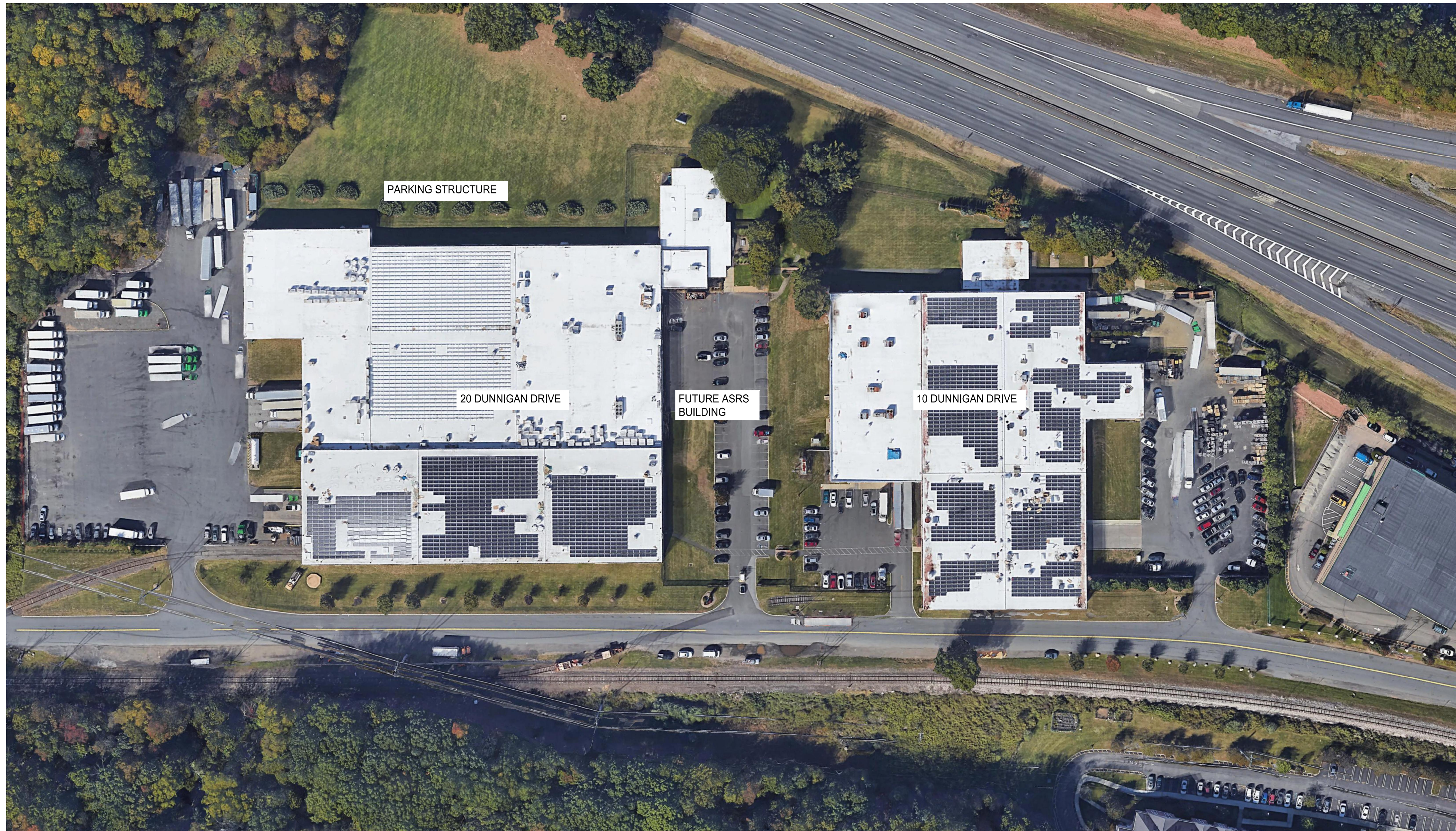


MANHATTAN BEER DISTRIBUTORS

20 DUNNIGAN DRIVE, SUFFERN, NY
STRUCTURAL REPAIRS

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SITE LOCATION MAP - OVERALL LAYOUT
(NOT TO SCALE)



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SCALE :	AS SHOWN

DRAWING TITLE :

