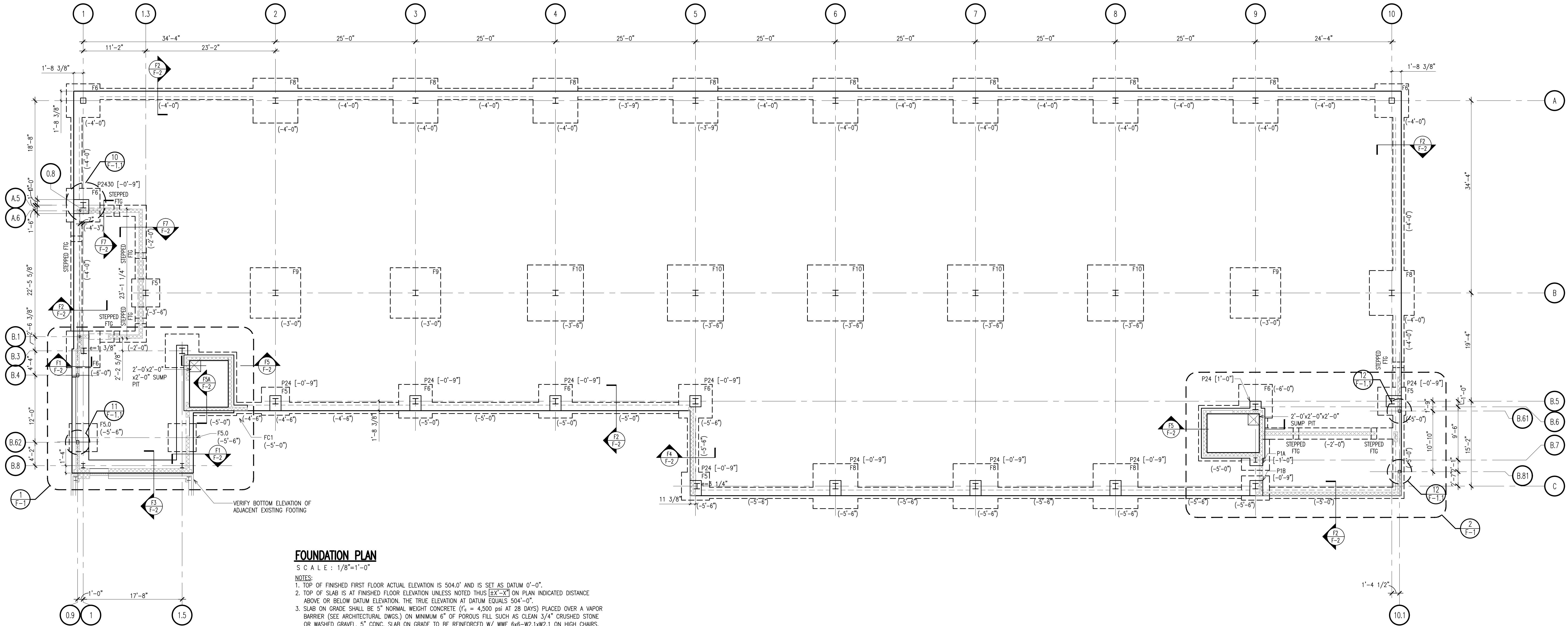
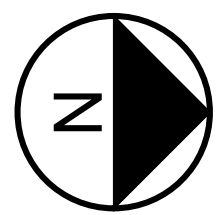




FOUNDATION PLAN

SCALE: 1/8"=1'-0"

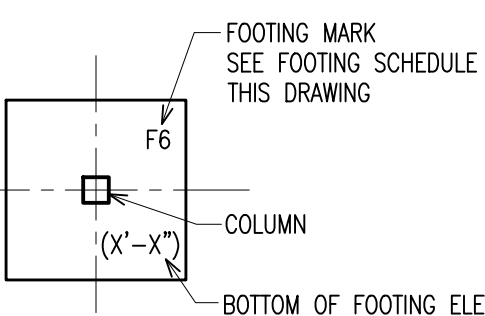
NOTES:

1. TOP OF FINISHED FIRST FLOOR ACTUAL ELEVATION IS 504.0' AND IS SET AS DATUM 0'-0".
2. TOP OF SLAB IS AT FINISHED FLOOR ELEVATION UNLESS NOTED THUS [EX-X] ON PLAN INDICATED DISTANCE ABOVE OR BELOW DATUM ELEVATION. THE TRUE ELEVATION AT DATUM EQUALS 504'-0".
3. SLAB ON GRADE SHALL BE 5" NORMAL WEIGHT CONCRETE (f_c = 4,500 psi AT 28 DAYS) PLACED OVER A VAPOR BARRIER (SEE ARCHITECTURAL DWGS.) ON MINIMUM 6" OF POROUS FILL SUCH AS CLEAN 3/4" CRUSHED STONE OR WASHED GRAVEL, 5" CONC. SLAB ON GRADE TO BE REINFORCED W/ WWF 6x6-W2.1xW2.1 ON HIGH CHAIRS.
4. PROVIDE FINISH AS SCHEDULE ON THE ARCHITECTURAL DRAWINGS TO ALL EXPOSED CONCRETE SURFACES.
5. FOOTING SHALL BEAR ON VIRGIN SOIL OR CONTROLLED COMPACTED FILL HAVING A MINIMUM BEARING CAPACITY OF 4,000 LBS. PER SQUARE FOOT. BOTTOM OF FOOTING ELEV. ARE SHOWN THUS (X'-X") ON PLAN AND ARE REFERENCED TO THE BUILDING'S DATUM. WHERE NEW BUILDING ABUTS EXISTING BUILDING, TOP OF EXISTING FOOTING ELEV. ARE SHOWN THUS (X'-X") ON PLAN AND ARE REFERENCED TO THE NEW BUILDING'S DATUM.
6. TOP OF PIERS ARE SHOWN THUS [...] ON PLAN AND ARE REFERENCED TO THE BUILDING'S DATUM.
7. F10 INDICATES FOOTING TYPE, SEE FOOTING SCHEDULE FOR SIZE AND REINFORCING.
8. P24 INDICATES PIER TYPE, SEE SCHEDULE FOR SIZE AND REINFORCING.
9. FOR TYPICAL DETAILS AND GENERAL NOTES, SEE DWGS. F-0 AND S-0.
10. FOR COLUMN SCHEDULE SEE DWG. S-0.
11. ALL COLUMN FOOTINGS SHALL BE CENTERED UNDER COLUMN CENTERLINES UNLESS NOTED OTHERWISE ON PLAN.

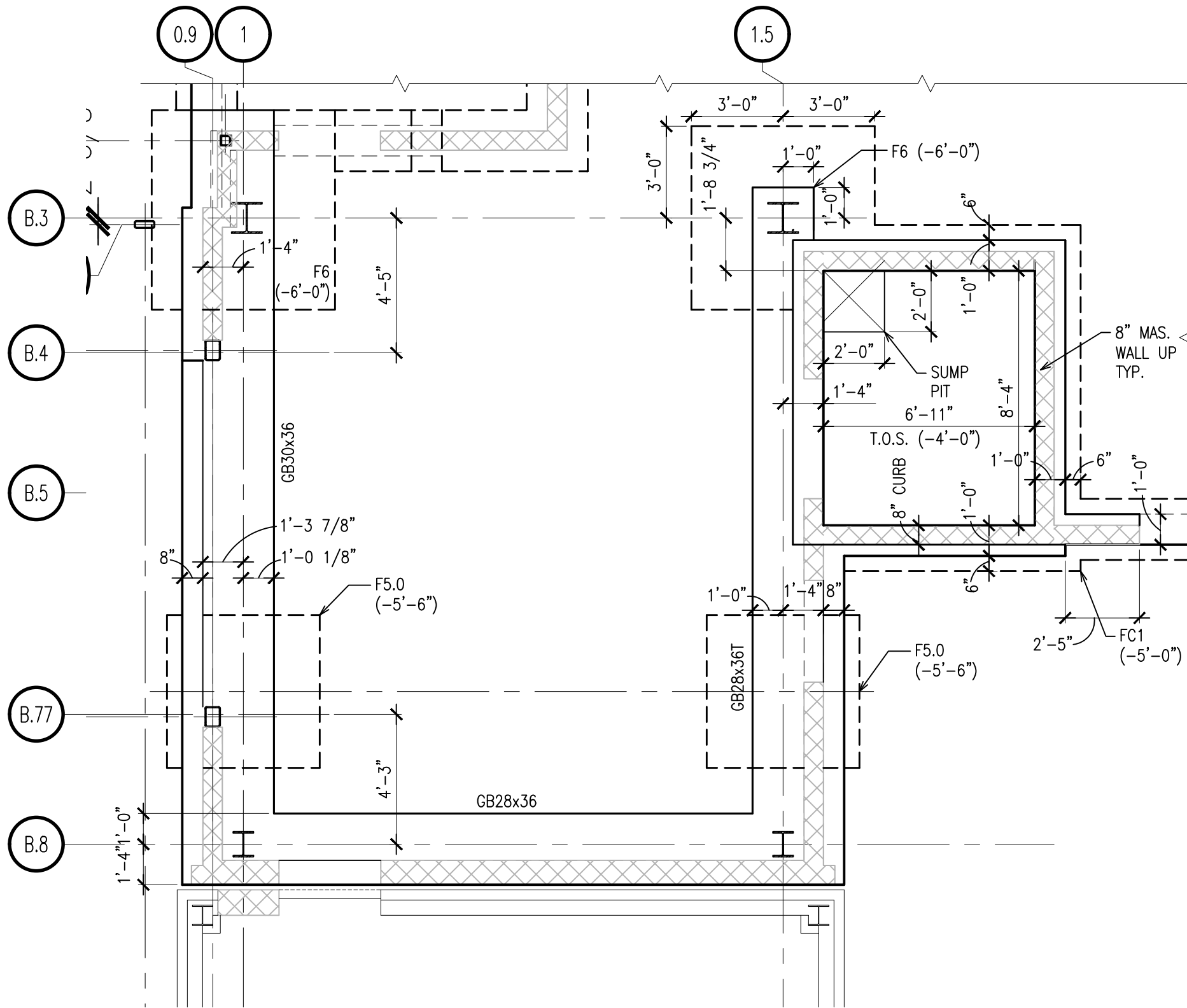
FOOTING SCHEDULE

NOTE: ALL FOOTINGS ARE CENTERED ON COLUMN LINES UNLESS NOTED WITH "O", SEE PLAN FOR OFFSET VALUE

FOOTING MARK	FOOTING SIZE	FOOTING THICKNESS	FOOTING REINFORCEMENT
F5	5'-0"x5'-0"	1'-6"	8-#5 E.W. BOTTOM
F6	6'-0"x6'-0"	2'-0"	6-#7 E.W. TOP & BOTTOM
F8	8'-0"x8'-0"	2'-0"	8-#7 E.W. BOTTOM
F9	9'-0"x9'-0"	2'-0"	9-#7 E.W. BOTTOM
F10	10'-0"x10'-0"	2'-6"	11-#7 E.W. BOTTOM
FSC	5'-0" SEE PLAN	1'-6"	#5@7" O.C. E.W. BOTTOM

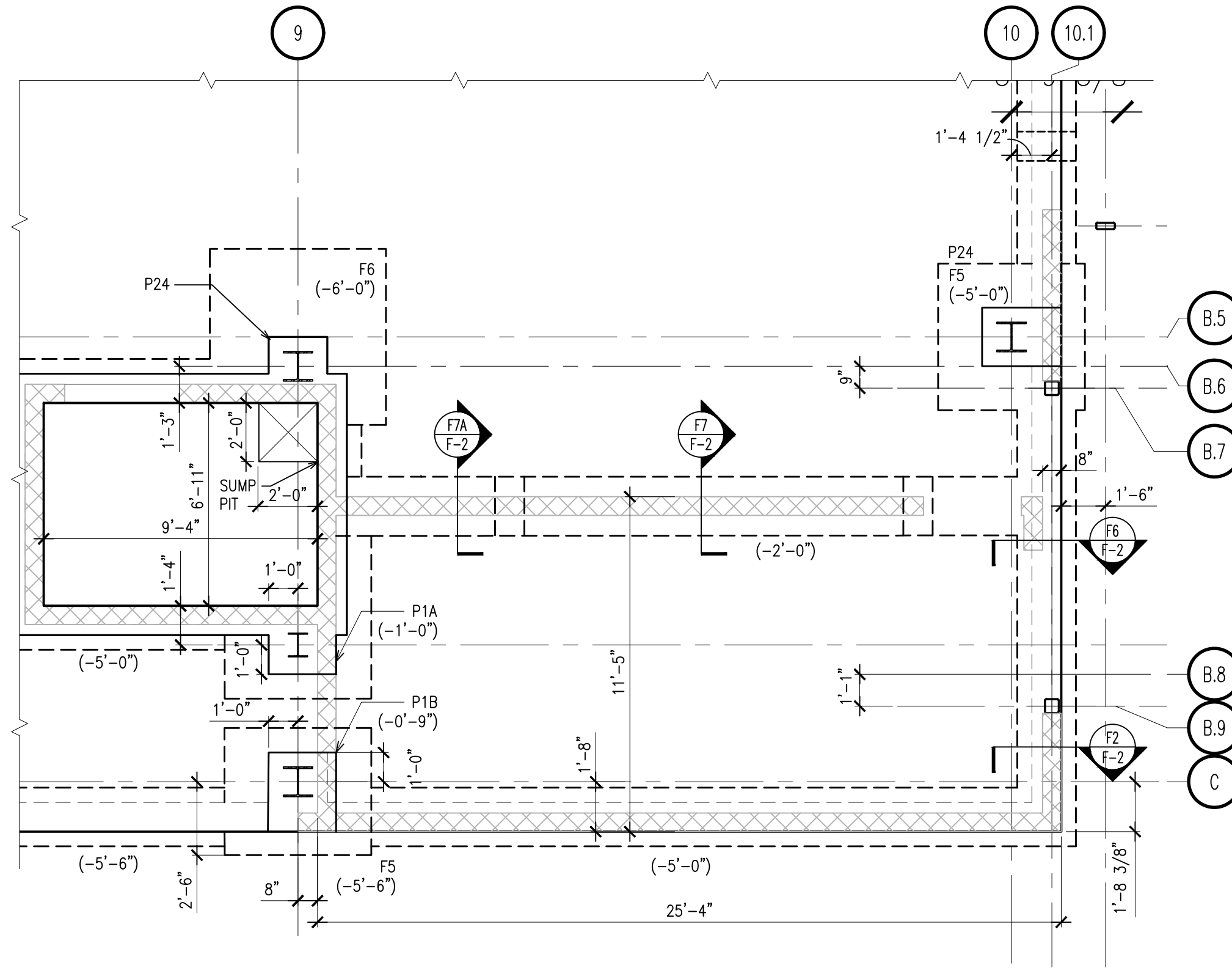


ALL FOOTINGS ARE DESIGNED FOR NET SAFE BEARING PRESSURE OF 4,000 PSF BASED ON RECOMMENDATIONS FROM MENARD USA.



1 ENLARGED FOUNDATION PLAN

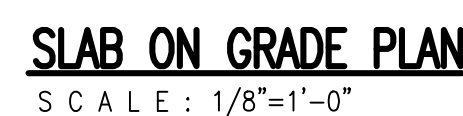
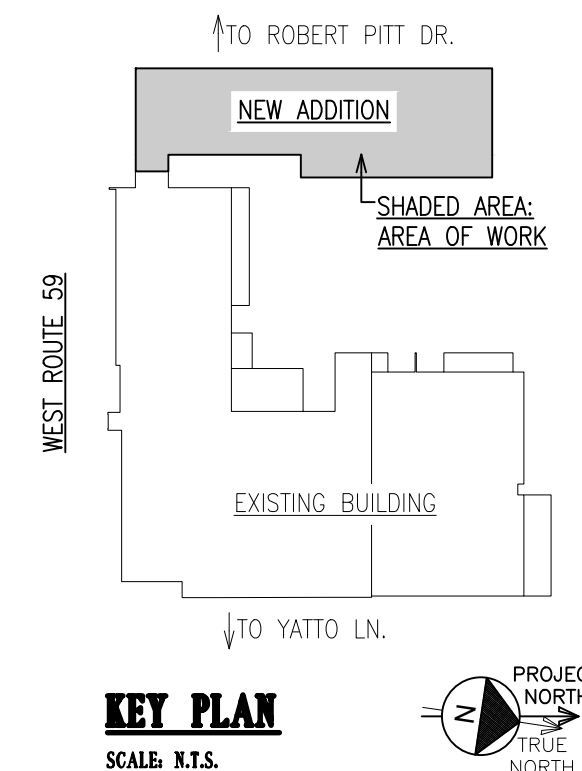
SCALE: 1/4"=1'-0"



2 ENLARGED FOUNDATION PLAN

SCALE: 1/4"=1'-0"

1 ISSUED FOR BUILDING DEPARTMENT FILING & BIDDING		04-28-25	
ORANGE AND ROCKLAND UTILITIES, INC. PEARL RIVER NEW YORK			
FOUNDATION PLAN AND DETAILS			
SVOC NEW OFFICE ADDITION & NEW PARKING			
TAX MAP ID: 68.52-1-2 3RD WEST ROUTE 54 VILLAGE OF SPRING VALLEY TOWN OF RAMAPO ROCKLAND COUNTY, NEW YORK			
DESIGN REVIEW		FILE NO.	
DATE		44 OF 95	
DESIGN REVIEW		1	
PEARL RIVER		ROCKLAND COUNTY, NEW YORK	
SNS Architects & Engineers, PC		John Lignos, AIA Lorin J. Sonenshine, AIA Steven Napoliitano, PE, PP Robert Nocella, AIA	
ONE PARAGON DRIVE . MONTVALE . NEW JERSEY . 07040			

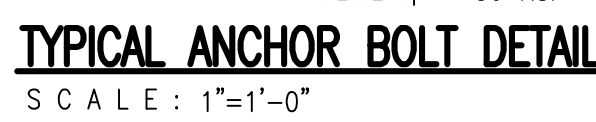


PLAN

BP3 BASE PLATE DETAIL

BP4 BASE PLATE DETAIL

COLUMN BASE PLATE ELEVATION



COLUMN SCHEDULE																																												
COLUMN MARK	A										A.5	B										B.3		B.5					B.6	B.7	B.8		C					B.4	B.61	B.62	B.61			
	1	2	3	4	5	6	7	8	9	10	1	1.3	2	3	4	5	6	7	8	9	10	1	1.5	2	3	4	5	10	9		1	1.5	5	6	7	8	9	0.9	10.1	0.9	10.1			
T.O.S. EL. @ ROOF LEVEL 40'-0" AT HP 39'-3 3/8" AT LP																																												
T.O.S. @ 3rd FLOOR 26'-7 1/2"																																												
T.O.S. @ 2nd FLOOR 13'-7 1/2"																																												
TOP OF FOOTING ELEV. -0'-0"																																												
-1'-0"																																												
-2'-0"																																												
BASE TYPE	2	1	1	1	1	1	1	1	1	2	2	2	1	1	1	1	1	1	1	1	2	2	2	2	2	2	2	2	2	2	3	3	3	2	1	1	1	2	4	4	4	4		

NOTE: (2) HSS 4x4x3/8 W/ BP5 LOCATION SEE PLAN

1	ISSUED FOR BUILDING DEPARTMENT FILING & BIDDING	04-28-25



ORANGE AND ROCKLAND UTILITIES, INC.
PEARL RIVER NEW YORK

APPROVED		SLAB PLAN & COLUMN SCHEDULE			
DATE		SVOC NEW OFFICE ADDITION & NEW PARKING			
☐ INFORMATION ☐ COMMENTS ☐ BID ☐ CONSTRUCTION ☐ _____		THE S&B OF S&B-C-2 390 WEST ROUTE 59 VILLAGE OF DANBURY TOWN OF DANBURY ROCKLAND COUNTY, NEW YORK			
SCALE	FILE NO.	DRAWING NUMBER		SHEET	REV.
AS NOTED	5593	F-1.1		45 OF 95	1

SINS ARCHITECTS & ENGINEERS
tel: 201.573.1767
fax: 201.573.0808
www.sins-arch-eng.com

