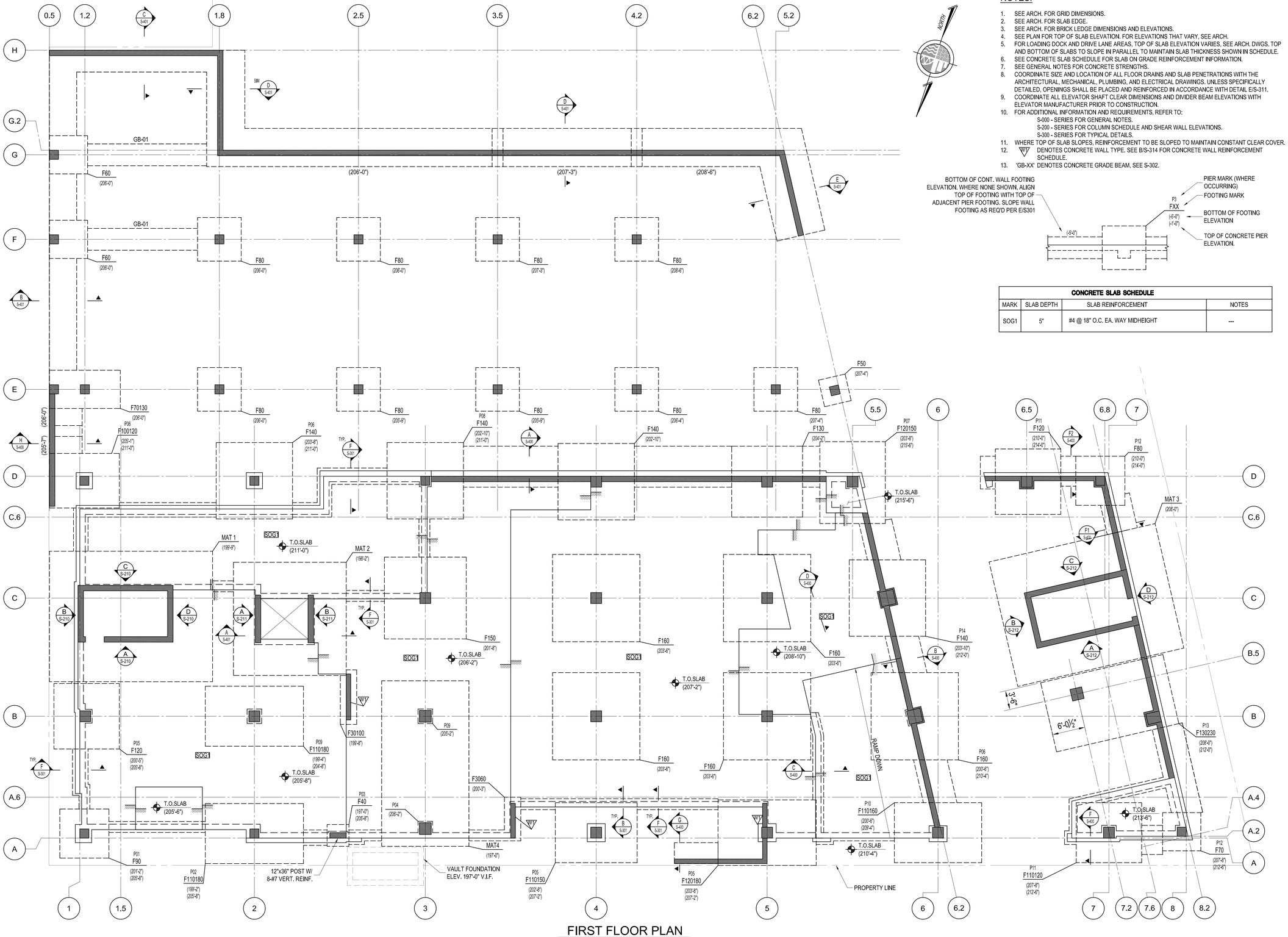

FIRST FLOOR PLAN

1/8" = 1'-0"

NOTES:

1. SEE ARCH. FOR GRID DIMENSIONS.
2. SEE ARCH. FOR SLAB EDGE.
3. SEE ARCH. FOR BRICK LEDGE DIMENSIONS AND ELEVATIONS.
4. SEE PLAN FOR TOP OF SLAB ELEVATION. FOR ELEVATIONS THAT VARY, SEE ARCH.
5. FOR LOADING DOCK AND DRIVE LANE AREAS, TOP OF SLAB ELEVATION VARIES, SEE ARCH. DWGS. TOP AND BOTTOM OF SLABS TO SLOPE IN PARALLEL TO MAINTAIN SLAB THICKNESS SHOWN IN SCHEDULE.
6. SEE CONCRETE SLAB SCHEDULE FOR SLAB ON GRADE REINFORCEMENT INFORMATION.
7. SEE GENERAL NOTES FOR CONCRETE STRENGTHS.
8. COORDINATE SIZE AND LOCATION OF ALL FLOOR DRAINS AND SLAB PENETRATIONS WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. UNLESS SPECIFICALLY DETAILED, OPENINGS SHALL BE PLACED AND REINFORCED IN ACCORDANCE WITH DETAIL E/S-311.
9. COORDINATE ALL ELEVATOR SHAFT CLEAR DIMENSIONS AND DIVIDER BEAM ELEVATIONS WITH ELEVATOR MANUFACTURER PRIOR TO CONSTRUCTION.
10. FOR ADDITIONAL INFORMATION AND REQUIREMENTS, REFER TO:
    S-000 - SERIES FOR GENERAL NOTES.
    S-200 - SERIES FOR COLUMN SCHEDULE AND SHEAR WALL ELEVATIONS.
    S-300 - SERIES FOR TYPICAL DETAILS.
11. WHERE TOP OF SLAB SLOPES, REINFORCEMENT TO BE SLOPED TO MAINTAIN CONSTANT CLEAR COVER.
12. ▽ DENOTES CONCRETE WALL TYPE. SEE B/S-314 FOR CONCRETE WALL REINFORCEMENT SCHEDULE.
13. 'GB-XX' DENOTES CONCRETE GRADE BEAM, SEE S-302.

CONCRETE SLAB SCHEDULE

MARK	SLAB DEPTH	SLAB REINFORCEMENT	NOTES
SOG1	5"	#4 @ 18" O.C. EA. WAY MIDHEIGHT	---

ISSUE FOR BID
APR 23, 2021

Rev. #	Revision Description	Date:
1	ISSUE FOR BID	04/23/2021

Project Description:
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WESTMORELAND LOFTS
136-158 WESTMORELAND AVE.
WHITE PLAINS, NY 10606

Owner/Developer:
136-158 WESTMORELAND, LLC
1485 5TH AVENUE, 24F
NEW YORK, NY 10035

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212 324-6300

Khachaturian Engineering Associates
Mechanical/Electrical/Plumbing Engineers
186 Wood Avenue South, First Floor
Iselin, NJ 08830
732 635-0044

Sheet Title:
FIRST FLOOR PLAN

Seal & Signature

Date: 01-27-2021
Scale: AS NOTED
Job#: 161162.00
Sheet Title: S-101

NOTES:

1. SEE ARCH. FOR GRID DIMENSIONS.
2. SEE ARCH. FOR SLAB EDGE.
3. SEE PLAN FOR TOP OF SLAB ELEVATION. FOR SLOPING AREAS AT PARKING, TOP AND BOTTOM OF SLABS TO SLOPE IN PARALLEL TO MAINTAIN SLAB THICKNESS SHOWN IN SCHEDULE.
4. SEE CONCRETE SLAB SCHEDULE FOR TWO WAY FLAT PLATE SIZE. SEE GENERAL NOTES FOR CONCRETE STRENGTHS AND ADDITIONAL REQUIREMENTS. SEE TYPICAL TWO-WAY SLAB DETAIL FOR BAR LENGTH UNLESS OTHERWISE NOTED.
5. QUANTITIES OF REINFORCING BARS IN FLAT PLATE SHALL BE AS SHOWN IN COLUMN AND MIDDLE STRIPS. SEE S-315 FOR COLUMN AND MIDDLE STRIPS WIDTHS.
6. SEE TYPICAL BAR LENGTH DETAILS ON SHEET S-311 FOR LENGTH OF BARS IN COLUMN AND MIDDLE STRIPS.
7. 'B-XXX' INDICATES REINFORCED CONCRETE BEAM. SEE CONCRETE BEAM SCHEDULE ON C/S-313 FOR SIZE AND REINFORCING OF CONCRETE BEAMS.
8. COORDINATE SIZE AND LOCATION OF ALL FLOOR DRAINS AND SLAB PENETRATIONS WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. UNLESS SPECIFICALLY DETAILED, OPENINGS SHALL BE PLACED AND REINFORCED IN ACCORDANCE WITH DETAIL E/S-311.
9. COORDINATE ALL ELEVATOR SHAFT CLEAR DIMENSIONS AND DIVIDER BEAM ELEVATIONS WITH ELEVATOR MANUFACTURER PRIOR TO CONSTRUCTION.
10. FOR ADDITIONAL INFORMATION AND REQUIREMENTS, REFER TO:
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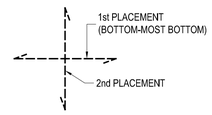
    - S-200 - SERIES FOR COLUMN SCHEDULE AND SHEAR WALL ELEVATIONS.
    - S-300 - SERIES FOR TYPICAL DETAILS.
11. WHERE TOP OF SLAB SLOPES, REINFORCEMENT TO SLOPE TO MAINTAIN CONSTANT CLEAR COVER.
12. 'SRX' INDICATES STUDRAIL REINFORCING REQUIRED, REFER TO DETAIL D/S-310.

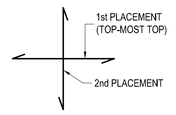
CONCRETE SLAB SCHEDULE

MARK	SLAB DEPTH	SLAB REINFORCEMENT	NOTES
S1	12"	#5 @ 12" O.C. EA. WAY BOT #5 @ 12" OC MIDDLE STRIP TOP BARS U.N.O.	SEE PLAN FOR TOP AND ADD'L BOT. REINF.
S2	10"	#5 @ 12" O.C. EA. WAY BOT #5 @ 12" OC MIDDLE STRIP TOP BARS U.N.O.	SEE PLAN FOR TOP AND ADD'L BOT. REINF.
S6	8"	#4@12" O.C. EA. WAY, TOP & BOT.	OUTERMOST BARS TO BE IN THE N-S DIRECTION

SLAB REINFORCING PLACEMENT ORDER



BOTTOM BARS (UON)



TOP BARS (UON)

MEZZANINE FLOOR PLAN
1/8" = 1'-0"

ISSUE FOR BID
APR 23, 2021

Rev. #	Revision Description	Date:
1	ISSUE FOR BID	04/23/2021

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Khachaturian Engineering Associates
Mechanical/Electrical/Plumbing Engineers
186 Wood Avenue South, First Floor
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732 635-0044

Sheet Title:
MEZZANINE FLOOR PLAN

Seal & Signature

Date: 01-27-2021
Scale: AS NOTED
Job#: 161162.00
Sheet Title: S-102

NORTH

NOTES:

1. SEE ARCH. FOR GRID DIMENSIONS.
2. SEE ARCH. FOR SLAB EDGE.
3. SEE PLAN FOR TOP OF SLAB ELEVATION.
4. SEE CONCRETE SLAB SCHEDULE FOR TWO WAY FLAT PLATE SIZE. SEE GENERAL NOTES FOR CONCRETE STRENGTHS AND ADDITIONAL REQUIREMENTS. SEE TYPICAL TWO-WAY SLAB DETAIL FOR BAR LENGTH UNLESS OTHERWISE NOTED.
5. QUANTITIES OF REINFORCING BARS IN FLAT PLATE SHALL BE AS SHOWN IN COLUMN AND MIDDLE STRIPS. SEE S-315 FOR COLUMN AND MIDDLE STRIPS WIDTHS.
6. SEE TYPICAL BAR LENGTH DETAILS ON SHEET S-311 FOR LENGTH OF BARS IN COLUMN AND MIDDLE STRIPS.
7. 'B-XXX' INDICATES REINFORCED CONCRETE BEAM. SEE CONCRETE BEAM SCHEDULE ON C/S-313 FOR SIZE AND REINFORCING OF CONCRETE BEAMS.
8. COORDINATE SIZE AND LOCATION OF ALL FLOOR DRAINS AND SLAB PENETRATIONS WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. UNLESS SPECIFICALLY DETAILED, OPENINGS SHALL BE PLACED AND REINFORCED IN ACCORDANCE WITH DETAIL E/S-311.
9. COORDINATE ALL ELEVATOR SHAFT CLEAR DIMENSIONS AND DIVIDER BEAM ELEVATIONS WITH ELEVATOR MANUFACTURER PRIOR TO CONSTRUCTION.
10. FOR ADDITIONAL INFORMATION AND REQUIREMENTS, REFER TO:
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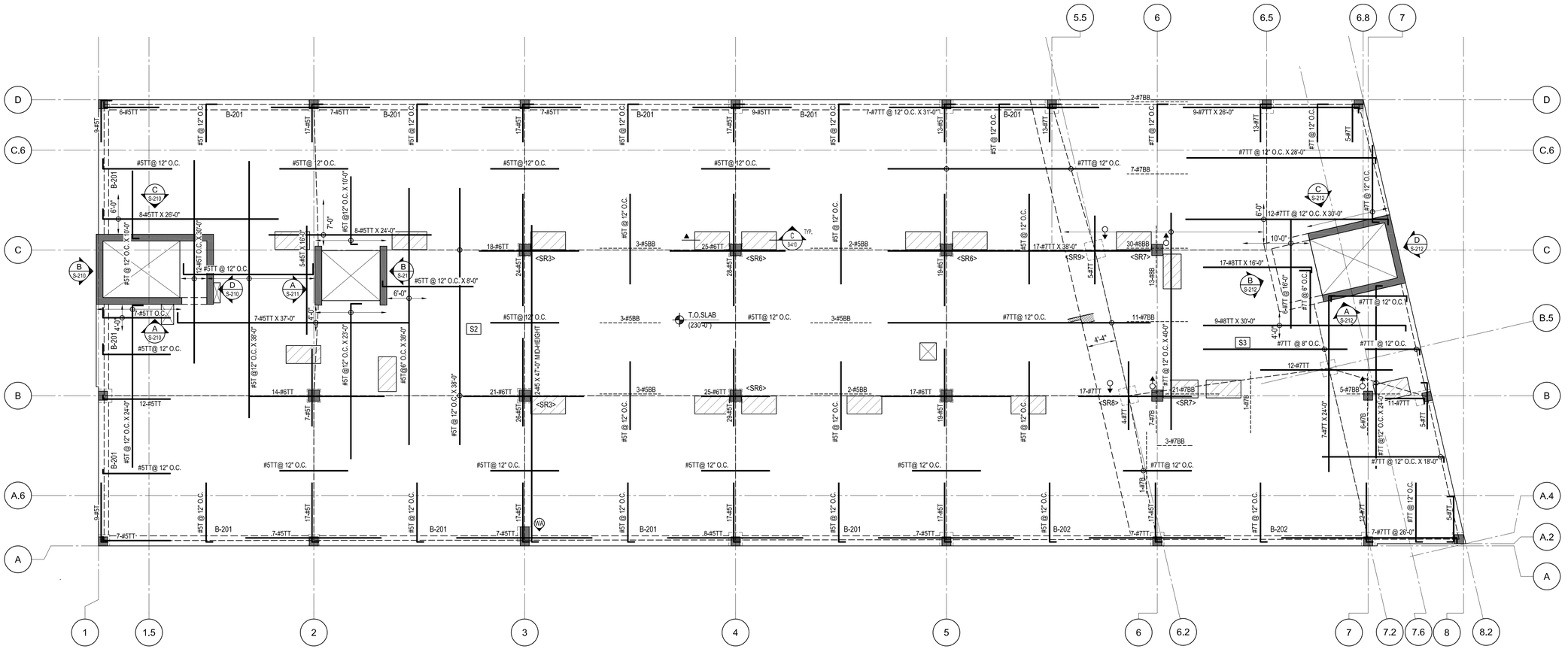
    S-200 - SERIES FOR COLUMN SCHEDULE AND SHEAR WALL ELEVATIONS.
    S-300 - SERIES FOR TYPICAL DETAILS.
11. 'SRX' INDICATES STUDRAIL REINFORCING REQUIRED, REFER TO DETAIL D/S-310.
12. ▨ DENOTES AREAS WITH A 2" SLAB DEPRESSION.
13. LB-XX DENOTES CONCRETE LINK BEAM, SEE J/S-312 AND SHEAR WALL ELEVATIONS.

CONCRETE SLAB SCHEDULE

MARK	SLAB DEPTH	SLAB REINFORCEMENT	NOTES
S2	10"	#5 @ 12" O.C. EA. WAY BOT #5 @ 12" OC MIDDLE STRIP TOP BARS U.N.O.	SEE PLAN FOR TOP AND ADD'L BOT. REINF.
S3	24"	#7 @ 12" O.C. EA. WAY BOT #7 @ 12" OC MIDDLE STRIP TOP BARS U.N.O.	SEE PLAN FOR TOP AND ADD'L BOT. REINF.

SLAB REINFORCING PLACEMENT ORDER

BOTTOM BARS (UON) TOP BARS (UON)



SECOND FLOOR PLAN

1/8" = 1'-0"

ISSUE FOR BID
APR 23, 2021

Rev. #	Revision Description	Date:
1	ISSUE FOR BID	04/23/2021

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Sheet Title:
SECOND FLOOR PLAN

Seal & Signature

Date: 01-27-2021
Scale: AS NOTED
Job#: 161162.00
Sheet Title: S-103

NOTES:

1. SEE ARCH. FOR GRID DIMENSIONS.
2. SEE ARCH. FOR SLAB EDGE.
3. SEE PLAN FOR TOP OF SLAB ELEVATION.
4. SEE CONCRETE SLAB SCHEDULE FOR TWO WAY FLAT PLATE SIZE. SEE GENERAL NOTES FOR CONCRETE STRENGTHS AND ADDITIONAL REQUIREMENTS. SEE TYPICAL TWO-WAY SLAB DETAIL FOR BAR LENGTH UNLESS OTHERWISE NOTED.
5. QUANTITIES OF REINFORCING BARS IN FLAT PLATE SHALL BE AS SHOWN IN COLUMN AND MIDDLE STRIPS. SEE S-315 FOR COLUMN AND MIDDLE STRIPS WIDTHS.
6. SEE TYPICAL BAR LENGTH DETAILS ON SHEET S-311 FOR LENGTH OF BARS IN COLUMN AND MIDDLE STRIPS.
7. 'B-XXX' INDICATES REINFORCED CONCRETE BEAM. SEE CONCRETE BEAM SCHEDULE ON C/S-313 FOR SIZE AND REINFORCING OF CONCRETE BEAMS.
8. COORDINATE SIZE AND LOCATION OF ALL FLOOR DRAINS AND SLAB PENETRATIONS WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. UNLESS SPECIFICALLY DETAILED, OPENINGS SHALL BE PLACED AND REINFORCED IN ACCORDANCE WITH DETAIL E/S-311.
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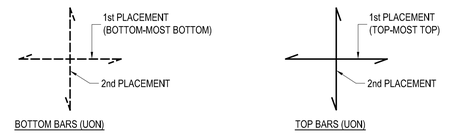
    S-300 - SERIES FOR TYPICAL DETAILS.
11. 'SRX' INDICATES STUDRAIL REINFORCING REQUIRED, REFER TO DETAIL D/S-310.
12. ▨ DENOTES AREAS WITH A 2" SLAB DEPRESSION.
13. LB-XX DENOTES CONCRETE LINK BEAM, SEE J/S-312 AND SHEAR WALL ELEVATIONS.

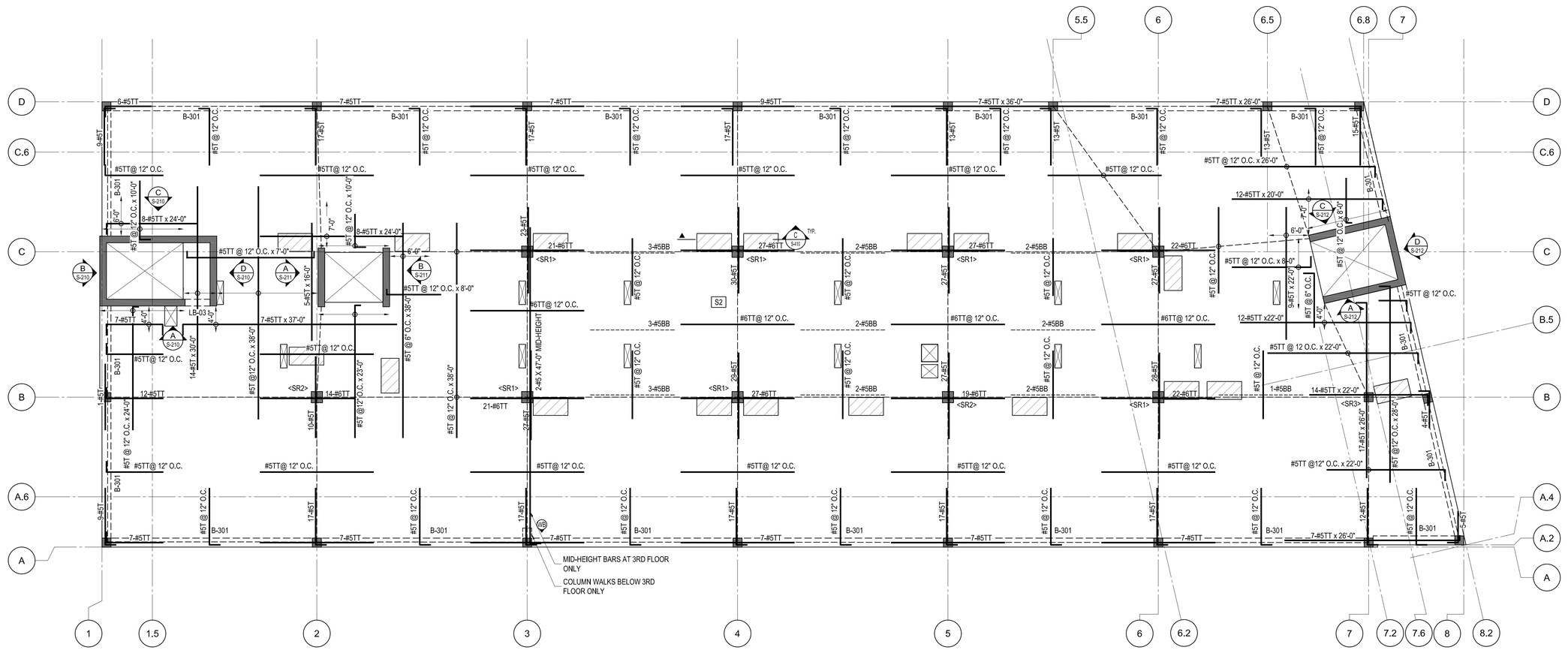
TOP OF SLAB ELEVATION	
FLOOR	ELEVATION
3RD	+ 240'-4"
4TH	+ 250'-8"

CONCRETE SLAB SCHEDULE

MARK	SLAB DEPTH	SLAB REINFORCEMENT	NOTES
S2	10"	#5 @ 12" O.C. EA. WAY BOT #5 @ 12" OC MIDDLE STRIP TOP BARS U.N.O.	SEE PLAN FOR TOP AND ADD'L BOT. REINF.