DRAFTED : A.S.
CHECKED : S.N.
PROJ. NO : 5593
DATE: NOVEMBER 25 2024

STATE OF NEW YORK
SEAL OF THE STATE ENGINEER
STEVEN NAPOLITANO, P.E.
PP Cert./Lic. No. 077923
SEAL PROFESSIONAL ENGINEER

ENGINEERING REVIEW		
DEPT.	SIGNATURE	DATE
CIVIL		
METERING		
RELAY		
TRANS. & SUB.		
OWN	DES	CHKD
DESIGN REVIEW		DATE

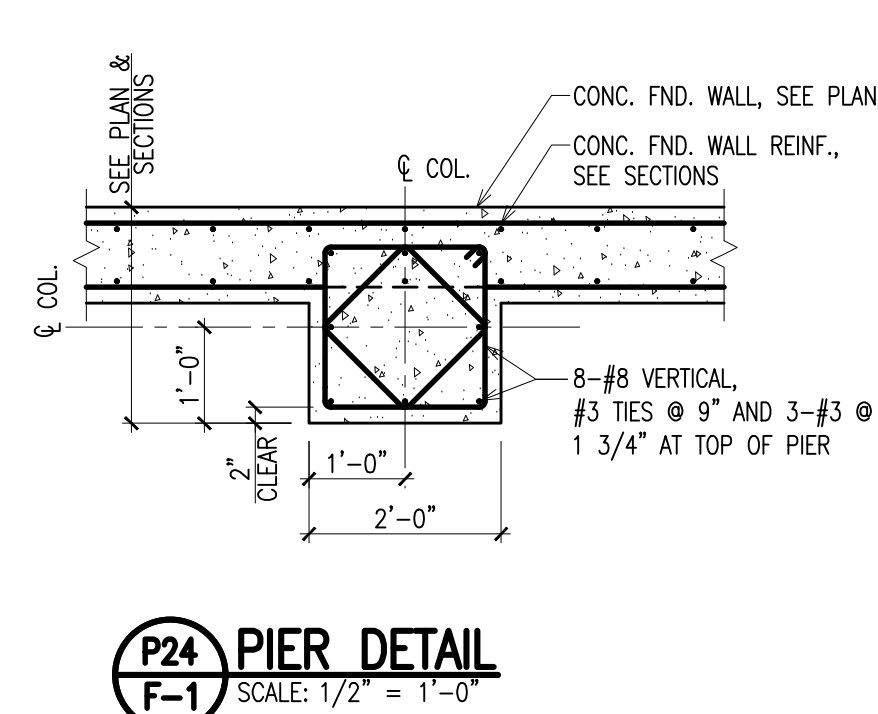
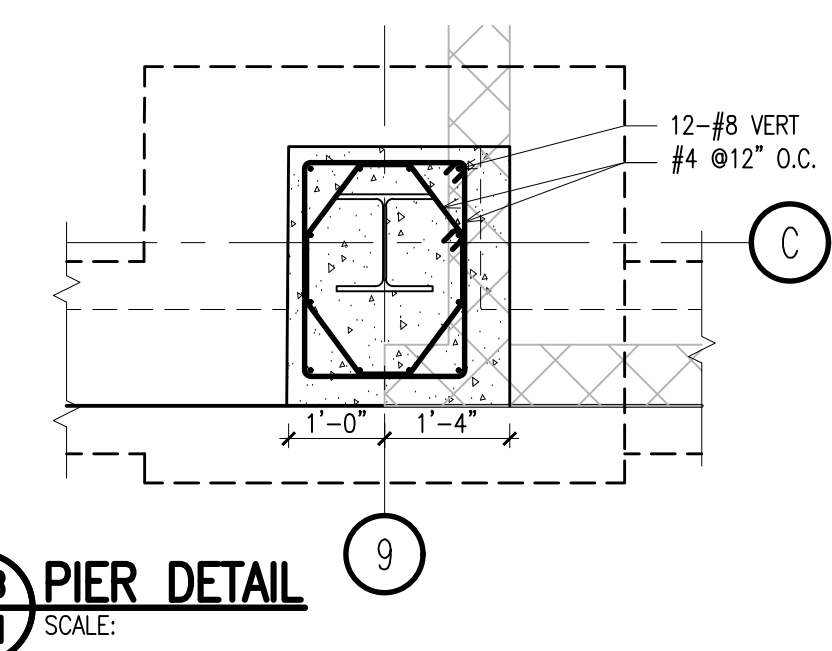
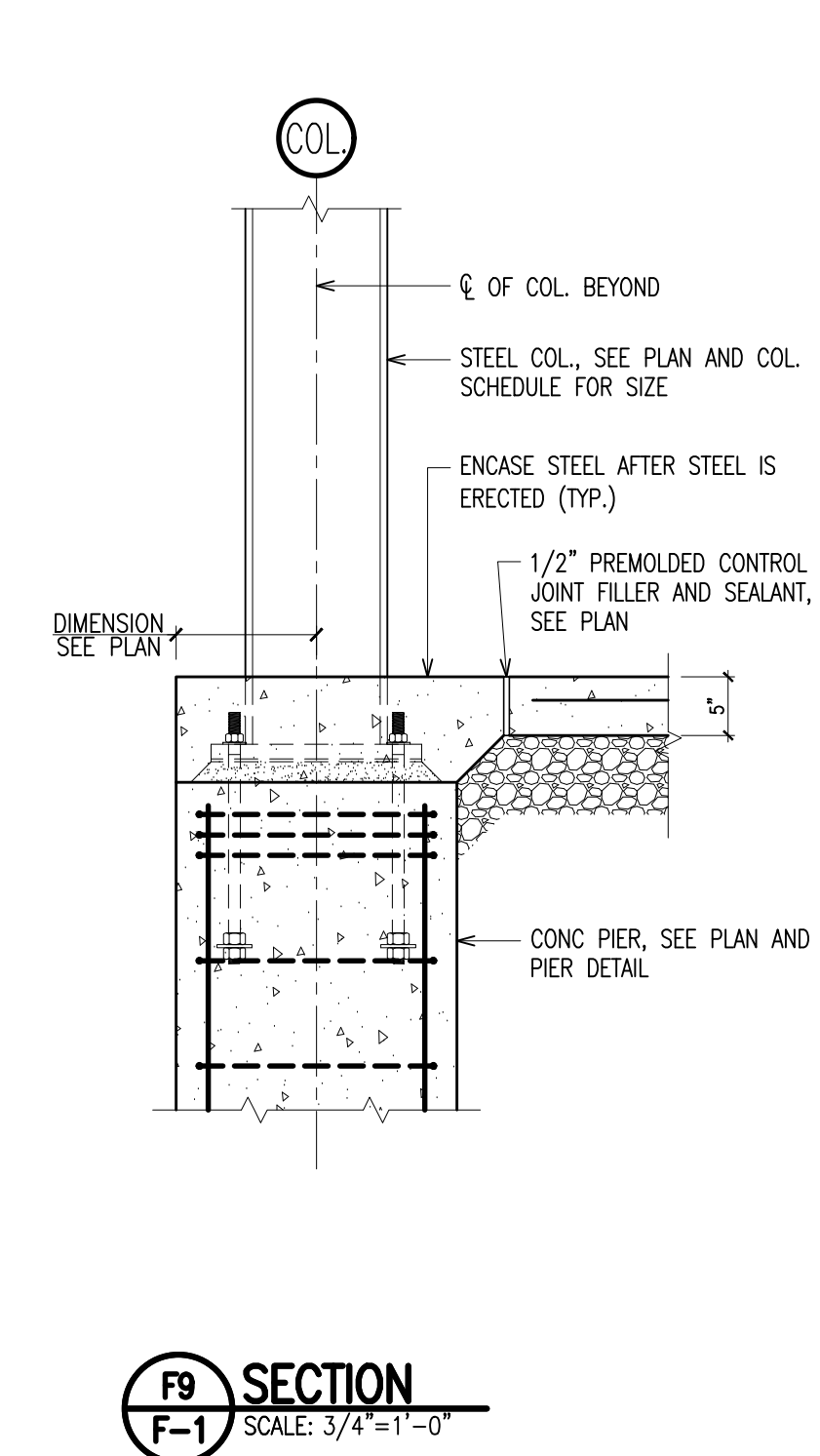
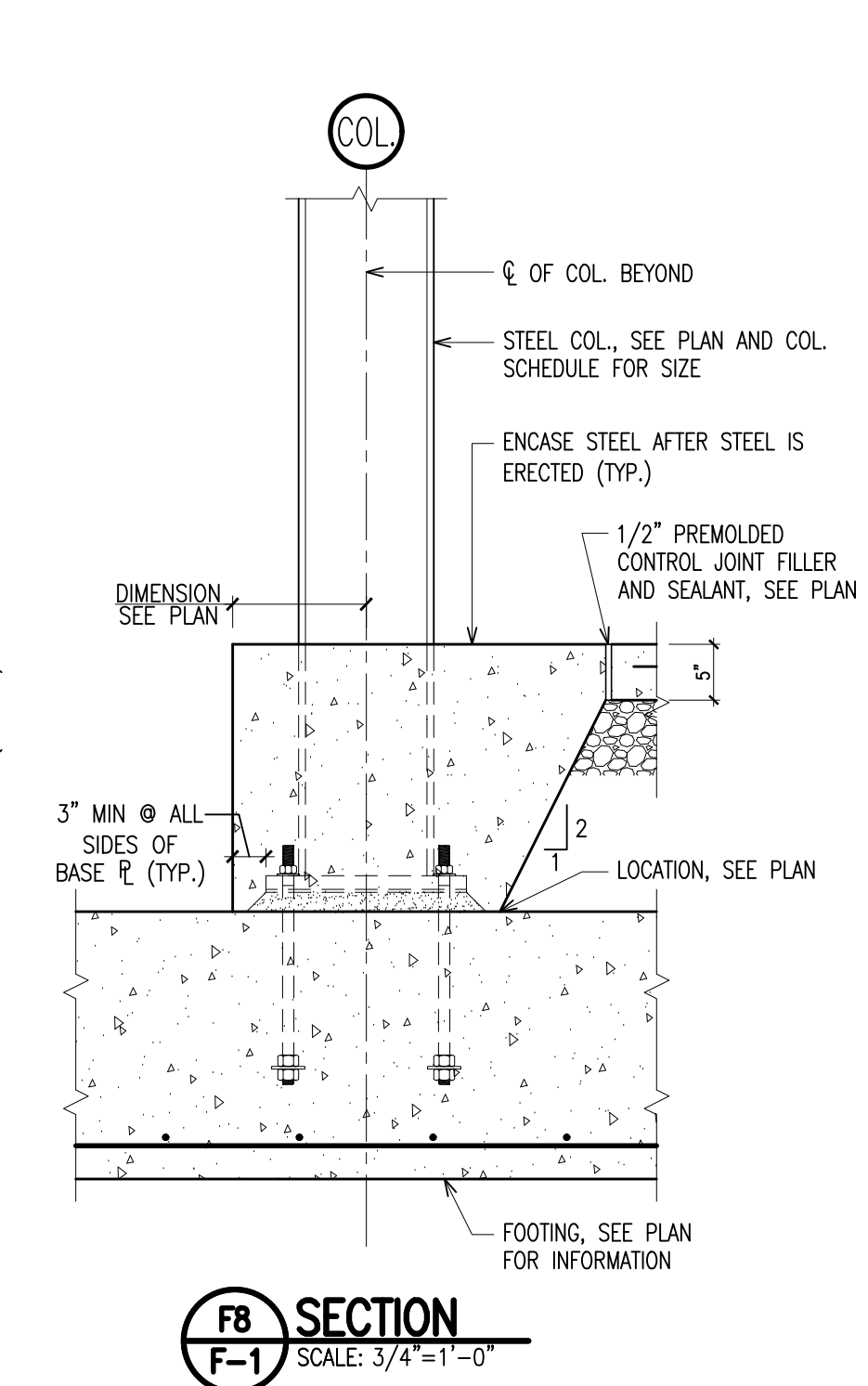
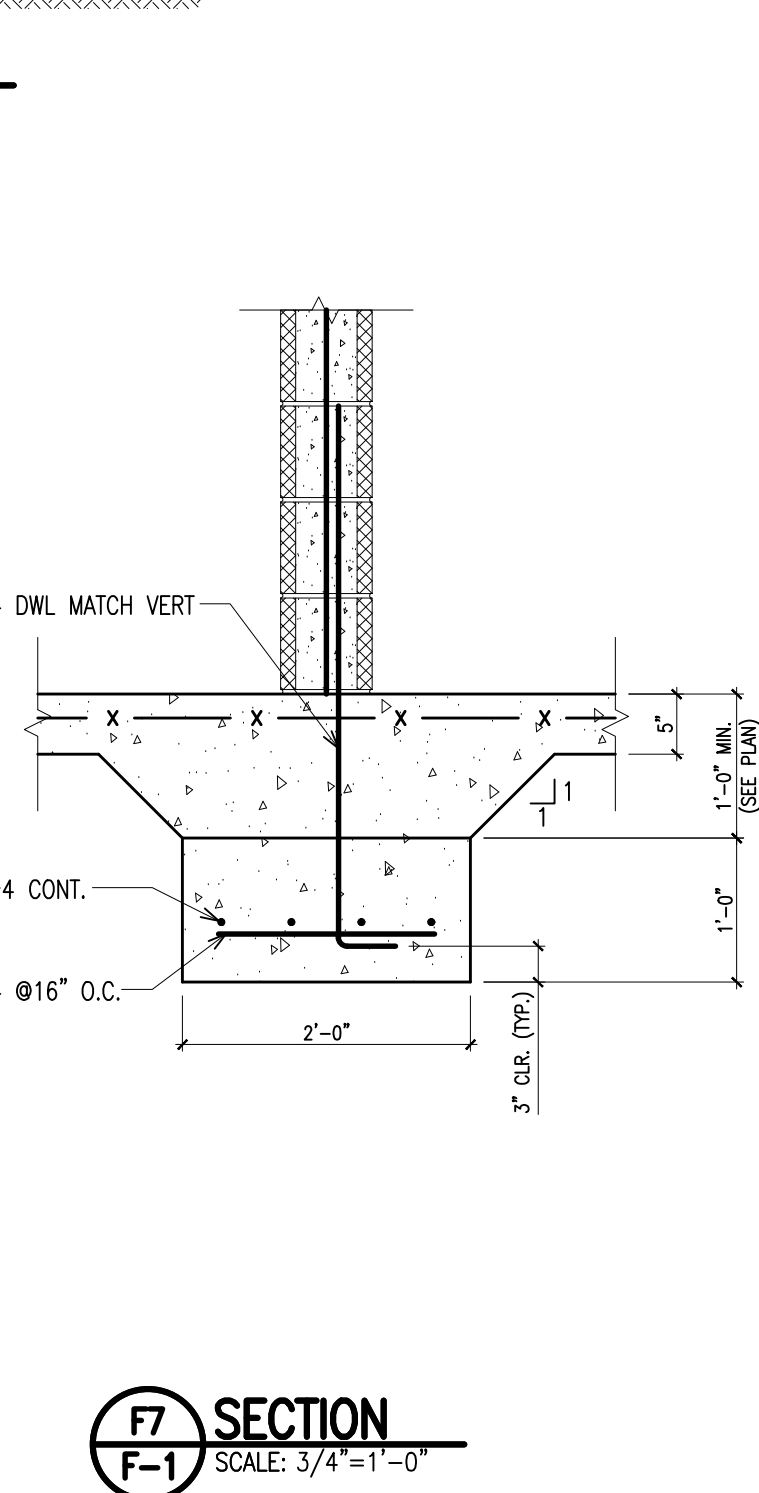
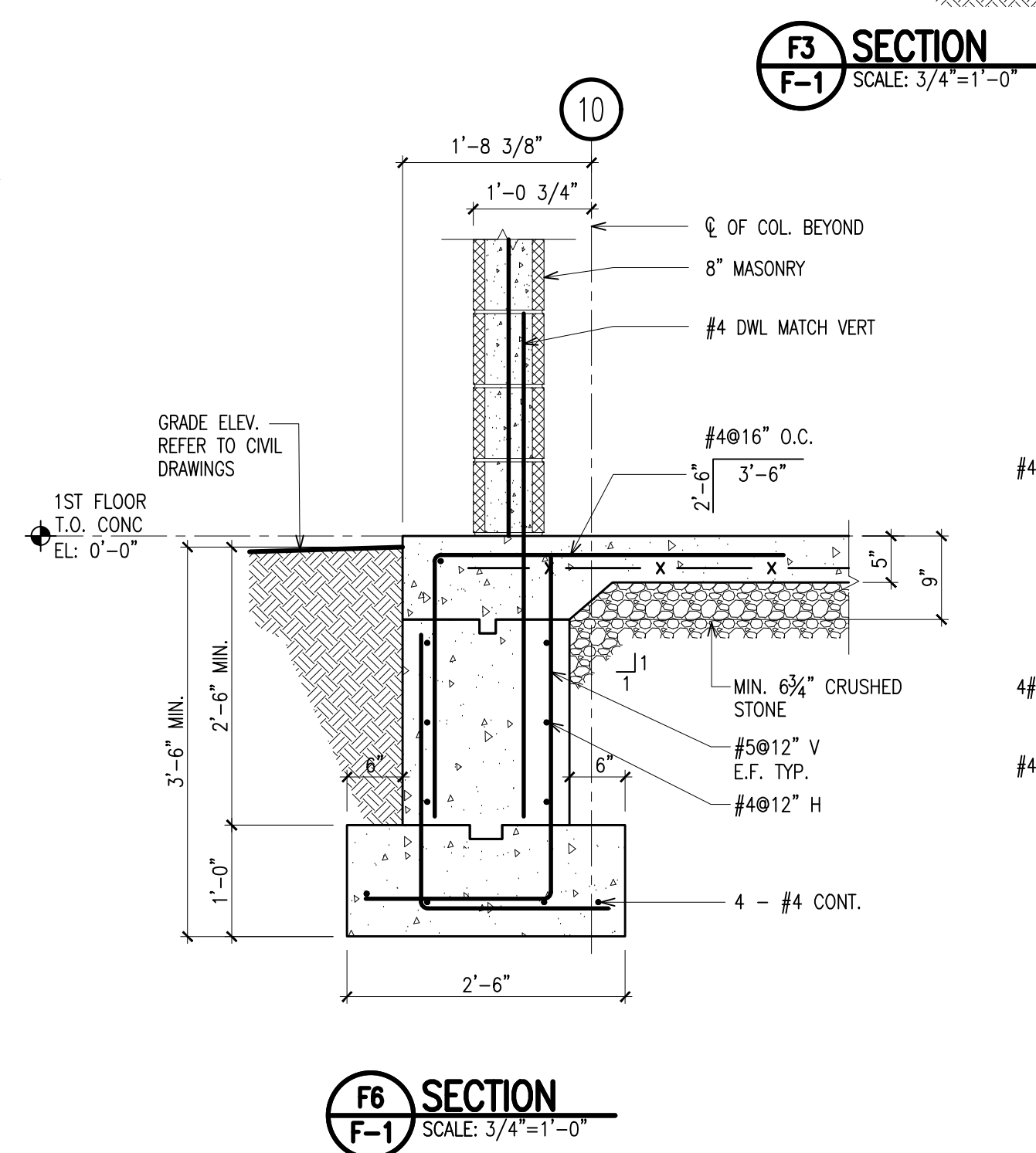
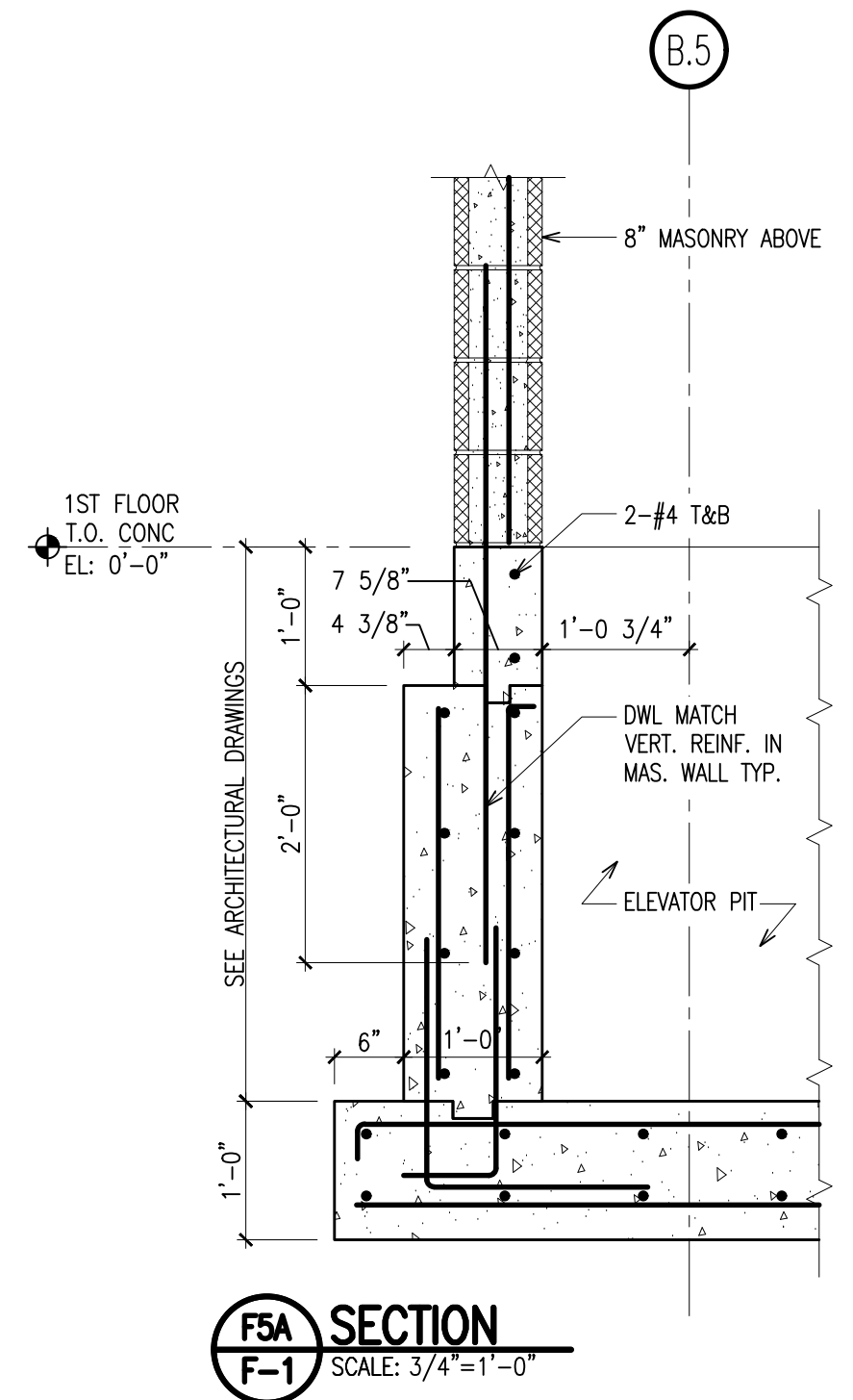
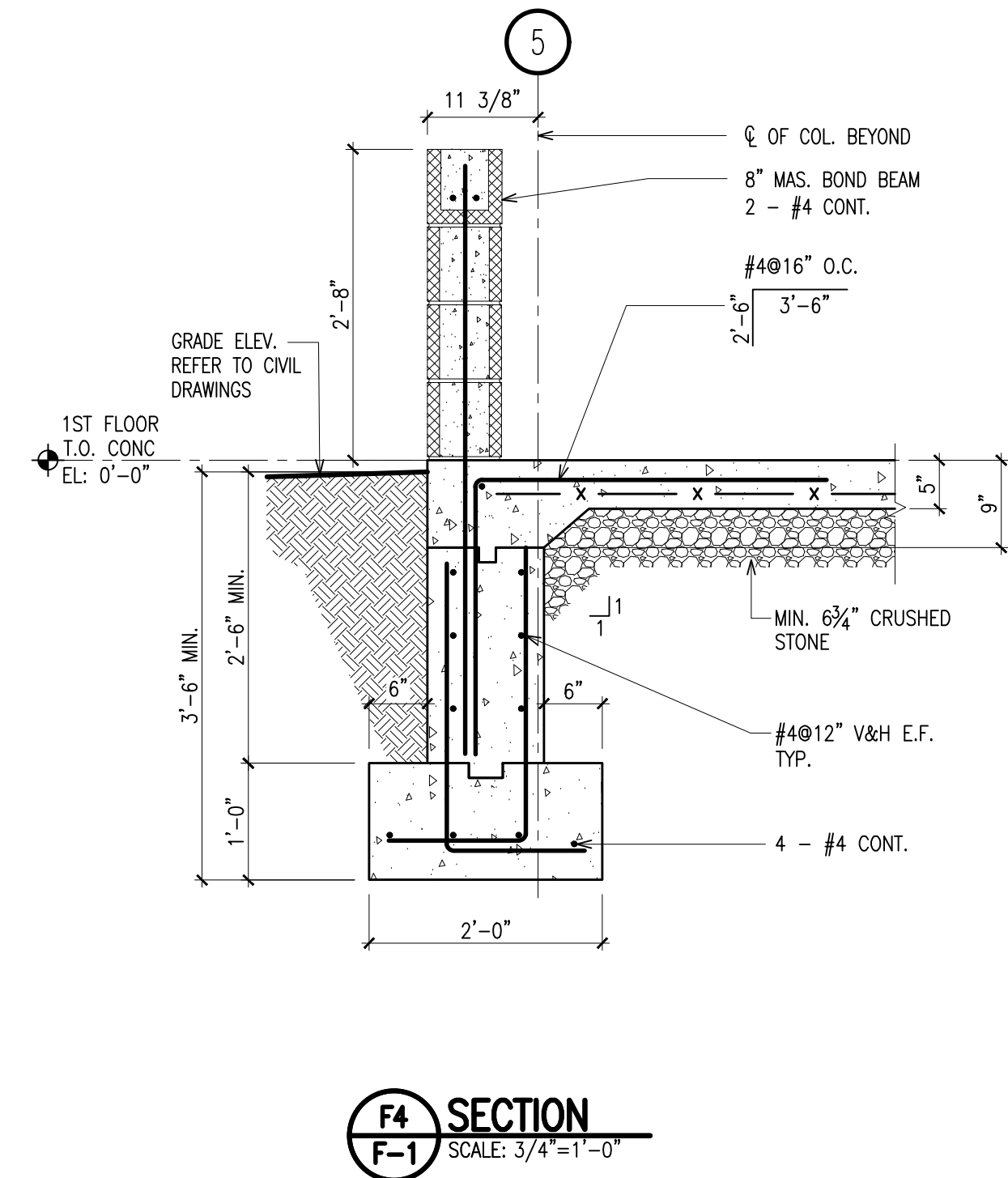
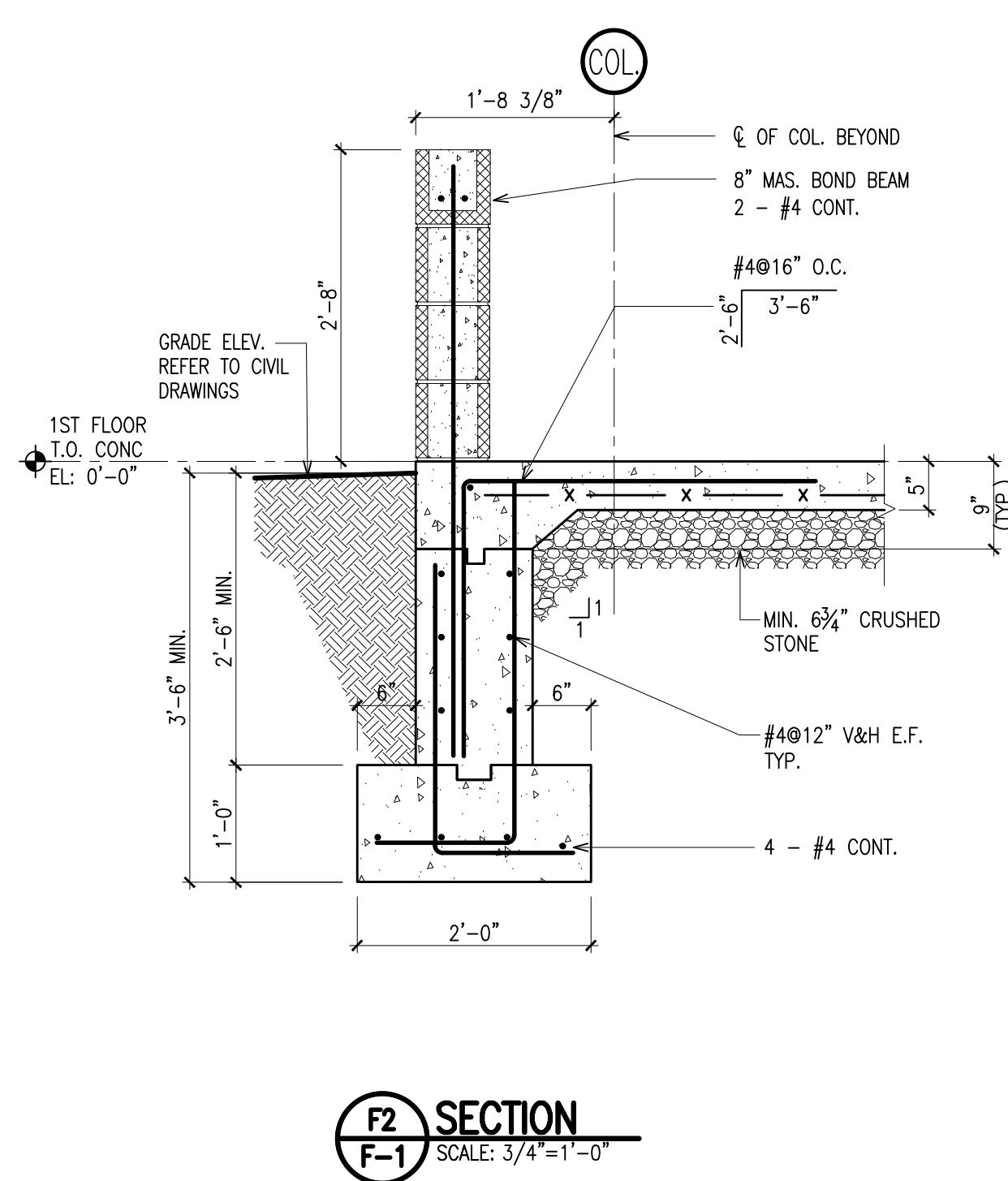
PEARL RIVER
SNS
ONE PARAGON DRIVE

Architects & Engineers, PC

ROCKLAND COUNTY, NEW YORK

- ▣ John Lignos, AIA
- ▣ Lorin J. Sonenshine, AIA
- Steven Napolitano, PE, PP
- ▣ Robert Nocella, AIA

MONTVALE NEW JERSEY 07045



1	ISSUED FOR BUILDING DEPARTMENT FILING & BIDDING		04-28-25

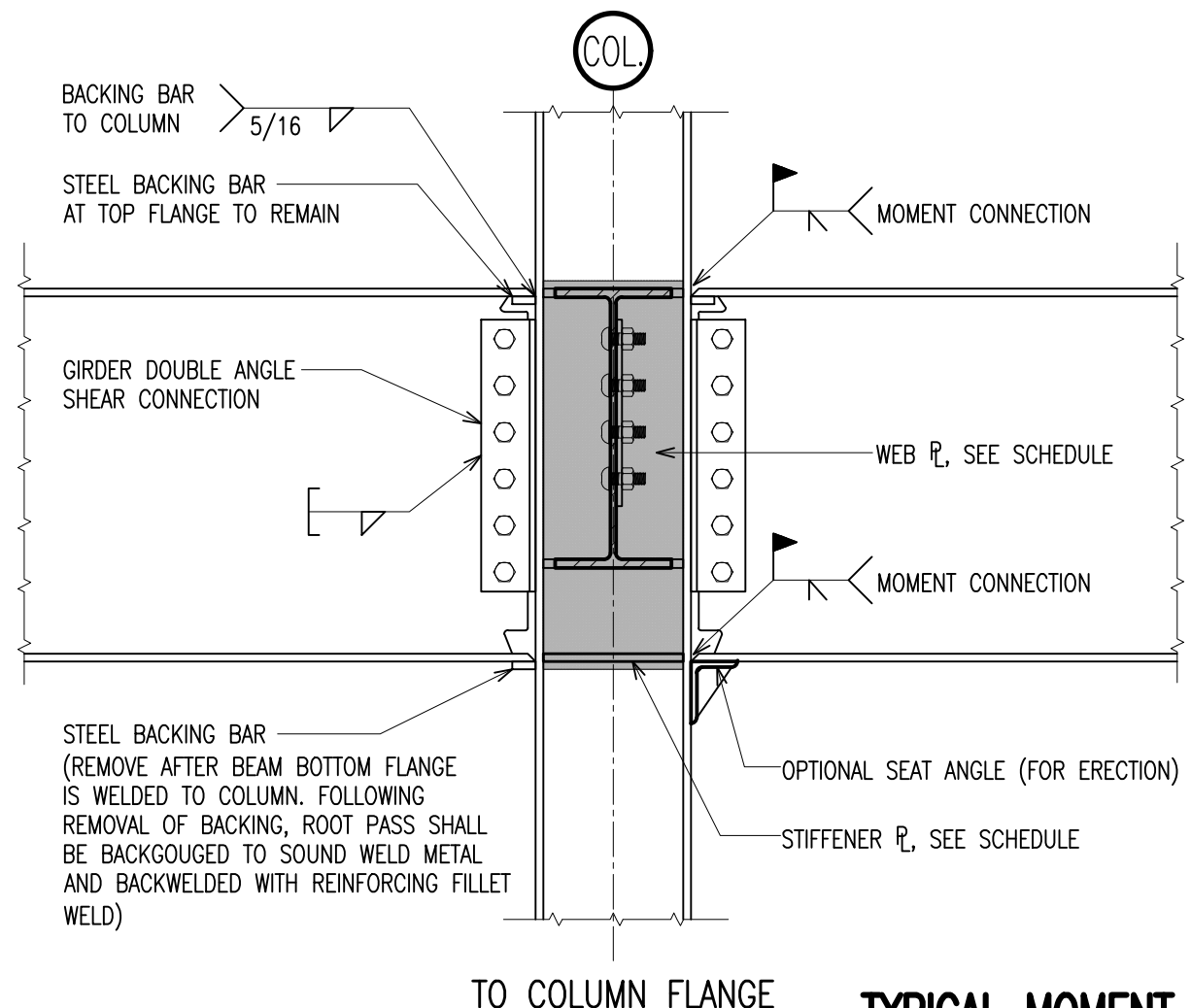
	ORANGE AND ROCKLAND UTILITIES, INC. PEARL RIVER NEW YORK			
	FOUNDATION SECTIONS			
APPROVED _____ DATE _____	SVOC NEW OFFICE ADDITION & NEW PARKING 745 MAP TO 5652-1 S 590 WEST ROUTE 59 VILLAGE OF SPRING VALLEY TOWN OF RAMAPO ROCKLAND COUNTY, NEW YORK			
☐ INFORMATION ☐ COMMENTS ☐ BID ☐ CONSTRUCTION _____				
SCALE _____ AS NOTED	FILE NO. _____ 5593	DRAWING NUMBER _____ F-2	SHEET _____ 46 OF 95	REV _____ 1

ROCKLAND COUNTY, NEW YORK

Architects & Engineers, PC

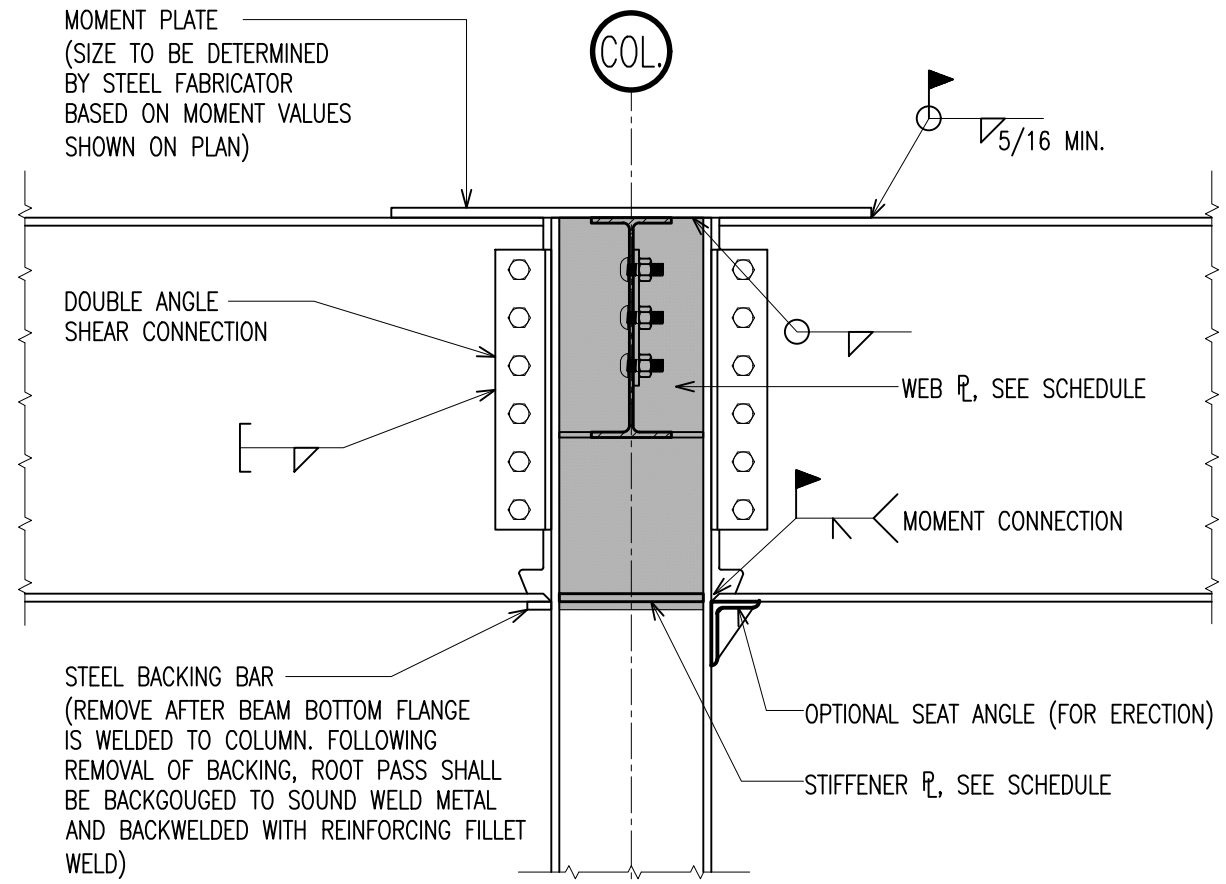
- ☐ John Lignos, AIA
- ☐ Lorin J. Sonenshine, AIA
- ☐ Steven Napolitano, PE, PP
- ☐ Robert Nocella, AIA

MONTVALE - NEW JERSEY - 07048



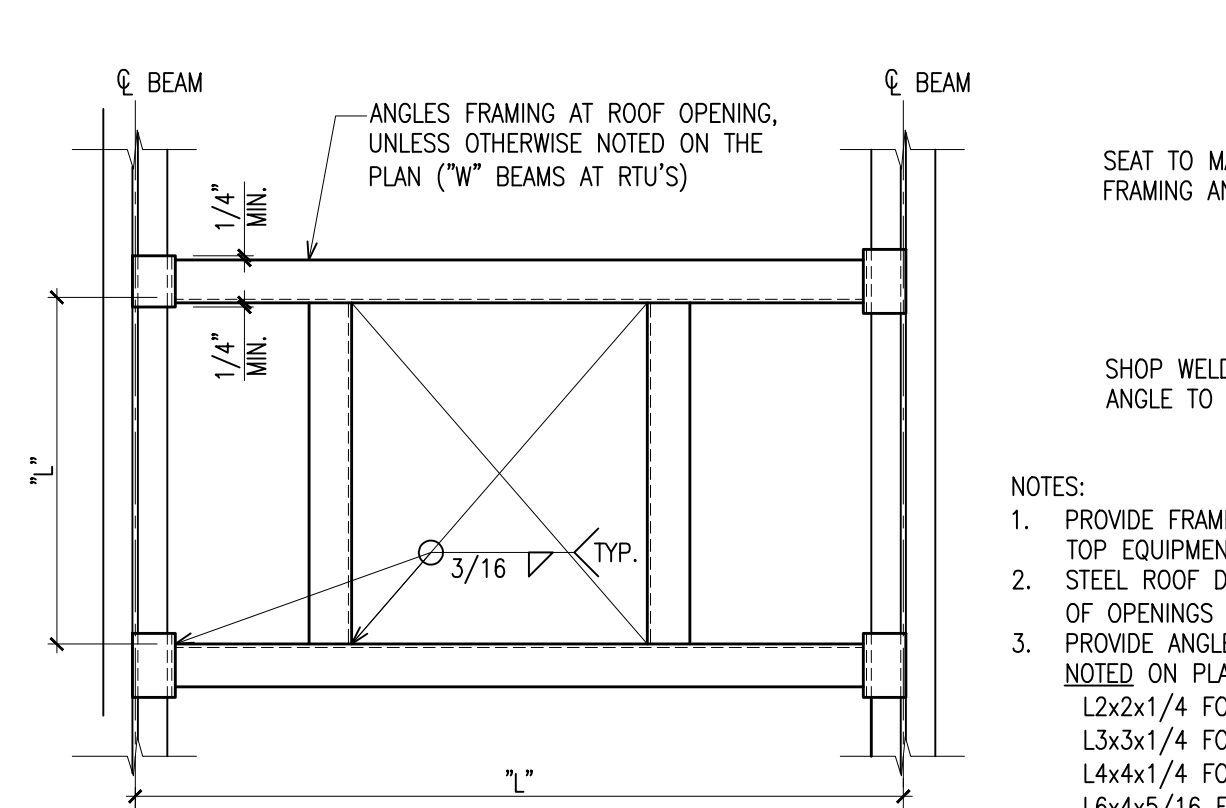
TYPICAL MOMENT CONNECTION AT FLOOR

SCALE: NTS
OTHER TYPES OF MOMENT CONNECTIONS ARE ACCEPTABLE, SUBJECT TO ENGINEER'S APPROVAL.
FOR MOMENT CONNECTION VALUES SEE PLANS



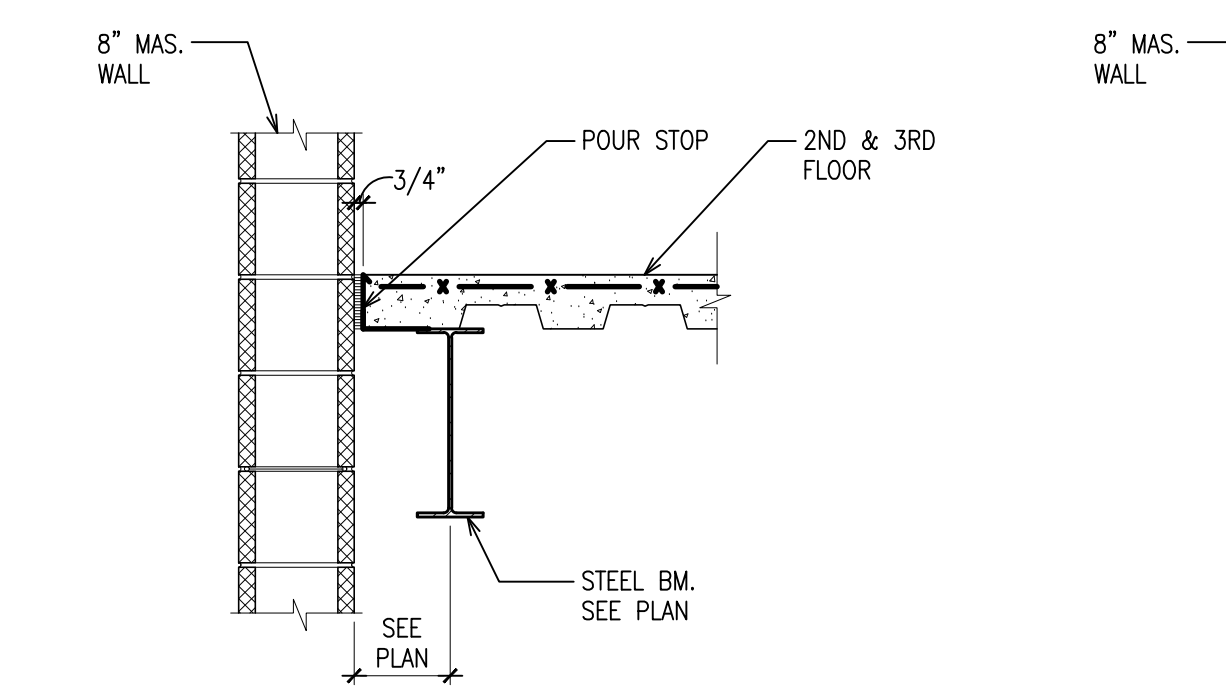
TYPICAL MOMENT CONNECTION AT ROOF

SCALE: NTS
OTHER TYPES OF MOMENT CONNECTIONS ARE ACCEPTABLE, SUBJECT TO ENGINEER'S APPROVAL.
FOR MOMENT CONNECTION VALUES SEE STRUCTURAL PLANS.



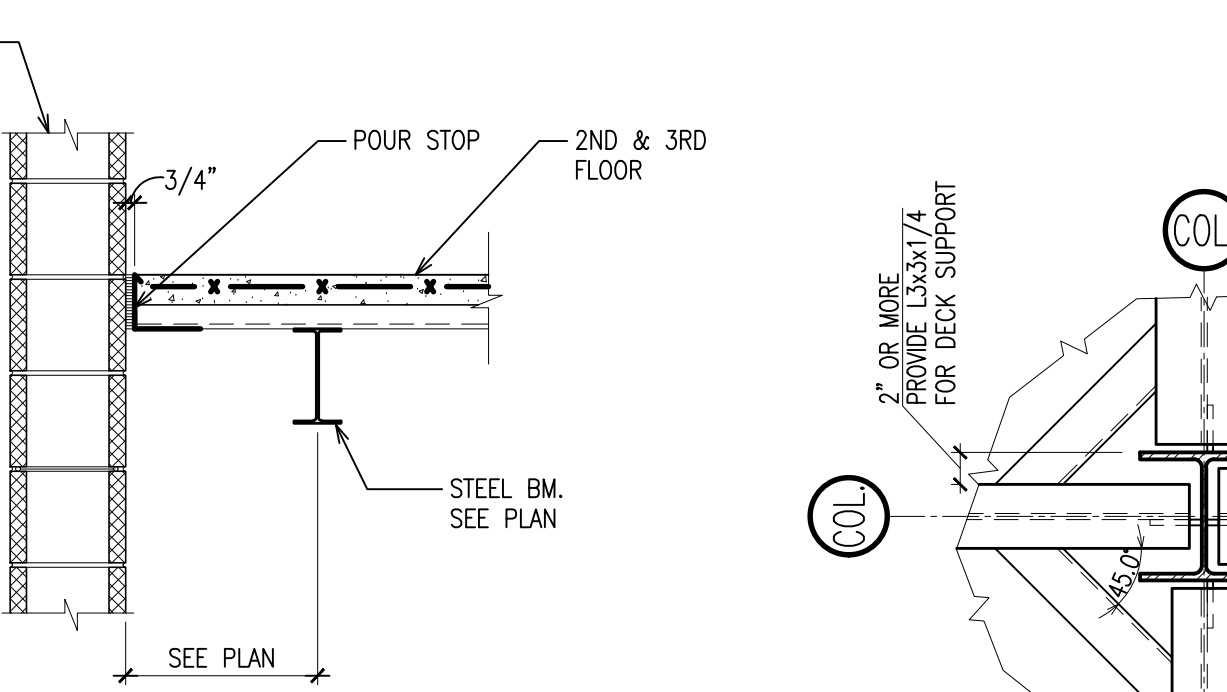
TYPICAL ROOF OPENING AND ROOF EQUIPMENT SUPPORT FRAMING DETAIL

SCALE: 1"=1'-0"



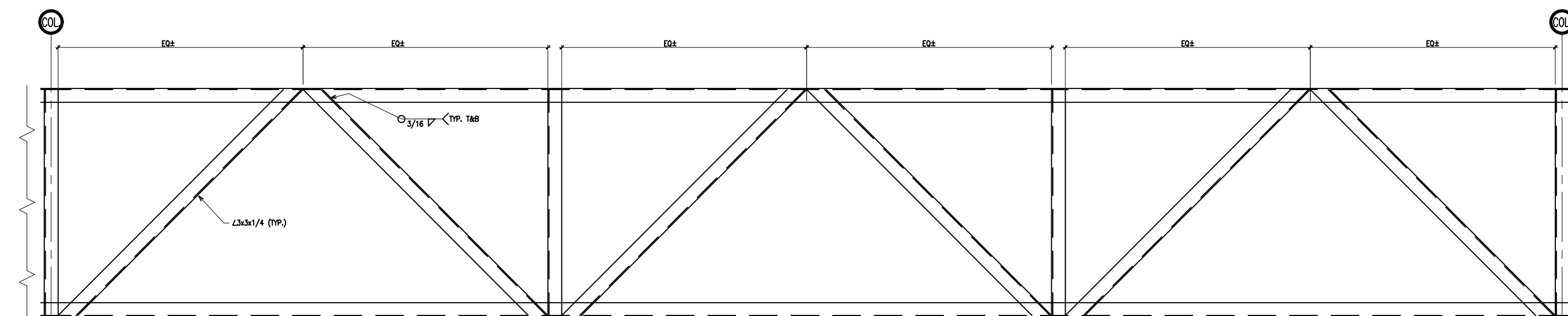
TYPICAL DETAIL BTW. SLAB & MASONRY WALL

SCALE: 3/4"=1'-0"



TYPICAL DETAIL BTW. SLAB & MASONRY WALL

SCALE: 3/4"=1'-0"



TYPICAL DETAIL OF VERTICAL BRACES

SCALE: 1/2"=1'-0"

(VB1 & VB2 & VB3)

JOINT	ROOF	3rd FLOOR	2nd FLOOR
1-B.8	-	-	-
1-B.3	-	-	-
1-A.5	-	-	-
1-A	-	-	-
1.3-B	-	-	-
1.5-C	-	-	-
1.6-B.3	-	-	-
2-B.6	-	-	-
2-B	STIFF R 3/8"	WEB R 1/4", STIFF R 3/8"	WEB R 1/4", STIFF R 3/8"
2-A	-	STIFF R 3/8"	WEB R 3/8", STIFF R 3/8"
3-B.6	-	-	-
3-B	-	WEB R 3/8", STIFF R 3/8"	WEB R 7/16", STIFF R 3/8"
3-A	-	STIFF R 3/8"	WEB R 1/4", STIFF R 3/8"
4-B.6	-	-	-
4-B	-	WEB R 3/8", STIFF R 3/8"	WEB R 3/8", STIFF R 3/8"
4-A	-	STIFF R 3/8"	STIFF R 3/8"
5-C	-	-	-
5-B.5	-	-	-
5-B	-	WEB R 3/8", STIFF R 3/8"	WEB R 7/16", STIFF R 3/8"
5-A	-	STIFF R 3/8"	STIFF R 3/8"
6-C	-	STIFF R 3/8"	STIFF R 3/8"
6-B	STIFF R 3/8"	STIFF R 1/2"	STIFF R 5/8"
6-A	-	STIFF R 3/8"	STIFF R 3/8"
7-C	-	STIFF R 3/8"	STIFF R 3/8"
7-B	STIFF R 3/8"	STIFF R 1/2"	STIFF R 5/8"
7-A	-	STIFF R 3/8"	STIFF R 3/8"
8-C	-	STIFF R 3/8"	STIFF R 3/8"
8-B	STIFF R 3/8"	STIFF R 1/2"	STIFF R 5/8"
8-A	-	STIFF R 3/8"	STIFF R 3/8"
9-C	-	-	-
9-B.6	-	-	-
9-B	-	WEB R 3/8", STIFF R 3/8"	WEB R 3/8", STIFF R 3/8"
9-A	-	STIFF R 3/8"	WEB R 1/4", STIFF R 3/8"
10-B.5	-	-	-
10-B	-	STIFF R 3/8"	WEB R 1/4", STIFF R 3/8"
10-A	-	STIFF R 3/8"	STIFF R 3/8"

BEAM SIZE	MOMENT (KFT)
W27x94	450
W27x84	435
W24x62	275
W24x55	200
W21x44	190
W18x40	170
W16x36	130
W16x31	125
W16x26	90
W14x22	70

DESIGN LOAD TABLE (VALUES IN PSF)	ROOF	3rd FLOOR	2nd FLOOR
LIVE LOAD	25*	80	80
ROOFING	1	-	-
RIGID INSULATION	3	-	-
METAL DECK	2	2	4
SOLAR PANELS	13	-	-
4 1/2" SLAB (110 PCF LIGHTWEIGHT CONCRETE)	0	35	35
STEEL FRAMING	2	3	3
CEILING (MESH OR ACOUSTICAL) + M/E/P	5	5	5
TOTAL LOAD (LL + DL)	51	125	127

* - MECHANICAL WITH SNOW DRAFT

2020 BUILDING CODE OF NEW YORK STATE (BASED ON IBC 2018)

SEISMIC NOTATIONS

RISK CATEGORY II
SEISMIC IMPORTANCE FACTOR I=1.0
MAPPED SPECTRAL RESPONSE ACCELERATION S_s=0.294
MAPPED SPECTRAL RESPONSE ACCELERATION S₁=0.06
SOIL SITE CLASS: D
SPECTRAL RESPONSE COEFFICIENT S_{ds}=0.31
SPECTRAL RESPONSE COEFFICIENT S_{d1}=0.10
SEISMIC DESIGN CATEGORY B
SEISMIC FORCE RESISTING SYSTEM: STEEL ORDINARY MOMENT FRAMES
DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN SYSTEMS
SEISMIC RESPONSE COEFFICIENT C_s=0.041
RESPONSE MODIFICATION FACTOR R=3.25
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE PER ASCE 7-16
DESIGN BASE SHEAR V_E=77 K

WIND NOTATIONS

BASIC WIND SPEED=115 mph
WIND LOAD IMPORTANCE FACTOR I=1.0
WIND EXPOSURE C
INTERNAL PRESSURE COEFFICIENT ±0.18
COMPONENTS & CLADDING AT ROOF AND WALLS VELOCITY PRESSURE: SIMPLIFIED PROCEDURE PER ASCE 7-16
MAIN WIND FORCE RESISTING SYSTEM ANALYSIS PROCEDURE: SIMPLIFIED PROCEDURE PER ASCE 7-16
DESIGN BASE SHEAR V_E=65 K, F_r=224 K

SNOW NOTATIONS

GROUND SNOW LOAD P_g=10 psf
FLAT ROOF SNOW LOAD P_f=20.0 psf
SNOW EXPOSURE FACTOR C_e=0.9
SNOW LOAD IMPORTANCE FACTOR I=1.0
THERMAL FACTOR C_t=1.0

INSPECTION TASKS PRIOR TO WELDING	CLAUSES
Welding procedure specifications (WPS) available	6.3
Manufacturer certifications for welding consumables available	6.2
Material identification (type/grade)	6.2
Welder identification system	6.4 (welder qualification) (identification system not required by AWS D1.1/D1.1M)
Fit-up of groove welds (including joint geometry)	6.5.2 6.5.22
Joint preparation	6.5.2
Dimensions (alignment, root opening, root face, bevel)	5.14
Cleanliness (condition of steel surfaces)	5.17
Tacking (lock weld quality and location)	5.9, 5.21.1.1
Backing type and fill (if applicable)	5.9, 5.21.1.1
Fit-up of CJP groove welds of HSS T-, Y- & K-joints without backing (including joint geometry)	9.11.2
Joint preparation	9.11.2
Dimensions (alignment, root opening, root face, bevel)	5.14
Cleanliness (condition of steel surfaces)	5.17
Tacking (lock weld quality and location)	5.17
Configuration and finish of access holes	6.5.2, 5.16 (also see Section J.16)
Fit-up of fillet welds	5.21.1 5.14 5.17
Dimensions (alignment, gaps at root)	5.21.1
Cleanliness (condition of steel surfaces)	5.14
Tacking (lock weld quality and location)	5.17
Check welding equipment	6.2.5.10

The words "all welds" in clause 6.5.1 clearly indicate that all welds are required to be inspected for size, length and location in order to ensure conformity. Chapter N follows the same principle in labeling these tasks "perform," which is defined as "perform these tasks for each welded joint or member."

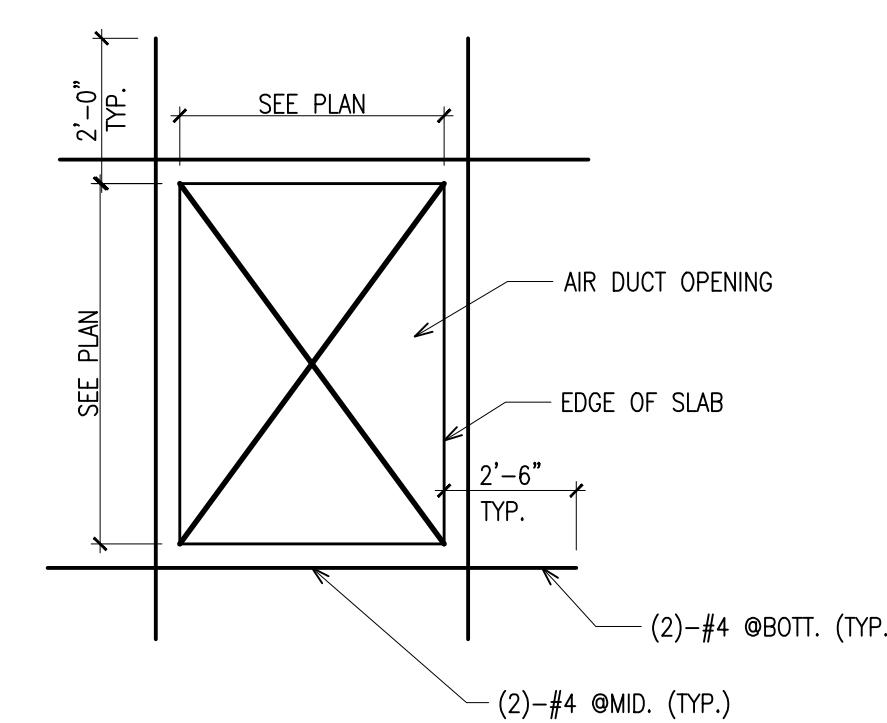
The words "suitable intervals" used in clause 6.5.2 characterize that it is not necessary to inspect these tasks for each weld, but as necessary to ensure that the applicable requirements of AWS D1.1/D1.1M are met. Following the same principles and terminology, Chapter N labels these tasks as "observe," which is defined as "observe these items on a random basis."

INSPECTION TASKS DURING WELDING	QC	QA
Control and handling of welding consumables	0	0
Packaging	0	0
Exposure control	0	0
No welding over cracked tack welds	0	0
Environmental conditions	0	0
Wind speed within limits	0	0
Precipitation and temperature	0	0
WPS followed	0	0
Settings on welding equipment	0	0
Travel speed	0	0
Selected welding materials	0	0
Shielding gas type/flow rate	0	0
Preheat applied	0	0
Interpass temperature maintained (min/max)	0	0
Proper position (F, V, H, OH)	0	0
Welding techniques	0	0
Interpass and final cleaning	0	0
Each pass within profile limitations	0	0
Each pass meets quality requirements	0	0
Placement and installation of steel headed stud anchors	P	P

INSPECTION TASKS AFTER WELDING	QC	QA
Welds cleaned	0	0
Size, length and location of welds	P	P
Welds meet visual acceptance criteria	P	P
Cross prohibition	P	P
Weld/boss-metal fusion	P	P
Croter cross section	P	P
Weld profiles	P	P
Weld size	P	P
Undercut	P	P
Porosity	P	P
Arc strikes	P	P
k-arec[a]	P	P
Weld access holes in rolled heavy shapes and built-up heavy shapes[b]	P	P
Backing removed and weld tabs removed (if required)	P	P
Repair activities	P	P
Document acceptance or rejection of welded joint or member	P	P
No prohibited welds have been added without the approval of the EOR	0	0

[a] When welding of doubler plates, continuity plates or stiffeners has been performed in the k-arec, visually inspect the web k-arec for cracks within 3 in. (75 mm) of the weld.

[b] After rolled heavy shapes (see Section A3.1c) and built-up heavy shapes (see Section A3.1d) are welded, visually inspect the weld access hole for cracks.



TYPICAL DUCT OPENING DETAIL

SCALE: NTS

FOR DUCT LOCATIONS AND SIZES SEE ARCHITECTURAL & MEP DWGS

DESIGN LOAD TABLE (VALUES IN PSF)	ROOF	3rd FLOOR	2nd FLOOR
LIVE LOAD	25*	80	80
ROOFING	1	-	-
RIGID INSULATION	3	-	-
METAL DECK	2	2	4
SOLAR PANELS	13	-	-
4 1/2" SLAB (110 PCF LIGHTWEIGHT CONCRETE)	0	35	35
STEEL FRAMING	2	3	3
CEILING (MESH OR ACOUSTICAL) + M/E/P	5	5	5
TOTAL LOAD (LL + DL)	51	125	127

* - MECHANICAL WITH SNOW DRAFT

2020 BUILDING CODE OF NEW YORK STATE (BASED ON IBC 2018)

SEISMIC NOTATIONS

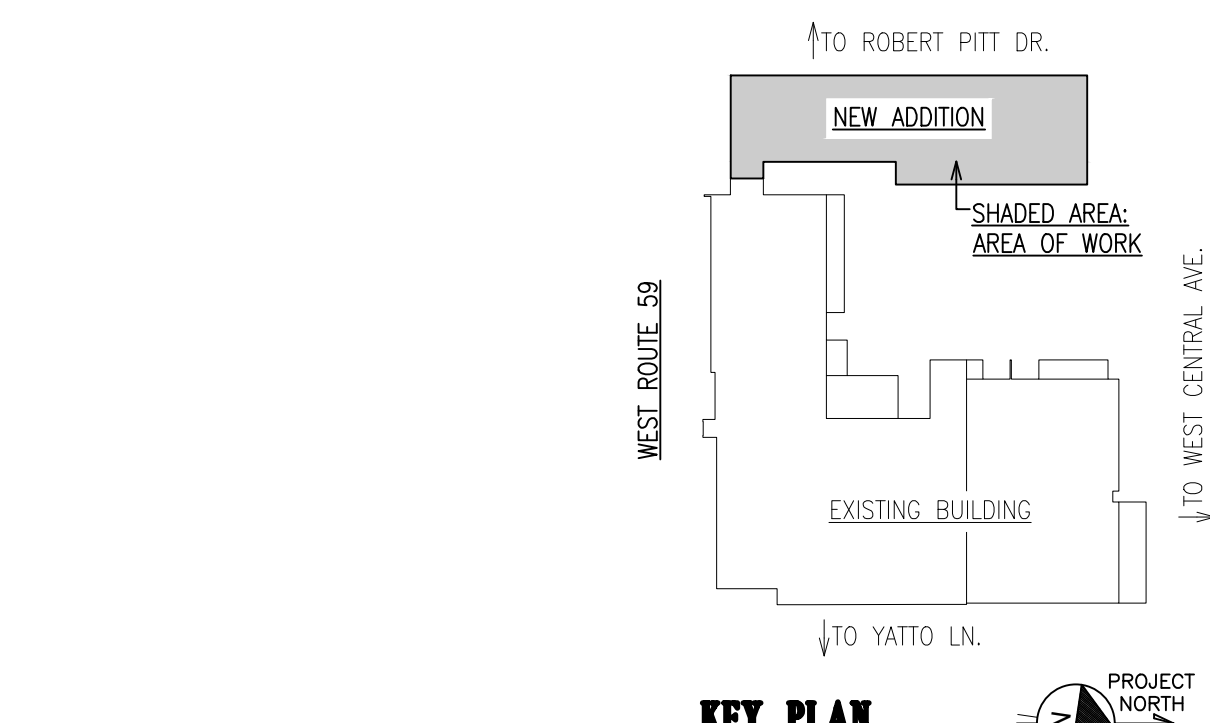
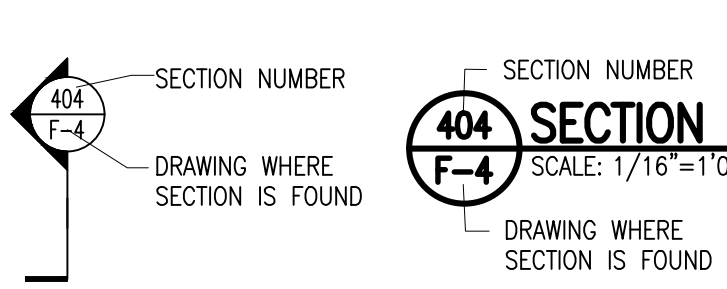
RISK CATEGORY II
SEISMIC IMPORTANCE FACTOR I=1.0
MAPPED SPECTRAL RESPONSE ACCELERATION S_s=0.294
MAPPED SPECTRAL RESPONSE ACCELERATION S₁=0.06
SOIL SITE CLASS: D
SPECTRAL RESPONSE COEFFICIENT S_{ds}=0.31
SPECTRAL RESPONSE COEFFICIENT S_{d1}=0.10
SEISMIC DESIGN CATEGORY B
SEISMIC FORCE RESISTING SYSTEM: STEEL ORDINARY MOMENT FRAMES
DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN SYSTEMS
SEISMIC RESPONSE COEFFICIENT C_s=0.041
RESPONSE MODIFICATION FACTOR R=3.25
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE PER ASCE 7-16
DESIGN BASE SHEAR V_E=77 K

WIND NOTATIONS

BASIC WIND SPEED=115 mph
WIND LOAD IMPORTANCE FACTOR I=1.0
WIND EXPOSURE C
INTERNAL PRESSURE COEFFICIENT ±0.18
COMPONENTS & CLADDING AT ROOF AND WALLS VELOCITY PRESSURE: SIMPLIFIED PROCEDURE PER ASCE 7-16
MAIN WIND FORCE RESISTING SYSTEM ANALYSIS PROCEDURE: SIMPLIFIED PROCEDURE PER ASCE 7-16
DESIGN BASE SHEAR V_E=65 K, F_r=224 K

SNOW NOTATIONS

GROUND SNOW LOAD P_g=10 psf
FLAT ROOF SNOW LOAD P_f=20.0 psf
SNOW EXPOSURE FACTOR C_e=0.9
SNOW LOAD IMPORTANCE FACTOR I=1.0
THERMAL FACTOR C_t=1.0



STEEL DECK NOTES:

- SUBMIT ONE (1) PRINT EACH OF SHOP DRAWING AND PDF ELECTRONIC COPY FOR ENGINEER / ARCHITECT REVIEW. ONLY PDF VERSION WILL BE RETURNED TO THE CONTRACTOR. SHOP DRAWINGS MUST BE REVIEWED AND APPROVED BY THE CONTRACTOR PRIOR TO SUBMISSION, AND ARE TO BEAR THE CONTRACTOR'S STAMP SIGNIFYING SUCH.
- ALL REVISIONS TO SHOP DRAWINGS AFTER THE FIRST SUBMISSION MUST BE SO IDENTIFIED ON SUBSEQUENT SUBMISSIONS.
- REVIEW OF SHOP DRAWINGS SHALL NOT RELIEVE THE CONTRACTOR OF ANY CONTRACT REQUIREMENTS, EVEN IF SUCH ITEMS ARE NOT SHOWN ON THE SHOP DRAWINGS.
- ANY CHANGES PROPOSED BY THE DETAILER MUST BE CLEARLY IDENTIFIED ON THE SHOP DRAWINGS AND MUST BE SUBSTITUTED BY THE SUBMISSION OF CALCULATIONS USED IN DESIGN OF SUCH CHANGE.
- DECK CONTRACTOR SHALL FURNISH AND INSTALL REINFORCING PLATES WELDED TO METAL DECK AT OPENINGS UP TO 9" INCLUSIVE. METAL DECK CONTRACTOR SHALL FURNISH AND INSTALL REINFORCING ANCHORS 2 1/2" x 2 1/2" x 1/4" ON 2 SIDES OF OPENINGS GREATER THAN 9" AND LESS THAN 18" EXCEPT WHERE STEEL FRAME IS PROVIDED TYPICAL AT ROOF AND FLOORS.
- DECK CONTRACTOR SHALL SHORE THE METAL DECK IF THE UNSHORED CLEAR SPAN OF THE DECK IS EXCEEDED DURING CONSTRUCTION.