SLAB REINFORCING PLACEMENT ORDER





THIRD AND FOURTH FLOOR PLAN

1/8" = 1'-0"

ISSUE FOR BID
APR 23, 2021

Rev. #	Revision Description	Date:
1	ISSUE FOR BID	04/23/2021

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186 Wood Avenue South, First Floor
Iselin, NJ 08830
732 635–0044

Sheet Title:
THIRD AND FOURTH FLOOR PLAN

Seal & Signature

Date: 01-27-2021
Scale: AS NOTED
Job#: 161162.00
Sheet Title: S-104

NORTH

NOTES:

1. SEE ARCH. FOR GRID DIMENSIONS.
2. SEE ARCH. FOR SLAB EDGE.
3. SEE PLAN FOR TOP OF SLAB ELEVATION.
4. SEE CONCRETE SLAB SCHEDULE FOR TWO WAY FLAT PLATE SIZE. SEE GENERAL NOTES FOR CONCRETE STRENGTHS AND ADDITIONAL REQUIREMENTS. SEE TYPICAL TWO-WAY SLAB DETAIL FOR BAR LENGTH UNLESS OTHERWISE NOTED.
5. QUANTITIES OF REINFORCING BARS IN FLAT PLATE SHALL BE AS SHOWN IN COLUMN AND MIDDLE STRIPS. SEE S-315 FOR COLUMN AND MIDDLE STRIPS WIDTHS.
6. SEE TYPICAL BAR LENGTH DETAILS ON SHEET S-XXX FOR LENGTH OF BARS IN COLUMN AND MIDDLE STRIPS.
7. 'B-XXX' INDICATES REINFORCED CONCRETE BEAM. SEE CONCRETE BEAM SCHEDULE ON C/S-313 FOR SIZE AND REINFORCING OF CONCRETE BEAMS.
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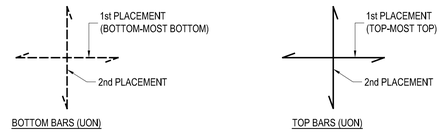
11. 'SRX' INDICATES STUDRAIL REINFORCING REQUIRED, REFER TO DETAIL D/S-310.
12. ▨ DENOTES AREAS WITH A 2" SLAB DEPRESSION.
13. LB-XX DENOTES CONCRETE LINK BEAM, SEE J/S-312 AND SHEAR WALL ELEVATIONS.

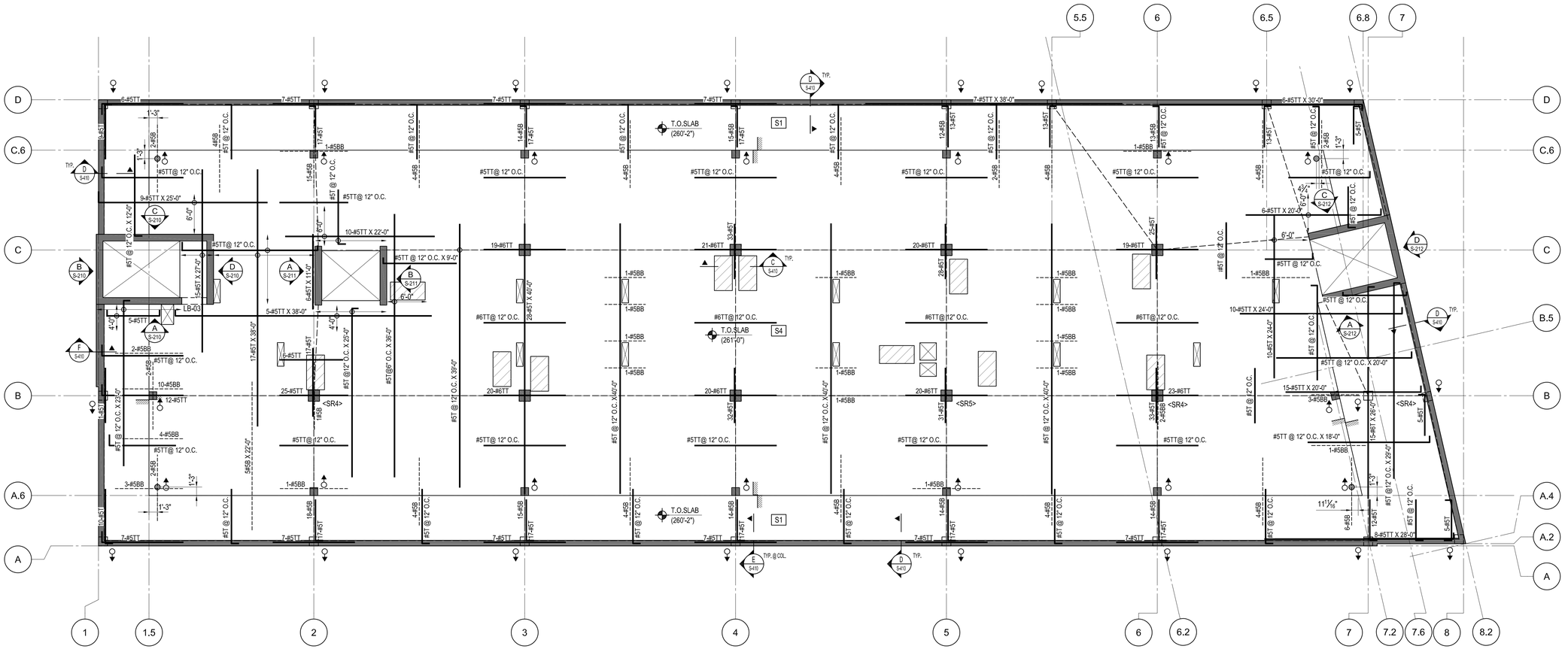
CONCRETE SLAB SCHEDULE

MARK	SLAB DEPTH	SLAB REINFORCEMENT	NOTES
S1	12"	#5 @ 12" O.C. EA. WAY BOT #5 @ 12" OC MIDDLE STRIP TOP BARS U.N.O.	SEE PLAN FOR TOP AND ADD'L BOT. REINF.
S4	14"	#5 @ 12" O.C. EA. WAY BOT #5 @ 12" OC MIDDLE STRIP TOP BARS U.N.O.	SEE PLAN FOR TOP AND ADD'L BOT. REINF.

SLAB REINFORCING PLACEMENT ORDER



BOTTOM BARS (UON) TOP BARS (UON)



FIFTH FLOOR PLAN

1/8" = 1'-0"

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1	ISSUE FOR BID	04/23/2021

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Khachaturian Engineering Associates
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186 Wood Avenue South, First Floor
Iselin, NJ 08830
732 635-0044

Sheet Title:
FIFTH FLOOR PLAN

Seal & Signature

Date:	01-27-2021
Scale:	AS NOTED
Job#:	161162.00
Sheet Title:	S-105

NORTH

NOTES:

1. SEE ARCH. FOR GRID DIMENSIONS.
2. SEE ARCH. FOR SLAB EDGE.
3. SEE ARCH. FOR STEEL POST LOCATIONS.
4. SEE PLAN FOR TOP OF SLAB ELEVATION.
5. SEE CONCRETE SLAB SCHEDULE FOR TWO WAY FLAT PLATE SIZE. SEE GENERAL NOTES FOR CONCRETE STRENGTHS AND ADDITIONAL REQUIREMENTS. SEE TYPICAL TWO-WAY SLAB DETAIL FOR BAR LENGTH UNLESS OTHERWISE NOTED.
6. QUANTITIES OF REINFORCING BARS IN FLAT PLATE SHALL BE AS SHOWN IN COLUMN AND MIDDLE STRIPS. SEE S-315 FOR COLUMN AND MIDDLE STRIPS WIDTHS.
7. SEE TYPICAL BAR LENGTH DETAILS ON SHEET S-311 FOR LENGTH OF BARS IN COLUMN AND MIDDLE STRIPS.
8. 'B-XXX' INDICATES REINFORCED CONCRETE BEAM. SEE CONCRETE BEAM SCHEDULE ON C/S-313 FOR SIZE AND REINFORCING OF CONCRETE BEAMS.
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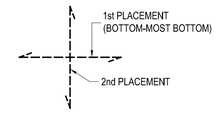
13. LB-XX DENOTES CONCRETE LINK BEAM, SEE J/S-312 AND SHEAR WALL ELEVATIONS.

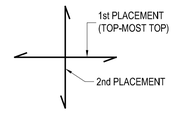
CONCRETE SLAB SCHEDULE

MARK	SLAB DEPTH	SLAB REINFORCEMENT	NOTES
S4	14"	#5 @ 12" O.C. EA. WAY BOT #5 @ 12" OC MIDDLE STRIP TOP BARS U.N.O.	SEE PLAN FOR TOP AND ADD'L BOT. REINF.

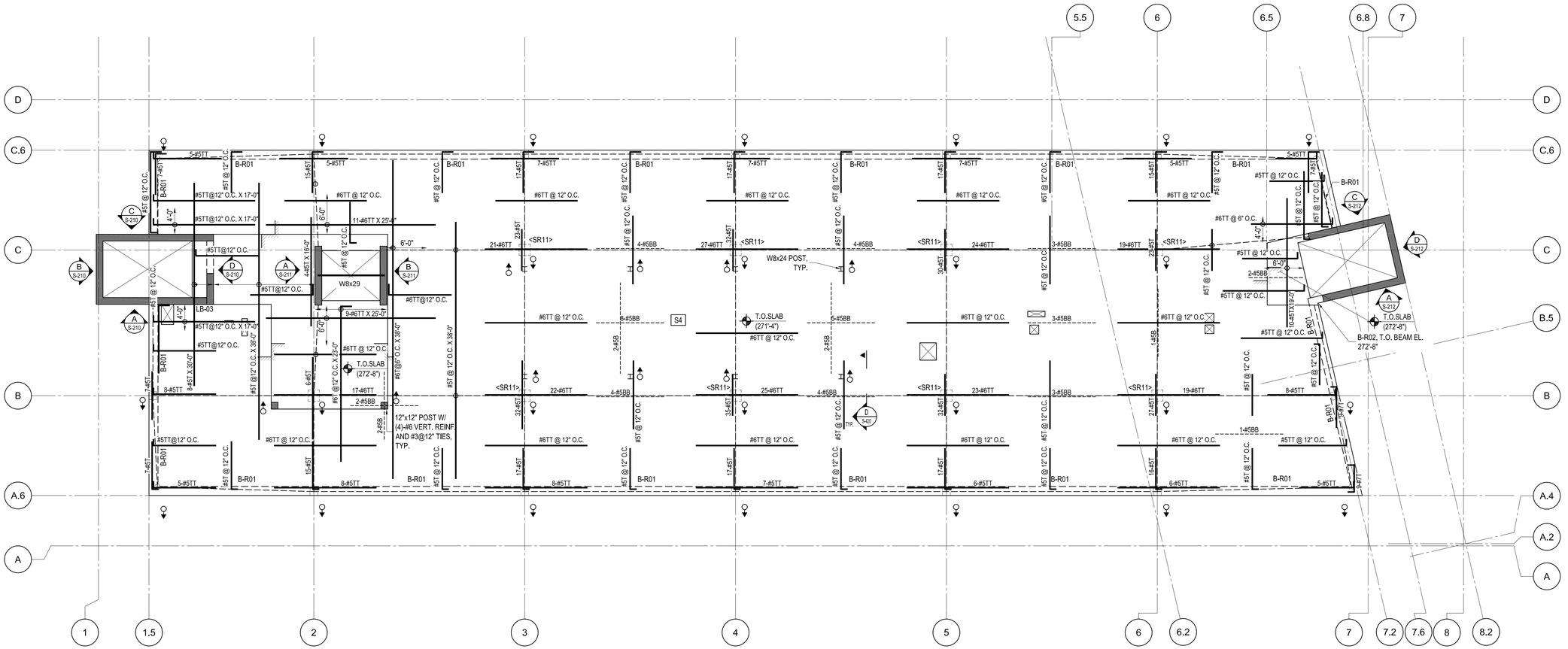
SLAB REINFORCING PLACEMENT ORDER



BOTTOM BARS (UON)



TOP BARS (UON)



ROOF FLOOR PLAN

1/8" = 1'-0"

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Sheet Title:
ROOF FLOOR PLAN

Seal & Signature

Date:	01-27-2021
Scale:	AS NOTED
Job#:	161162.00
Sheet Title:	S-106

NORTH

NOTES:

1. SEE ARCH. FOR GRID DIMENSIONS.
2. SEE ARCH. FOR SLAB EDGE.
3. SEE ARCH. FOR STEEL POST LOCATIONS.
4. SEE PLAN FOR TOP OF SLAB ELEVATION.
5. SEE CONCRETE SLAB SCHEDULE FOR TWO WAY FLAT PLATE SIZE. SEE GENERAL NOTES FOR CONCRETE STRENGTHS AND ADDITIONAL REQUIREMENTS. SEE TYPICAL TWO-WAY SLAB DETAIL FOR BAR LENGTH UNLESS OTHERWISE NOTED.
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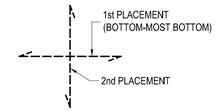
8. 'B-XXX' INDICATES REINFORCED CONCRETE BEAM. SEE CONCRETE BEAM SCHEDULE ON C/S-313 FOR SIZE AND REINFORCING OF CONCRETE BEAMS.
9. COORDINATE SIZE AND LOCATION OF ALL FLOOR DRAINS AND SLAB PENETRATIONS WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. UNLESS SPECIFICALLY DETAILED, OPENINGS SHALL BE PLACED AND REINFORCED IN ACCORDANCE WITH DETAIL E/S-311.
10. COORDINATE ALL ELEVATOR SHAFT CLEAR DIMENSIONS AND DIVIDER BEAM ELEVATIONS WITH ELEVATOR MANUFACTURER PRIOR TO CONSTRUCTION.
11. FOR ADDITIONAL INFORMATION AND REQUIREMENTS, REFER TO:
    - S-000 - SERIES FOR GENERAL NOTES.
    - S-200 - SERIES FOR COLUMN SCHEDULE AND SHEAR WALL ELEVATIONS.
    - S-300 - SERIES FOR TYPICAL DETAILS.
12. 'SRX' INDICATES STUDRAIL REINFORCING REQUIRED, REFER TO DETAIL D/S-310.
13. ⟷ DENOTES 1.5IN CELLULAR DECK, 18/18GA
14. LB-XX DENOTES CONCRETE LINK BEAM, SEE J/S-312 AND SHEAR WALL ELEVATIONS.

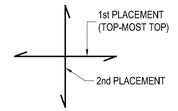
CONCRETE SLAB SCHEDULE

MARK	SLAB DEPTH	SLAB REINFORCEMENT	NOTES
S7	8"	#4 @ 12" O.C. EA. WAY BOT #5 @ 12" O.C. EA. WAY TOP	

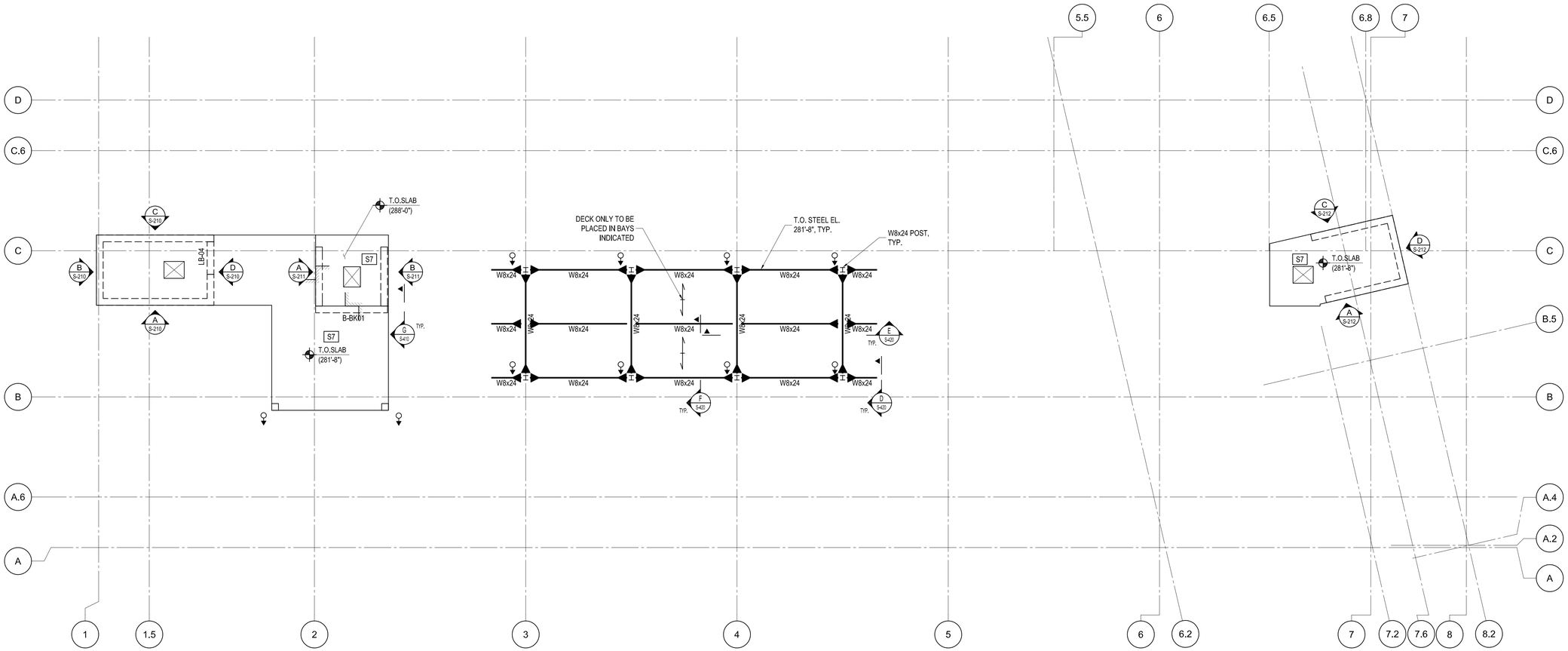
SLAB REINFORCING PLACEMENT ORDER



BOTTOM BARS (UON)



TOP BARS (UON)



BULKHEAD FLOOR PLAN

1/8" = 1'-0"

ISSUE FOR BID
APR 23, 2021

Rev. #	Revision Description	Date:
1	ISSUE FOR BID	04/23/2021

Project Description:
PROPOSED MIXED USE BUILDING:
WESTMORELAND LOFTS
136-158 WESTMORELAND AVE.
WHITE PLAINS, NY 10606

Owner/Developer:
136-158 WESTMORELAND, LLC
1485 5TH AVENUE, 24F
NEW YORK, NY 10035

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186 Wood Avenue South, First Floor
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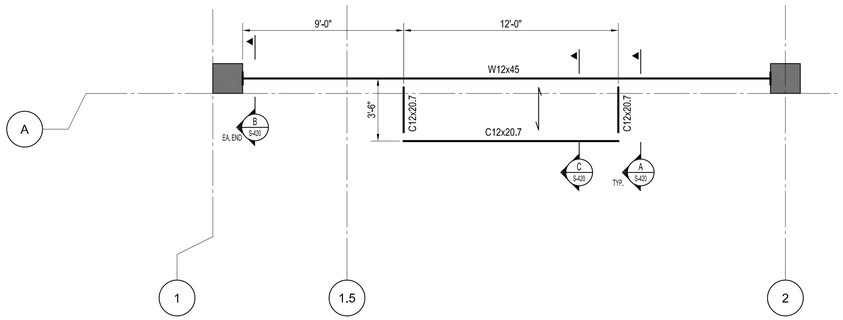
732 635-0044

Sheet Title:
BULKHEAD FLOOR PLAN

Seal & Signature

Date: 01-27-2021
Scale: AS NOTED
Job#: 161162.00
Sheet Title: S-107



A CANOPY PART PLAN

A / S-110

1/4" = 1'-0"

NOTES:

1. ALL EXTERIOR STRUCTURAL STEEL AND CONNECTIONS TO BE GALVANIZED.
2. TOP OF STEEL EL. 214'-10" U.O.N.
3. ⟵⟶ DENOTES SPAN DIRECTION OF 1.5 IN 18GA ROOF DECK.