- MATERIALS AND DESIGN-COMPOSITE FLOOR DECK SHALL BE TYPE 2 VUL 20 GAUGE AS MANUFACTURED BY VULCRAFT, FROM STEEL CONFORMING TO ASTM A1008 OR ASTM A653 WITH MINIMUM YIELD STRENGTH OF 50 KSI. DECK SHALL BE DETAILED FOR A THREE OR MORE CONTINUOUS SPANS CONDITION (EXCEPT TWO SPAN LIMITATION OF THE FRAMING PLAN). DEFLECTION RELATIVE TO SUPPORT BEAMS AND CAUSED BY THE DEAD LOAD OF WET CONCRETE AND DECK SHALL NOT EXCEED L (DECK SPAN)/180 OR 3/4". LIVE LOAD CAPACITIES SHALL BE CALCULATED IN ACCORDANCE WITH THE "SDI COMPOSITE DECK DESIGN HANDBOOK". LIVE LOAD DEFLECTIONS SHALL NOT EXCEED 1/360 OF THE SPAN.
- FINISHES-STEEL ROOF DECK SHALL BE CONFORM TO THE REQUIREMENTS OF ASTM A653 COATING CLASS 60.
- TOLERANCES-MANUFACTURER'S STANDARD TOLERANCES APPLY. (DECK SHEET LENGTH IS PLUS OR MINUS 1/2"). BASE STEEL THICKNESS SHALL BE GREATER THAN OR EQUAL TO 95% OF THE DESIGN THICKNESS.
- INSTALLATION-STEEL DECK SHALL BE ERECTED AND FASTENED IN ACCORDANCE WITH PROJECT SPECIFICATIONS, THE APPROVED ERECTION LAYOUTS, AND THE "SDI MANUAL OF CONSTRUCTION WITH STEEL DECK", TO FORM A WORKING PLATFORM. IMMEDIATELY FASTEN SHEETS TO THE SUPPORTS. WELDS TO SUPPORTS SHALL BE 5/8" ARC PUDDLE WELDS WITH WELD SPACING OF 12" MAXIMUM. SIDE LAPS ARE TO BE WELDED AT A MAXIMUM SPACING OF 36" ON CENTER FOR SPANS OVER 5'-0". DECK SHALL BE BUTTED OVER SUPPORTS. FLOOR OPENINGS LOCATED AND DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE CUT BY THE FLOOR DECK CONTRACTOR. HOLES FOR OTHER TRADES PLUS ANY REINFORCING FOR THESE HOLES SHALL BE CUT AND REINFORCED BY THE OTHER TRADES. ALL SHEETS FROM OPENED BUNDLES MUST BE FASTENED BEFORE THE END OF THE WORKING DAY. BUNDLES MUST BE LEFT SECURED TO PREVENT WIND BLOWING THE INDIVIDUAL SHEETS. MAXIMUM UNSHORED SPAN, SLAB THICKNESS AND CONCRETE DENSITY SHALL BE POSTED ON THE ERECTION DRAWINGS. MINIMUM END BEARING IS 1'-1/2".
- CONCRETE-CONCRETE SHALL BE LIGHTWEIGHT (110 PCF). PLACEMENT OF CONCRETE SHALL CONFORM TO THE APPLICABLE SECTIONS OF THE ACI SPECIFICATIONS AND THE "SDI MANUAL OF CONSTRUCTION WITH STEEL DECK". IF BUGGIES ARE USED, THE DECK SHALL BE PLANKED TO PREVENT DAMAGE. CONTRACTOR SHALL NOT EXCEED THE SDI CONSTRUCTION LOADS DURING EITHER CONCRETE PLACEMENT OR FINISHING. CALCIUM CHLORIDE (OR ANY ADMIXTURE CONTAINING CHLORIDE SALTS) SHALL NOT BE USED IN CONCRETE PLACED CURE.
- SITE STORAGE-IF SITE STORAGE IS NEEDED, THE BUNDLES OF DECK SHALL BE STORED OFF THE GROUND FOR A THREE OR MORE CONTINUOUS SPANS CONDITION (EXCEPT TWO SPAN LIMITATION OF THE FRAMING PLAN). THE DEFLECTION OF THE DECK UNDER DESIGN LIVE LOAD SHALL NOT EXCEED 1/240 OF THE SPAN OR 1", WHICHEVER IS LESS. SECTION PROPERTIES USED IN DETERMINING STRESS AND DEFLECTION SHALL HAVE BEEN CALCULATED IN ACCORDANCE WITH AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS". COEFFICIENTS FOR MOMENTS AND DEFLECTIONS SHALL CONFORM TO SDI "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS AND THE AISI/SDI-RD1.0 STANDARD FOR STEEL ROOF DECK".

- MATERIALS AND DESIGN-STEEL DECK SHALL BE TYPE 3N, 20 GAGE (SEE ROOF FRAMING PLAN FOR LOCATION), AS MANUFACTURED BY VULCRAFT AND SHALL BE MADE FROM STEEL CONFORMING TO ASTM A653 GRADE 40. THE MINIMUM YIELD STRENGTH IS 50,000 PSI. DECK SHALL BE DETAILED FOR A THREE OR MORE CONTINUOUS SPANS CONDITION (EXCEPT TWO SPAN LIMITATION OF THE FRAMING PLAN). THE DEFLECTION OF THE DECK UNDER DESIGN LIVE LOAD SHALL NOT EXCEED 1/240 OF THE SPAN OR 1", WHICHEVER IS LESS. SECTION PROPERTIES USED IN DETERMINING STRESS AND DEFLECTION SHALL HAVE BEEN CALCULATED IN ACCORDANCE WITH AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS". COEFFICIENTS FOR MOMENTS AND DEFLECTIONS SHALL CONFORM TO SDI "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS AND THE AISI/SDI-RD1.0 STANDARD FOR STEEL ROOF DECK".
- FINISHES-STEEL ROOF DECK SHALL CONFORM TO THE REQUIREMENTS OF ASTM A653 COATING CLASS 60.
- INSTALLATION-STEEL DECK SHALL BE ERECTED AND FASTENED IN ACCORDANCE WITH PROJECT SPECIFICATIONS, THE APPROVED ERECTION LAYOUTS, AND THE "SDI MANUAL OF CONSTRUCTION WITH STEEL DECK". CUTTING OPENINGS THROUGH THE DECK LESS THAN 25 SQUARE FOOT IN AREA, AND ALL SKEW CUTTING, SHALL BE PERFORMED IN THE FIELD. ARC PUDDLE WELDS SHALL BE AT LEAST 5/8" INCH IN DIAMETER, OR ELONGATED HAVING AN EQUAL PERIMETER. FASTENING AT SUPPORTS SHALL BE AT LEAST EDGE RIBS. SPACE ADDITIONAL WELDS SHALL BE 12 INCHES APART BUT NO MORE THAN 18 INCHES. SIDE LAPS OF INDIVIDUAL SHEETS MUST BE FASTENED TOGETHER BEFORE SUPPORTS FOR SPANS GREATER THAN 5 FEET, THE FASTENER SPACING SHALL NOT EXCEED 3 FEET ON CENTER. END LAPS (WHEN REQUIRED) OF SHEETS SHALL BE A MINIMUM OF 2 INCHES (1 INCH ON EACH SIDE OF ITS CENTERLINE) AND SHALL OCCUR OVER SUPPORTS. WHEN SUPPORTS ARE STEEL JOISTS, THE MINIMUM END LAP SHALL BE 4 INCHES. END LAPS SHALL BE WELDED TO SUPPORTS AT 6" O/C. MINIMUM END BEARING IS 1'-1/2".
- ACCESSORIES- RIDGE AND VALLEY COVER PLATES AND FLAT PLATES AT CHANGE OF DECK DIRECTION (WHEN ATTACHED DIRECTLY TO THE STEEL DECK AND AS SHOWN ON CONTRACT DOCUMENTS) SHALL BE FURNISHED AND INSTALLED BY DECK CONTRACTOR. COVER PLATES SHALL BE 20 GAUGE MIN. AND FASTENED TO DECK ON BOTH SIDES OF JOINT WITH SELF-TAPPING SCREWS AT 12" O/C. METAL DECK CONTRACTOR SHALL FURNISH AND INSTALL CLOSE-LOCK METAL CLADDING AT ALL ROOF OPENINGS. METAL DECK CONTRACTOR SHALL PROVIDE 2 1/2" x 2 1/2" x 1/4" ANGLE FRAMING AT ROOF DRAINS AND OVERFLOW DRAINS. METAL DECK CONTRACTOR SHALL VERIFY ALL ROOF OPENING DIMENSIONS WITH APPROVED ROOF UNIT(S) MANUFACTURER'S CUTS AND SHOP DRAWINGS.

SPECIAL INSPECTIONS

- PER NEW YORK STATE BUILDING CODE 2020 EDITION 1705.2.1 STRUCTURAL, STEEL SPECIAL INSPECTIONS AND NON-DESTRUCTIVE TESTING OF THE STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES AND PORTIONS THEREOF AND SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISI 360 (2016).

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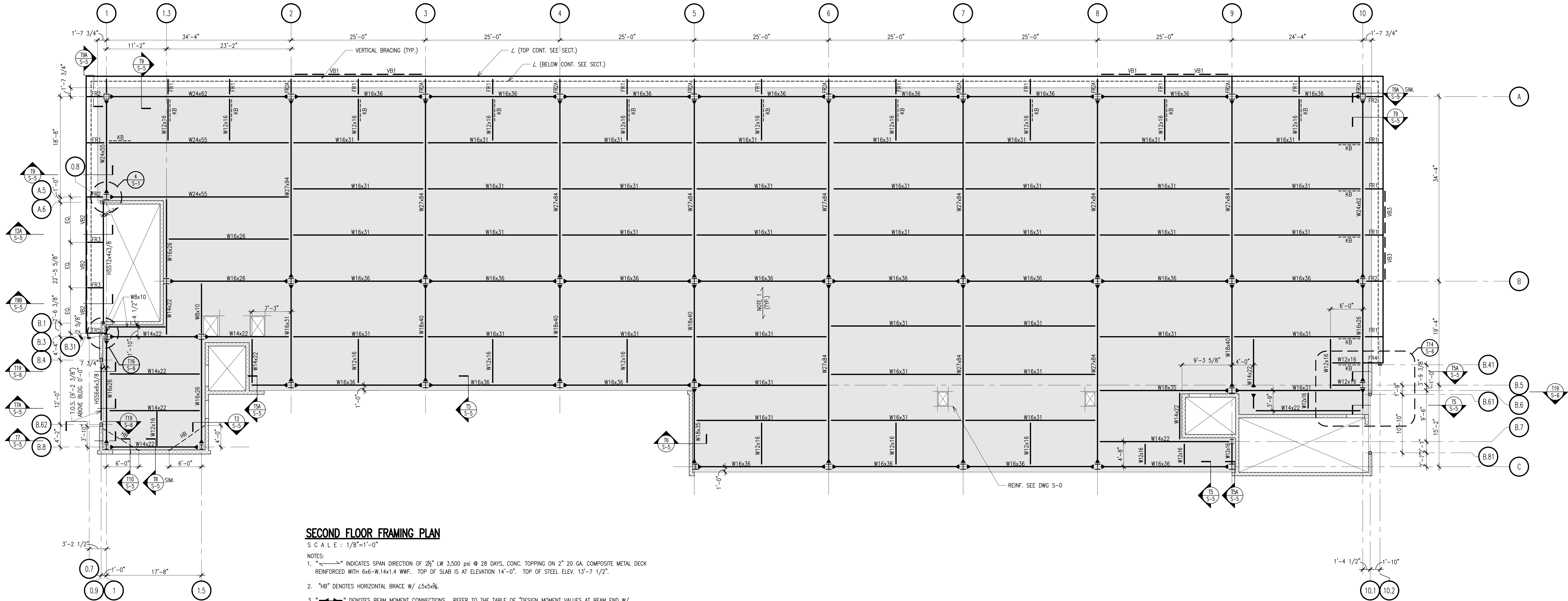
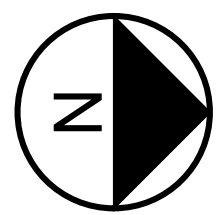
PEARL RIVER NEW YORK

NOTES AND TYPICAL DETAILS	FILE NO.	SHEET	REV.
SVOC NEW OFFICE ADDITION & NEW PARKING	5593	S-0	47 OF 95

ENGINEERING REVIEW	DATE	DATE	DATE
DEPT.	SIGNATURE	DATE	DATE
CIVIL			
METERS			
RELAY			
TRANS			
SLAB			
DATE: NOVEMBER 25 2024			
DESIGN REVIEW	DATE	DATE	DATE
DESIGN REVIEW	DATE	DATE	DATE

SNS ARCHITECTS & ENGINEERS 100 WEST ROUTE 50 VILLAGE OF SPRING VALLEY TOWN OF RAMAPO ROCKLAND COUNTY, NEW YORK	PEARL RIVER ROCKLAND COUNTY, NEW YORK
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JOHN LIGNOS, AIA LORIN J. SONENSHINE, AIA STEVEN NAPOLITANO, PE, PP ROBERT NACCILLA, AIA

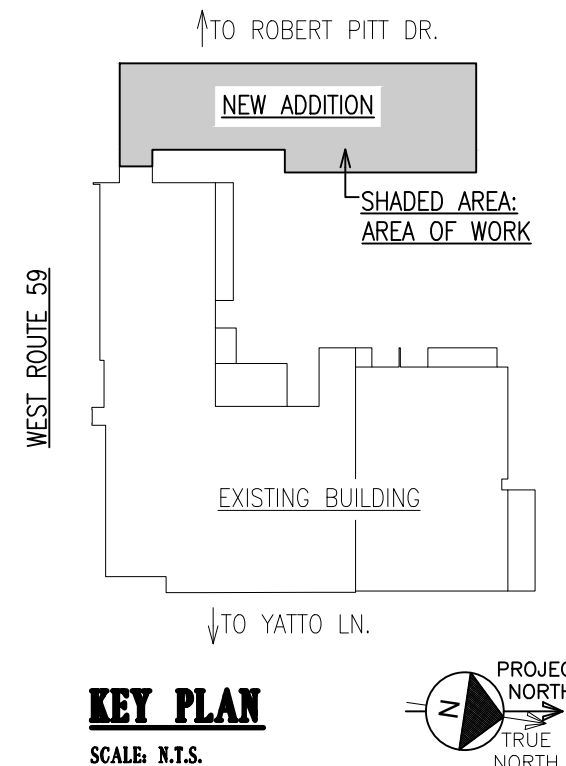


SECOND FLOOR FRAMING PLAN

SCALE: 1/8"=1'-0"

NOTES:

1. " " INDICATES SPAN DIRECTION OF 2½" LW 3,500 psi @ 28 DAYS, CONC. TOPPING ON 2" 20 GA. COMPOSITE METAL DECK REINFORCED WITH 6x6-W14x1.4 WWF. TOP OF SLAB IS AT ELEVATION 14'-0". TOP OF STEEL ELEV. 13'-7 1/2".
2. "HB" DENOTES HORIZONTAL BRACE W/ ½x5x½.
3. " " DENOTES BEAM MOMENT CONNECTIONS. REFER TO THE TABLE OF "DESIGN MOMENT VALUES AT BEAM END W/ MOMENT CONNECTION" ON DWG. S-0 FOR MOMENT CONNECTIONS DESIGN, U.O.N.
4. " " INDICATES CONCRETE SLAB AREAS.
5. VB** INDICATES VERTICAL BRACE, SEE DETAIL AT DRAWING S-0 FOR MORE INFO.



KEY PLAN
SCALE: 1/8"=1'-0"

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PEARL RIVER NEW YORK

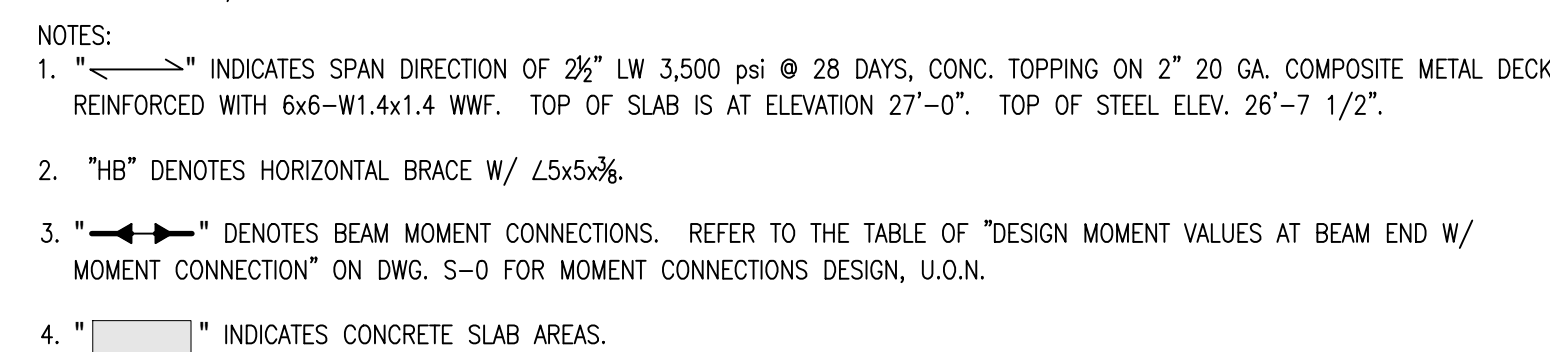
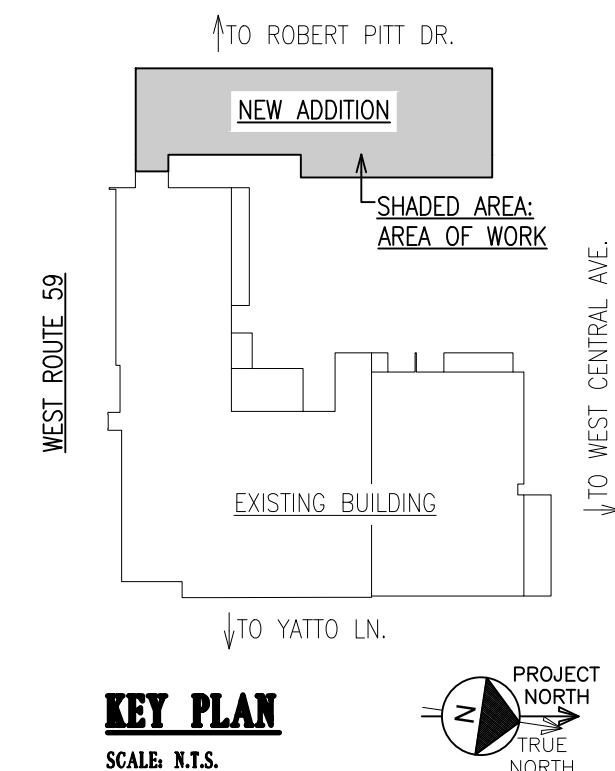
SNS ARCHITECTS & ENGINEERS 400 WEST ROUTE 59 PEARL RIVER, NY 10955 TEL: 845.338.1700 FAX: 845.338.0808 WWW.SNS-ARCH.COM	ENGINEERING REVIEW DEPT. CIVIL SIGNATURE DATE DATE	APPROVED DATE DATE	SECOND FLOOR FRAMING PLAN AND DETAILS SVOC NEW OFFICE ADDITION & NEW PARKING TAX MAP ID 68.62-1-2 SHEEP KILLS ROUTE 59 VILLAGE OF SPRING VALLEY TOWN OF RAMAPO ROCKLAND COUNTY, NEW YORK
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DRAFTED : A.S. CHECKED : S.N. PROJ. NO : 5593 DATE: NOVEMBER 25, 2024	OWN DES CHD SCALE AS NOTED	FILE NO. 5593	DRAWING NUMBER S-1	SHEET 48 OF 95	REV. 1
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PEARL RIVER
ROCKLAND COUNTY, NEW YORK

SNS Architects & Engineers, PC
ONE PARAGON DRIVE . MONTVALE . NEW JERSEY . 07040

John Lignos, AIA
Lorin J. Sonenshine, AIA
Steven Napolitano, PE, PP
Robert Nocella, AIA

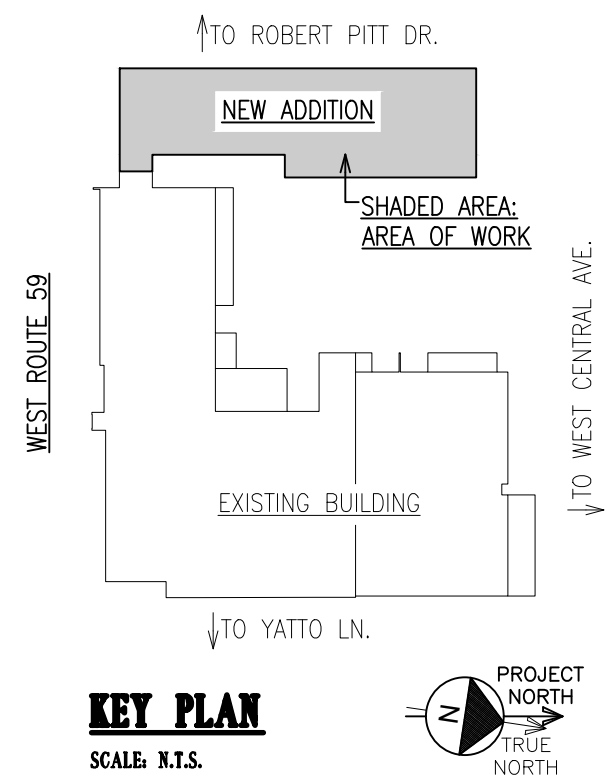
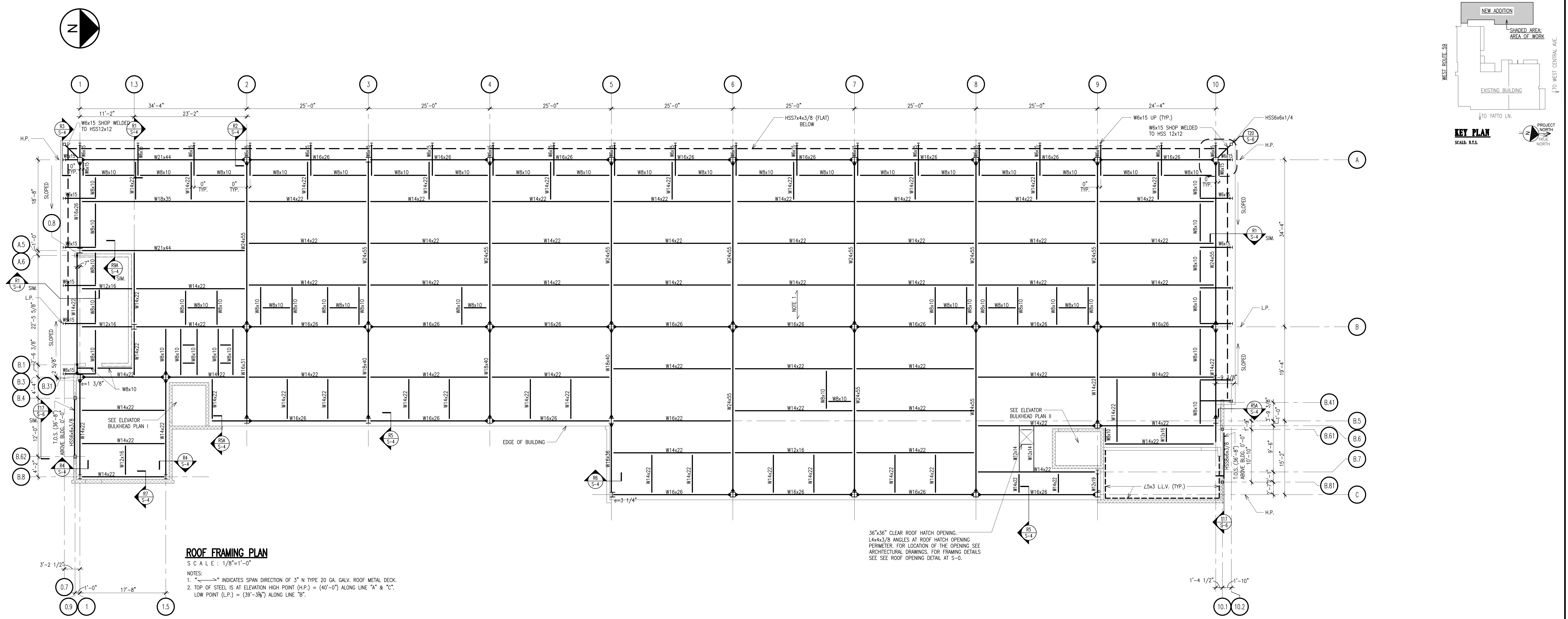