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SUPERSTRUCTURE PART PLANS

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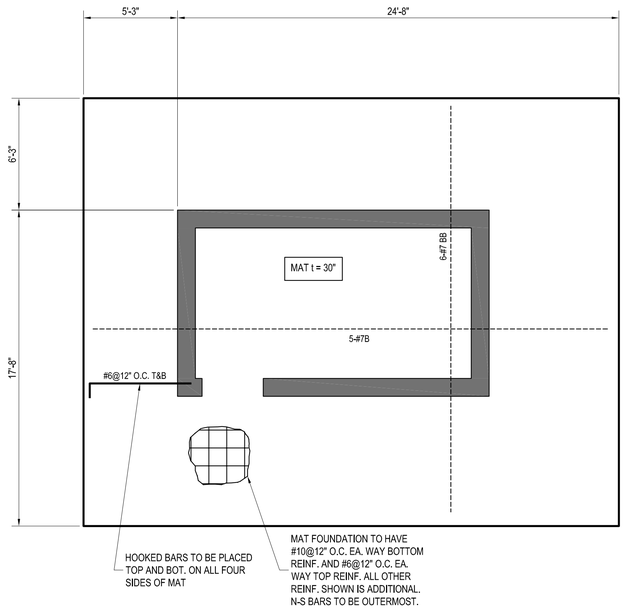
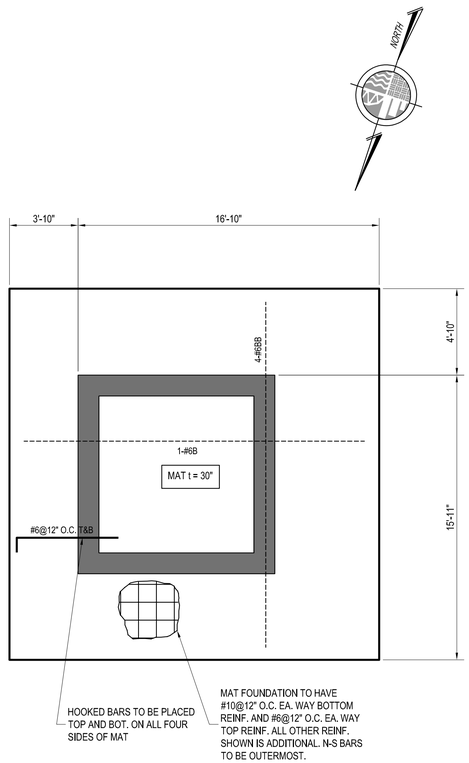
Sheet Title: S-110



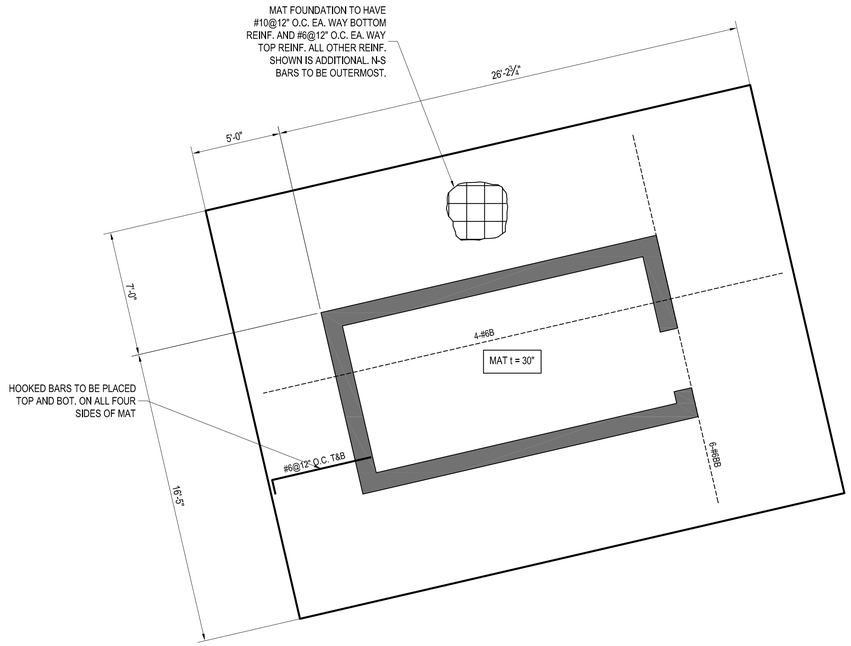
A S-120 MAT 1 PART PLAN

1/4" = 1'-0"



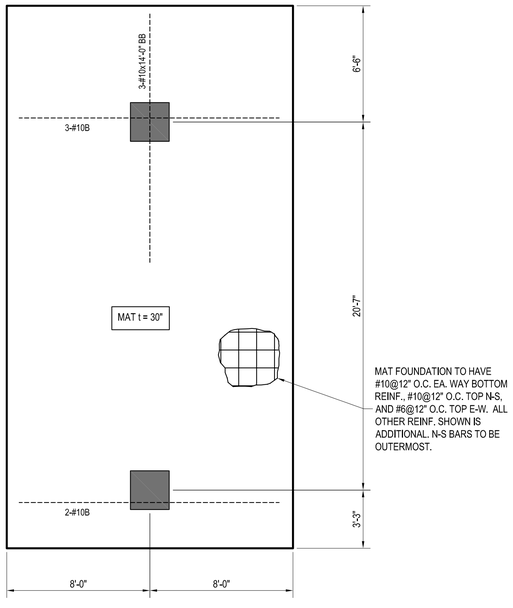
B S-120 MAT 2 PART PLAN

1/4" = 1'-0"



C S-120 MAT 3 PART PLAN

1/4" = 1'-0"



D S-120 MAT 4 PART PLAN

1/4" = 1'-0"

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1	ISSUE FOR BID	04/23/2021

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732 635-0044

Sheet Title:

FOUNDATION MAT PART PLANS

Seal & Signature

Date: 01-27-2021

Scale: AS NOTED

Job#: 161162.00

Sheet Title: S-120

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APR 23, 2021

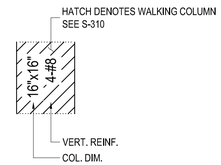
COLUMN SCHEDULE - CONCRETE

FLOOR LEVEL \ COLUMN NO.	A							A.2	A.6							B										B.5
	A - 1	A - 2	A - 3	A - 4	A - 5	A - 6	A - 7	A.2 - 8	A.6 - 1.5*	A.6 - 2	A.6 - 3	A.6 - 4	A.6 - 5	A.6 - 6	A.6 - 7.2*	B - 1	B - 1.5	B - 2	B - 3	B - 4	B - 5	B - 6	B - 6.2	B - 7	B - 7.2	B - 8.2	B.5 - 7.2
BULKHEAD FLOOR																										
ROOF FLOOR																										
FIFTH LEVEL FLOOR									10" Ø 4-#7	14"x14" 4-#7	14"x14" 4-#7	14"x14" 4-#7	14"x14" 4-#7	14"x14" 4-#7	10" Ø 4-#7		14"x14" 4-#7	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9			14"x14" 4-#7		
FOURTH LEVEL FLOOR	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	TRANSFER	TRANSFER	TRANSFER	TRANSFER	TRANSFER	TRANSFER	TRANSFER	16"x16" 4-#8	TRANSFER	↕	↕	↕	↕	↕		16"x16" 4-#8	TRANSFER	16"x16" 4-#8	
THIRD LEVEL FLOOR	↕	↕	16"x16" 4-#8	↕	↕	↕	↕	↕								↕		↕	↕	↕	↕	↕		↕		↕	
SECOND LEVEL FLOOR	16"x16" 4-#8	16"x16" 4-#8	16"x34"	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8								16"x16" 4-#8		20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9		16"x16" 4-#8		16"x16" 4-#8	
MEZZANINE FLOOR	20"x20" 8-#9	20"x20" 12-#9	26"x26" 16-#10	24"x24" 12-#10	24"x24" 8-#10	24"x24" 8-#10	24"x24" 12-#9	20"x20" 4-#9								24"x24" 12-#10		24"x24" 8-#8	26"x26" 20-#10	24"x24" 12-#10	24"x24" 12-#9	TRANSFER	30"x30" 12-#10	TRANSFER		30"x24" 16-#9	26"x26" 12-#10
FIRST FLOOR	20"x20" 8-#9	20"x20" 12-#9	26"x26" 16-#10	24"x24" 12-#10	24"x24" 8-#10	24"x24" 8-#10	24"x24" 12-#9	20"x20" 4-#9								24"x24" 12-#10		24"x24" 8-#8	26"x26" 20-#10	24"x24" 12-#10	24"x24" 12-#9		30"x30" 12-#10			30"x24" 16-#9	26"x26" 12-#10
COLUMN LOAD (KIPS)	265	629	563	513	506	564	426	108								436		714	779	847	820		815			351	384

NOTES

1. COLUMN LOADS SHOWN ARE SERVICE LEVEL (ASD) LOADS AND SHOWN IN KIPS. TENSION CONDITION ARE INDICATED THUS (-000K).
2. LIVE LOADS HAVE BEEN REDUCED AS ALLOWED BY BUILDING CODE.
3. FOR COLUMN REINFORCING GRADES AND OTHER REQUIREMENTS, SEE GENERAL NOTES.
4. SEE FRAMING PLANS FOR COLUMN LOCATIONS RELATIVE TO GRID LINE, AND ORIENTATION OF COLUMN.
5. COLUMN VERTICAL REINFORCING BAR SPLICES SHALL BE COMPRESSION LAP SPLICES UNLESS OTHERWISE INDICATED.
6. SEE "CONCRETE REBAR LAP SPLICE & HOOK LENGTH" SCHEDULE FOR SPLICE LENGTHS.
7. FOR TIE SIZE AND SPACING, SEE DETAIL A/S-310.
8. '*' DENOTES GRID INTERSECTION NEAREST COLUMN. SEE PLAN FOR COLUMN LOCATION.

COLUMN SCHEDULE LEGEND



COLUMN SCHEDULE - CONCRETE (CONT.)

FLOOR LEVEL \ COLUMN NO.	C					C.6							D								E							F					G	
	C - 3	C - 4	C - 5	C - 6	C - 6.2	C.6 - 1.5*	C.6 - 2	C.6 - 3	C.6 - 4	C.6 - 5	C.6 - 6	C.6 - 7.6*	D - 1	D - 2	D - 3	D - 4	D - 5	D - 5.5	D - 6.5	D - 6.8	E - 0.5	E - 1.2	E - 1.8	E - 2.5	E - 3.5	E - 4.2	E - 5.2	F - 0.5	F - 1.8	F - 2.5	F - 3.5	F - 4.2	G - 0.5	
BULKHEAD FLOOR																																	
ROOF FLOOR																																	
FIFTH LEVEL FLOOR	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9		10" Ø 4-#7	14"x14" 4-#7	14"x14" 4-#7	14"x14" 4-#7	14"x14" 4-#7	14"x14" 4-#7	10" Ø 4-#7																					
FOURTH LEVEL FLOOR	↕	↕	↕	↕		TRANSFER	TRANSFER	TRANSFER	TRANSFER	TRANSFER	TRANSFER	TRANSFER	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8													
THIRD LEVEL FLOOR													↕	↕	↕	↕	↕	↕	↕	↕													
SECOND LEVEL FLOOR	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9									16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8	16"x16" 4-#8													
MEZZANINE FLOOR	24"x24" 8-#9	24"x24" 12-#10	24"x24" 8-#10	TRANSFER	32"x32" 12-#9								20"x20" 4-#9	20"x20" 8-#10	20"x20" 4-#9	24"x24" 16-#10	24"x24" 8-#10	24"x24" 8-#9	24"x24" 8-#10	20"x20" 4-#9													
FIRST FLOOR	24"x24" 8-#9	24"x24" 12-#10	24"x24" 8-#10		32"x32" 12-#9								20"x20" 4-#9	20"x20" 8-#10	20"x20" 4-#9	24"x24" 16-#10	24"x24" 8-#10	24"x24" 8-#9	24"x24" 8-#10	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	20"x20" 4-#9	
COLUMN LOAD (KIPS)	756	854	784		546								264	571	560	519	394	423	433	106	15	89	129	120	120	126	99	71	131	108	110	123	51	

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Sheet Title:
CONCRETE COLUMN SCHEDULE

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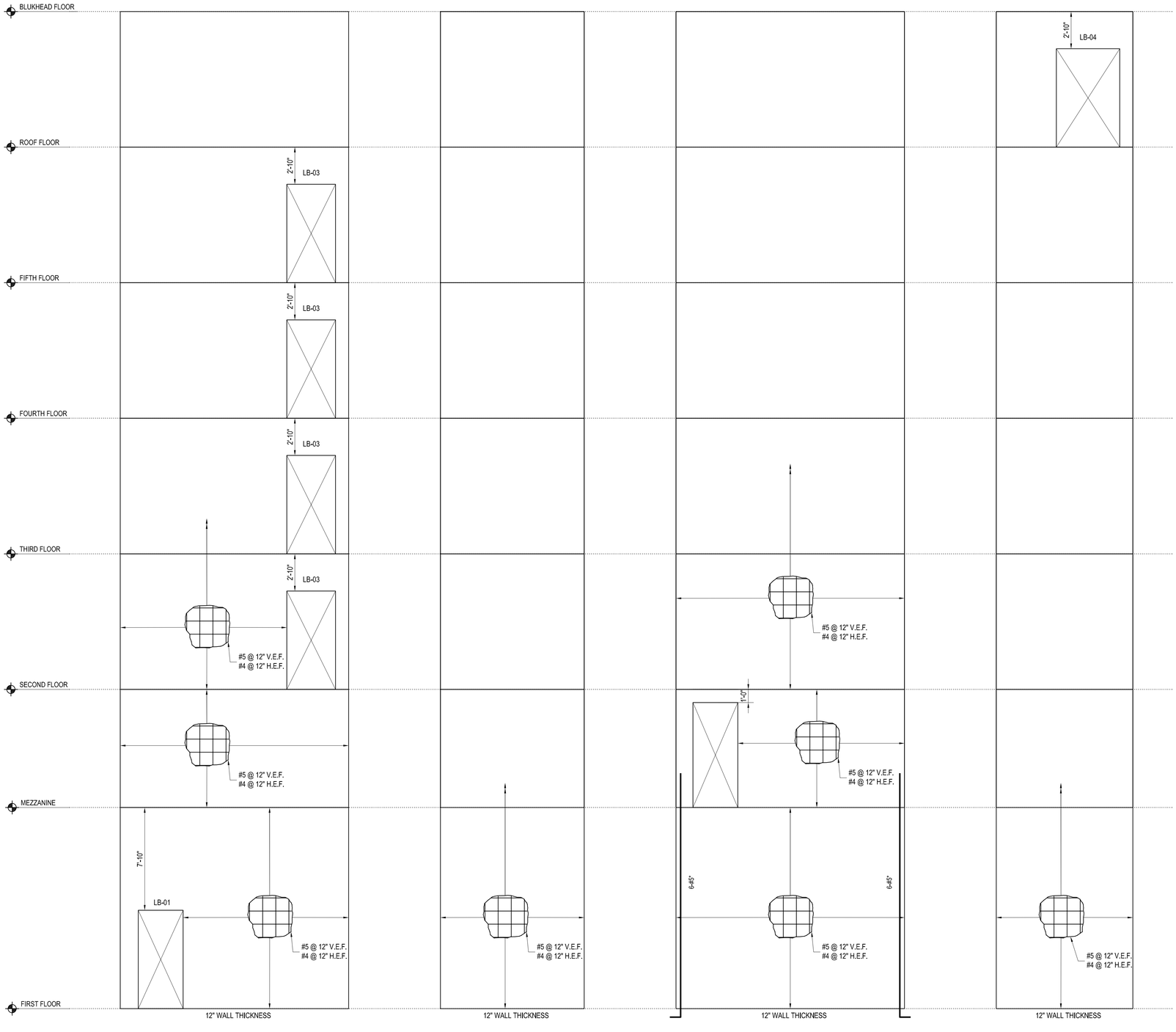

A S-210 SHEAR WALL ELEVATION A N.T.S.

B S-210 SHEAR WALL ELEVATION B N.T.S.

C S-210 SHEAR WALL ELEVATION C N.T.S.

D S-210 SHEAR WALL ELEVATION C N.T.S.

SHEAR WALL NOTES

1. ALL REINFORCING BAR SPLICES SHALL BE "CLASS B" TENSION LAP SPLICES. SEE DRAWING S-302 FOR SPLICE LENGTH TABLES
2. VERTICAL DOWELS PROJECTING OUT OF THE FOUNDATION WALLS SHALL MATCH THE SIZE, QUANTITY, AND CONFIGURATION OF THE VERTICAL REINFORCING BARS IN THE SHEAR WALLS. ALL DOWEL BARS SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WITH A STANDARD 90° HOOK UNLESS OTHERWISE NOTED.
3. ' * ' DENOTES CHORD REINFORCING. ALL CHORD REINFORCING SHALL BE PLACED IN TWO (2) ROWS WITH BAR PAIRS SPACED AT 6" O.C. UNLESS OTHERWISE INDICATED ON ELEVATIONS OR DETAILS.
4. CHORD REINFORCING LOCATED AT ENDS OF SHEAR WALLS SHALL BE ARRANGED PER DETAIL D/S-313.
5. CHORD REINFORCING LOCATED AT WALL CORNERS SHALL BE ARRANGED PER DETAIL E/S-313 UNLESS SPECIFICALLY DETAILED OTHERWISE.
6. LINK BEAMS INDICATED THUS: LBXX SHALL BE REINFORCED AS INDICATED IN THE LINK BEAM SCHEDULE ON DRAWING S-312.

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Sheet Title:
SHEAR WALL ELEVATION I

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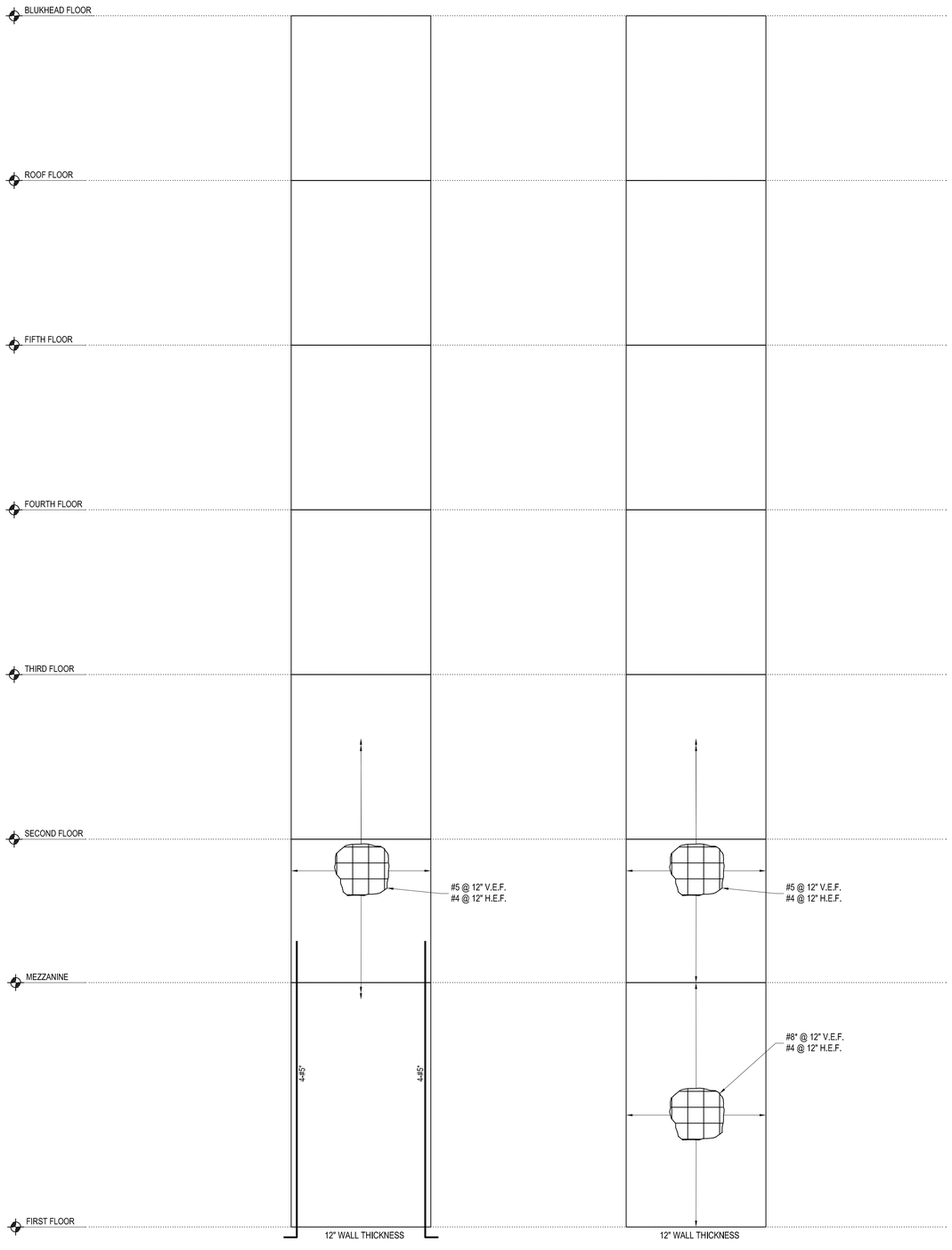

A S-211 SHEAR WALL ELEVATION A N.T.S.

B S-211 SHEAR WALL ELEVATION B N.T.S.

SHEAR WALL NOTES

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2. VERTICAL DOWELS PROJECTING OUT OF THE FOUNDATION WALLS SHALL MATCH THE SIZE, QUANTITY, AND CONFIGURATION OF THE VERTICAL REINFORCING BARS IN THE SHEAR WALLS. ALL DOWEL BARS SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WITH A STANDARD 90° HOOK UNLESS OTHERWISE NOTED.
3. ' * ' DENOTES CHORD REINFORCING. ALL CHORD REINFORCING SHALL BE PLACED IN TWO (2) ROWS WITH BAR PAIRS SPACED AT 6" O.C. UNLESS OTHERWISE INDICATED ON ELEVATIONS OR DETAILS.
4. CHORD REINFORCING LOCATED AT ENDS OF SHEAR WALLS SHALL BE ARRANGED PER DETAIL D/S-313.
5. CHORD REINFORCING LOCATED AT WALL CORNERS SHALL BE ARRANGED PER DETAIL E/S-313 UNLESS SPECIFICALLY DETAILED OTHERWISE.
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SHEAR WALL ELEVATION II

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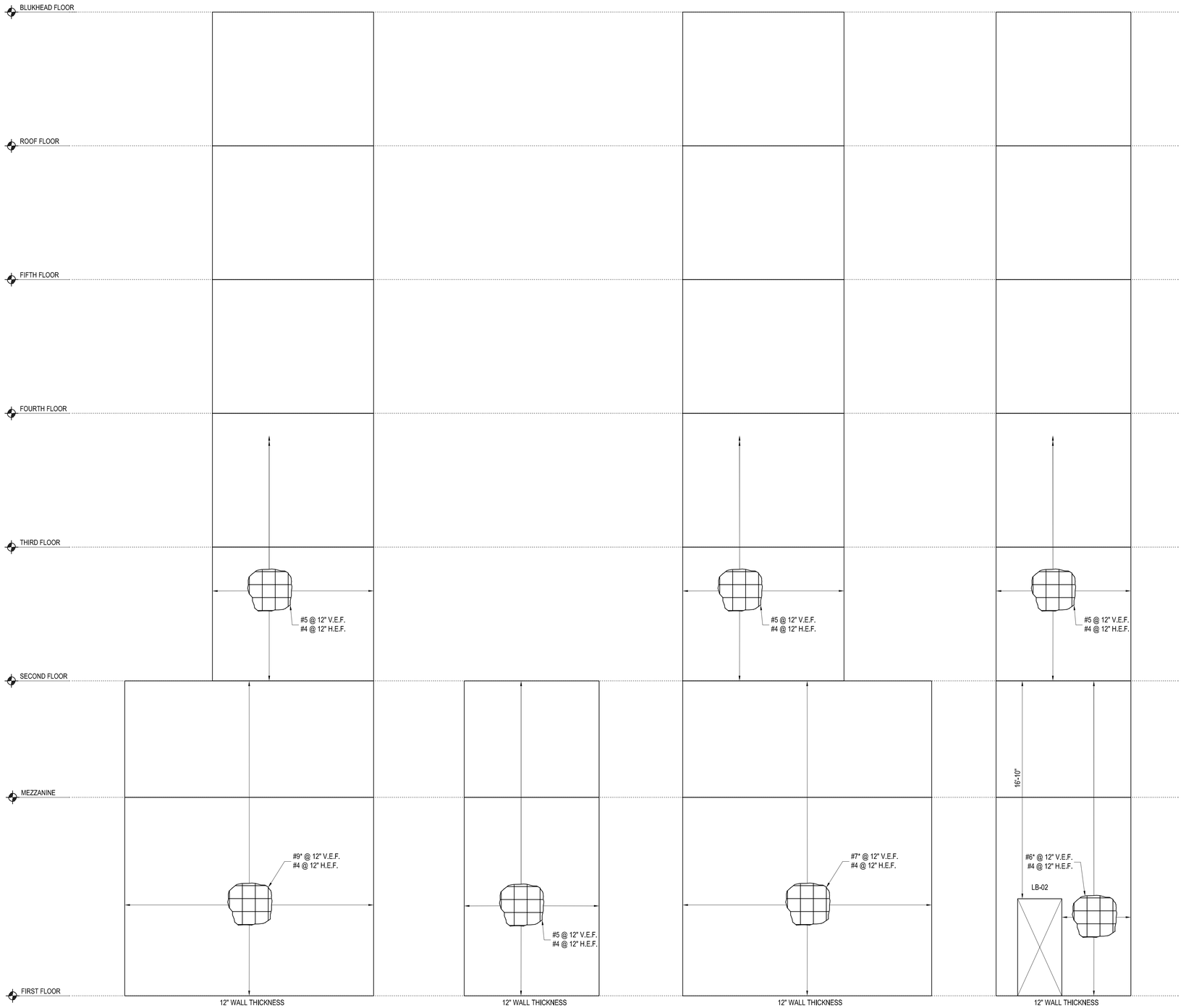

A S-212 SHEAR WALL ELEVATION A N.T.S.