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DATE: NOVEMBER 25 2024

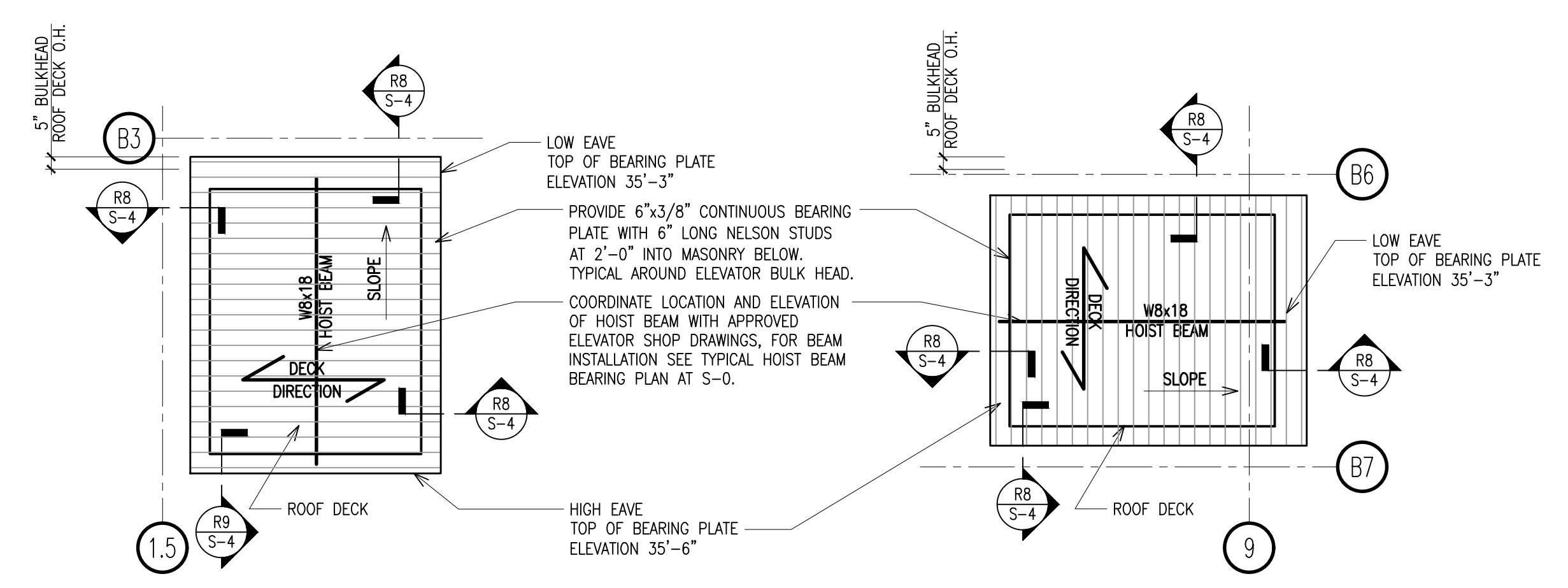


Steven Napolitano, PE,
PP Cert./Lic. No.
NY 077323



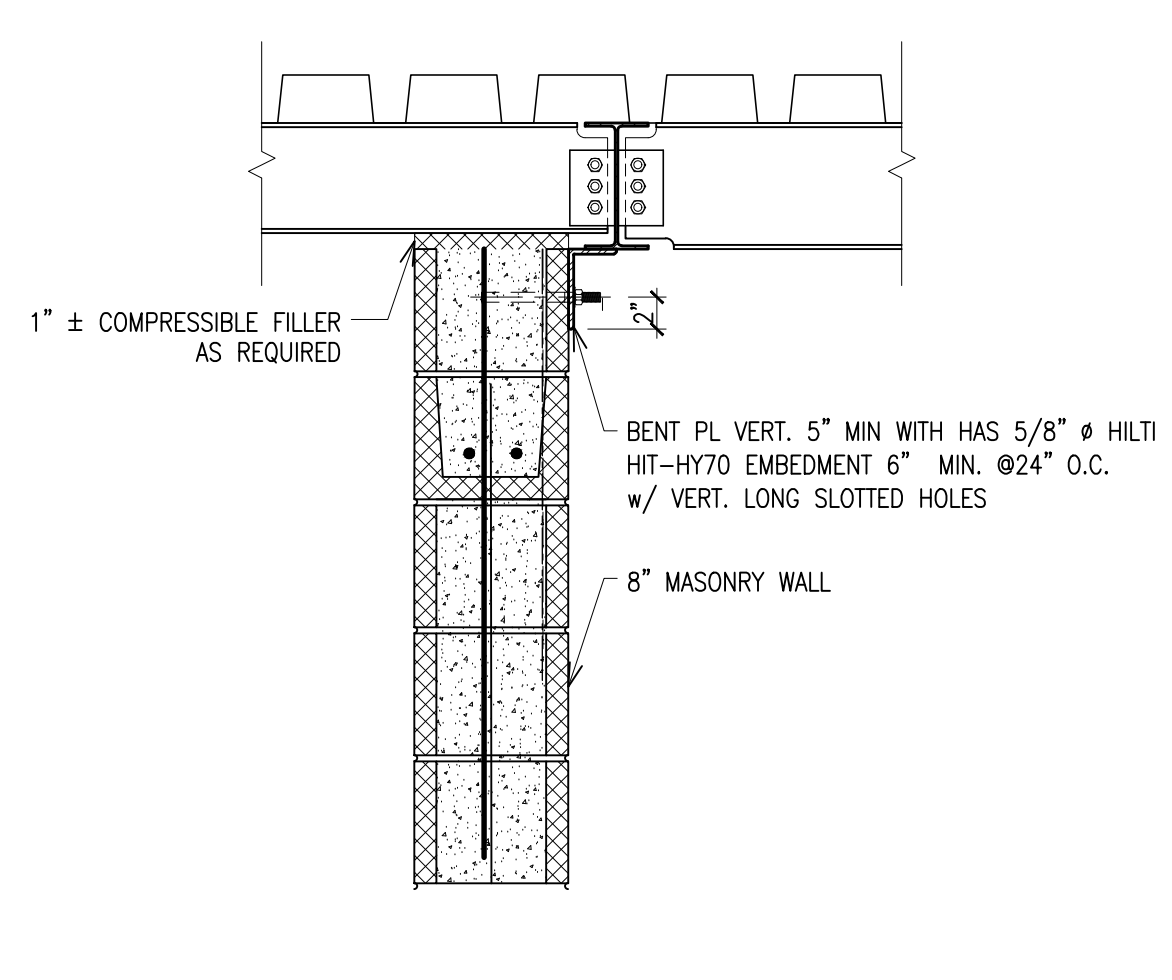
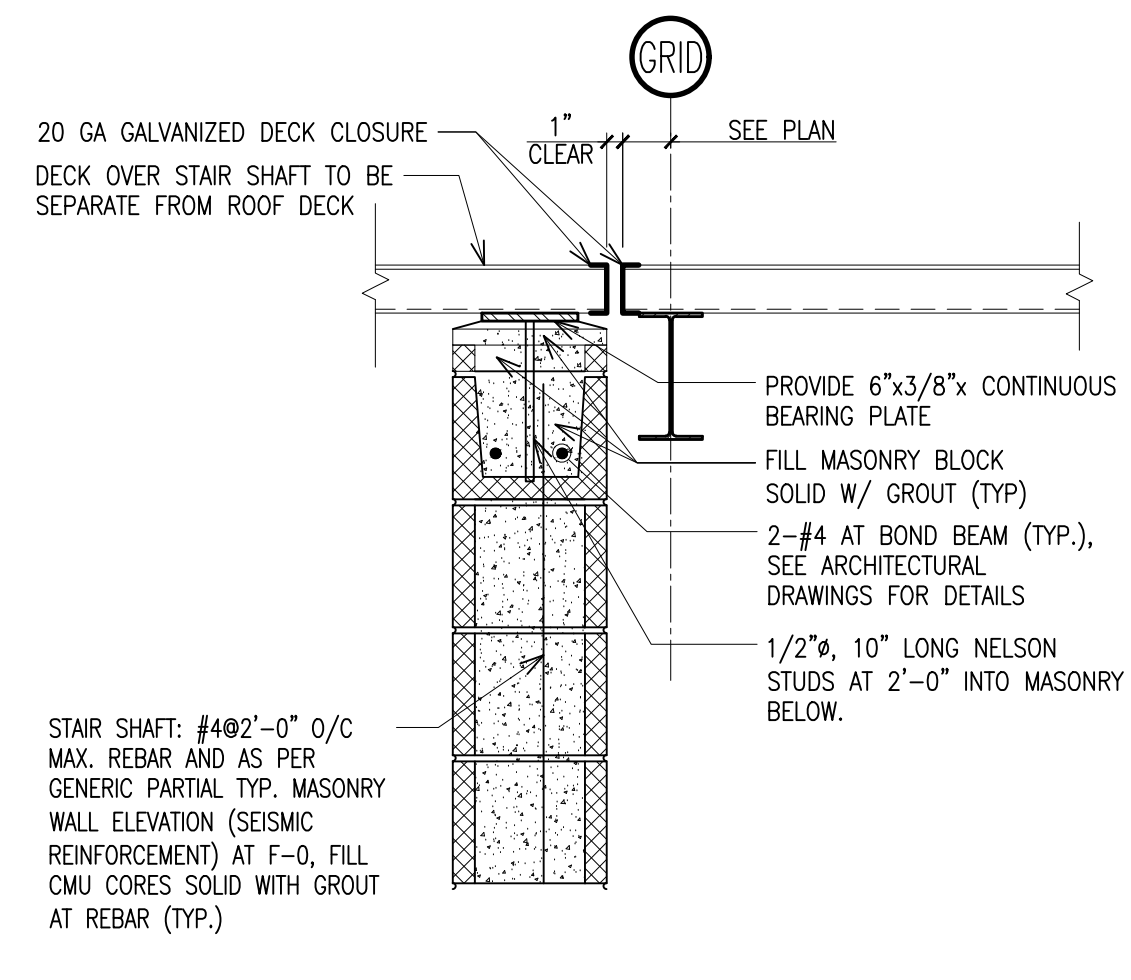
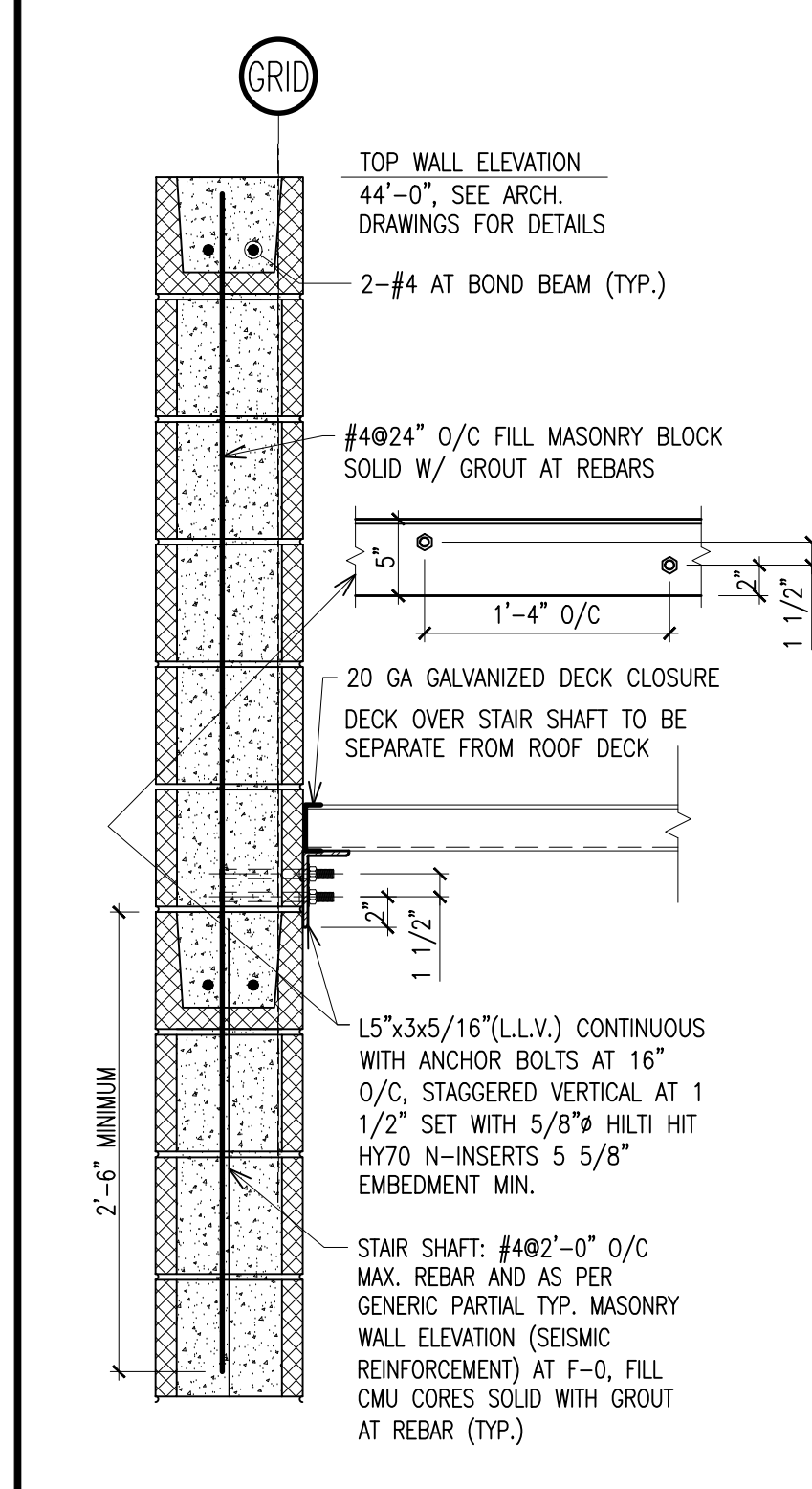
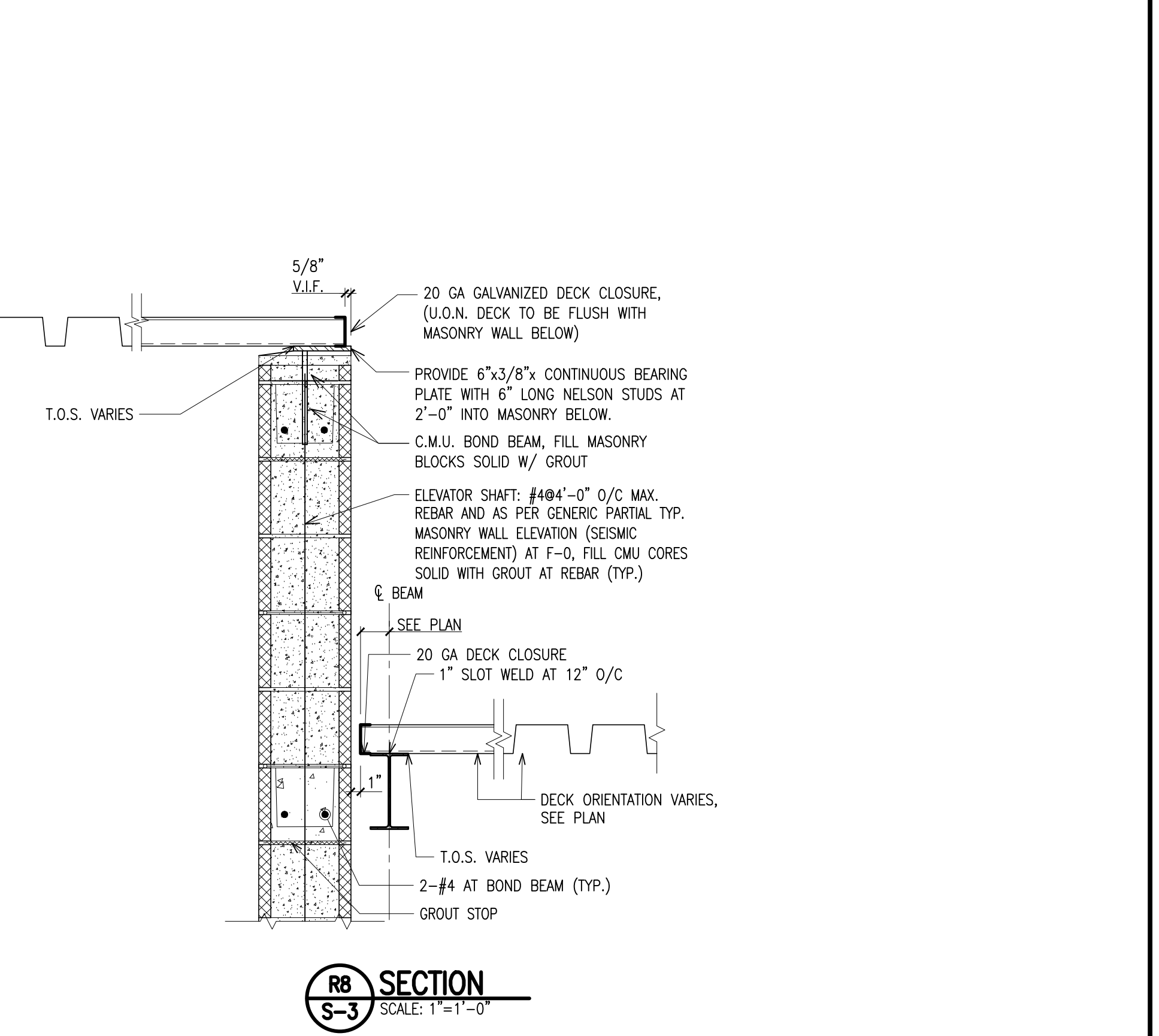
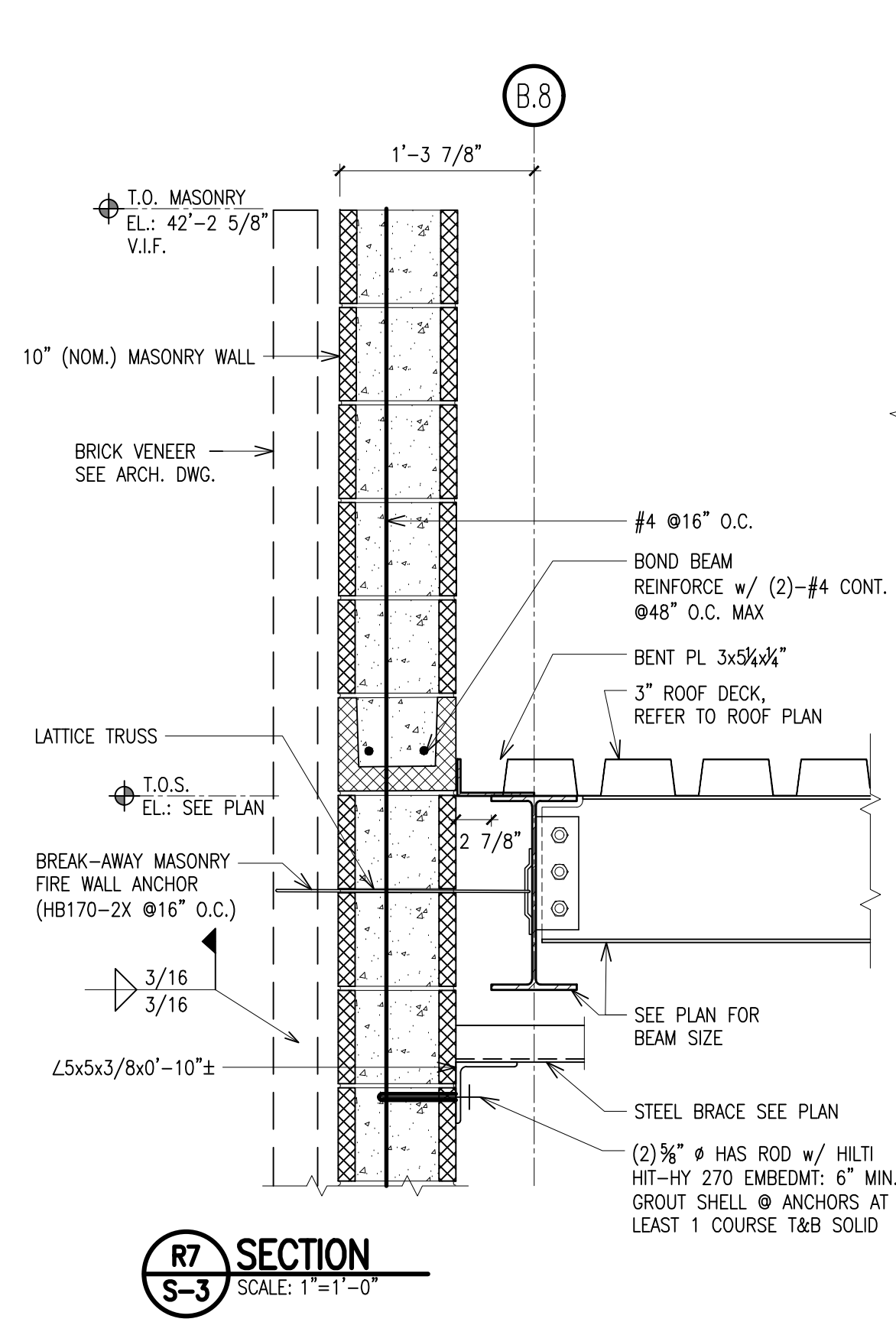
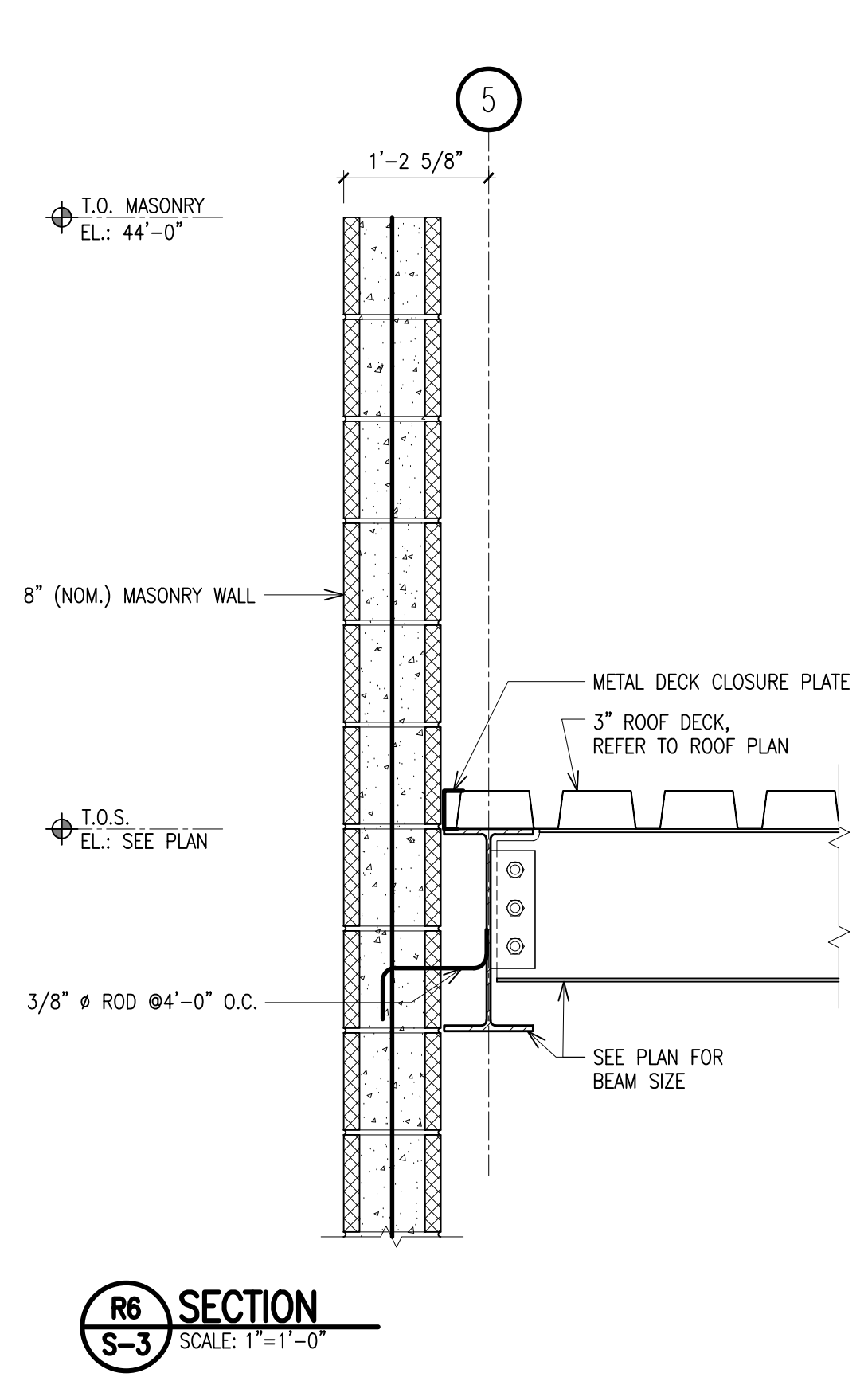
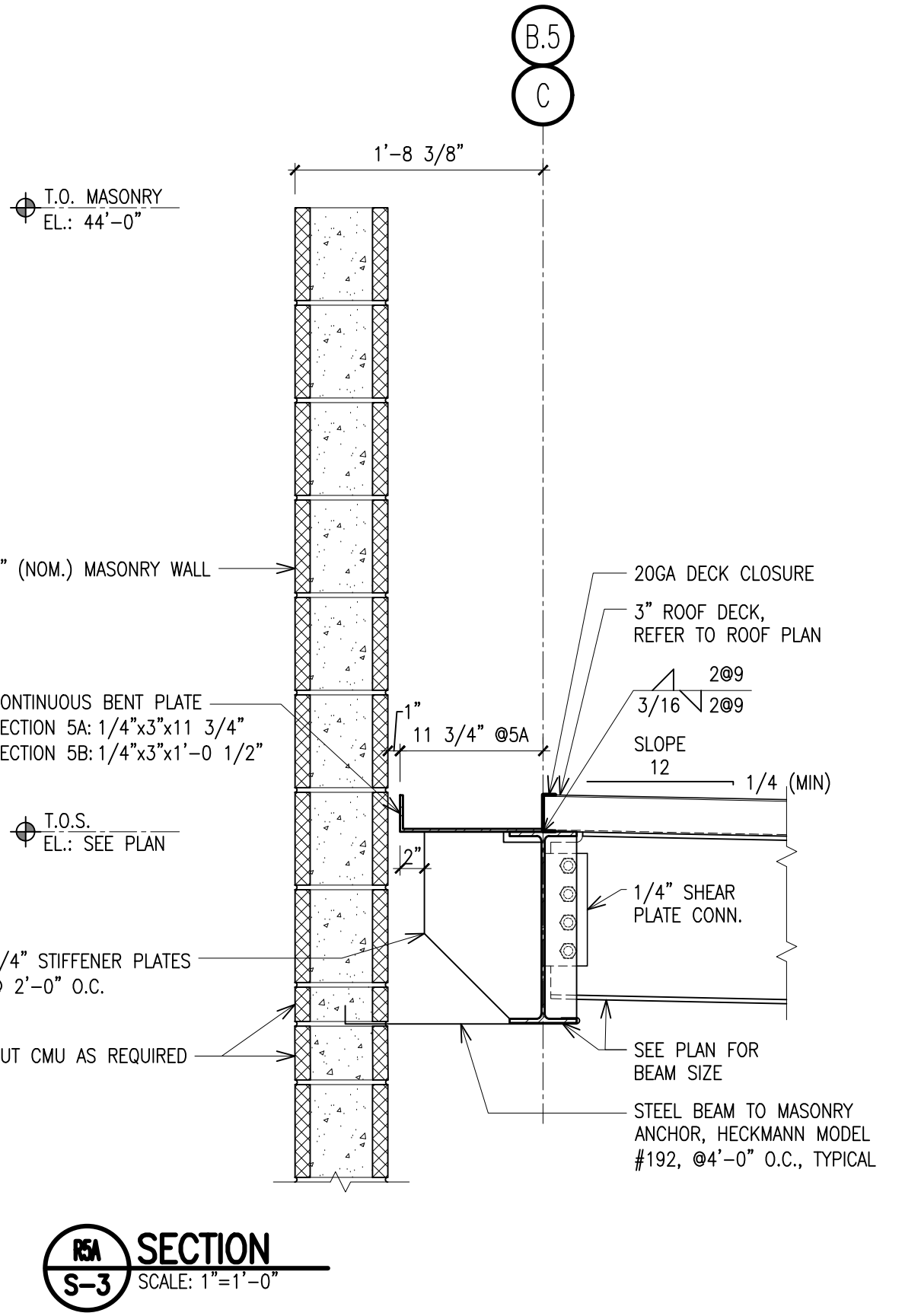
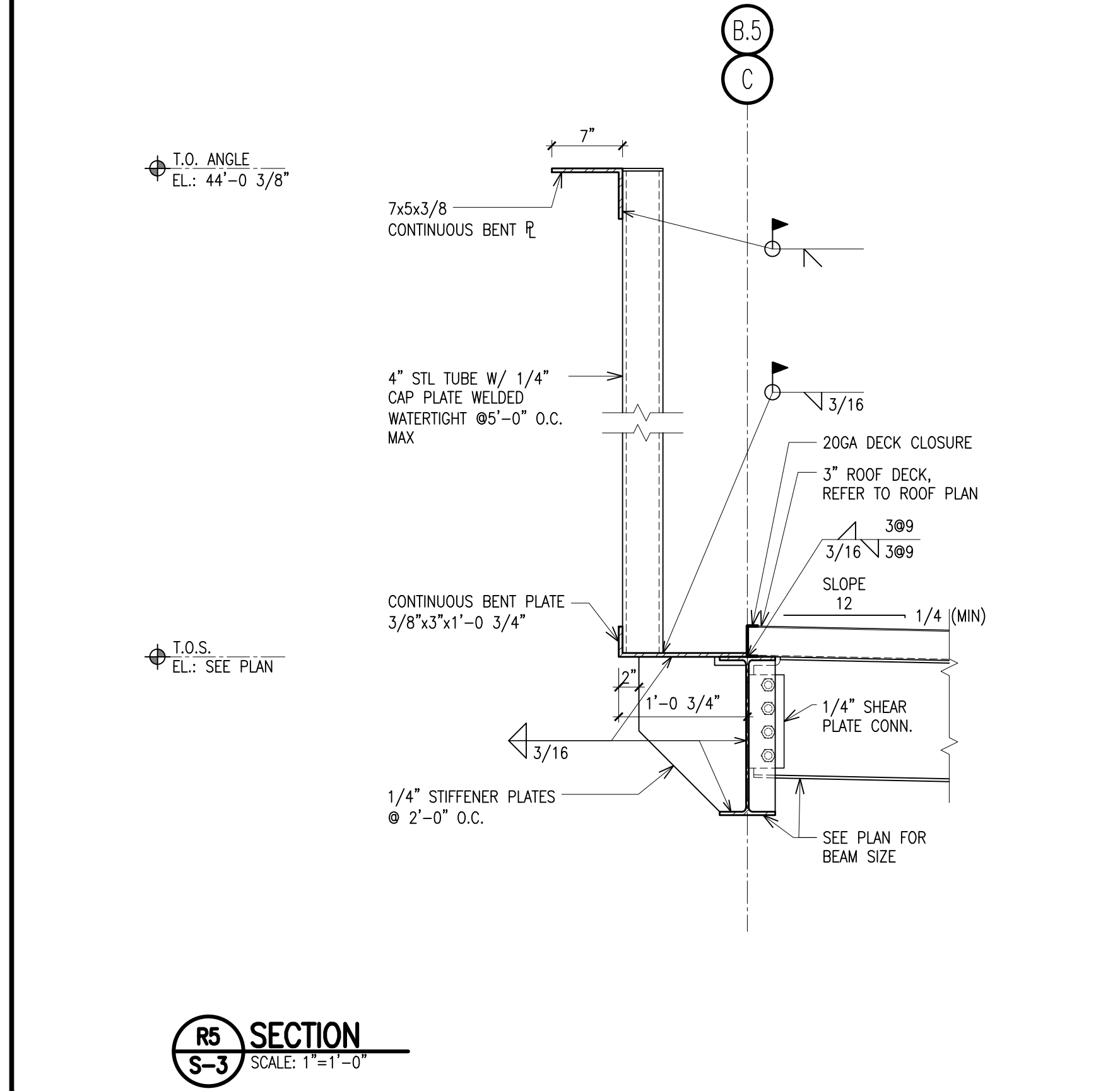
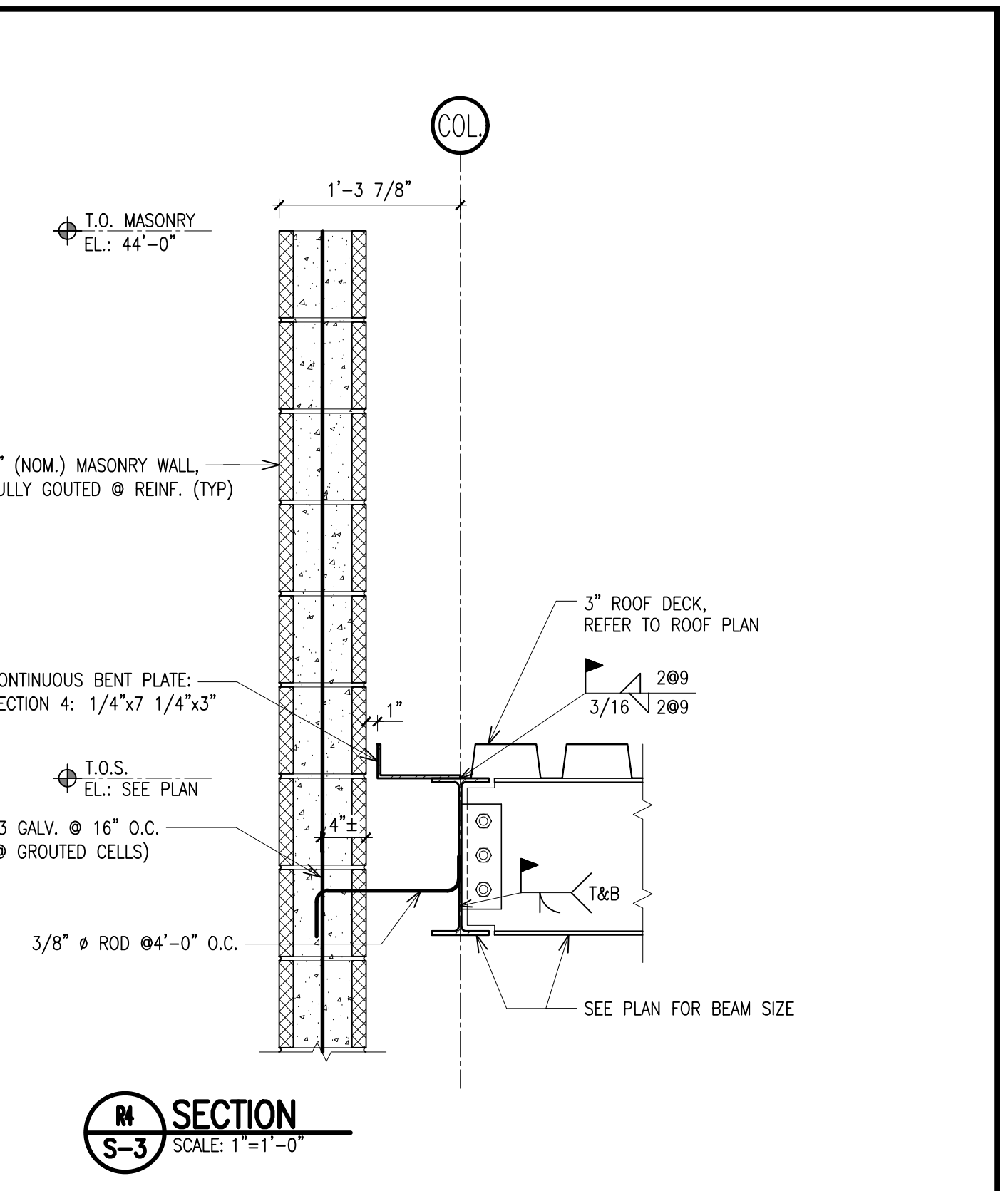
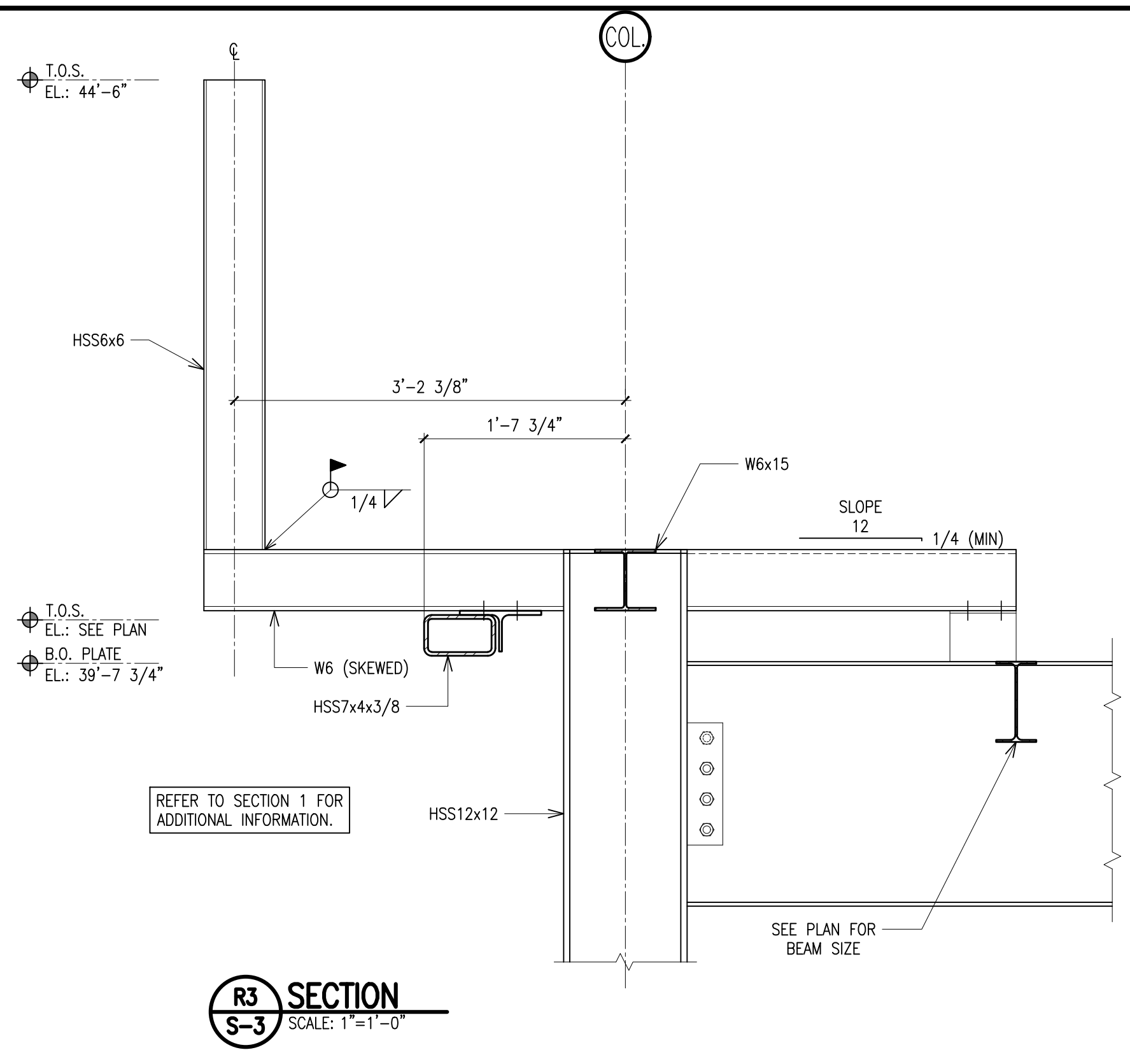
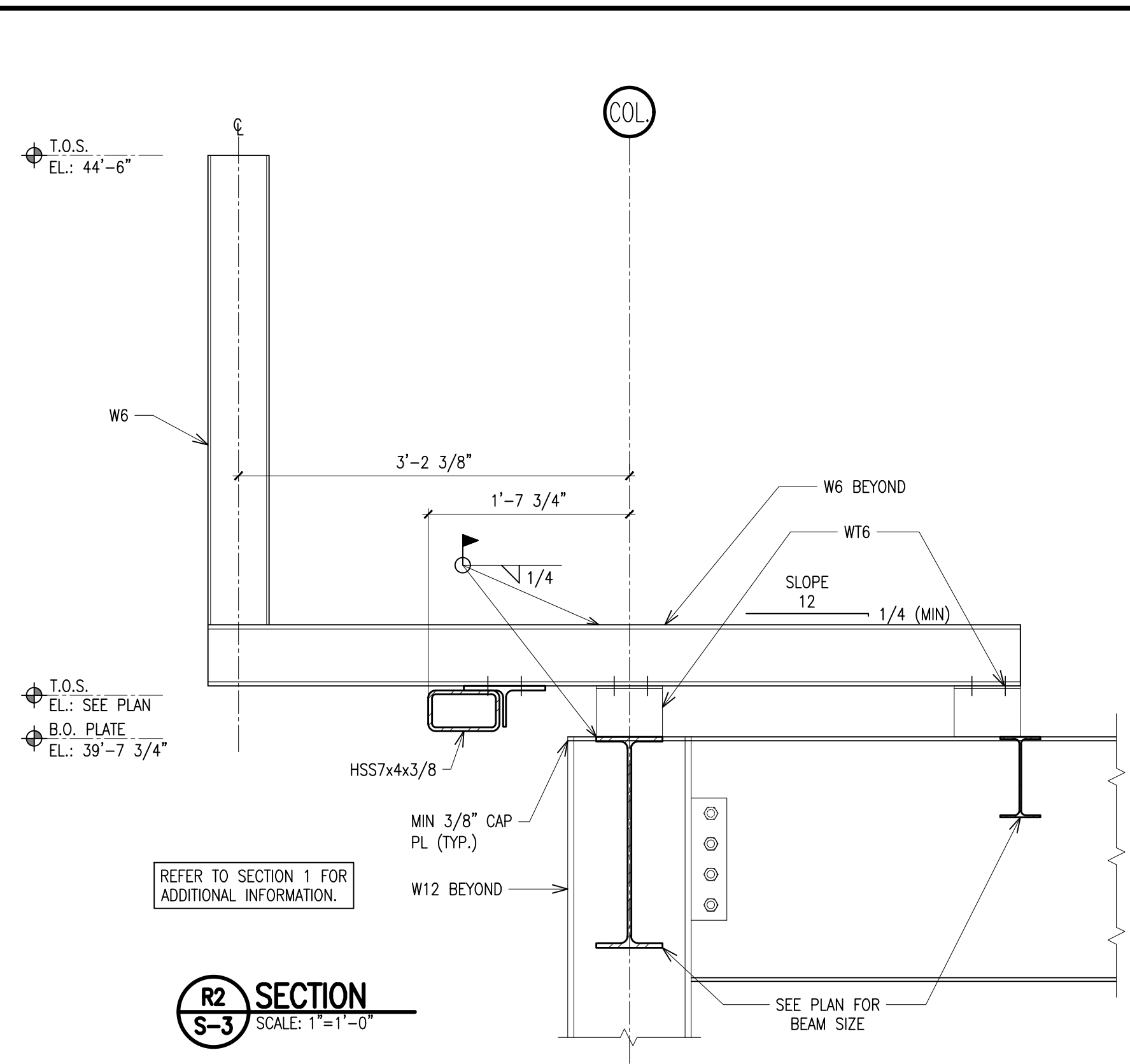
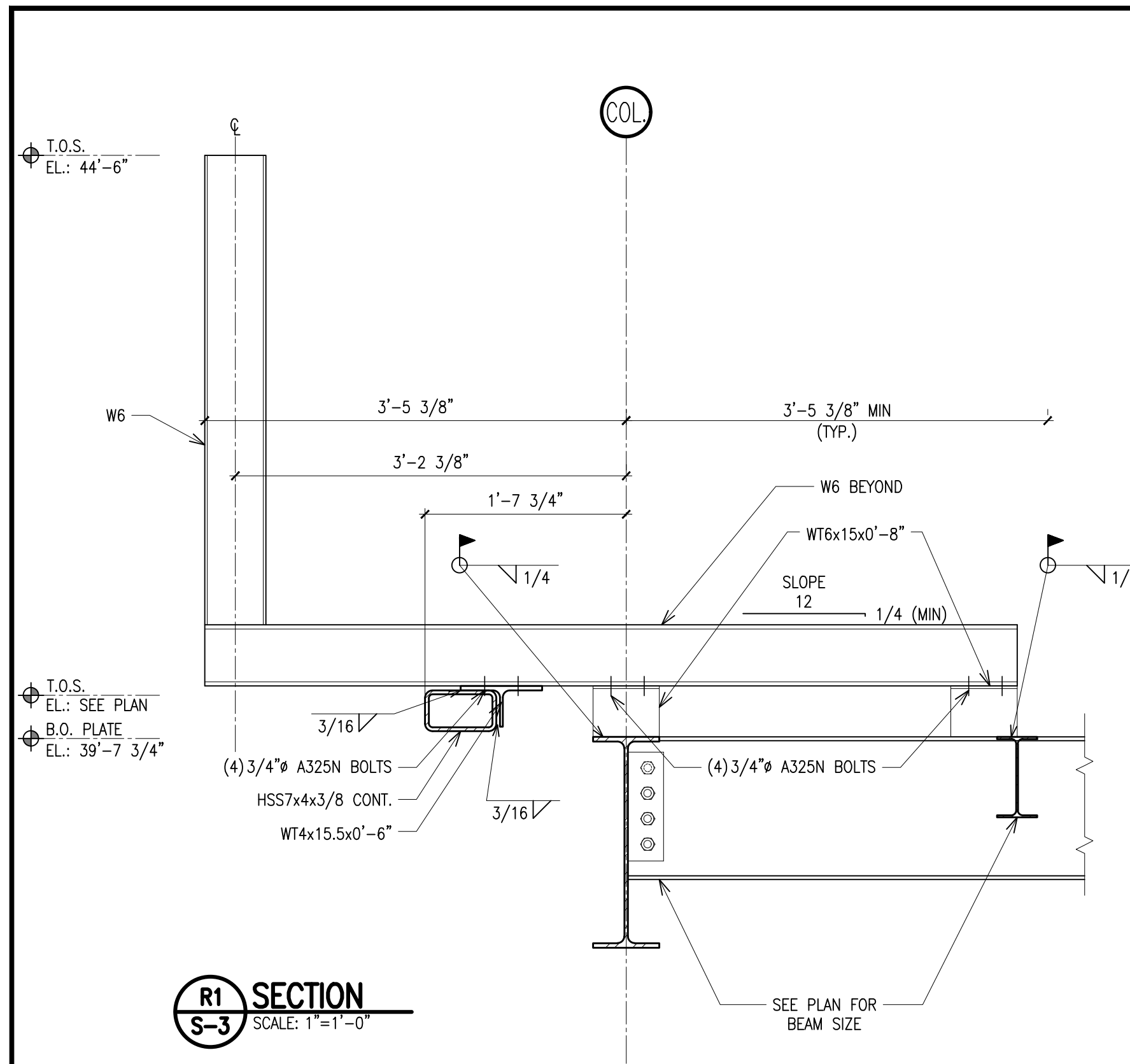
ROOF FRAMING PLAN
SCALE: 1/8"=1'-0"
NOTES:
1. " " INDICATES SPAN DIRECTION OF 3" N TYPE 20 GA. GALV. ROOF METAL DECK.
2. TOP OF STEEL IS AT ELEVATION HIGH POINT (H.P.) = (40'-0") ALONG LINE "A" & "C".
LOW POINT (L.P.) = (39'-3 3/4") ALONG LINE "B".

36'x36' CLEAR ROOF HATCH OPENING.
1/4x3/8 ANGLES AT ROOF HATCH OPENING PERIMETER. FOR LOCATION OF THE OPENING SEE ARCHITECTURAL DRAWINGS. FOR FRAMING DETAILS SEE SEE ROOF OPENING DETAIL AT S-0.



ELEVATOR BULKHEAD PLAN I
SCALE: 1/4"=1'-0"
ELEVATOR BULKHEAD PLAN II
SCALE: 1/4"=1'-0"

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ORANGE AND ROCKLAND UTILITIES, INC. PEARL RIVER NEW YORK			
ROOF FRAMING PLAN AND DETAILS		SVOC NEW OFFICE ADDITION & NEW PARKING	
TAX MAP ID 68.62-1-2 3RD WEST ROUTE 59 VILLAGE OF SPRING VALLEY TOWN OF RAMAPO ROCKLAND COUNTY, NEW YORK		FILE NO. 5593 DRAWING NUMBER S-3 SHEET 50 OF 95 REV. 1	
SNS ARCHITECTS & ENGINEERS 401.573.1767 401.573.0808 www.sns-arch.com DRAFTED: A.S. CHECKED: S.N. PROJ. NO.: 5593 DATE: NOVEMBER 25, 2024		ENGINEERING REVIEW DEPT. CIVIL SIGNATURE DATE DATE: NOVEMBER 25, 2024	
APPROVED DATE DATE: NOVEMBER 25, 2024		SCALE: AS NOTED	
PEARL RIVER ROCKLAND COUNTY, NEW YORK		PEARL RIVER ROCKLAND COUNTY, NEW YORK	
Steven Napolitano, P.E. P.E. Cert./Lic. No. 077323		John Lignos, AIA Lorin J. Sonenshine, AIA Steven Napolitano, P.E., P.P. Robert Nocella, AIA	