B S-212 SHEAR WALL ELEVATION B N.T.S.

C S-212 SHEAR WALL ELEVATION C N.T.S.

D S-212 SHEAR WALL ELEVATION C N.T.S.

SHEAR WALL NOTES

1. ALL REINFORCING BAR SPLICES SHALL BE "CLASS B" TENSION LAP SPLICES. SEE DRAWING S-302 FOR SPLICE LENGTH TABLES
2. VERTICAL DOWELS PROJECTING OUT OF THE FOUNDATION WALLS SHALL MATCH THE SIZE, QUANTITY, AND CONFIGURATION OF THE VERTICAL REINFORCING BARS IN THE SHEAR WALLS. ALL DOWEL BARS SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WITH A STANDARD 90° HOOK UNLESS OTHERWISE NOTED.
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4. CHORD REINFORCING LOCATED AT ENDS OF SHEAR WALLS SHALL BE ARRANGED PER DETAIL D/S-313.
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WESTMORELAND LOFTS

136-158 WESTMORELAND AVE.

WHITE PLAINS, NY 10606

Owner/Developer:

136-158 WESTMORELAND, LLC

1485 5TH AVENUE, 24F

NEW YORK, NY 10035

Papp Architects
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188 East Post Road, White Plains, NY 10601
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JMC Site Development Consultants
Civil Engineer
120 Bedford Road
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McLaren Engineering Group
Structural Engineer
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Khachaturian Engineering Associates
Mechanical/Electrical/Plumbing Engineers
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732 635–0044

Sheet Title:

SHEAR WALL ELEVATION III

Seal & Signature

Date: 01-27-2021

Scale: AS NOTED

Job#: 161162.00

Sheet Title: S-212

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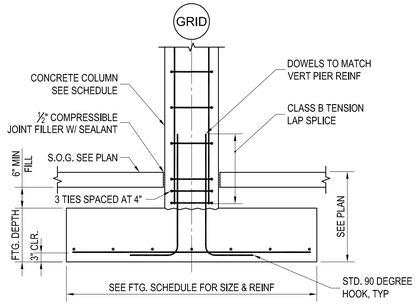
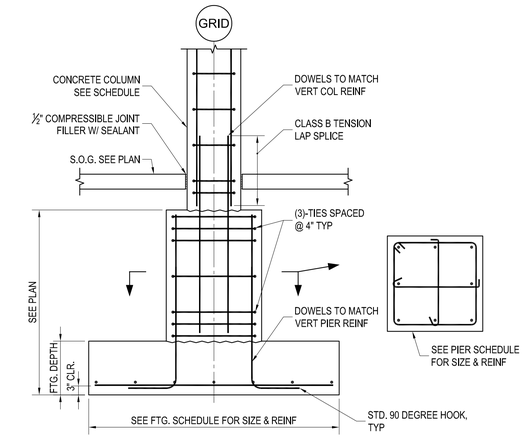
APR 23, 2021



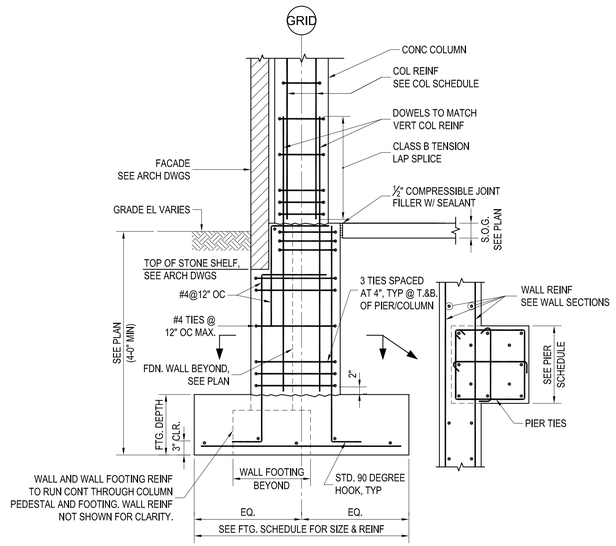
NOTES:
1. WHERE CONCRETE OR REINFORCED CMU WALLS BEAR ON FOOTING, INSTALL WALL DOWELS INTO FOOTING.
2. INSTALL WALL FOOTING REINFORCEMENT CONTINUOUS THROUGH INTERSECTING FOOTINGS.

A S-300 TYP INTERIOR FOOTING DETAIL NTS

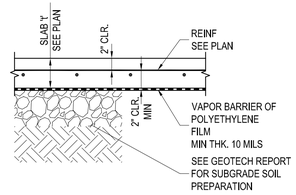


NOTES:
1. WHERE CONCRETE OR REINFORCED CMU WALLS BEAR ON FOOTING, INSTALL WALL DOWELS INTO FOOTING.
2. INSTALL WALL FOOTING REINFORCEMENT CONTINUOUS THROUGH INTERSECTING FOOTINGS.
3. OMIT PEDESTAL WHERE NOT REQUIRED.

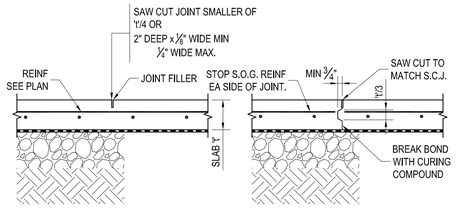
B S-300 TYP INTERIOR PIER & FOOTING DETAIL NTS



C S-300 TYP EXTERIOR FOOTING WITH PEDESTAL DETAIL NTS



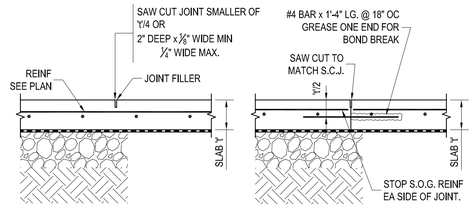
D S-300 TYP SLAB ON GRADE DETAIL NTS



SAW CUT JOINT (S.C.J.) CONSTRUCTION JOINT (C.J.)

NOTES:
1. JOINTS MUST BE CUT WITHIN 12 HOURS AFTER PLACEMENT OF CONCRETE.
2. SAW CUT JOINTS SHALL BE LOCATED AT COLUMN CENTERLINES. PROVIDE ADDITIONAL SAW CUT JOINTS AS REQUIRED THAT SPACING DOES NOT EXCEED 36 x 't'.
3. CONTRACTOR TO PROVIDE JOINT LAYOUT DRAWING FOR REVIEW AND APPROVAL BY THE ARCHITECT AND ENGINEER.
4. JOINTS MUST BE BULKHEADED AND DEFINE LIMITS OF POUR PANELS.

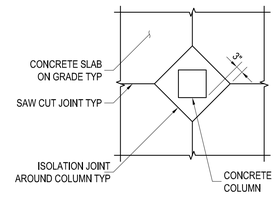
E S-300 TYP SLAB ON GRADE JOINT DETAIL NTS



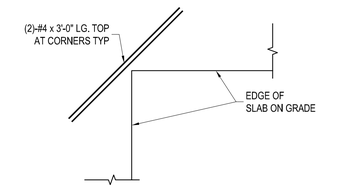
SAW CUT JOINT (S.C.J.) CONSTRUCTION JOINT (C.J.)

NOTES:
1. JOINTS MUST BE CUT WITHIN 12 HOURS AFTER PLACEMENT OF CONCRETE.
2. SAW CUT JOINTS SHALL BE LOCATED AT COLUMN CENTERLINES. PROVIDE ADDITIONAL SAW CUT JOINTS AS REQUIRED THAT SPACING DOES NOT EXCEED 36 x 't'.
3. CONTRACTOR TO PROVIDE JOINT LAYOUT DRAWING FOR REVIEW AND APPROVAL BY THE ARCHITECT AND ENGINEER.
4. JOINTS MUST BE BULKHEADED AND DEFINE LIMITS OF POUR PANELS.

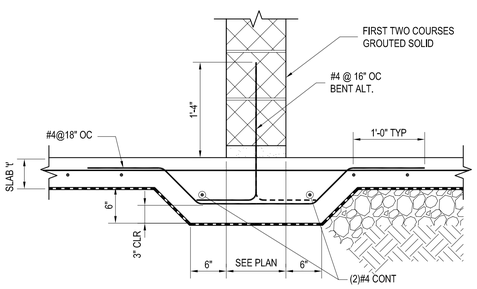
F S-300 TYP SLAB ON GRADE JOINT DETAIL NTS



G S-300 TYP SLAB JOINTS AROUND CONC COLUMN DETAIL NTS

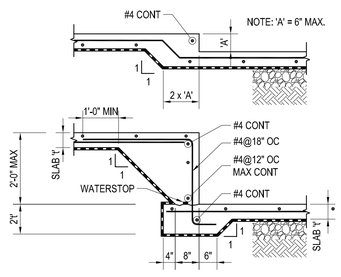


H S-300 TYP S.O.G. REINFORCING AT RE-ENTRANT CORNERS DETAIL NTS

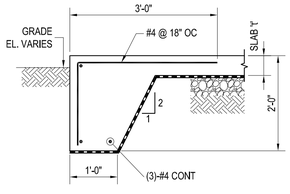


NOTE : PROVIDE AT ALL INTERIOR MASONRY WALLS U.O.N.

J S-300 TYP CMU WALL ON SOG DETAIL NTS



K S-300 TYP DEPRESSION AT SLAB ON GRADE DETAIL NTS



L S-300 TYP TURNED DOWN SLAB DETAIL NTS

Rev. #	Revision Description	Date:
1	ISSUE FOR BID	04/23/2021

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Sheet Title:
TYP.CONCRETE FOUNDATION DETAILS I

Seal & Signature

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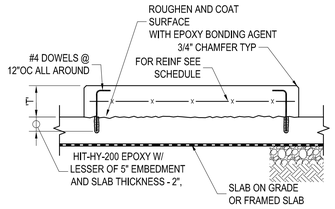
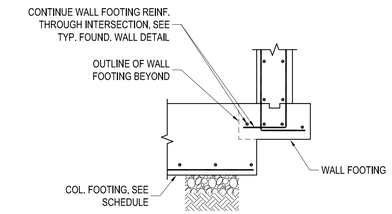
APR 23, 2021



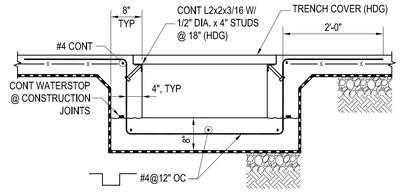
'T'	REINFORCEMENT
4"	1 LAYER 6 x 6 x W1.4 x W1.4 W.W.F.
6"	1 LAYER 6 x 6 x W2.9 x W2.9 W.W.F.
8"	2 LAYERS 6 x 6 x W2.9 x W2.9 W.W.F.
12"	#4 @ 12" EA WAY TOP & BOTTOM

NOTES:
1. FOR DIMENSION AND THICKNESS "T", SEE MECHANICAL DRAWINGS.
2. FOR LOCATION SEE MECHANICAL & ARCHITECTURAL DRAWINGS.
3. CAST MECHANICAL SLAB AFTER BASE IS POURED & CURED.
4. PADS LESS THAN 8" THICK MAY BE FIBER-REINFORCED IN PLACE OF W.W.F.
5. ALL EXTERIOR PAD REINFORCEMENT SHALL BE HOT DIP GALVANIZED

A S-301 TYP MECH. EQUIPMENT PAD DETAIL NTS

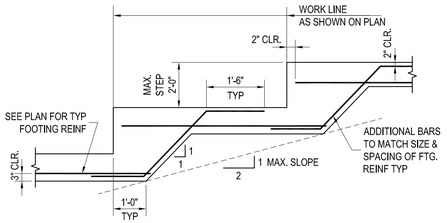


B S-301 TYP. WALL FOOTING-COLUMN FOOTING PARTIAL OVERLAP NTS

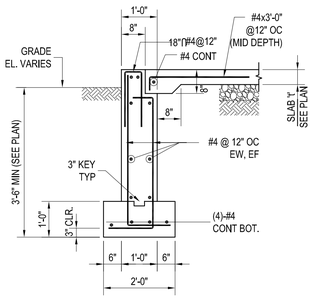


NOTE : SEE ARCH DRAWINGS AND M.E.P. DWGS FOR TRENCH DRAIN LOCATIONS AND DIMS.

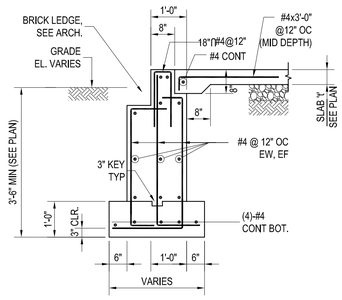
C S-301 TYP TRENCH DRAIN AT SLAB ON GRADE DETAIL NTS



E S-301 TYP STEPPED FOOTING DETAIL NTS

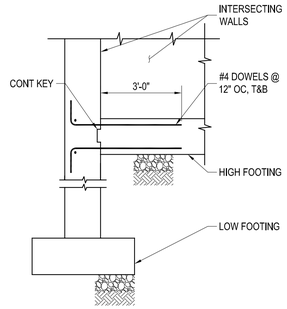


W/O BRICK LEDGE



W/ BRICK LEDGE

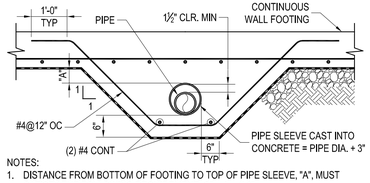
F S-301 TYPICAL FOUNDATION WALL NTS



G S-301 TYPICAL HIGH WALL FOOTING CONN. TO INTERSECTING WALL NTS

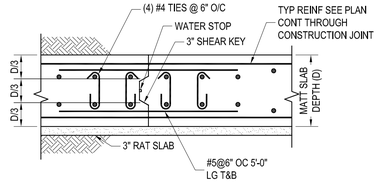
FOOTING SCHEDULE (4ksf BEARING)					
MARK	LENGTH	WIDTH	DEPTH	REINFORCEMENT	REMARKS
F60	6'-0"	6'-0"	1'-2"	(6)-#6 EW (BOT.)	
F70	7'-0"	7'-0"	1'-2"	(7)-#6 EW (BOT.)	
F80	8'-0"	8'-0"	1'-2"	(8)-#7 EW (BOT.)	
F90	9'-0"	9'-0"	1'-4"	(9)-#7 EW (BOT.)	
F100	10'-0"	10'-0"	1'-6"	(11)-#7 EW (BOT.)	
F110	11'-0"	11'-0"	1'-8"	(11)-#8 EW (BOT.)	
F120	12'-0"	12'-0"	1'-10"	(12)-#8 EW (BOT.)	
F130	13'-0"	13'-0"	2'-0"	(13)-#9 EW (BOT.)	
F140	14'-0"	14'-0"	2'-4"	(15)-#9 EW (BOT.)	
F150	15'-0"	15'-0"	2'-6"	(15)-#10 EW (BOT.)	
F160	16'-0"	16'-0"	2'-8"	(16)-#10 EW (BOT.)	
F3060	3'-0"	6'-0"	1'-0"	(4)-#5 LONG BARS (BOT.) (7)-#5 SHORT BARS (BOT.)	
F30100	3'-0"	10'-0"	1'-0"	(4)-#5 LONG BARS (BOT.) (11)-#5 SHORT BARS (BOT.)	
F70130	7'-0"	13'-0"	1'-2"	(8)-#4 LONG BARS (TOP) (14)-#4 SHORT BARS (TOP) (8)-#6 LONG BARS (BOT.) (14)-#6 SHORT BARS (BOT.)	
F100120	10'-0"	12'-10"	1'-8"	(11)-#5 LONG BARS (TOP) (13)-#5 SHORT BARS (BOT.) (12)-#8 EW (BOT.)	
F110120	11'-0"	12'-0"	2'-0"	(12)-#8 EW (BOT.)	
F110150	11'-0"	15'-0"	2'-0"	(14)-#9 EW (BOT.)	
F110160	11'-0"	16'-0"	2'-0"	(14)-#9 EW (BOT.)	
F110180	11'-0"	18'-0"	2'-4"	(18)-#9 LONG BARS (BOT.) (14)-#9 SHORT BARS (BOT.)	
F120150	12'-0"	15'-0"	2'-6"	(20)-#10 LONG BARS (BOT.) (16)-#10 SHORT BARS (BOT.)	
F120180	12'-0"	18'-0"	2'-2"	(18)-#9 LONG BARS (BOT.) (14)-#9 SHORT BARS (BOT.)	
F130140	13'-0"	14'-0"	2'-0"	(14)-#9 EW (BOT.)	
F130230	13'-0"	23'-0"	2'-6"	(17)-#11 LONG BARS (TOP) (23)-#11 SHORT BARS (TOP) (26)-#11 LONG BARS (BOT.) (28)-#11 SHORT BARS (BOT.)	HOOK ALL BARS

H S-301 FOOTING SCHEDULE NTS



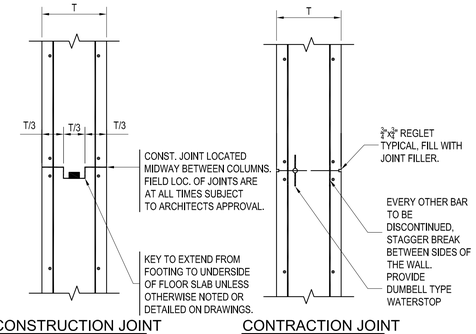
NOTES:
1. DISTANCE FROM BOTTOM OF FOOTING TO TOP OF PIPE SLEEVE, "A", MUST SATISFY THE FOLLOWING REQUIREMENT: 0" < A < 1'-0".
2. WHERE TOP OF PIPE SLEEVE IS MORE THAN 1'-0" BELOW THE BOTTOM OF FOOTING ELEVATION SHOWN ON PLAN, STEP FOOTING DOWN TO PERMIT PIPE TO PASS DIRECTLY BELOW FOOTING AS SHOWN.
3. WHERE TOP OF PIPE SLEEVE IS ABOVE THE BOTTOM OF FOOTING ELEVATION SHOWN ON PLAN, STEP FOOTING DOWN TO PERMIT PIPE TO PASS THROUGH WALL ABOVE FOOTING.
4. SEE MEP AND CIVIL DRAWINGS FOR SIZE, LOCATION, AND ELEVATION OF PIPES.

J S-301 TYP PIPE BELOW WALL FOOTING DETAIL NTS



NOTE: CONSTRUCTION JOINTS TO BE IN MIDDLE THIRD OF COLUMN STRIPS. PROVIDE PROPOSED LOCATIONS IN SUBMITTAL FOR REVIEW

K S-301 MAT SLAB TYPICAL CONSTRUCTION JOINT DETAIL NTS



NOTES:
1. DISTANCE BETWEEN CONTRACTION JOINTS TO BE A MAXIMUM OF 40 FEET.
2. CONTRACTION JOINT SHALL BE PLACED NO FARTHER THAN 10'-0" FROM ANY CORNER.
3. FOR WATERPROOFING, SEE ARCHITECTURAL DRAWINGS.
4. CONTRACTOR TO PROVIDE PLAN OF JOINT LOCATIONS FOR REVIEW

L S-301 TYPICAL WALL JOINT DETAILS NTS

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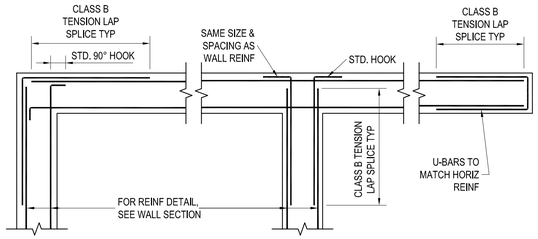
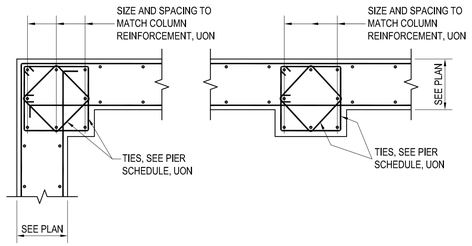


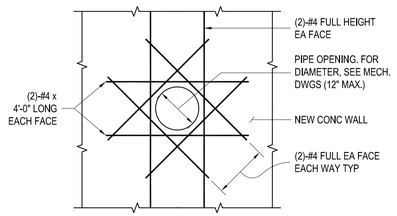
NOTE: VERTICAL REINFORCEMENT NOT SHOWN FOR CLARITY.

A S-302 TYP WALL INTERSECTION DETAIL NTS



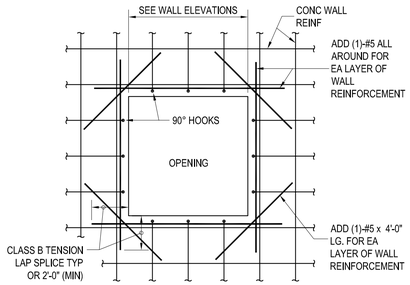
NOTE: SEE WALL SECTIONS FOR WALL REINF

B S-302 TYP PIER AT WALL DETAIL NTS



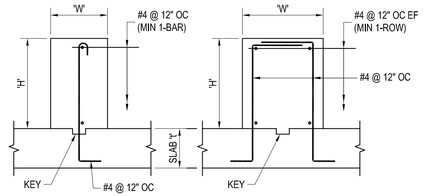
NOTE: COORDINATE OPENING SIZES AND LOCATIONS WITH CERTIFIED MECHANICAL SHOP DRAWINGS.

C S-302 TYP SLEEVE THROUGH WALL DETAIL NTS



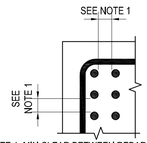
NOTE: ONE-HALF OF VERTICAL AND HORIZONTAL REINFORCING BAR AREA (MINIMUM) INTERRUPTED BY OPENING SHALL BE PROVIDED EACH SIDE OF OPENING IN THE SAME FACE. (FULL LENGTH BARS)

D S-302 TYP WALL OPENING REINFORCEMENT DETAIL NTS



NOTE: REFERENCE ARCH DWGS FOR SIZE AND LOCATION OF CONC CURBS.

E S-302 TYP CONCRETE CURB DETAIL NTS



NOTE 1: MIN CLEAR BETWEEN REBAR LAYERS IS THE GREATER OF 1" AND db, FOR TYP MEMBERS, AND THE GREATER OF 1.5" AND 1.5db FOR COMPRESSION MEMBERS

F S-302 TYP MULTIPLE REBAR LAYERS DETAIL NTS

GRADE BEAM SCHEDULE						
MARK	WIDTH	DEPTH	REINFORCEMENT		SHEAR STIRRUPS	COMMENTS
			TOP BARS	BOTTOM BARS		
GB01	4'-0"	1'-2"	(8) #9	(8) #5	4L-#4@8"	EXTEND STIRRUPS TO GRID 0.5

G S-302 GRADE BEAM SCHEDULE NTS

PIER SCHEDULE					
MARK	LENGTH	WIDTH	VERT REINF	TIES	COMMENTS
P01	3'-0"	3'-4.5"	16-#9	#3@18"	
P02	2'-7"	3'-4.5"	16-#9	#3@18"	
P03	1'-3"	3'-6"	8-#8	#3@14"	
P04	2'-8"	3'-1"	16-#10	#3@18"	
P05	3'-4"	3'-4"	16-#9	#3@18"	
P06	3'-0"	3'-0"	12-#10	#3@18"	
P07	2'-3"	3'-2"	12-#9	#3@18"	
P08	2'-2"	2'-10"	8-#10	#3@18"	
P09	2'-8"	2'-8"	20-#10	#3@18"	
P10	2'-11"	3'-3"	12-#10	#3@18"	
P11	2'-8"	3'-0"	12-#9	#3@18"	
P12	2'-6"	2'-8"	12-#9	#3@18"	
P13	2'-8"	3'-6"	16-#9	#3@18"	
P14	3'-2"	3'-2"	16-#9	#3@18"	

K S-302 PIER SCHEDULE NTS

TABLE 1.1
TENSION LAP SPLICE LENGTHS (CLASS B MINIMUM)
3/4" COVER (IN)

BAR \ f'c	BAR(KSI)	3,000	4,000	5,000	6,000	7,000	8,000	9,000	≥ 10,000
#3	60	13	12	12	12	12	12	12	12
#4	60	22	19	17	16	15	15	15	15
#5	60	32	28	25	23	21	20	19	19
#6	60	43	37	34	31	28	27	25	24
#7	60	69	60	54	49	46	43	40	38
#8	60	86	74	67	61	56	53	50	47
#9	60	104	90	81	74	68	64	60	57
#10	60	125	108	97	88	82	77	72	69
#11	75	183	158	142	129	120	112	106	100

TABLE 1.2
TENSION LAP SPLICE LENGTHS (CLASS B MINIMUM)
1 1/2" COVER (IN)

BAR \ f'c	BAR(KSI)	3,000	4,000	5,000	6,000	7,000	8,000	9,000	≥ 10,000
#3	60	13	12	12	12	12	12	12	12
#4	60	18	15	15	15	15	15	15	15
#5	60	22	19	19	19	19	19	19	19
#6	60	26	23	23	23	23	23	23	23
#7	60	43	37	33	30	28	27	27	27
#8	60	54	47	42	38	35	33	31	30
#9	60	66	58	52	47	44	41	39	37
#10	60	81	70	63	58	53	50	47	45
#11	75	121	105	94	86	79	74	70	66

NOTE: USE TABLE 1.1 IF BAR SPACING IS LESS THAN 4" O/C UP TO #8, 5" OC FOR #9 OR GREATER

TABLE 1.1-1.2 NOTES:
1. TABLES 1.1-1.2 CONFORM TO ACI 318. TABULATED VALUES ASSUME UNCOATED REINFORCEMENT AND NORMAL WEIGHT CONCRETE (144-150PCF).
2. LENGTHS TABULATED MUST BE MULTIPLIED BY THE FOLLOWING MODIFICATION FACTORS:
   A. TOP BARS 1.3
   ("TOP" IS DEFINED BY ACI 318 AS HORIZONTAL BARS HAVING MORE THAN 12 INCHES OF FRESH CONCRETE CAST BELOW THE BAR).
   B. LIGHTWEIGHT CONCRETE 1.3
   C. EPOXY-COATED BARS:
      1.) BARS WITH COVER < 3db, OR WITH CLEAR SPACING < 6db 1.5
      FOR BOTTOM & VERTICAL BARS. 1.3 FOR "TOP" BARS.*
      2.) ALL OTHER CONDITIONS 1.2
      * FOR EPOXY-COATED "TOP" BARS THE MAXIMUM FOR COMBINED FACTORS = 1.7
3. LAP SPLICES ARE NOT PERMITTED FOR #14 AND #18. USE MECHANICAL CONNECTIONS OR WELDED SPLICES. LAP SPLICES OF #14 AND #18 BARS TO #11 AND SMALLER BARS ARE PERMITTED.

H S-302 REBAR SPLICE TABLES NTS

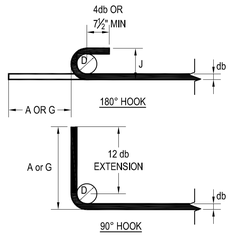
TABLE 2
COMPRESSION LAP SPLICES (IN)

BAR \ fy	60 KSI (30db)	75 KSI (44db)	80 KSI (48db)
#3	12	17	18
#4	15	22	24
#5	19	28	30
#6	23	33	36
#7	27	39	42
#8	30	44	48
#9	34	50	55
#10	39	56	61
#11	43	62	68
#14	SEE NOTE 2		
#18	SEE NOTE 2		

TABLE 2 NOTES:
1. TABLE 2 CONFORMS TO ACI 318. TABULATED VALUES ASSUME NORMAL WEIGHT CONCRETE (144-150PCF) WITH f'c OF 3,000 PSI OR GREATER AND UNCOATED REINFORCEMENT.
2. LAP SPLICES ARE NOT PERMITTED FOR #14 AND #18, USE MECHANICAL CONNECTIONS OR WELDED SPLICES. LAP SPLICES OF #14 AND #18 BARS TO #11 AND SMALLER BARS ARE PERMITTED PER ACI 318

TABLE 3
END HOOK LENGTHS (IN)

BAR	180° HOOKS			90° HOOKS
	D	A or G	J	A or G
#3	2¼	5	3	6
#4	3	6	4	8
#5	3¾	7	5	10
#6	4½	8	6	12
#7	5¼	10	7	14
#8	6	11	8	16
#9	9½	15	11¾	19
#10	10¾	17	13¼	22
#11	12	19	14¾	24
#14	18¼	27	21¾	31
#18	24	36	30½	41



J S-302 END HOOK LENGTH NTS

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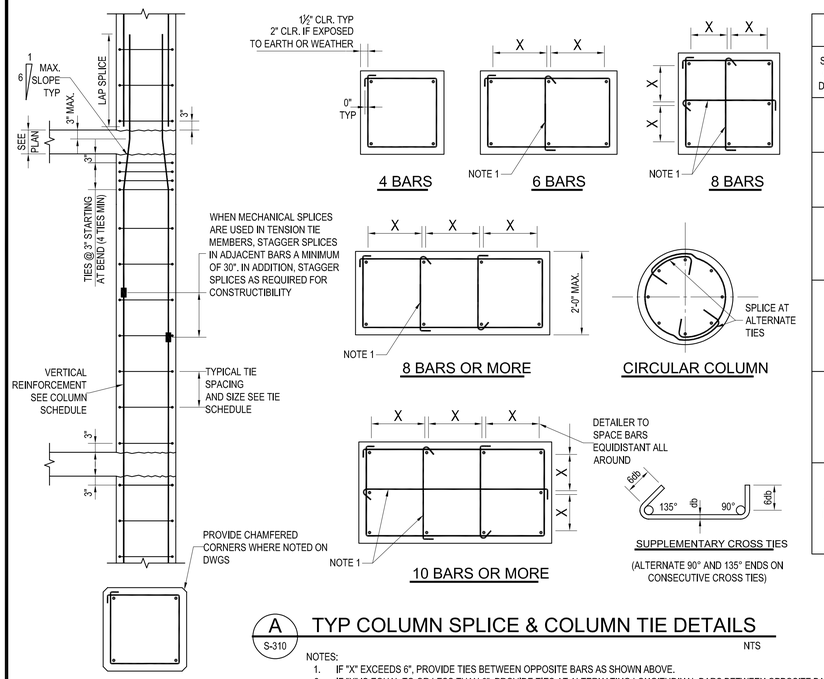
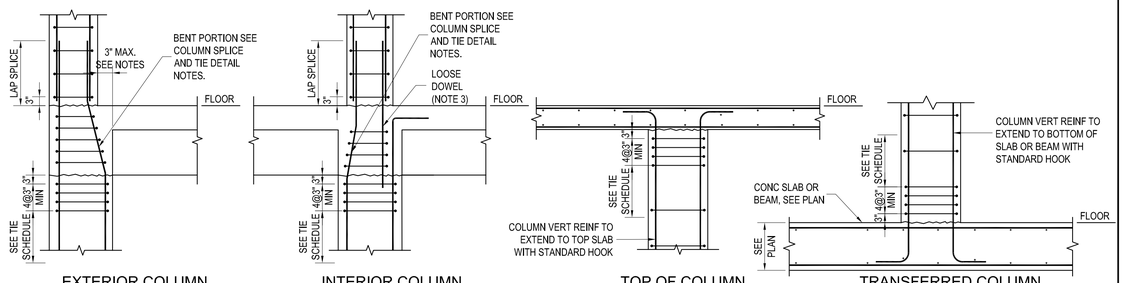


TIE SCHEDULE		
SMALLEST COLUMN DIMENSION	SIZE OF VERT REINFORCING STEEL	SIZE AND SPACING OF TIES
12"	#5	#3@10"
	#6 TO #10	#3@12"
	#11 & LARGER	#4@12"
14"	#6	#3@12"
	#7 TO #10	#3@14"
	#11 & LARGER	#4@14"
16"	#6	#3@12"
	#7	#3@14"
	#8 TO #10	#3@16"
	#11 & LARGER	#4@16"
18"	#6	#3@12"
	#7	#3@14"
	#8	#3@16"
	#9 & #10	#3@18"
	#11 & LARGER	#4@18"
20"	#6	#3@12"
	#7	#3@14"
	#8	#3@16"
	#9 & #10	#3@18"
	#11 & LARGER	#4@18"
22" & LARGER	#6	#3@12"
	#7	#3@14"
	#8	#3@16"
	#9 & #10	#3@18"
	#11 & LARGER	#4@18"

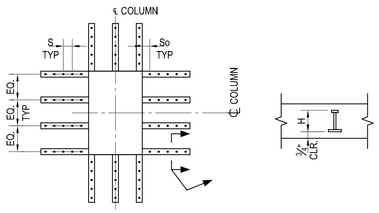
A S-310 TYP COLUMN SPLICE & COLUMN TIE DETAILS NTS

NOTES:
1. IF "X" EXCEEDS 6", PROVIDE TIES BETWEEN OPPOSITE BARS AS SHOWN ABOVE.
2. IF "X" IS EQUAL TO OR LESS THAN 6", PROVIDE TIES AT ALTERNATING LONGITUDINAL BARS BETWEEN OPPOSITE BARS.
3. WHERE LONGITUDINAL BARS ARE OFFSET, TIES OR PARTS OF FLOOR CONSTRUCTION SHALL BE PROVIDED TO RESIST 1.5 TIMES THE HORIZONTAL COMPONENT OF THE FORCE IN THE INCLINED PORTION OF THE OFFSET BAR.
4. TIES SHALL BE DESIGNED FOR SHEAR IF REQUIRED.



NOTES:
1. BENT PORTION OF COLUMN REINFORCEMENT SHALL BE WITHIN THE SHALLOWEST MEMBER FRAMING INTO THE COLUMN.
2. MAXIMUM SLOPE SHALL BE 1:6.
3. IF SLOPE REQUIRED IS GREATER THAN 1:6 OR THE OFFSET IS MORE THAN 3", STOP VERTICAL BARS 3" BELOW TOP OF FLOOR CONSTRUCTION AND PROVIDE LOOSE DOWELS. LENGTH OF DOWELS LAP SPLICED WITH LONGITUDINAL BARS. TOTAL AREA OF EXTENDED BARS AND/OR DOWELS SHALL EQUAL AREA OF VERTICAL REINFORCEMENT OF COLUMN ABOVE.
4. WHERE BEAMS FRAME FROM FOUR DIRECTIONS INTO COLUMN, TERMINATE TIES 3" BELOW LOWEST REINFORCEMENT IN SHALLOWEST BEAM.
5. LAP SPLICES SHALL SATISFY REQUIREMENTS FOR ALL FACTORED LOAD COMBINATIONS. IF LOAD COMBINATIONS INDUCE TENSION IN SOME COLUMN BARS, EACH BAR SPLICE SHALL BE DESIGNED FOR THE MAXIMUM CALULATED BAR TENSILE FORCE.

B S-310 TYP COLUMN SPLICE DETAIL NTS

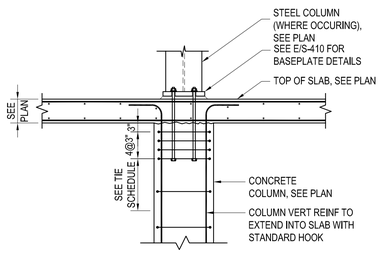


TYPICAL STUDRAIL ARRANGEMENT

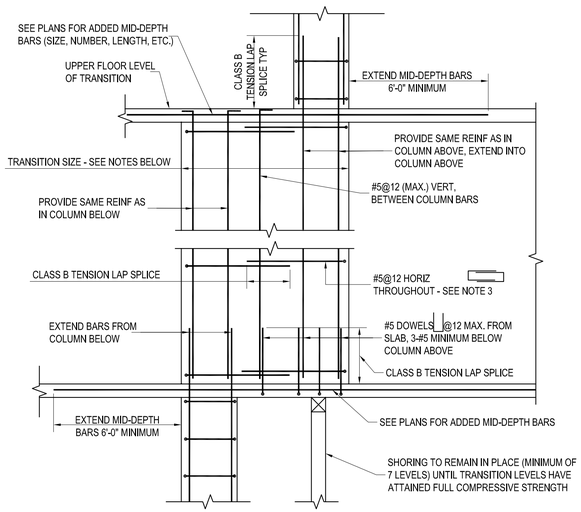
NOTES:
1. INSTALL A MINIMUM OF 2 STUDRAILS PER COLUMN FACE.
2. WHERE EDGE OF SLAB IS FLUSH WITH COLUMN FACE, STUDRAILS SHALL BE INSTALLED AT LEAST 2" FROM SLAB EDGE.
3. SEE SPECIFICATIONS FOR STUDRAIL MANUFACTURER INFORMATION.
4. SR7 STUD RAILS TO BE PLACED ABOVE OVERLAPPING STUD RAILS.
5. REDUCE STUD RAIL DEPTH TO H = 8.5" WHERE STUD RAIL EXTENDS INTO 10" SLAB.
6. STUD RAILS TO BE INSTALLED AT 3 COLUMN FACES ENTIRELY WITHIN 24" SLAB.

STUDRAIL REINFORCING SCHEDULE								
MARK	# COLUMN FACES	# RAILS PER COLUMN	STUDS PER RAIL	H	STUD DIA.	So	S	COMMENT
SR1	4	3 E.F.	14	6.5"	1/2"	3.25"	3.25"	
SR2	4	3 E.F.	6	8.5"	3/4"	4.25"	6.375"	
SR3	4	3 E.F.	5	6.5"	3/4"	3.25"	4.875"	
SR4	4	2 E.F.	5	10.5"	3/4"	5.25"	7.875"	
SR5	4	2 E.F.	3	12.5"	3/4"	6.25"	9.375"	
SR6	4	3 E.F.	7	6.5"	3/4"	3.25"	4.875"	
SR7	4	2 E.F.	8	20.0"	3/4"	9.875"	7.25"	SEE NOTE 4
SR8	4	5 E.F.	7	20.5"	3/4"	9.00"	9.00"	SEE NOTE 5
SR9	3	5 E.F.	14	20.5"	3/4"	8.00"	8.00"	SEE NOTE 6
SR10	4	3 E.F.	4	8.5"	3/4"	4.25"	6.375"	
SR11	4	3 E.F.	5	11.25"	3/4"	5.625"	8.5"	

D S-310 TYP STUDRAIL DETAIL AND SCHEDULE NTS

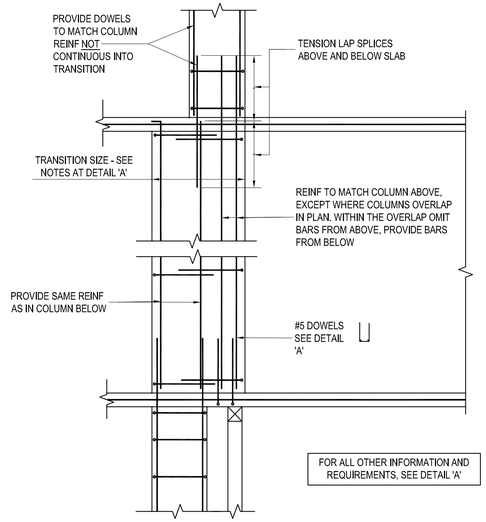


C S-310 TYP STEEL COLUMN ON CONCRETE SLAB DETAIL NTS



E S-310 DETAIL A - ELEVATION OF "WALKING" COLUMN TRANSITION NTS

NOTES:
1. FOR DIMENSIONS AND LOCATION OF TRANSITION SEE COLUMN LOCATION PLANS AND/OR COLUMN SCHEDULE.
2. VERTICAL REINFORCEMENT IN TRANSITION TO BE AS SHOWN IN DETAILS 'A' AND 'B' UNLESS THE COLUMN SCHEDULE CALLS FOR SPECIFIC VERTICAL REINFORCEMENT WITHIN TRANSITIONS.
3. IN ADDITION TO #5@12 HORIZONTAL HOOPS, PROVE HOOPS AND CROSSTIES TO SATISFY TIEING REQUIREMENTS OF TYPICAL COLUMN DETAILS FOR ALL VERTICAL REINFORCEMENT.



F S-310 DETAIL B - ELEVATION OF "WALKING" COLUMN TRANSITION NTS

NOTE: USE THIS DETAIL WHERE COLUMNS ABOVE AND BELOW TRANSITION OVERLAP IN PLAN

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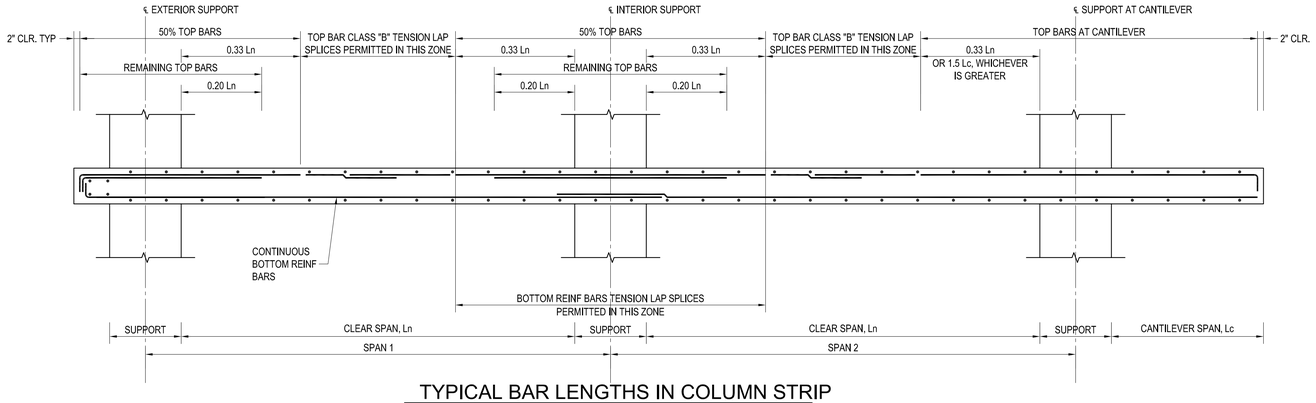
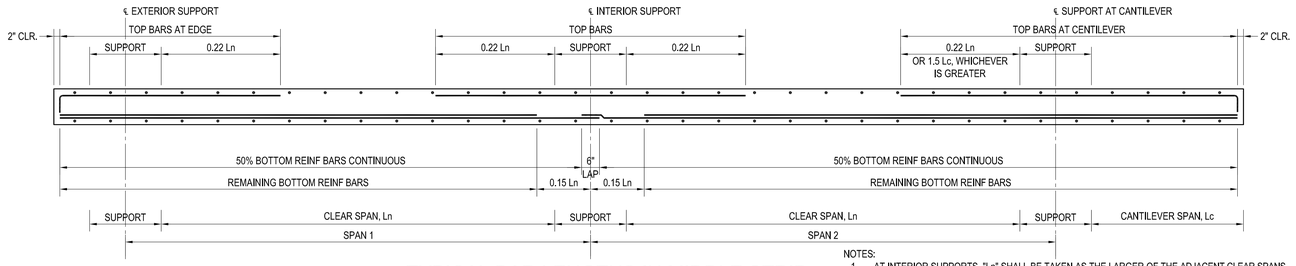


TYPICAL BAR LENGTHS IN COLUMN STRIP



TYPICAL BAR LENGTHS IN MIDDLE STRIP

A S-311 TYP TWO WAY SLAB REINFORCING DETAIL

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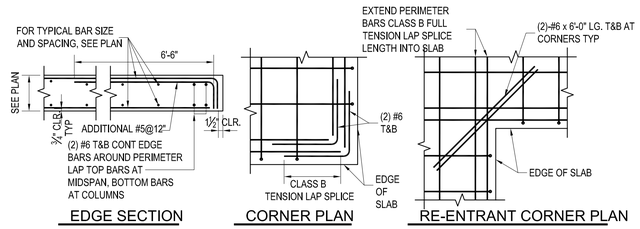
NOTES:
1. AT INTERIOR SUPPORTS, "Ln" SHALL BE TAKEN AS THE LARGER OF THE ADJACENT CLEAR SPANS
2. WHEN INDICATED ON PLAN, TOP BARS SHALL BE CONTINUOUS AT MID-SPAN OR TENSION LAP SPLICED IN THE ZONE INDICATED IN DETAIL.
3. PROVIDE A MINIMUM OF (2) CONTINUOUS BOTTOM BARS IN EACH DIRECTION WITHIN THE REGION BOUND BY THE VERTICAL BARS. PROVIDE A STANDARD 90 OR 180 DEGREE HOOK AT EXTERIOR COLUMN FOR THESE BARS.
4. CONTINUOUS TOP BARS (IF REQUIRED) SHALL BE LAPPED AT THE CENTER OF THE SLAB (CLASS B TENSION LAP SPLICE).
5. CONTINUOUS BOTTOM BARS SHALL BE LAPPED AT THE SUPPORTS (CLASS B TENSION LAP SPLICE).
6. BAR LENGTHS SHOWN ON PLAN GOVERN OVER THE SLAB FRACTIONS SHOWN IN THIS DETAIL.
7. SEE PLAN FOR ORDER OF BAR LAYERING.
8. LAP SPLICES SHALL BE STAGGERED

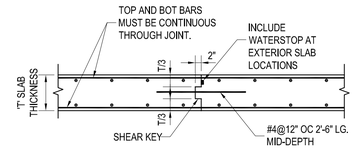


EDGE SECTION | CORNER PLAN | RE-ENTRANT CORNER PLAN

NOTE: PROVIDE STD END HOOK, TILT AS NECESSARY TO FIT IN SLAB. AT CONTRACTOR'S OPTION, PROVIDE 180 DEGREE HOOK IF STD 180 HOOK FITS IN SLAB AND CONFORMS TO CLEAR COVER REQUIREMENTS.

B S-311 TYP REINFORCING AT SLAB EDGE DETAIL

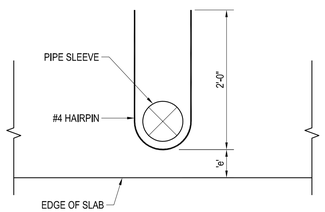
NTS



NOTES:
1. SLAB CONSTRUCTION JOINTS MUST BE LOCATED WITHIN MIDDLE THIRD OF SLAB SPAN.
2. CONTRACTOR TO PROVIDE SUBMITTAL WITH PROPOSED CONSTRUCTION JOINT LOCATION FOR REVIEW
3. SURFACE OF CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, ALL CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER REMOVED.