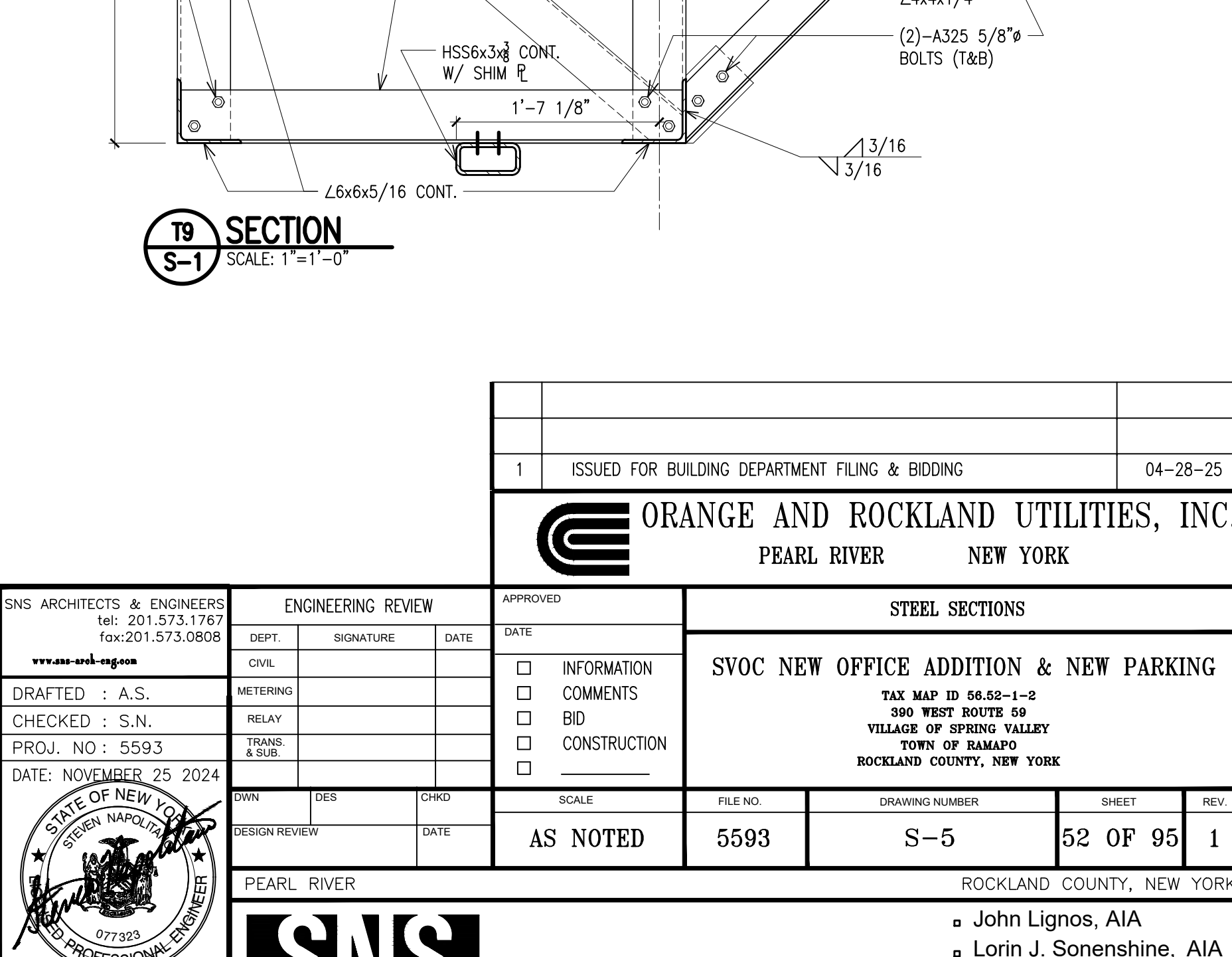
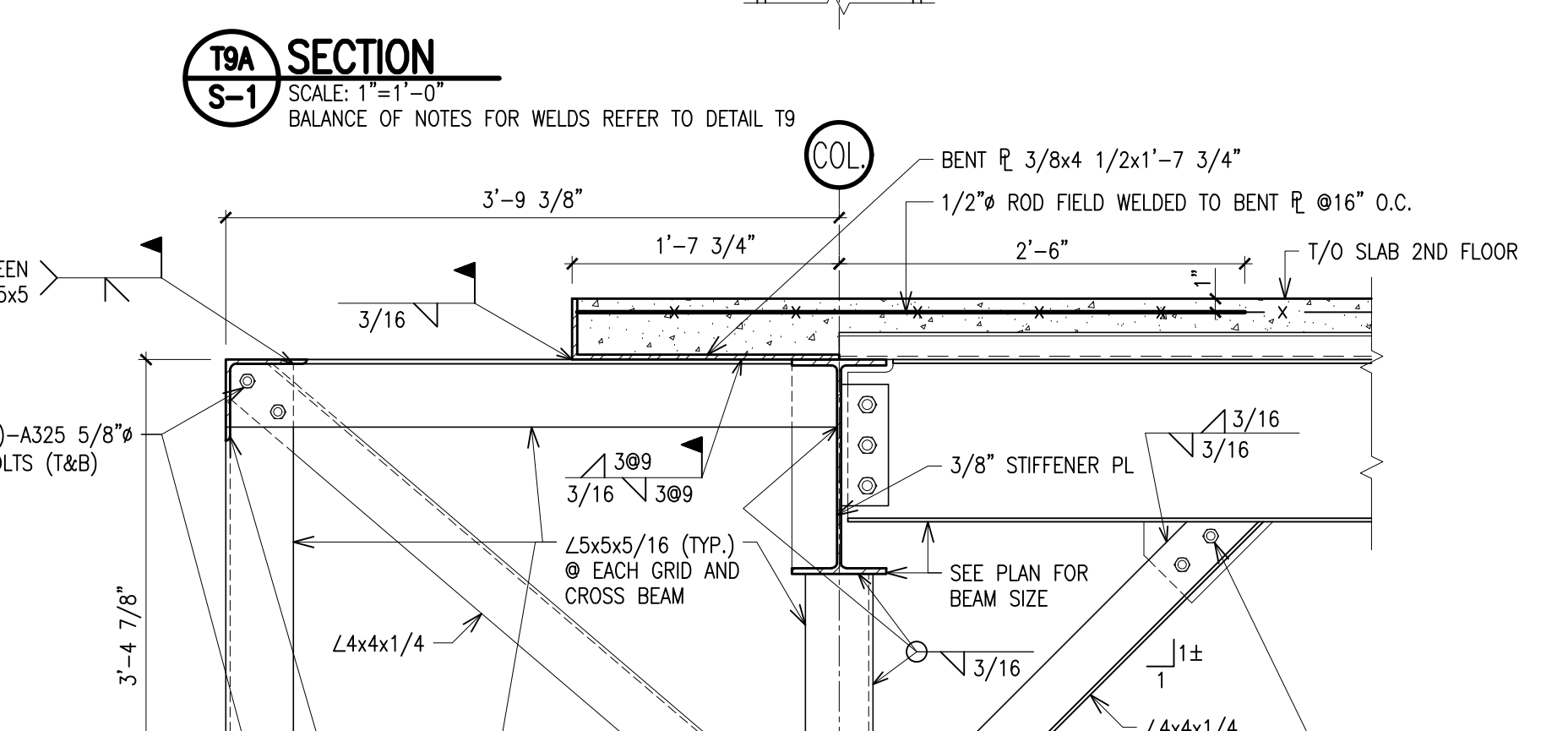
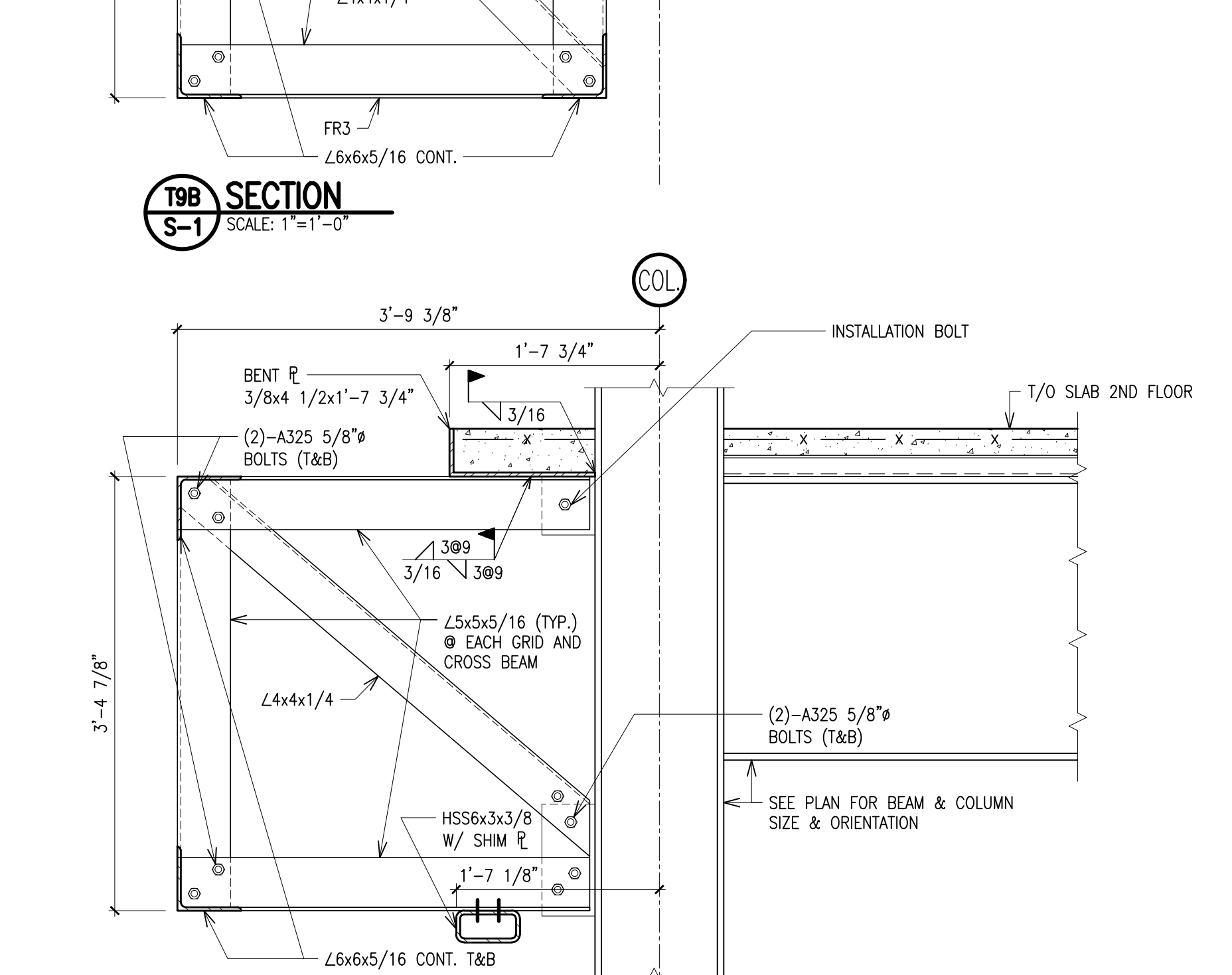
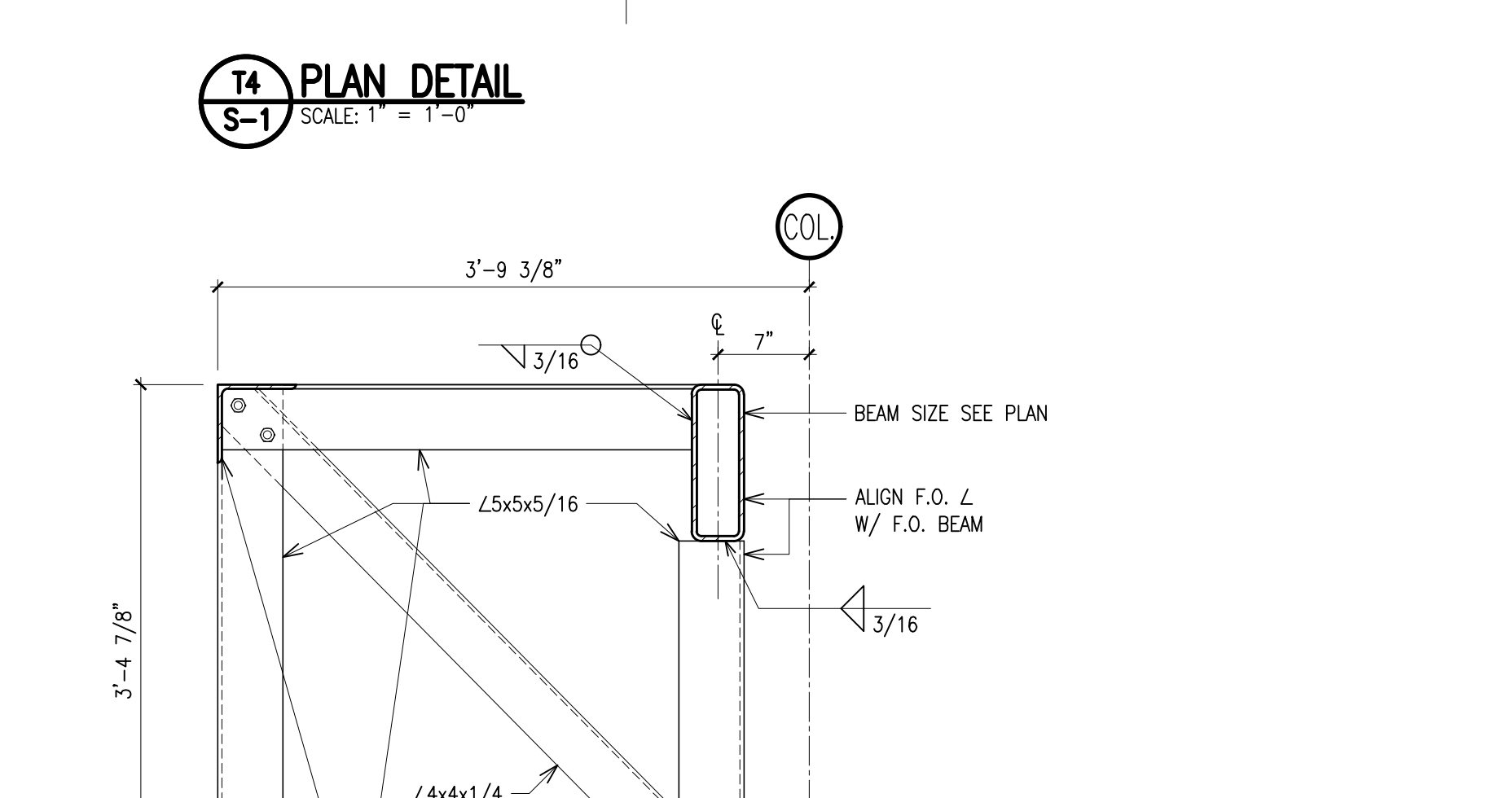
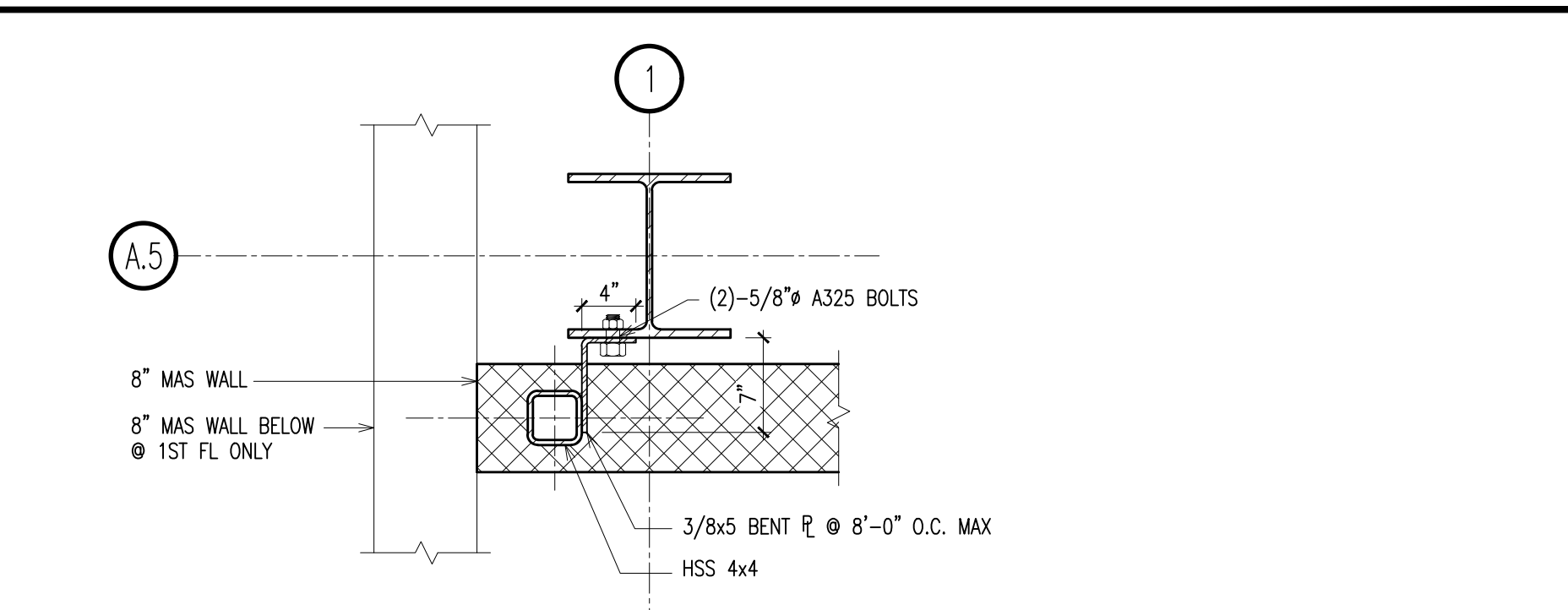
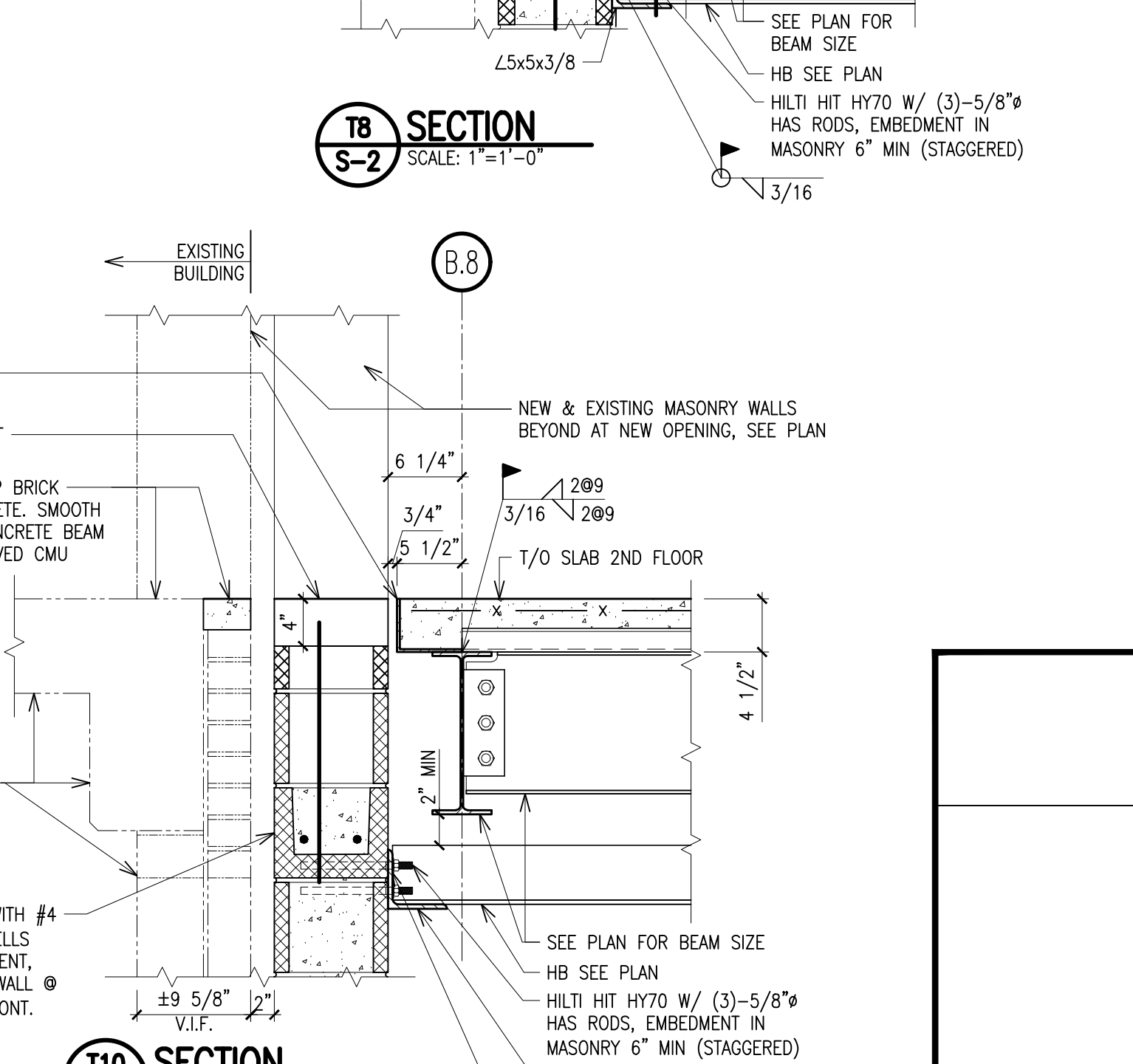
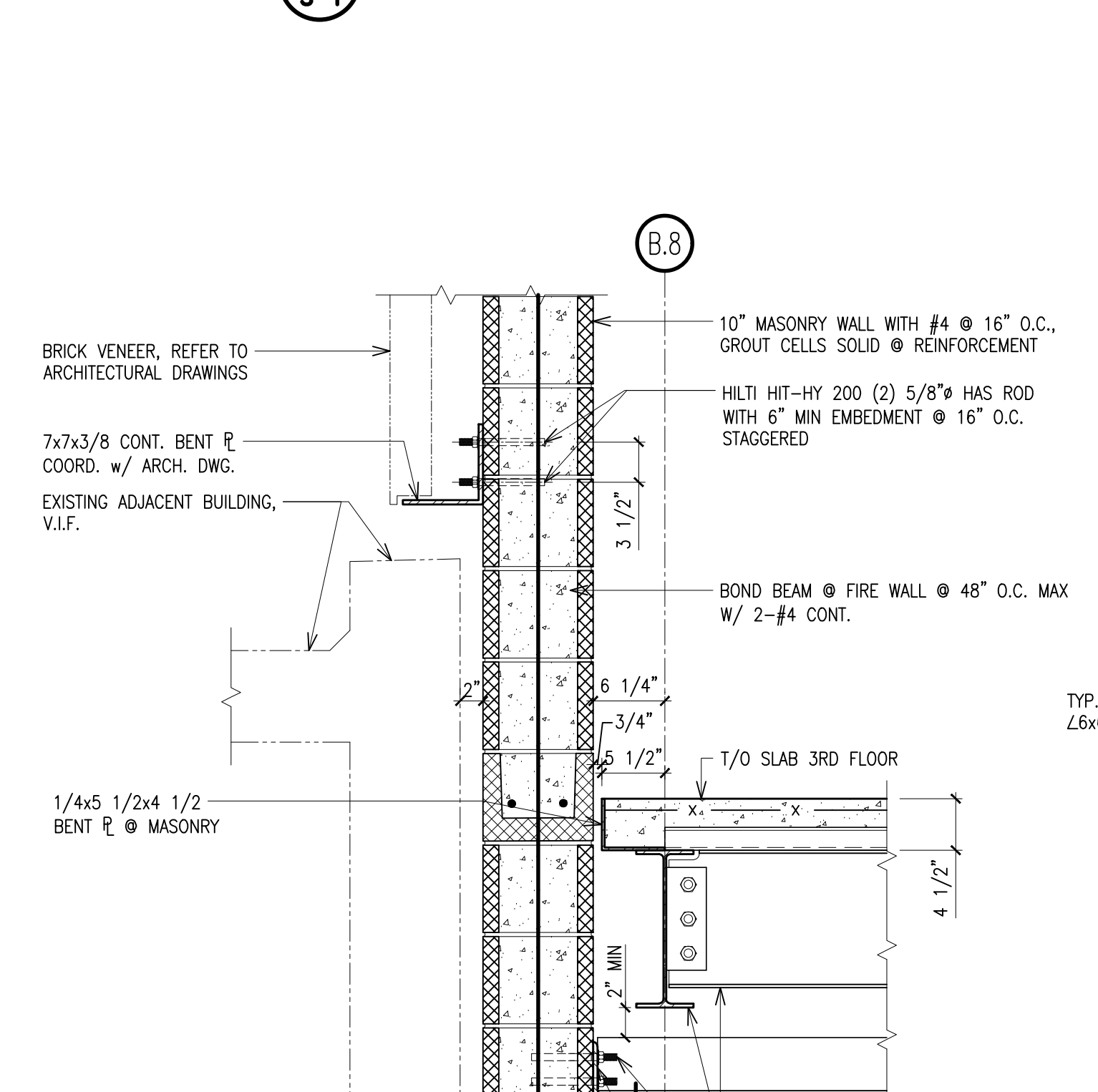
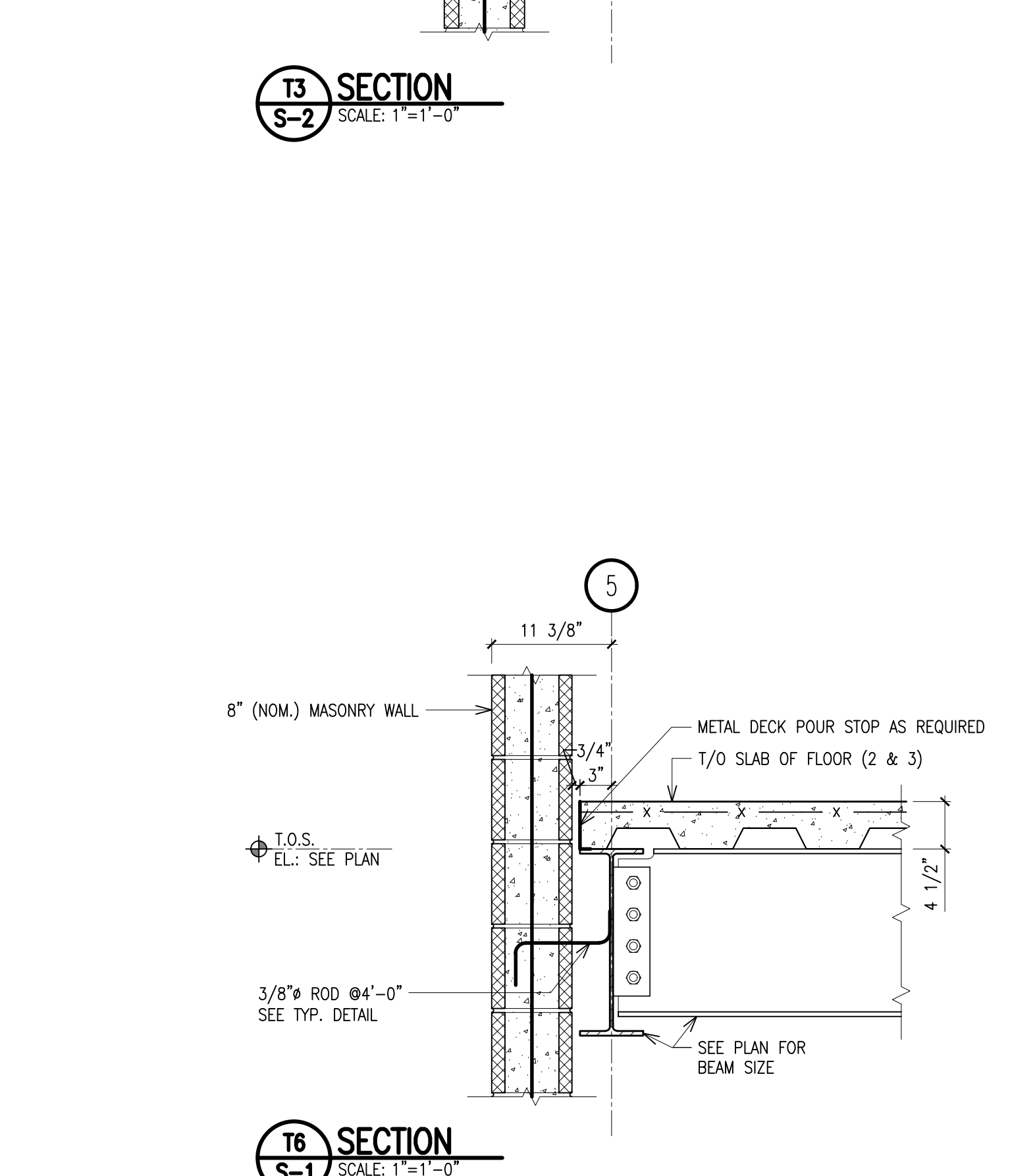
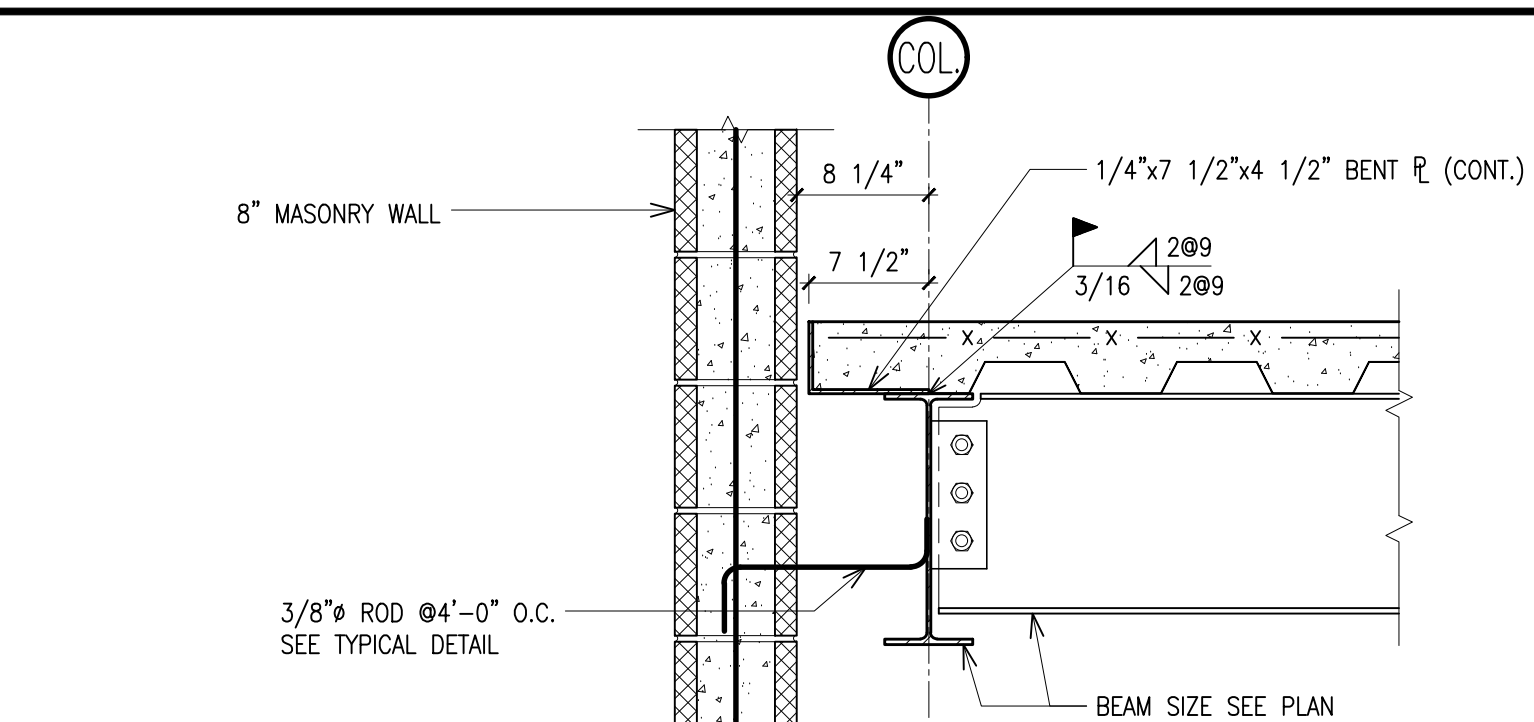
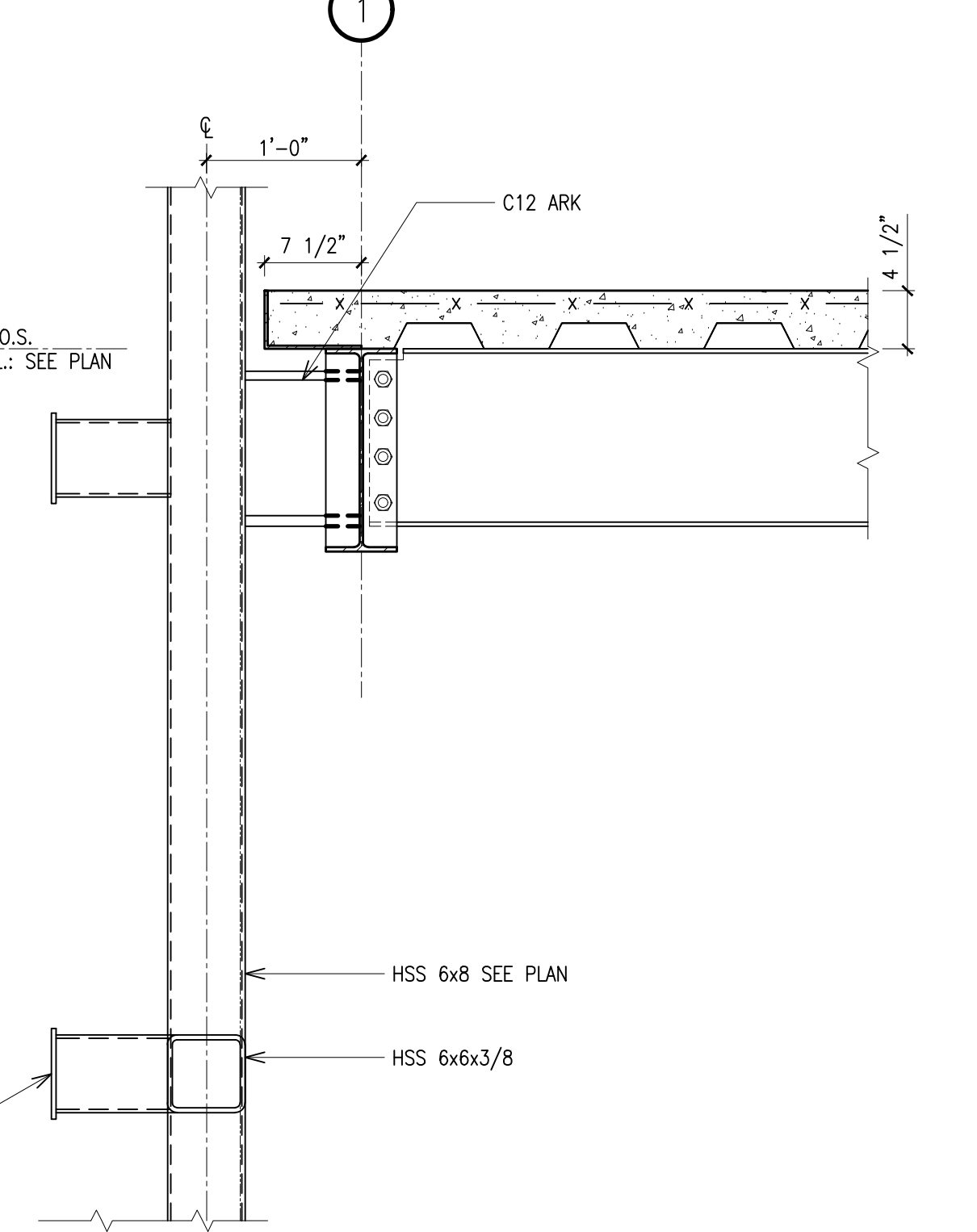
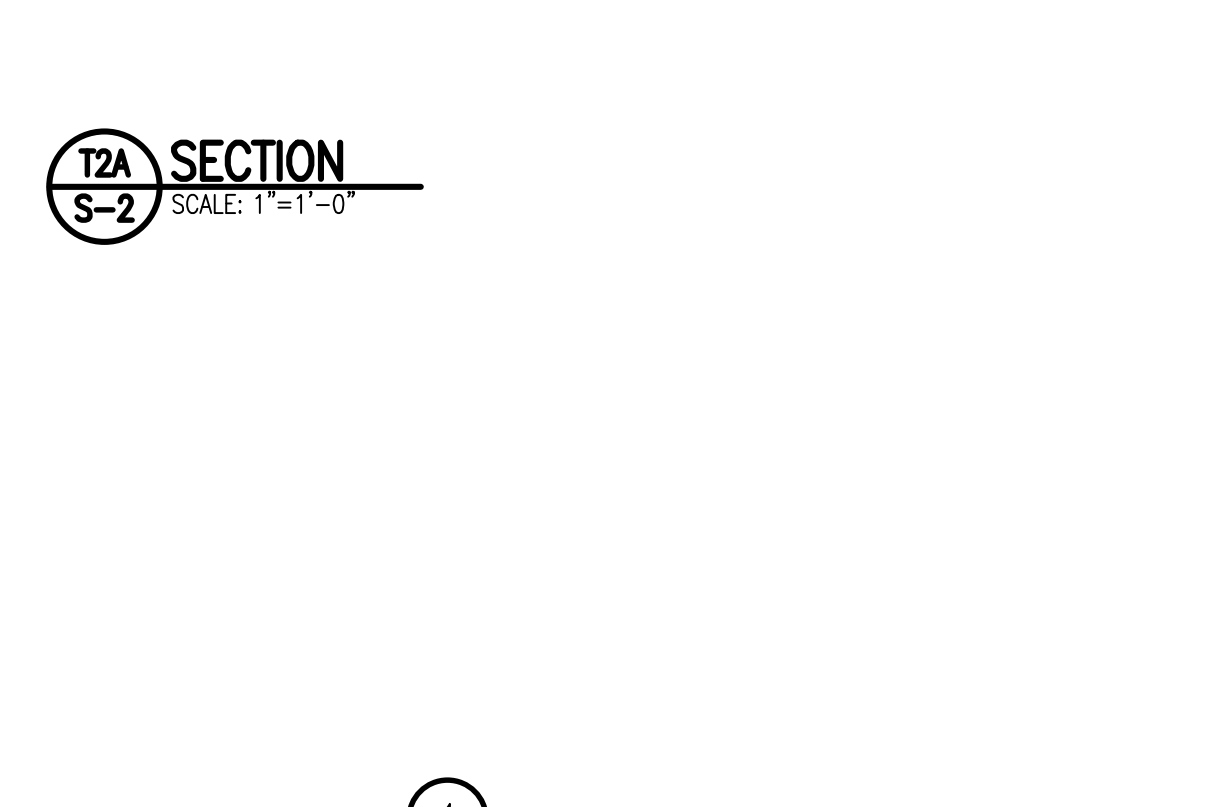
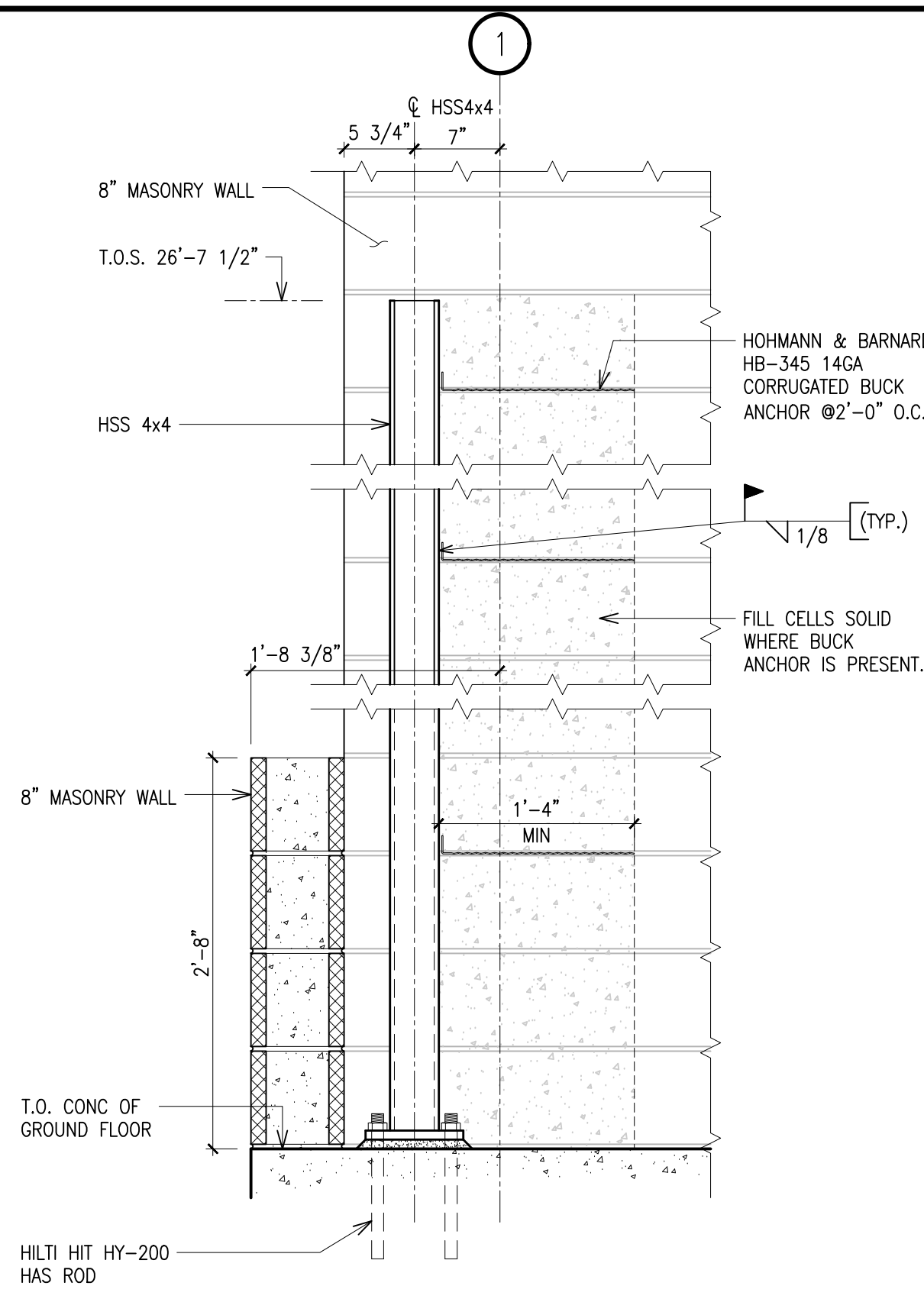
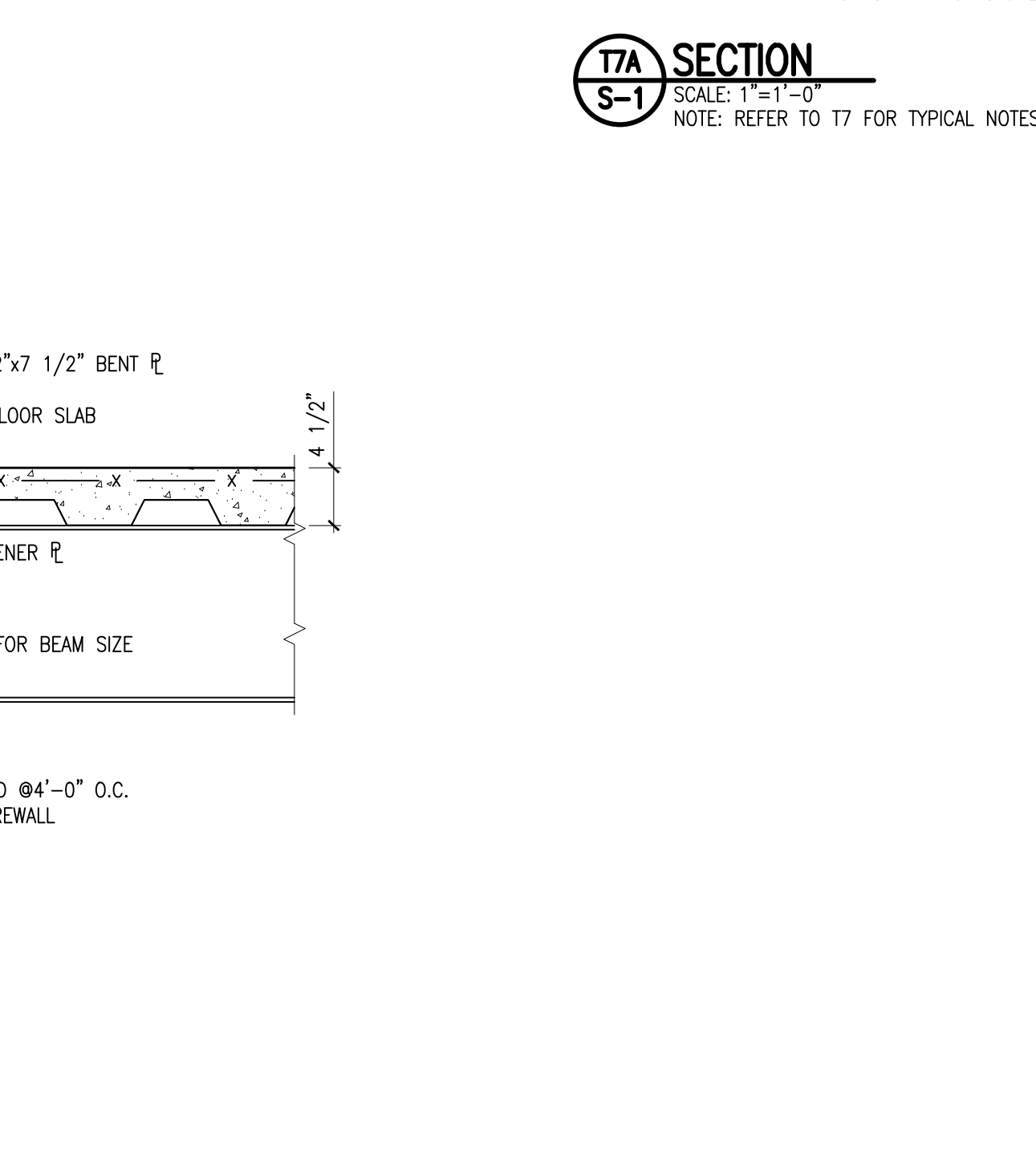
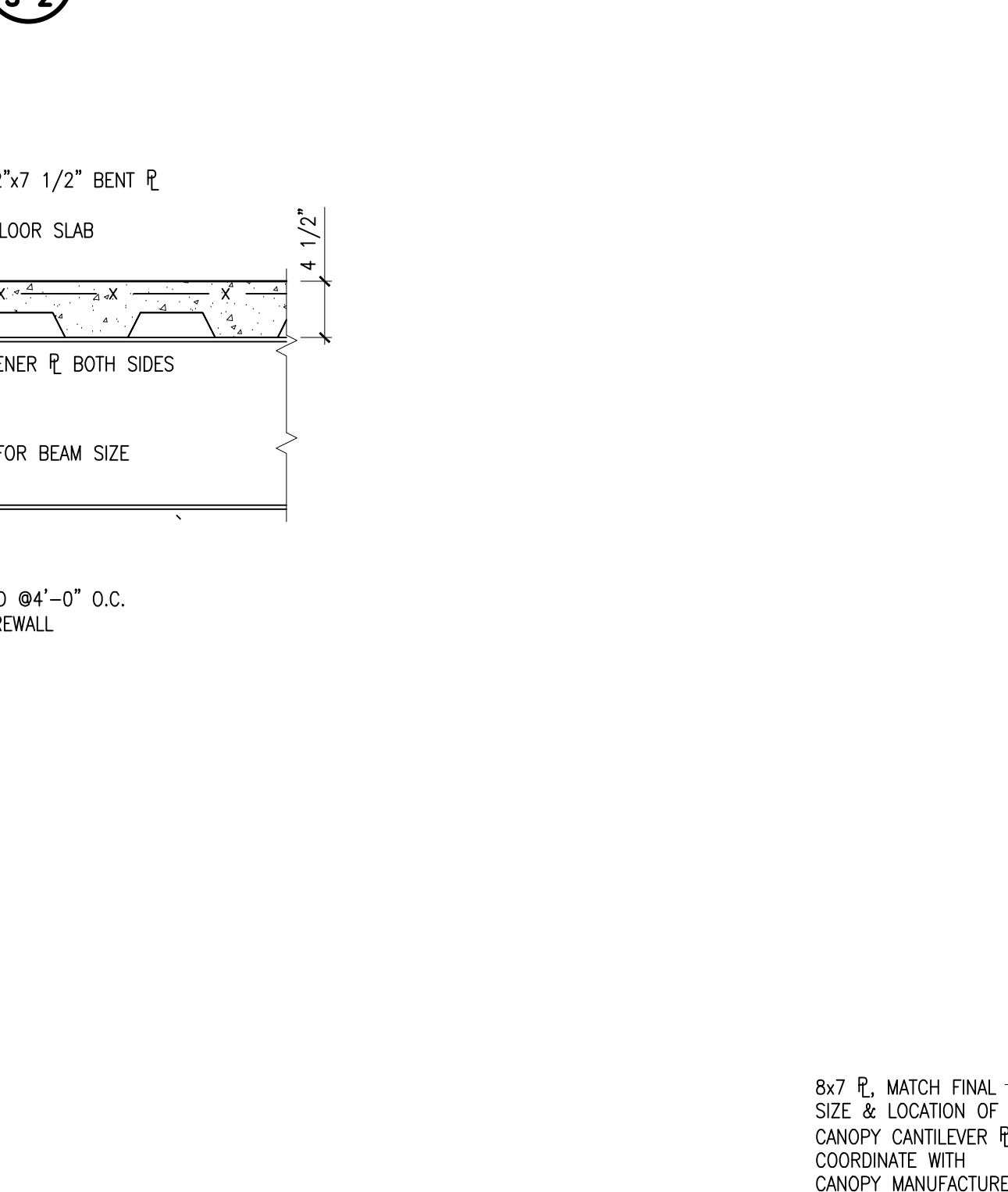
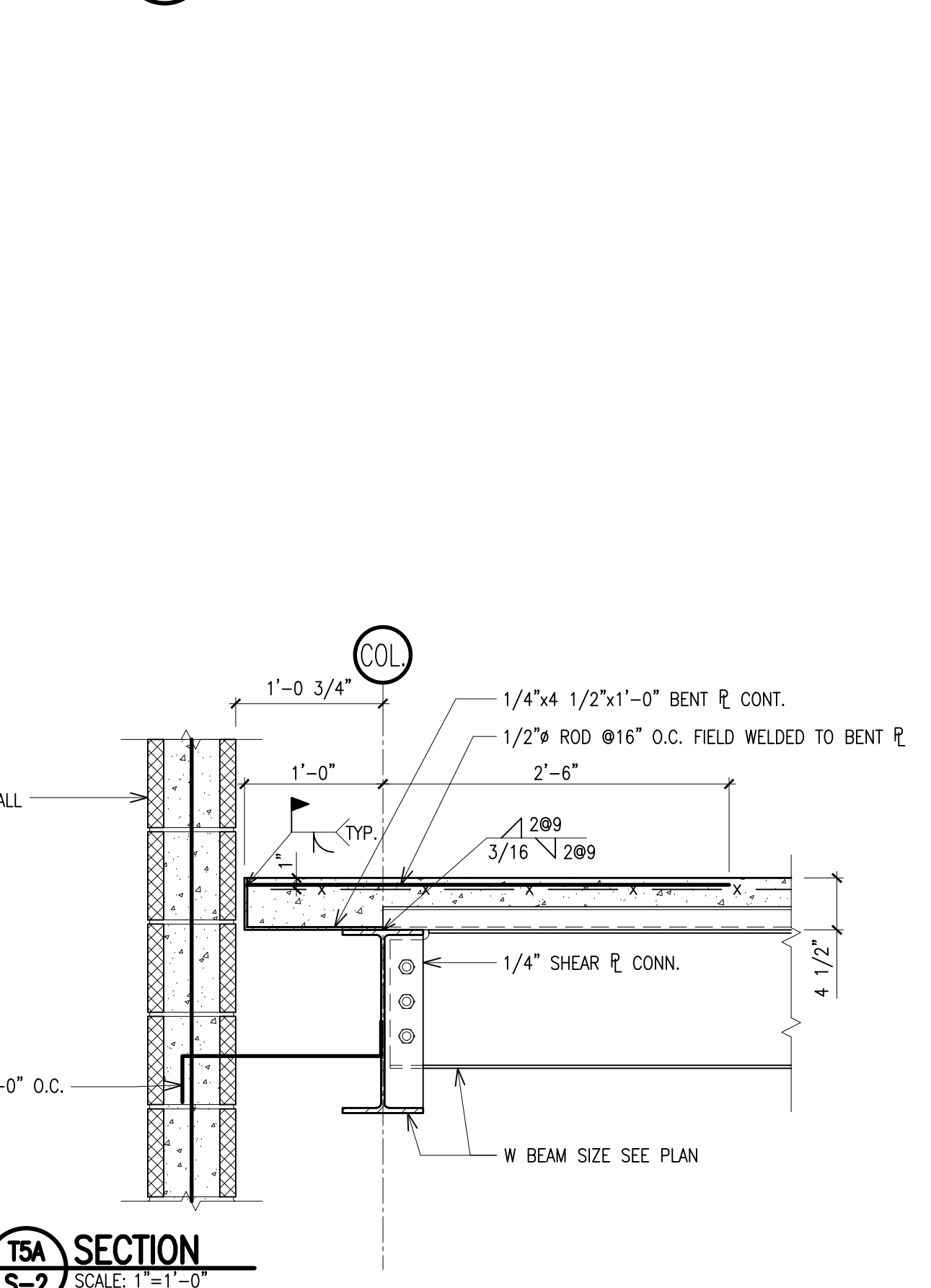
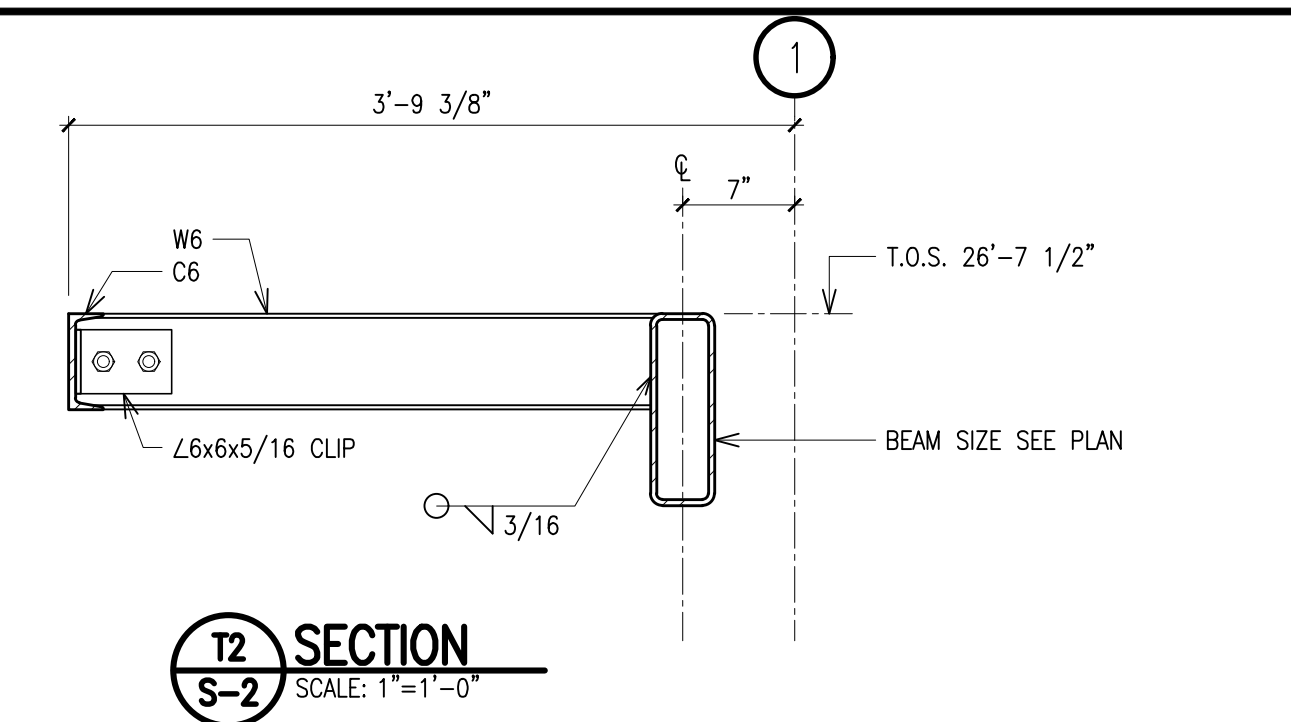
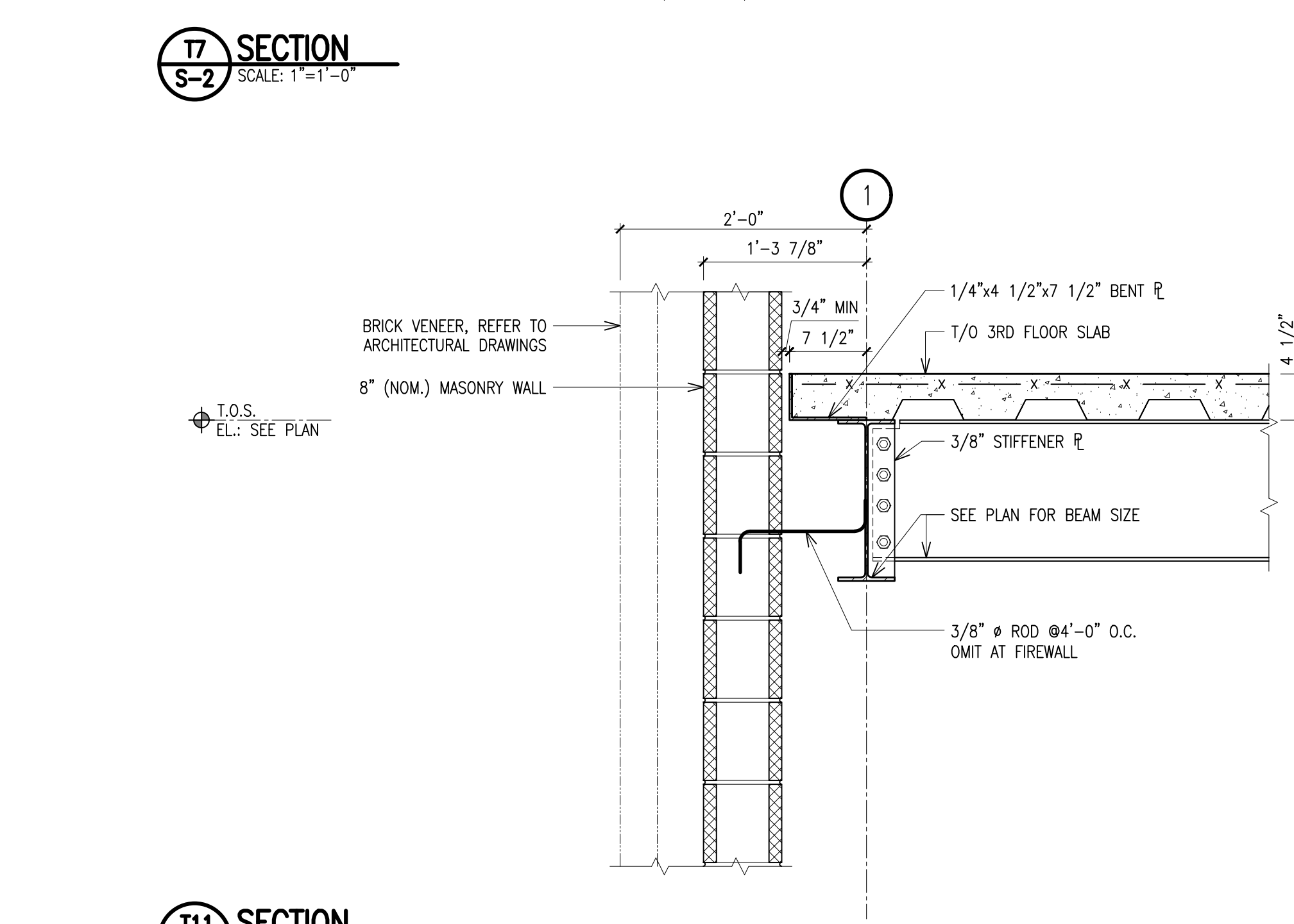
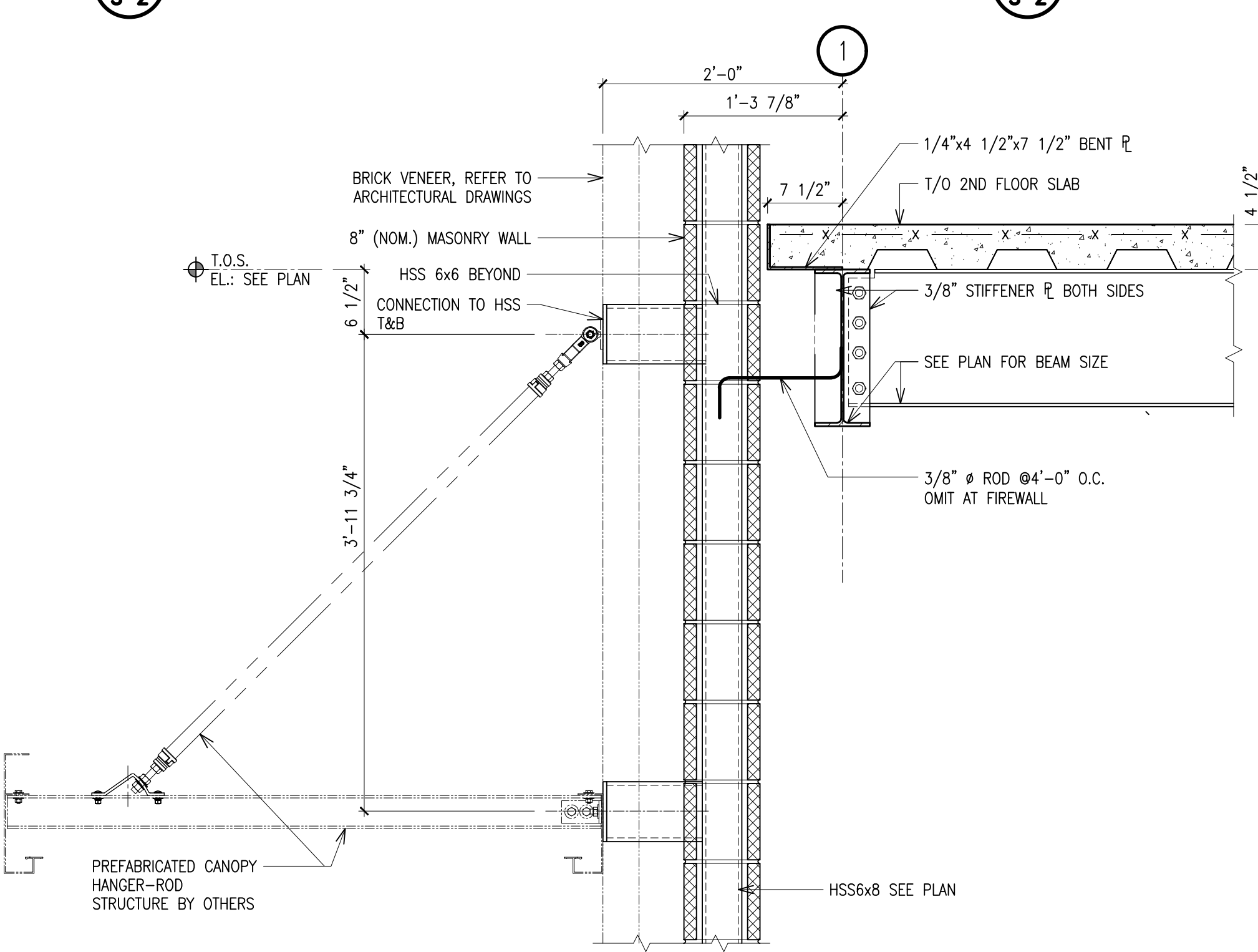
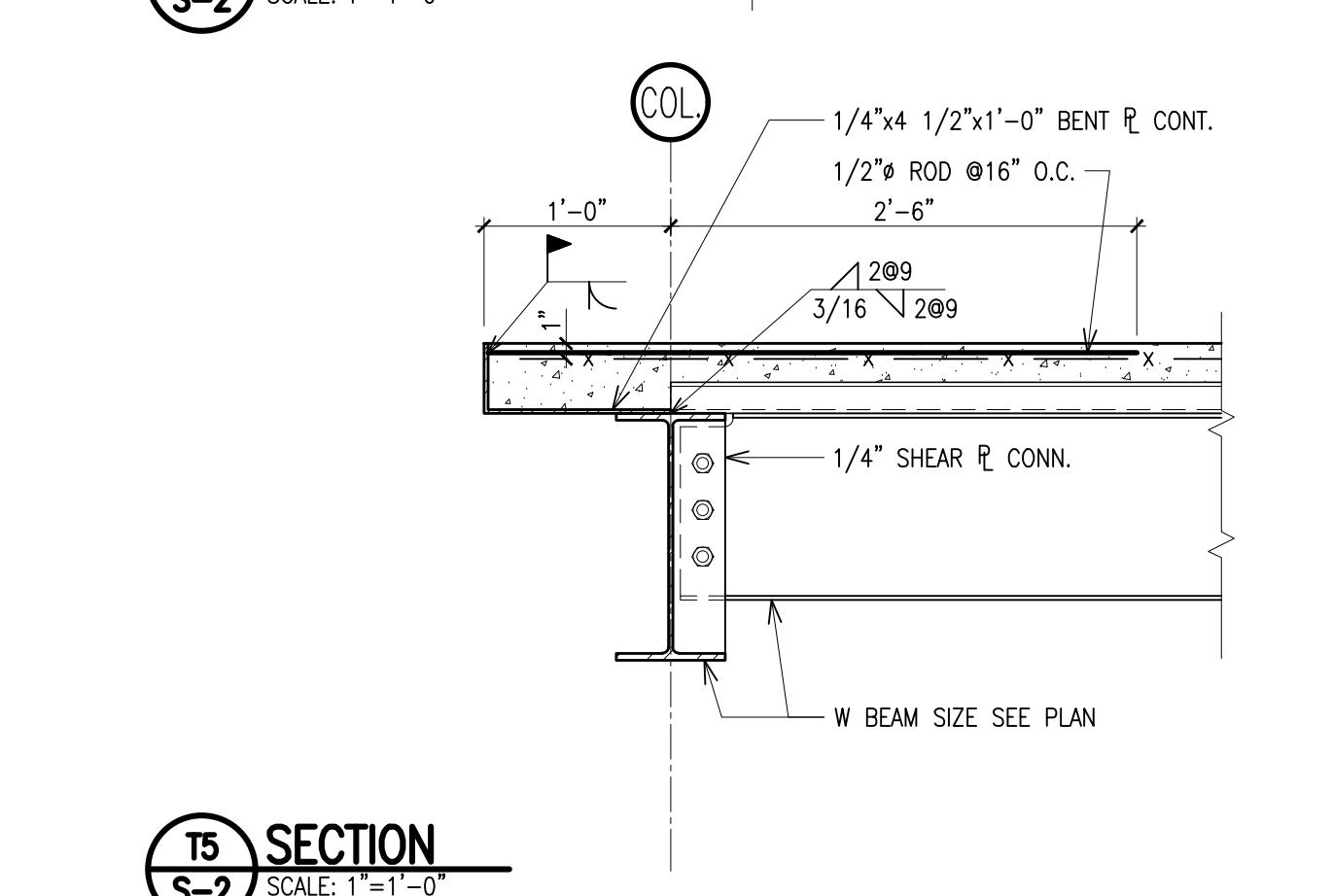
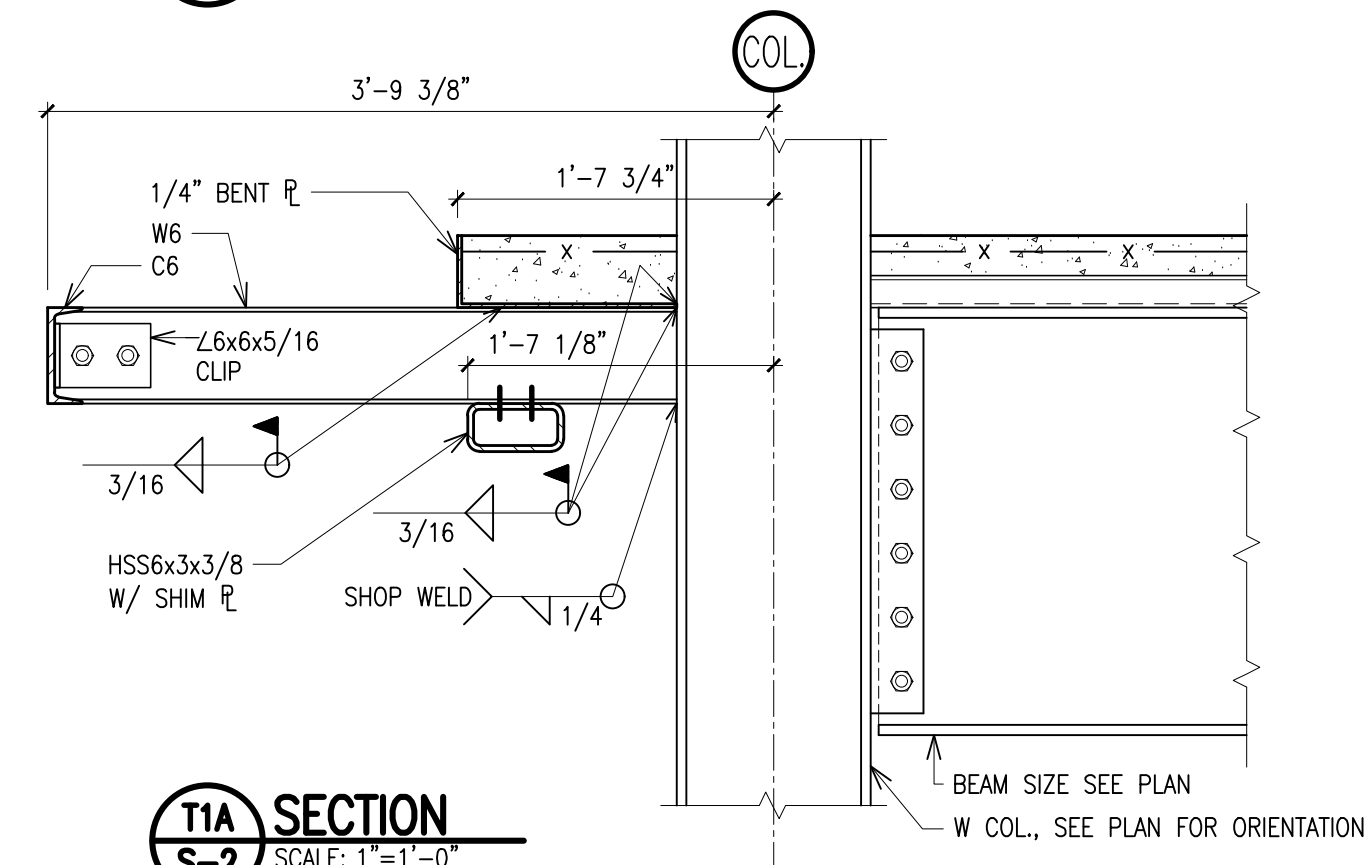
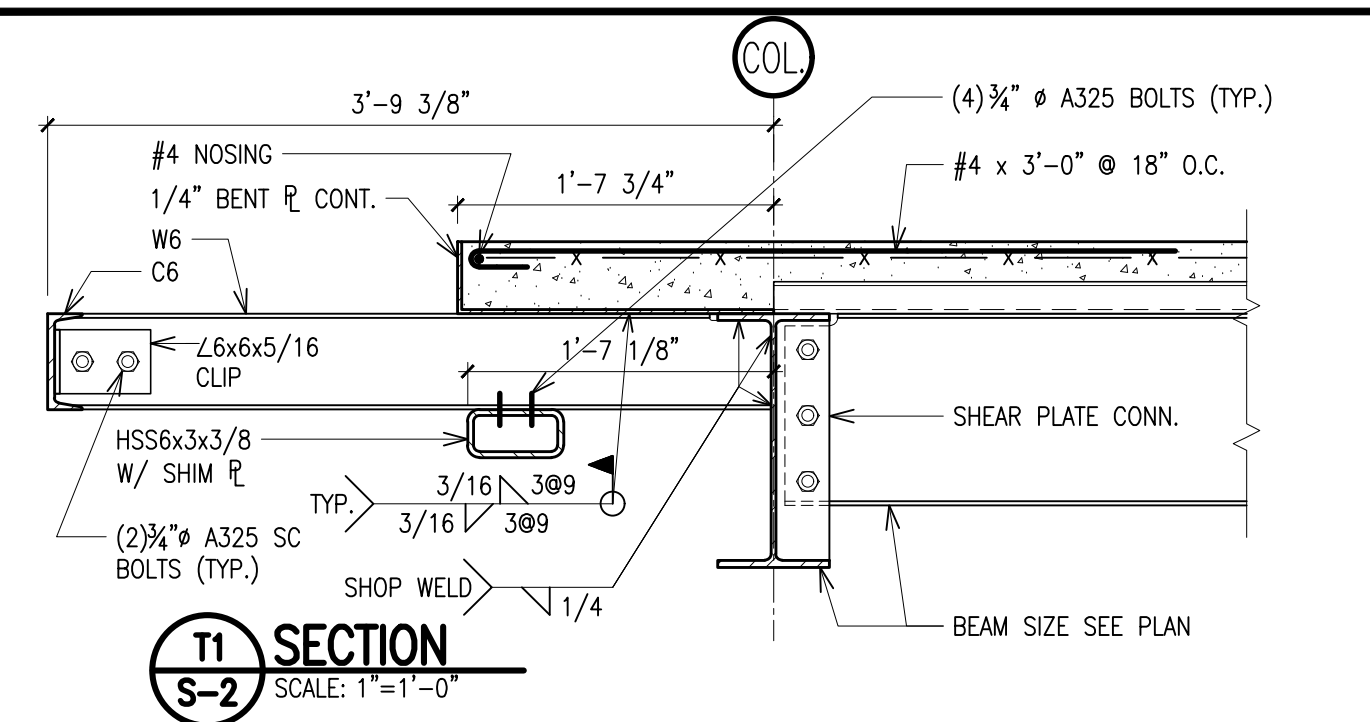