C S-311 TYP CONCRETE FRAMED SLAB CONSTRUCTION JOINT DETAIL

NTS

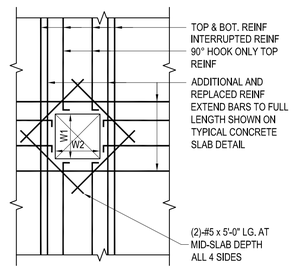


NOTES:
1. 'e' MUST BE 2" MINIMUM.
2. FOR 'e' LESS THAN 6", ADD #4 HAIRPIN AS SHOWN.

D S-311 TYP PENETRATION AT SLAB EDGE DETAIL

NTS

NOTES:
1. IF REINF INTERRUPTED (TOP OR BOTTOM) 1/2 THE EQUIVALENT AREA SHALL BE REPLACED ON EACH SIDE OF OPENING ((1)-#5 TOP & BOTTOM MINIMUM EACH SIDE).
2. IF OPENING IS SMALLER THAN SPACING OF SLAB BARS, NO ADDITIONAL REINF IS REQ'D. SLAB BARS TO BE PLACED AT EDGES OF OPENING.
3. IF OPENING IS NEAR OR AGAINST EXTERIOR BEAMS OR COLUMNS & REINF CANNOT BE EXTENDED, HOOK ADDITIONAL BARS.



E S-311 TYP CONC SLAB OPENING DETAIL

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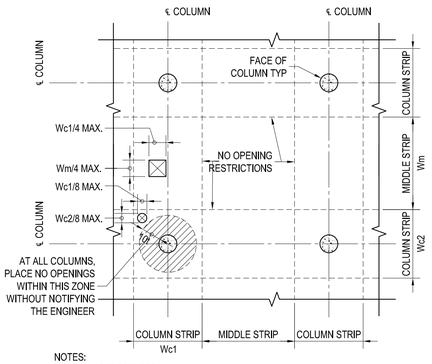

NOTES:
1. t = SLAB THICKNESS
2. SEE PLAN FOR CENTERLINES OF COLUMN STRIPS AND TYPICAL REINFORCEMENT PLANS FOR EXTENTS OF CENTER AND MIDDLE STRIPS.
3. ALL REINFORCEMENT INTERRUPTED BY OPENINGS SHALL BE ADDED TO EITHER SIDE OF THE OPENING. SEE TYPICAL CONC SLAB OPENING DETAIL.

F S-311 TYP TWO-WAY SLAB OPENING RESTRICTIONS DETAIL

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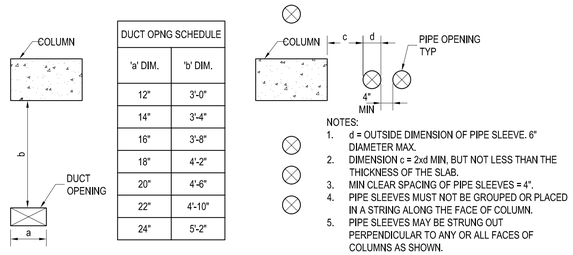

DUCT OPENING | PIPE SLEEVES

NOTES:
1. d = OUTSIDE DIMENSION OF PIPE SLEEVE. 6" DIAMETER MAX.
2. DIMENSION c = 2xd MIN. BUT NOT LESS THAN THE THICKNESS OF THE SLAB.
3. MIN CLEAR SPACING OF PIPE SLEEVES = 4".
4. PIPE SLEEVES MUST NOT BE GROUPED OR PLACED IN A STRING ALONG THE FACE OF COLUMN.
5. PIPE SLEEVES MAY BE STRUNG OUT PERPENDICULAR TO ANY OR ALL FACES OF COLUMNS AS SHOWN.

NOTE: ALL OPENINGS WITHIN 10 x SLAB THICKNESS MUST BE SUBMITTED TO THE ENGINEER FOR APPROVAL. THE CRITERIA LISTED IN THIS DETAIL CONSTITUTE OPENING CONFIGURATIONS LIKELY TO BE APPROVED BY THE ENGINEER OF RECORD.

G S-311 TYP TWO-WAY SLAB OPENING RESTRICTIONS NEAR COL DETAIL

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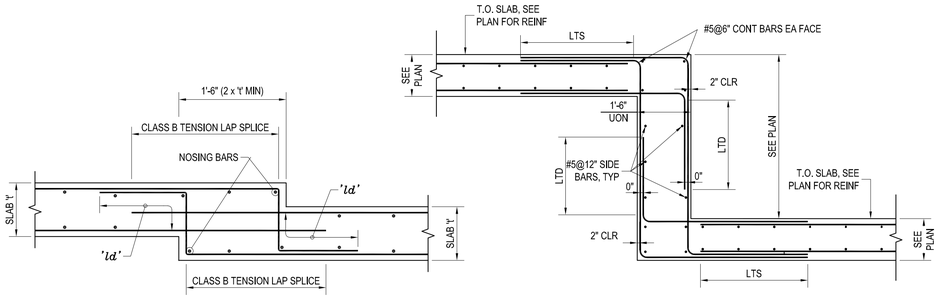
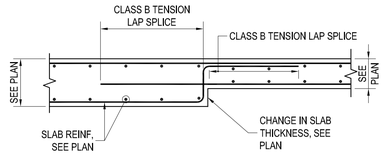


NOTE: SEE PLAN FOR SLAB THICKNESS AND REINFORCEMENT.

SLAB STEP LESS THAN SLAB THICKNESS

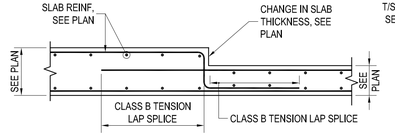
SLAB STEP GREATER THAN SLAB THICKNESS

A S-312 TYP REINFORCED CONCRETE SLAB FOLD DETAIL NTS



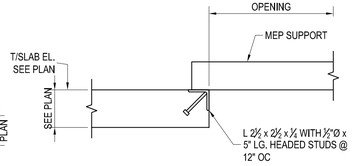
NOTE: SEE PLAN FOR SLAB THICKNESS AND REINFORCEMENT.

B S-312 TYP CHANGE IN SLAB THICKNESS DETAIL NTS



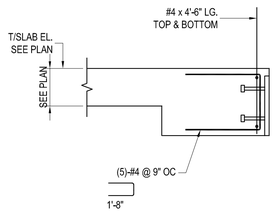
NOTE: SEE PLAN FOR SLAB THICKNESS AND REINFORCEMENT.

C S-312 TYP CHANGE IN SLAB THICKNESS DETAIL NTS

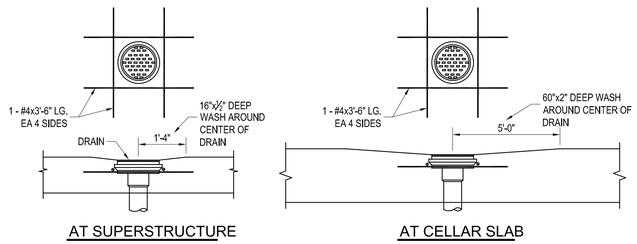


NOTE: INSTALL EMBEDS ON SHORT SIDES OF OPENINGS ONLY, UON

D S-312 TYP EMBED AT MECH. OPENING DETAIL NTS

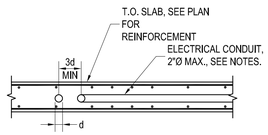


E S-312 TYP ADDITIONAL REINF AT EMBEDS DETAIL NTS



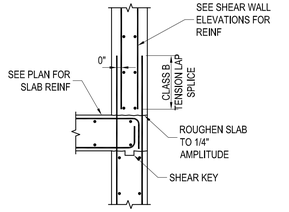
NOTE: SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR DRAIN LOCATIONS AND DETAILS

F S-312 TYP FLOOR DRAIN IN CONC SLAB DETAIL NTS

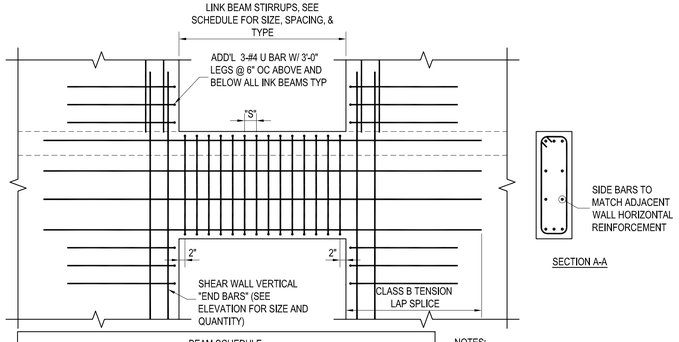


NOTES:
1. CONTRACTOR TO PROVIDE A PLAN SHOWING ALL PROPOSED CONDUIT SIZES AND LOCATIONS FOR ENGINEER REVIEW AND APPROVAL PRIOR TO INSTALLING ANY CONDUIT.
2. ALUMINUM CONDUIT IS NOT PERMITTED. CONDUIT MUST BE UNCOATED OR GALVANIZED IRON OR STEEL NOT THINNER THAN STANDARD SCHEDULE 40 STEEL PIPE.
3. MAXIMUM CONDUIT SIZE ALLOWED IN SLAB IS 2".
4. CONDUIT SHALL BE INSTALLED WITHIN THE MIDDLE ⅓ OF THE SLAB DEPTH.
5. CONDUIT SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS OR WIDTHS ON CENTER.
6. CONDUIT CANNOT BE PLACED WHERE IT WOULD INTERFERE OR DISPLACE REINFORCEMENT FROM ITS PROPER LOCATION.
7. CONDUIT SHALL BE INSTALLED NO CLOSER THAN 2 x SLAB THICKNESS OF A COLUMN.
8. CONDUIT CANNOT BE TIED TO REINFORCING STEEL. SEPARATE CHAIRS MUST BE PROVIDED FOR CONDUITS.
9. ONLY ONE LAYER OF CONDUIT MAY CROSS OVER ANOTHER LAYER OF CONDUIT AT ANY ONE LOCATION AND CROSSING CONDUIT SHALL OCCUR ONLY WITHIN THE MIDDLE HALF OF THE SLAB SPAN.

G S-312 TYP CONDUIT IN SLAB DETAIL NTS



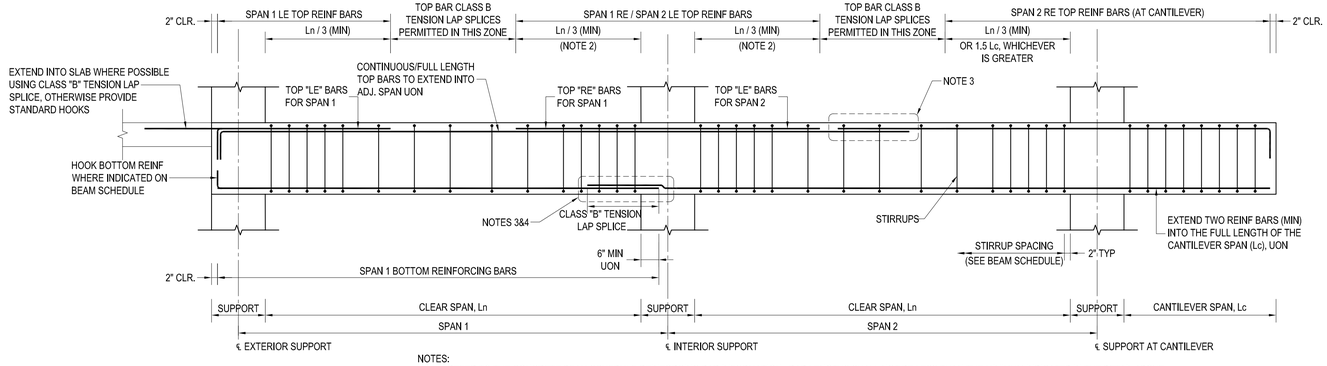
H S-312 TYP SLAB AT SHEAR WALLS DETAIL NTS



BEAM SCHEDULE				
MARK	REINFORCEMENT TOP & BOTTOM BARS CONT	STIRRUPS SIZE	STIRRUPS TYPICAL SPACING, "S"	REMARKS
LB01	6-#6	#5	12" OC - 2 LEG	BARS TO BE PLACED IN 2 LAYERS
LB02	12-#6	#5	12" OC - 2 LEG	BARS TO BE PLACED IN 4 LAYERS
LB03	3-#6	#5	12" OC - 2 LEG	---
LB04	3-#6	#5	12" OC - 2 LEG	---

NOTES:
1. FOR LINK BEAM DEPTH, SEE SHEAR WALL ELEVATIONS.
2. FOR LINK BEAM WIDTH, MATCH SHEAR WALL THICKNESS UON
3. BEAMS SHALL BE CONSTRUCTED MONOLITHICALLY WITH SUPPORTING MEMBERS AT EACH END.
4. WHERE FULL TENSION LAP SPLICE IS NOT POSSIBLE DUE TO TERMINATION OF WALL ADJACENT TO LINK BEAM, PROVIDE STD HOOK.

J S-312 TYP LINK BEAM DETAIL NTS



NOTES:
1. REFER TO THE CONCRETE BEAM SCHEDULE REINFORCING BAR INFORMATION. BAR LENGTHS SHALL NOT BE LESS THAN THE MINIMUM INDICATED IN THE DETAIL ABOVE.
2. FOR TOP BARS LOCATED AT A CONTINUOUS SUPPORT, "Ln" REFERS TO THE LONGER OF THE ADJACENT CLEAR SPANS.
3. FOR SPANDREL BEAMS (ALONG SLAB EDGES) A MINIMUM OF 1/6 OF THE TOP REINFORCING STEEL SHALL BE CONTINUOUS FULL LENGTH OF THE SPAN (BASED ON THE LARGER OF THE RIGHT OR LEFT END GROSS AREA, MIN OF (2) BARS) UNLESS OTHERWISE NOTED. EXTEND AT LEAST 1/4 OF THE BOTTOM REINFORCING STEEL FROM THE BEAM WITH THE LARGER GROSS AREA OF BOTTOM REINFORCING STEEL INTO THE ADJACENT SPAN (MIN OF (2) BARS).
4. FOR NON-SPANDREL BEAMS WHERE "TYPE 1" STIRRUPS ARE USED, EXTEND AT LEAST ¾ OF THE BOTTOM REINFORCING STEEL FROM THE BEAM WITH THE LARGER GROSS AREA OF BOTTOM REINFORCING STEEL INTO THE ADJACENT SPAN AND LAP USING A CLASS "B" TENSION LAP SPLICE (MIN OF (2) BARS). FOR ALL OTHER STIRRUPS TYPES, EXTEND BOTTOM BARS A MINIMUM OF 6" INTO THE SUPPORT UNLESS OTHERWISE NOTED.
5. TOP REINFORCING BAR LAP SPLICES SHALL BE LOCATED IN THE MIDDLE THIRD OF THE CLEAR SPAN. BOTTOM REINFORCING LAP SPLICES SHALL BE LOCATED AT OR WITHIN A DISTANCE OF Ln / 6 FROM THE SUPPORT.
6. COLUMN REINFORCING NOT SHOWN THIS DETAIL FOR CLARITY.

K S-312 TYP CONCRETE BEAM REINFORCING DETAIL NTS

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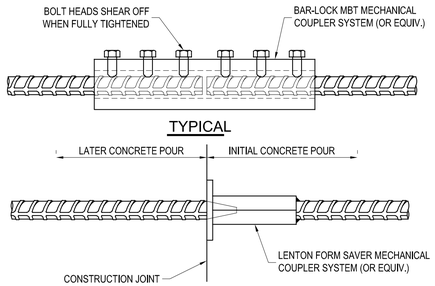
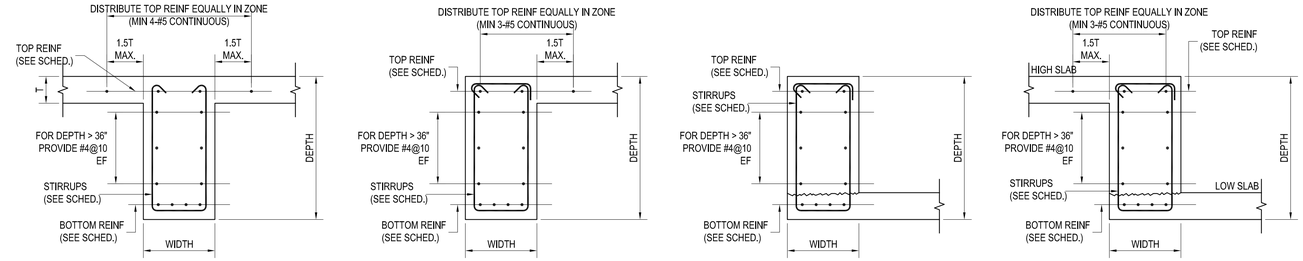


AT CONSTRUCTION JOINTS

NOTE: ALL MECHANICAL REINFORCEMENT SPLICES MUST DEVELOP 125% OF YIELD STRENGTH OF REINFORCEMENT.

A S-313 TYP MECHANICAL REINFORCEMENT SPLICE DETAIL

NTS



TYPICAL BEAM | INTERIOR/EXTERIOR EDGE BEAM | UPTURNED BEAM | BEAM AT STEP

NOTE: SLAB REINFORCING NOT SHOWN FOR CLARITY

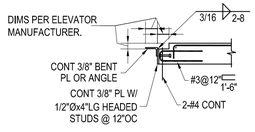
B S-313 TYP CONCRETE BEAM SECTIONS

NTS

CONCRETE BEAM SCHEDULE						
MARK	WIDTH	DEPTH	REINFORCEMENT			COMMENTS
			TOP BARS	BOTTOM BARS	SHEAR STIRRUPS	
B-M01	1'-2"	3'-0"	(2) #8	(3) #9	2L #3 @ 12" o.c.	
B-M02	2'-0"	3'-0"	(5) #9	(5) #9	2L #4 @ 8" o.c.	(5) #6 CONT. EA. SIDE
B-M03	2'-0"	2'-10"	(3) #9	(3) #9	2L #4 @ 12" o.c.	(2) #5 CONT. EA. SIDE
B-M04	1'-8"	2'-0"	(4) #5	(8) #6*	2L #4 @ 9" o.c.	*2 ROWS
B-M05	1'-0"	2'-8"	(2) #5	(4) #6*	2L #3@ 12" o.c.	*2 ROWS
B-M06	1'-8"	3'-0"	(3) #9	(3) #9	2L #4 @ 12" o.c.	(4) #5 CONT. EA. SIDE
B-M07	1'-0"	3'-0"	(2) #5	(4) #7*	2L #4@ 12" o.c.	*2 ROWS
B-201	1'-8"	2'-0"	(3) #8	(3) #8	2L #3 @ 8" o.c.	
B-202	1'-8"	4'-4"	(3) #8	(3) #8	2L #3 @ 8" o.c.	
B-301	1'-4"	1'-10"	(3) #9	(3) #9	2L #3@ 8" o.c.	DEPTH TO BE 1'-8" SUPPORTING 4TH FLOOR
B-R01	1'-2"	1'-9"	(3) #7	(3) #7	2L #3@ 8" o.c.	
B-R02	1'-2"	3'-1"	(3) #7	(3) #7	2L #3@ 12" o.c.	EXTEND BEAM TOP & BOT REINF. 1.3Ld INTO WALL
B-BK01	1'-0"	1'-6"	(3) #5	(3) #5	2L #3@ 6" o.c.	

C S-313 CONCRETE BEAM SCHEDULE

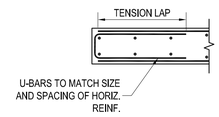
NTS



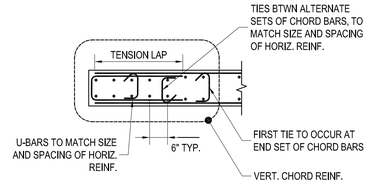
(SEE PLANS FOR OTHER SLAB REINF NOT SHOWN)

F S-313 TYP SILL AT ELEV. DOOR DETAIL

NTS



W/O CHORD REINF.



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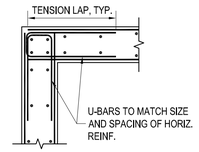

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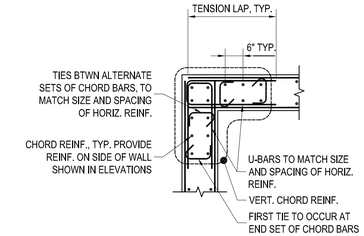

W/ CHORD REINF.

D S-313 SHEAR WALL END

NTS

E S-313 SHEAR WALL CORNER

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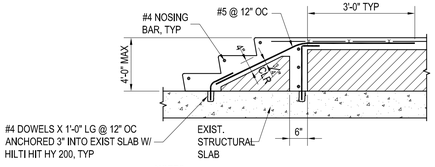
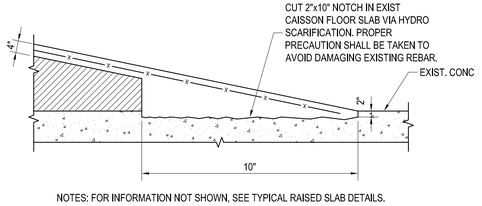


NOTES:
1. FOR INFORMATION NOT SHOWN, SEE TYPICAL RAISED SLAB DETAILS THIS SHEET.
2. CONTRACTOR SHALL TAKE PROPER PRECAUTIONS TO AVOID OVERDRILLING EPOXY ANCHOR HOLES INTO BASEMENT SLAB BEYOND EMBEDMENT DEPTH SPECIFIED.

G S-313 TYP RAISED SLAB DETAIL @ STAIR DETAIL

NTS



NOTES: FOR INFORMATION NOT SHOWN, SEE TYPICAL RAISED SLAB DETAILS.

H S-313 TYP RAMP BASE DETAIL

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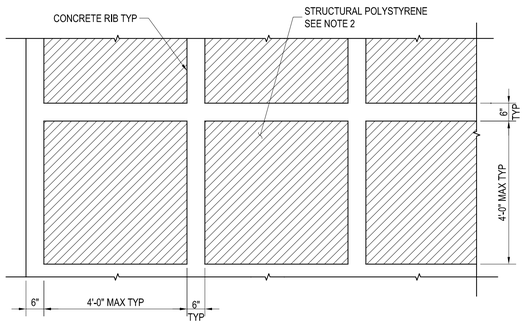


RAISED SLAB THICKNESS 'T' > 6"

NOTES:
1. TOPPING SLABS SHALL BE LIGHTWEIGHT STRUCTURAL CONCRETE.
2. STRUCTURAL POLYSTYRENE SHALL BE OWENS CORNING FOAMULAR 400 HIGH DENSITY EXTRUDED POLYSTYRENE INSULATION OR APPROVED EQUAL: MIN REQUIRED COMPRESSIVE STRENGTH = 40 PSI (PER ASTM D1621), MIN REQUIRED COMPRESSIVE MODULUS = 1400 PSI (PER ASTM D1621), MIN REQUIRED INSULATION FOUNDATION MODULUS "K" = 300 PCI.