R9 SECTION S-3 SCALE: 1"=1'-0"

R10 SECTION S-3 SCALE: 1"=1'-0"

R11 SECTION S-3 SCALE: 1"=1'-0"

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ORANGE AND ROCKLAND UTILITIES, INC. PEARL RIVER NEW YORK			
SNS ARCHITECTS & ENGINEERS 1401 201.573.1767 FAX: 201.573.0808 www.sns-arch.com		ENGINEERING REVIEW DEPT. SIGNATURE DATE CIVIL METEERING RELAY CHECKED : S.N. PROJ. NO : 5593 DATE: NOVEMBER 25 2024	
DRAFTED : A.S. CHECKED : S.N. PROJ. NO : 5593 DATE: NOVEMBER 25 2024		APPROVED INFORMATION COMMENTS BID CONSTRUCTION	
DESIGN REVIEW DATE PEARL RIVER		SCALE AS NOTED	
FILE NO. 5593		DRAWING NUMBER S-4	
SHEET 51 OF 95		REV 1	
ROCKLAND COUNTY, NEW YORK John Lignos, AIA Lorin J. Sonenshine, AIA Steven Napolitano, PE, PP Robert Nocella, AIA		SNS Architects & Engineers, PC ONE PARAGON DRIVE MONTVALE NEW JERSEY 07040	



1		ISSUED FOR BUILDING DEPARTMENT FILING & BIDDING		04-28-25	
ORANGE AND ROCKLAND UTILITIES, INC. PEARL RIVER NEW YORK					
STEEL SECTIONS					
SVOC NEW OFFICE ADDITION & NEW PARKING					
TAX MAP ID 68.52-1-2 3RD WEST ROUTE 68 VILLAGE OF SPRING VALLEY TOWN OF RAMAPO ROCKLAND COUNTY, NEW YORK					
SWS ARCHITECTS & ENGINEERS 201.573.1767 FAX: 201.573.0808 www.sas-arch.com DRAFTED: A.S. CHECKED: S.N. PROJ. NO.: 5593 DATE: NOVEMBER 25, 2024		ENGINEERING REVIEW DEPT. CIVIL SIGNATURE DATE		APPROVED DATE INFORMATION COMMENTS BID CONSTRUCTION	
DESIGN REVIEW DATE		SCALE AS NOTED		FILE NO. 5593	
SHEET S-5		DRAWING NUMBER S-5		SHEET 52 OF 95	
PEARL RIVER		ROCKLAND COUNTY, NEW YORK		JOHN LIGNOS, AIA LORIN J. SONENSHINE, AIA STEVEN NAPOLITANO, P.E., P.P. ROBERT NOCELLA, AIA	

SNS ARCHITECTS & ENGINEERS tel: 201.573.1767 fax: 201.573.0808 www.sns-ae.com	ENGINEERING REVIEW			APPROVED		STEEL SECTIONS		
	DEPT.	SIGNATURE	DATE	DATE	SVOC NEW OFFICE ADDITION & NEW PARKING 			
	CIVIL			☐ INFORMATION ☐ COMMENTS ☐ BID ☐ CONSTRUCTION ☐ _____				
	METERING							
HETRAY								
	CHECKED : S.N.	TITLE:			TAX MAP 19 58.52-1-2 500 WEST ROUTE 59 VILLAGE OF SPRING VALLEY TOWN OF RAMAPO ROCKLAND COUNTY, NEW YORK			
	PROJ. NO.: 5593	SCALE						
	DATE: NOVEMBER, 25, 2024	FILE NO.	DRAWING NUMBER	SHEET				REV.
	OWN	DES	ORD	AS NOTED				
	DESIGN REVIEW		DATE					
	PEARL RIVER			ROCKLAND COUNTY, NEW YORK				
 SNS Architects & Engineers, PC ONE PARAGON DRIVE MONTAILE NEW JERSEY 07648 ■ John Lignos, AIA ■ Lorin J. Sonenshine, AIA ■ Steven Napolitano, PE, PP ■ Robert Nocella, AIA								
Steven Napolitano, PE, PE Cert./Lic. No. NJ 077320								