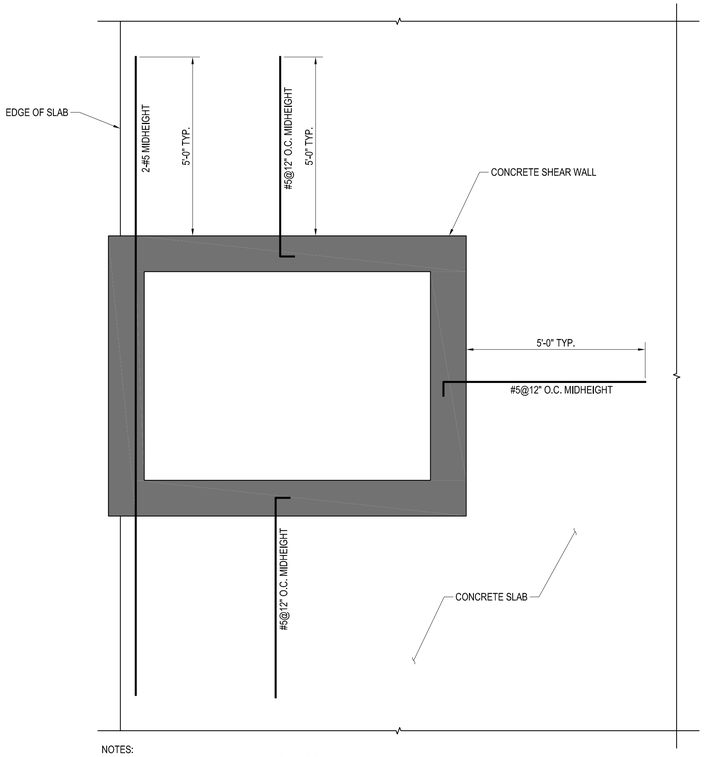
A S-314 TYP CONCRETE TOPPING SLAB DETAIL

NTS

CONCRETE WALL SCHEDULE			
MARK	THICKNESS	VERT. REINF.	HORIZ. REINF.
W1	10"	#5@12" EF	#4@12" EF
W2			
W3			

B S-314 CONCRETE WALL REINFORCEMENT SCHEDULE

NTS



NOTES:
1. SLAB REINF. SHOWN IS IN ADDITION TO SLAB REINF. SHOWN ON PLANS.
2. SEE S-210 SERIES FOR SHEAR WALL REINF.

C S-314 ADDITIONAL SLAB REINF. AT SHEAR WALLS

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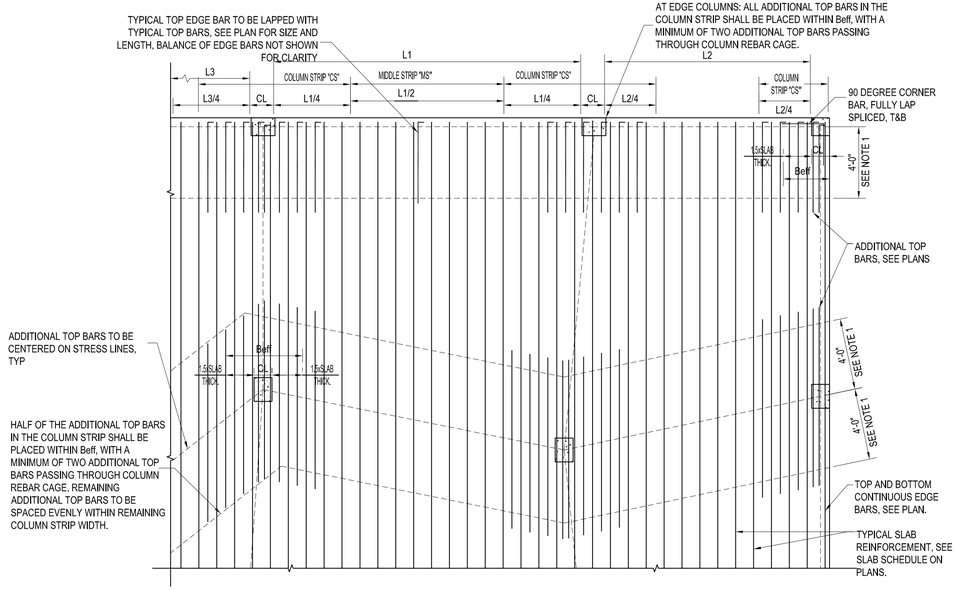
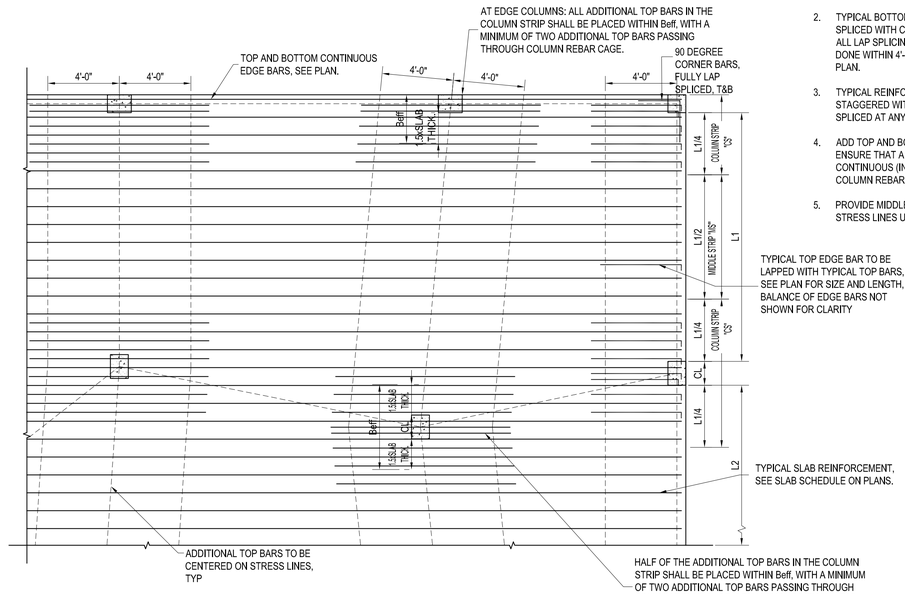


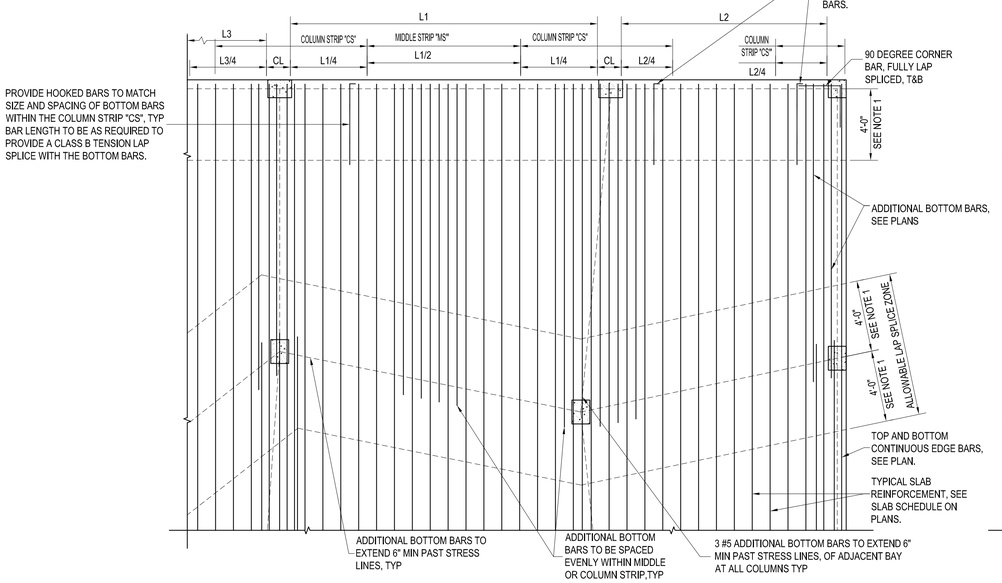
A S-315 FLAT PLATE REBAR PLAN
NORTH / SOUTH TOP MAT ARRANGEMENT
NTS



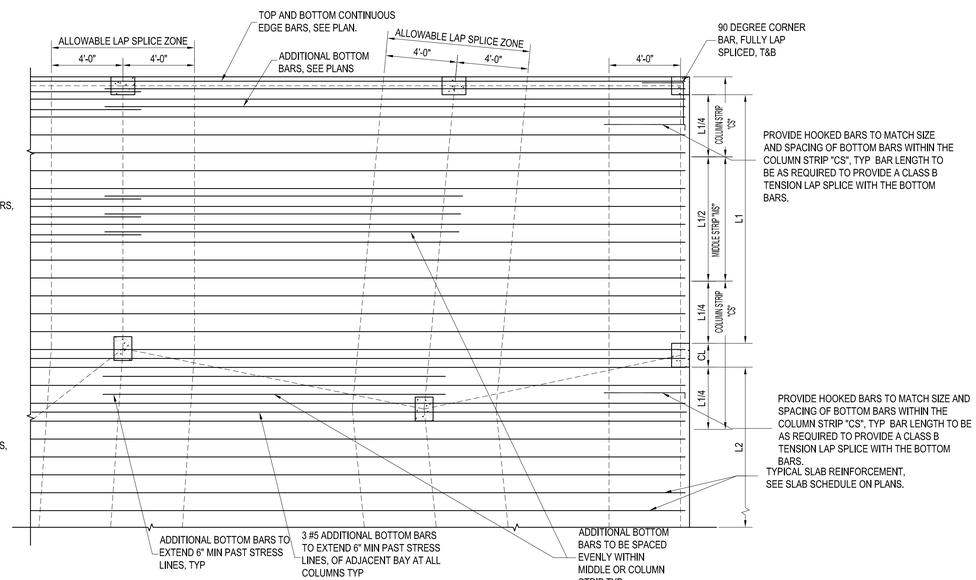
B S-315 FLAT PLATE REBAR PLAN
EAST / WEST TOP MAT ARRANGEMENT
NTS

GENERAL NOTES:

1. TYPICAL TOP MAT REINFORCEMENT SHALL BE LAP SPLICED WITH CLASS B TENSION LAP SPLICE LENGTHS. NO LAP SPLICING OF REINFORCEMENT IS ALLOWED WITHIN 4'-0" OF THE STRESS LINES SHOWN ON PLAN.
2. TYPICAL BOTTOM MAT REINFORCEMENT SHALL BE LAP SPLICED WITH CLASS B TENSION LAP SPLICE LENGTHS. ALL LAP SPLICING OF REINFORCEMENT SHALL BE DONE WITHIN 4'-0" OF THE STRESS LINES SHOWN ON PLAN.
3. TYPICAL REINFORCEMENT LAP SPLICES SHALL BE STAGGERED WITH NO MORE THAN 50% OF THE BARS SPLICED AT ANY LOCATION.
4. ADD TOP AND BOTTOM BARS AS NECESSARY TO ENSURE THAT A MINIMUM OF (2) BARS ARE PLACED CONTINUOUS (IN EACH DIRECTION) WITHIN THE COLUMN REBAR CAGE.
5. PROVIDE MIDDLE STRIP TOP BARS CENTERED ON STRESS LINES U.O.N.



C S-315 FLAT PLATE REBAR PLAN
NORTH / SOUTH BOTTOM MAT ARRANGEMENT
NTS



D S-315 FLAT PLATE REBAR PLAN
EAST / WEST BOTTOM MAT ARRANGEMENT
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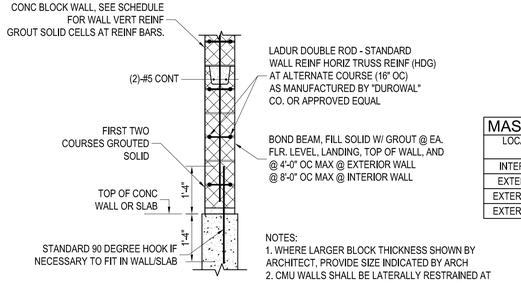
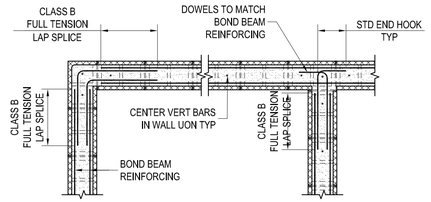


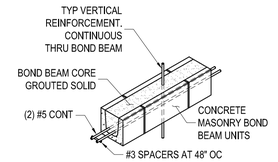
MASONRY WALL REINFORCEMENT SCHEDULE			
LOCATION & HEIGHT OF WALL	MINIMUM BLOCK THICKNESS	VERTICAL REINFORCEMENT	DOWELS FROM WALL OR SLAB
INTERIOR UP TO 20'-0"	8"	#4@48"	#4@48"
EXTERIOR UP TO 10'-0"	8"	#5@32"	#5@16"
EXTERIOR 10'-1" TO 15'-0"	10"	#5@16"	#5@16"
EXTERIOR 15'-1" TO 20'-0"	12"	#5@16"	#5@16"

NOTES:
1. WHERE LARGER BLOCK THICKNESS SHOWN BY ARCHITECT, PROVIDE SIZE INDICATED BY ARCH
2. CMU WALLS SHALL BE LATERALLY RESTRAINED AT BASE AND TOP OF WALL AS SHOWN ON TYPICAL DETAILS.
3. PROVIDE (1) #5 VERT BAR WITHIN 8" OF ENDS AND CORNERS OF WALLS AND WITHIN 8" OF CONTROL JOINTS.

A S-330 TYP CMU WALL REINFORCING DETAIL — NTS

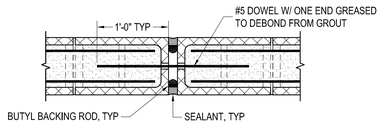


B S-330 TYP CMU WALL BOND BEAM JOINT DETAIL — NTS



NOTES:
1. BOND BEAMS LOCATED AT FLOOR AND ROOF LEVELS TO HAVE REINFORCING CONTINUOUS THROUGH CONTROL JOINTS.
2. INTERMEDIATE BOND BEAMS TO HAVE HORIZONTAL DISCONTINUOUS REINFORCING AT CONTROL JOINTS.

C S-330 TYP CMU BOND BEAM DETAIL — NTS



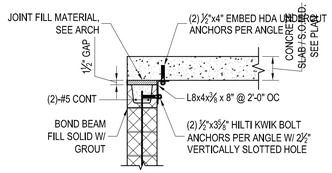
NOTE: SEE "TYP CMU CONTROL JOINT DETAIL FOR CONTROL JOINT SPACING CRITERIA

D S-330 TYP INTERMEDIATE BOND BEAM CMU WALL CONTROL JOINT DETAIL — NTS

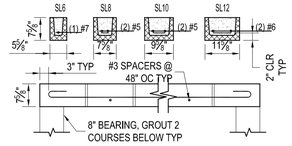
MASONRY LAP SPLICE LENGTHS (f'm = 1500PSI, Fy = 60,000PSI)					
BAR	1½" MIN CLR.	BAR CENTERED IN BLOCK			
		6" CMU	8" CMU	10" CMU	12" CMU
#3	23	16	16	16	16
#4	41	26	21	21	21
#5	63	42	28	26	26
#6	118	82	53	40	40
#7	X	115	73	54	48
#8	X	X	113	82	76
#9	X	X	X	107	103

NOTES:
1. TABULATED VALUES ASSUME UNCOATED REINFORCEMENT. SPLICE LENGTH OF EPOXY COATED BARS SHALL BE TAKEN AS 150 PERCENT OF THE VALUES PROVIDED ABOVE.
2. AT CONTRACTOR'S OPTION, AND FOR VALUES NOT PROVIDED, MECHANICAL SPLICES MAY BE USED. MECHNICAL SPLICES SHALL HAVE THE BARS CONNECTED TO DEVELOP AT LEAST 125% OF THE YIELD STRENGTH, Fy, OF THE BAR.

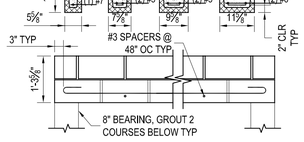
E S-330 MASONRY LAP SPLICE SCHEDULE — NTS



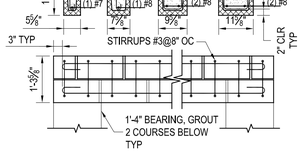
F S-330 TYP EXTERIOR CMU WALL HEAD RESTRAINT DETAIL — NTS



SHORT SPAN LINTELS (MAXIMUM SPAN 8'-0")



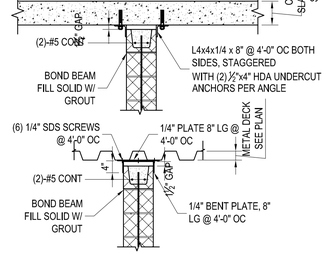
MEDIUM SPAN LINTELS (MAXIMUM SPAN 12'-0")



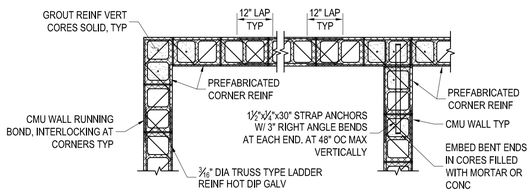
LONG SPAN LINTELS (MAXIMUM SPAN 16'-0")

NOTES:
1. GROUT SOLID ALL MASONRY UNITS.
2. ALL REINFORCING BARS ARE HOOKED AT THE ENDS.
3. MAXIMUM SPANS DO NOT APPLY TO LOAD BEARING WALLS.
4. BOND PATTERN OF LINTEL TO BE RUNNING BOND UON.
5. BOTTOM OF LINTEL SHALL BE SMOOTH MASONRY WITH NO CORES EXPOSED.

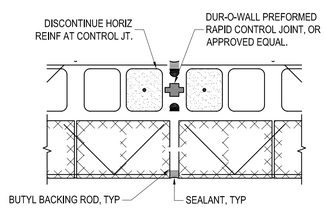
G S-330 TYP CMU BOND BEAM LINTEL DETAIL FOR INTERIOR CONDITIONS — NTS



H S-330 TYP INTERIOR CMU WALL HEAD RESTRAINT DETAIL — NTS

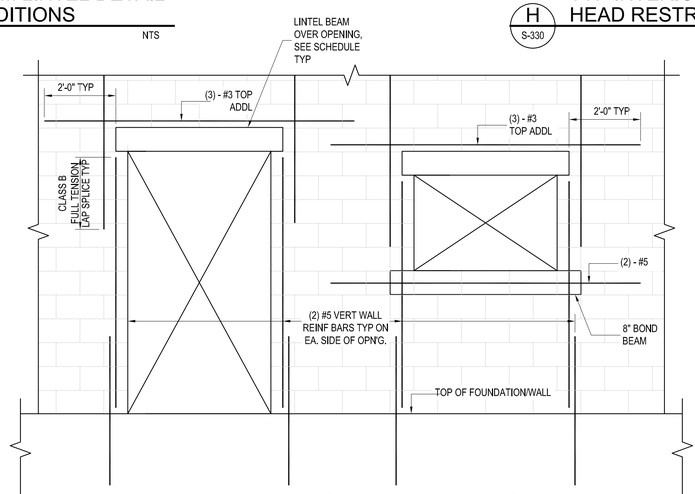


J S-330 TYP CMU WALL JOINT DETAIL — NTS



CONTROL JOINT SPACING CRITERIA
1. INSTALL VERTICAL CONTROL JOINTS IN ALL MASONRY WALLS AS FOLLOWS:
   - MAX SPACING, "S" = LESS OF 1.5xWALL HEIGHT OR 24'-0".
   - AT CHANGES IN WALL HEIGHTS OR THICKNESS.
   - INTERSECTIONS OF WALLS WITH COLUMNS, PIERS AND PILASTERS.
   - AT JOINTS IN FOUNDATIONS, FLOORS, OR ROOF.
   - WITHIN S/2 OF CORNERS OF WALLS OR INTERSECTIONS.
   - NO CLOSER THAN 2'-0" TO EDGE OF ANY OPENING IN WALL.
2. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS OF ALL CONTROL JOINTS IN MASONRY WALLS; WHERE DISCREPANCIES EXISTING BETWEEN CONTROL JOINT SPACING REQUIREMENTS ON ARCHITECTURAL DRAWINGS AND SPACING REQUIREMENTS INDICATED IN THIS DETAIL. CONTRACTOR SHALL NOTIFY THE ARCHITECT AND ENGINEER TO RESOLVE DISCREPANCY.
3. FOR FIRE RATINGS GREATER THAN 2 HOURS, REPLACE PREFORMED GASKET WITH CERAMIC FIBER FELT (ALUMINA-SILICA FIBER).

K S-330 TYP CMU WALL CONTROL JOINT DETAIL — NTS



L S-330 TYP WALL OPENING IN REINFORCED MASONRY WALL-ELEVATION DETAIL — NTS

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1	ISSUE FOR BID	04/23/2021

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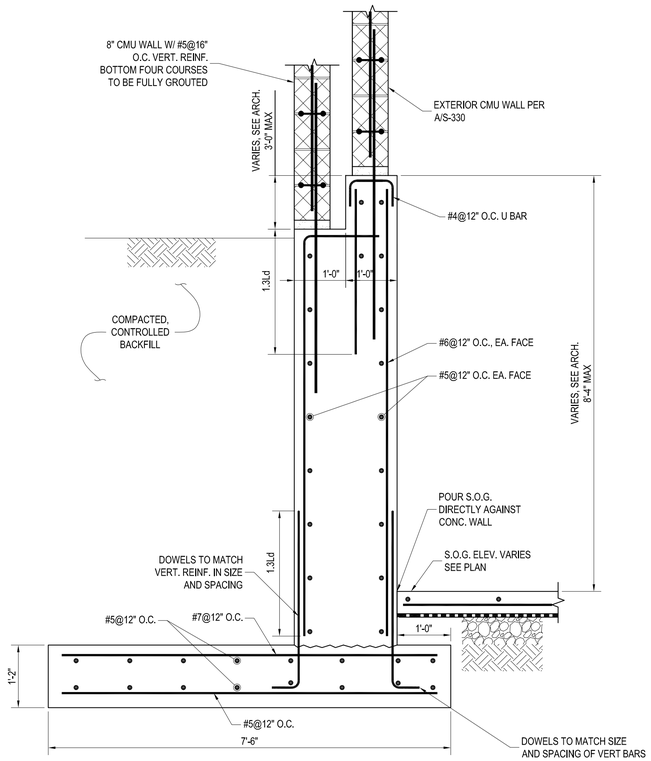
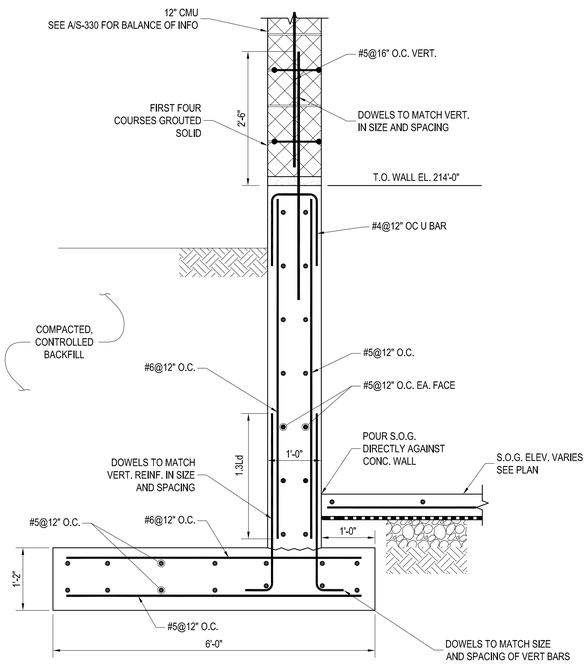


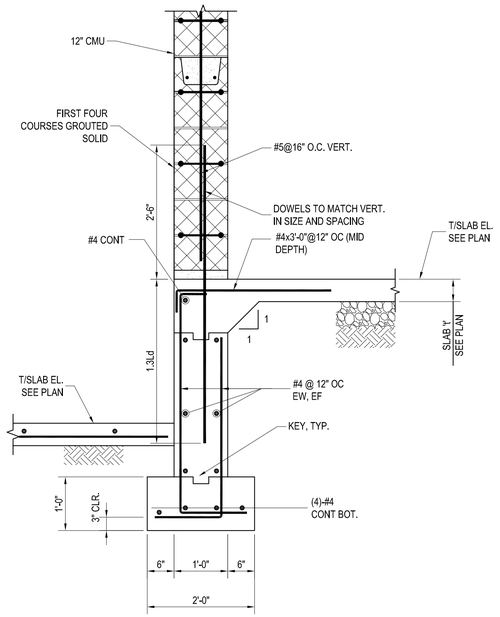
A S-400 CANT. RETAINING WALL

NTS



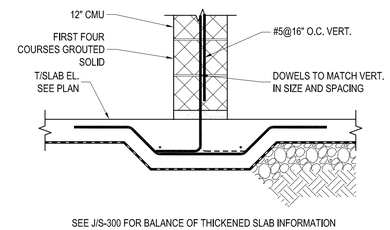
B S-400 LOADING DOCK WALL 1

NTS



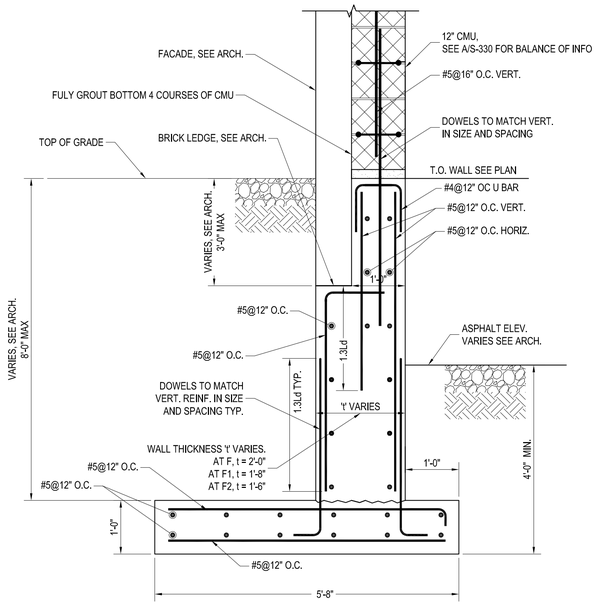
C S-400 LOADING DOCK WALL 2

NTS



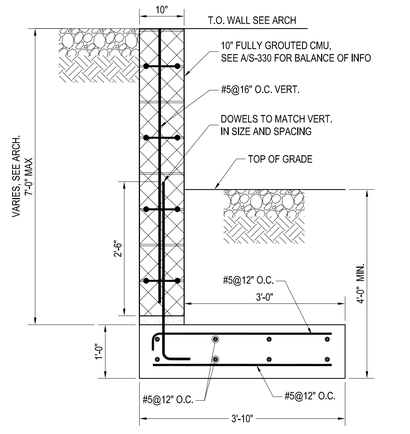
D S-400 LOADING DOCK WALL 3

NTS



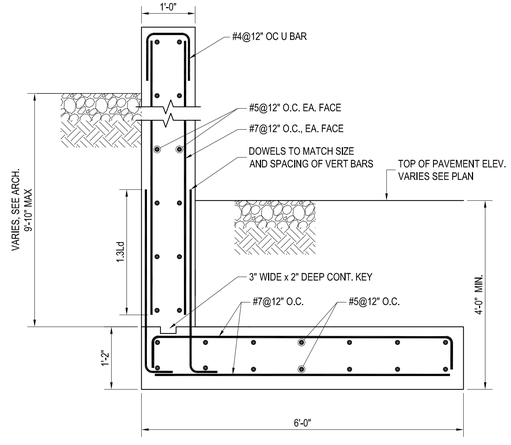
F2 S-400 F1 S-400 F S-400 CANT. RETAINING WALL

NTS



G S-400 CANT. RETAINING WALL

NTS



H S-400 CANT. RETAINING WALL

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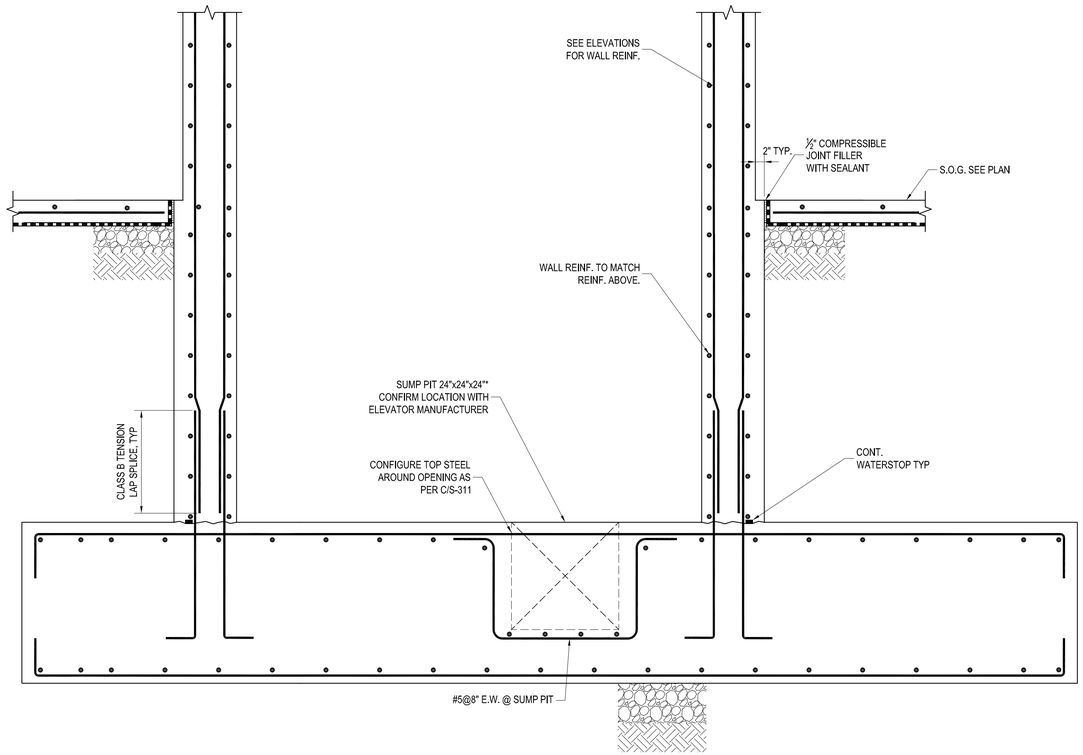
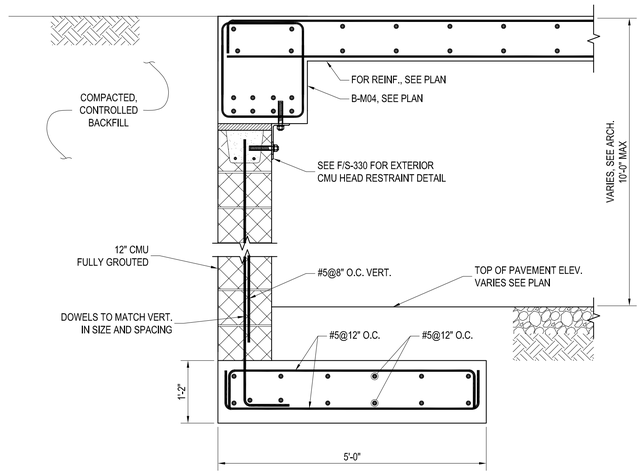
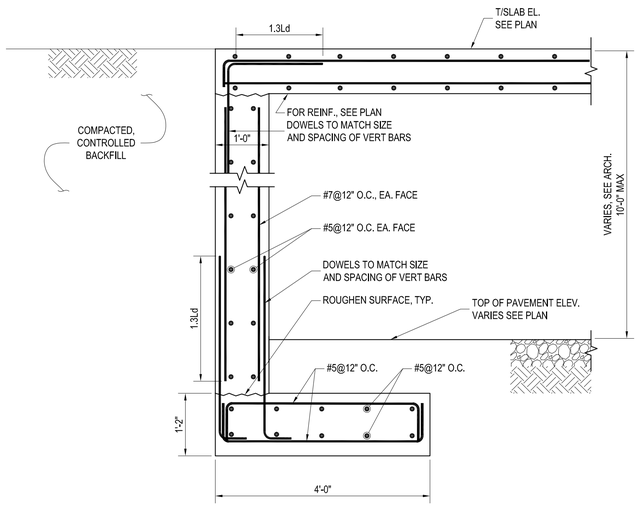


A S-401 SECTION AT ELEVATOR PIT
NTS



B S-401 PARKING GARAGE WALL 1
NTS



C S-401 PARKING GARAGE WALL 2
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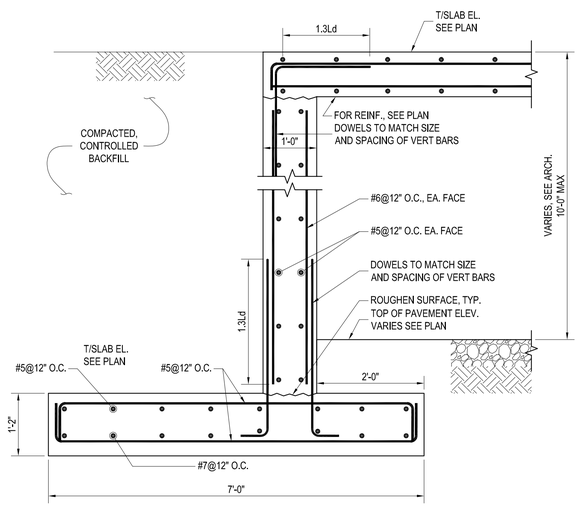

D S-401 PARKING GARAGE WALL 3
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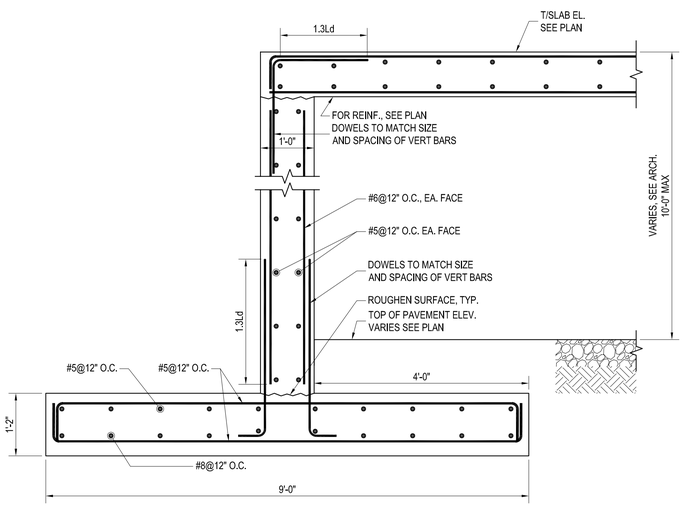

E S-401 PARKING GARAGE WALL 4
NTS

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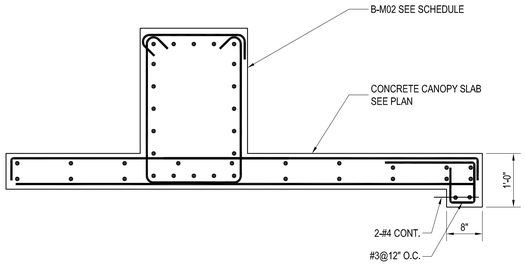
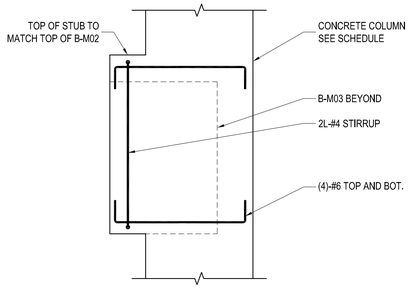
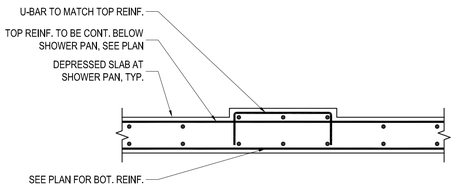


A SECTION AT CONC. CANOPY
S-410 NTS



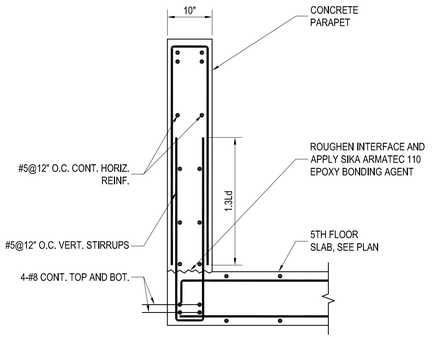
NOTE: SEE BEAM SCHEDULE FOR B-M02 AND B-M03 REINF., NOT SHOWN IN DETAIL FOR CLARITY

B SECTION AT COLUMN STUB
S-410 NTS

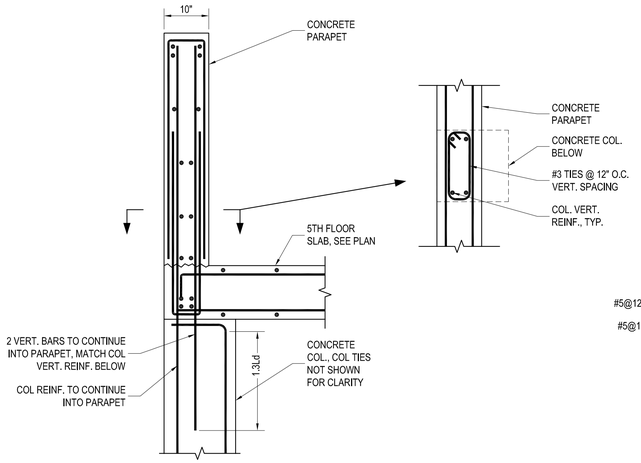


NOTE: FOR SINGLE SHOWER PAN, SEE TYP. CHANGE IN SLAB THICKNESS DETAIL

C SLAB REINF. AT DOUBLE SHOWER PAN
S-410 NTS

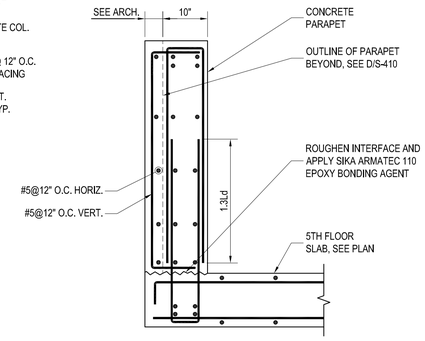


D 5TH FLOOR PARAPET DETAIL
S-410 NTS

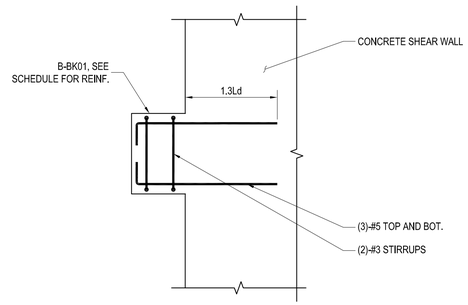


NOTE: SEE D/S-410 FOR BALANCE OF INFORMATION.

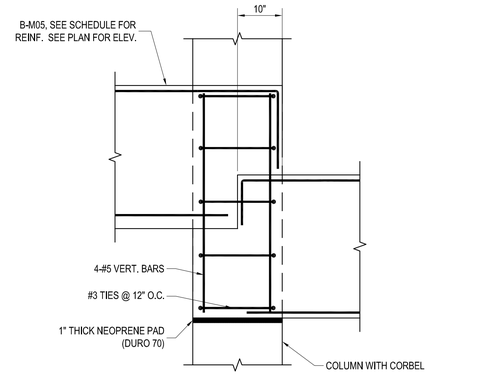
E COLUMN - PARAPET CONNECTION
S-410 NTS



F PARAPET THICKENING DETAIL
S-410 NTS

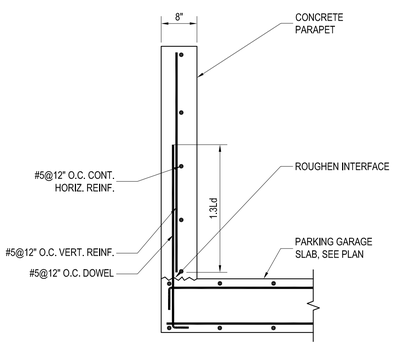


G BEAM STUB AT BULKHEAD
S-410 NTS

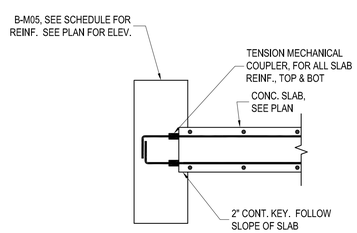


NOTE:BEAM SHEAR REINFORCEMENT AND SLAB NOT SHOWN FOR CLARITY

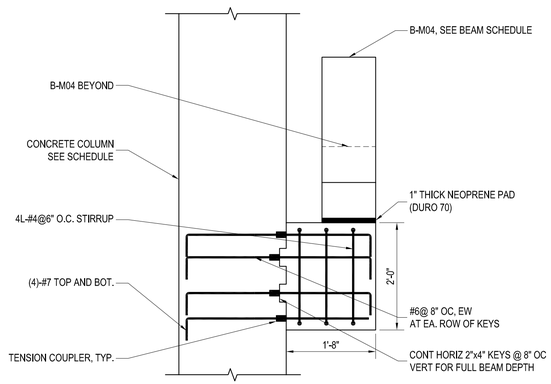
H GARAGE BEAM AT CORBEL
S-410 NTS



J PARKING GARAGE PARAPET DETAIL
S-410 NTS



K GARAGE BEAM SUPP. SLAB
S-410 NTS



NOTE: B-M04 REINFORCEMENT AND SLAB NOT SHOWN IN DETAIL FOR CLARITY

L GARAGE BEAM SUPP. SLAB
S-410 NTS

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SUPERSTRUCTURE CONCRETE DETAILS I

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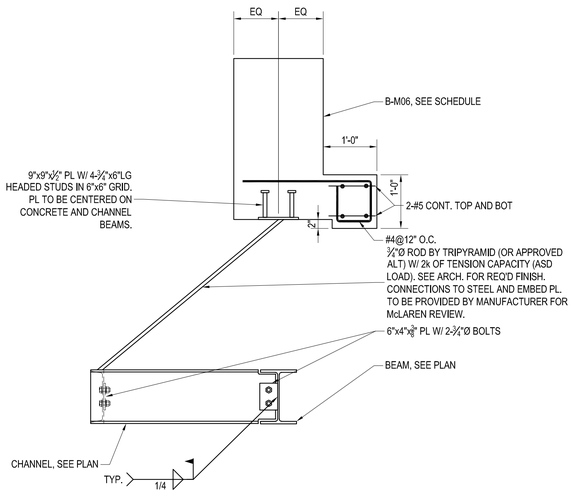
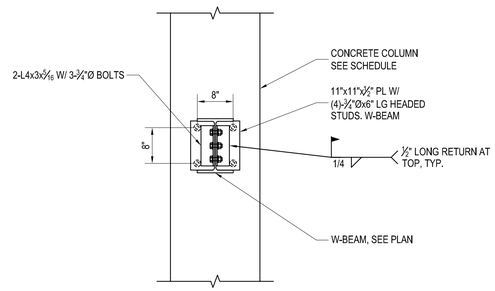


A S-420 SECTION AT CANOPY II

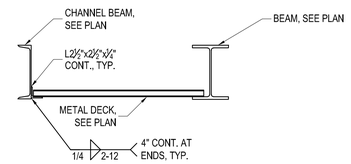
NTS



NOTE: ALL EXTERIOR STEEL AND CONNECTIONS TO BE GALVANIZED

B S-420 SECTION AT CANOPY EMBED

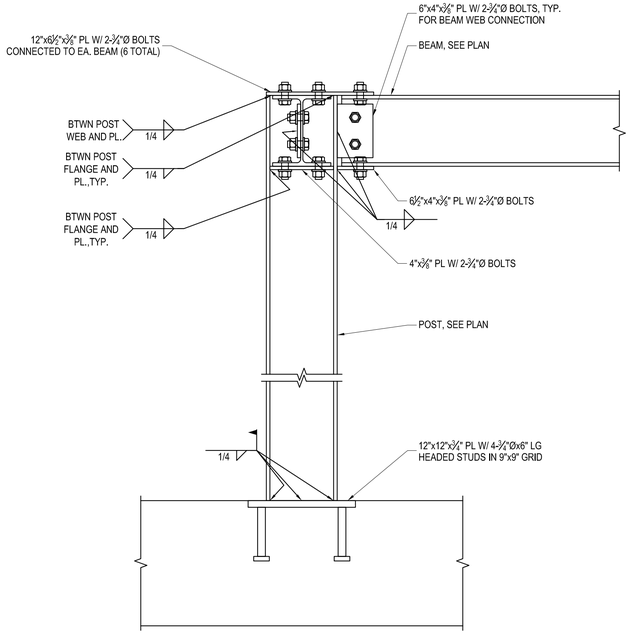
NTS



NOTE: ALL EXTERIOR STEEL AND CONNECTIONS TO BE GALVANIZED

C S-420 SECTION AT CANOPY I

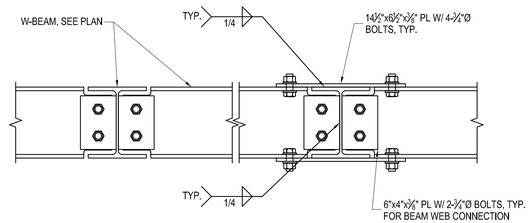
NTS



NOTES:
1. SLAB REINF. NOT SHOWN FOR CLARITY
2. ALL STEEL AND CONNECTIONS EXPOSED TO EXTERIOR TO BE GALVANIZED

D S-420 DETAIL AT TRELLIS POST

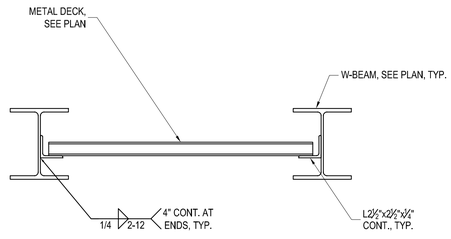
NTS



NOTES:
1. ALL STEEL AND CONNECTIONS EXPOSED TO EXTERIOR TO BE GALVANIZED

E S-420 TRELLIS MOMENT AND SHEAR CONNECTIONS

NTS



NOTES:
1. ALL STEEL AND CONNECTIONS EXPOSED TO EXTERIOR TO BE GALVANIZED

F S-420 TRELLIS METAL DECK SUPPORT

NTS

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