COVER SHEET

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S-000

1 OF 60

GENERAL NOTES

- DESIGN AND CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE FOLLOWING CODES:
 - NEW YORK STATE BUILDING CODE.
 - AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318).
 - AMERICAN CONCRETE INSTITUTE SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI-301)
 - AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL AND CODE.
 - OCCUPATIONAL HEALTH SAFETY ADMINISTRATION (OSHA) CODE.
 - AMERICAN NATIONAL STANDARDS FOR CONSTRUCTION (ANSI).
 - USE LATEST EDITIONS INCLUDING ADDENDA, SUPPLEMENTS, AMENDMENTS, REVISIONS AND ERRATA U.N.O., EXCEPT AS MODIFIED BY THE CONTRACT DOCUMENTS.
 - THE AMERICAN WELDING SOCIETY CODE (A.W.S. D1.1).
 - STEEL DECK INSTITUTE (SDI)
 - STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS, LOAD AND WEIGHT TABLES (SJI)
 - AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
- PLANS AND APPLICATION FORMS MUST BE FILED AND APPROVED BY THE LOCAL BUILDING DEPARTMENT BEFORE STARTING ANY WORK OR ORDERING ANY MATERIALS.
- CONTRACTOR TO PROPERLY NOTIFY BUILDING DEPARTMENT PRIOR TO ANY WORK REQUIRING INSPECTION BY THE BUILDING DEPARTMENT AS CALLED FOR IN THE NEW YORK STATE BUILDING CODE.
- A TESTING AND INSPECTION AGENCY ACCEPTABLE TO THE STRUCTURAL ENGINEER AND RETAINED BY THE OWNER SHALL PERFORM ALL REQUIRED TESTING AND INSPECTIONS IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE REGULATIONS (SEE SPECIAL INSPECTION NOTES).
- THE CONTRACTOR IS RESPONSIBLE FOR ARRANGING WITH TESTING AGENCY WHEN INSPECTION SHALL BE PERFORMED. THE CONTRACTOR SHALL PROVIDE AMPLE TIME FOR THE TESTING AGENCY TO COMPLETE THEIR WORK.
- TESTING AND INSPECTION OF SOILS, CONCRETE, REBARS, STEEL, WELDING AND H.S. BOLTING, ETC., SHALL BE IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE. (SEE SPECIAL INSPECTION NOTES)
- CONTRACTOR SHALL PROVIDE, INSTALL, AND MAINTAIN ALL NECESSARY FALSEWORK, SHORING, BRACING AND ALL OTHER TEMPORARY CONSTRUCTION AND EQUIPMENT REQUIRED AND SHALL BE RESPONSIBLE FOR SITE SAFETY, STRUCTURAL STABILITY AND PROTECTION OF THE PUBLIC, STRUCTURES, AND PROPERTY INCLUDING PROTECTION FROM THE ELEMENTS DURING ANY PHASE OF THE WORK. SHORING DESIGN SHALL BE PERFORMED BY A NEW YORK STATE LICENSED P.E.
- THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELEVATOR DRAWINGS, AND SPECIFICATIONS. IN ADDITION, REFER TO THESE DRAWINGS FOR ADDITIONAL STRUCTURAL WORK NOT SHOWN ON STRUCTURAL DRAWINGS SUCH AS OPENINGS, INSERTS, SLEEVES, CURBS, ETC.
- AS PART OF THE CONTRACTOR'S SCOPE OF WORK, THE CONTRACTOR SHALL REPLACE, REPAIR AND PATCH TO ITS ORIGINAL CONDITION ANY AREAS ALTERED OR DAMAGED DURING PROCESS OF WORK AT NO ADDITIONAL COST TO THE OWNER AND WITH ALL NECESSARY GUARANTEES AND WARRANTIES.
- THE CONTRACTOR SHALL TAKE FIELD MEASUREMENTS, ELEVATIONS, OPENINGS AND VERIFY FIELD CONDITIONS AND SHALL CAREFULLY COMPARE SUCH FIELD MEASUREMENTS AND CONDITIONS AND OTHER INFORMATION KNOWN TO THE CONTRACTOR, WITH THE CONTRACT DOCUMENTS, BEFORE COMMENCING WORK AND PRIOR TO PREPARING SHOP DRAWINGS AND ORDERING MATERIALS AND DETAILS. ERRORS, DISCREPANCIES, INCONSISTENCIES, OR OMISSIONS DISCOVERED WHICH MAY INTERFERE WITH THE PROPER EXECUTION OF THE WORK, SHALL BE REPORTED TO THE ENGINEER IMMEDIATELY. DEVIATIONS FROM THESE PLANS MAY OCCUR DUE TO FIELD CONDITIONS.
- THE CONTRACTOR SHALL FILE AND OBTAIN AND PAY FOR ALL REQUIRED PERMITS, FEES AND LICENSES BEFORE STARTING ANY WORK OR ORDERING ANY MATERIAL. DO NOT BEGIN CONSTRUCTION UNLESS AND UNTIL A WRITTEN PERMIT HAS BEEN ISSUED IN ACCORDANCE WITH THE REQUIREMENTS OF THE BUILDING CODE. SUBMIT ORIGINAL COPIES TO OWNER BEFORE PROCEEDING.
- THESE DRAWINGS REPRESENT STRUCTURAL COMPONENTS IN THEIR FINAL AND FINISHED STATE. CONSTRUCTION PROCEDURE, METHODS, SAFETY PRECAUTIONS OR MECHANICAL REQUIREMENTS USED TO ERRECT THEM ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR OR SUB-CONTRACTOR DOING THE WORK, AND SHOULD FOLLOW CHAPTER 33 OF THE NEW YORK STATE BUILDING CODE.
- THE CONTRACTOR IS CAUTIONED TO MAKE CONTINUOUS OBSERVATIONS AND SAFEGUARDS OF THE EXISTING STRUCTURES DURING THE PERFORMANCE OF HIS WORK. SHOULD HE BECOME AWARE OF ANY EXISTING UNFORESEEN STRUCTURAL CONDITIONS OR SITUATIONS THAT REQUIRE FURTHER INVESTIGATION OR STUDY, (SUCH AS EXCESSIVE DETERIORATION, CRACKS IN MASONRY OR SLABS, MOVEMENT, ADDITIONAL DEFLECTION, ETC.), HE SHALL IMMEDIATELY NOTIFY THE ENGINEER/ARCHITECT OF RECORD.
- SHOP DRAWINGS AND SAMPLE PRODUCTS ARE REQUIRED FOR ALL WORK AND SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY ARE STAMPED 'APPROVED' AND INITIALED BY THE CONTRACTOR ATTESTING THAT IT CONFORMS TO THE CONTRACT DOCUMENTS. SHOP DRAWINGS SHALL BE PREPARED, SIGNED AND SEALED BY A NEW YORK STATE LICENSED P.E. RETAINED BY THE CONTRACTOR.
- THE CONTRACTOR SHALL RETAIN HIS OWN CONSULTANTS, INCLUDING STRUCTURAL CONSULTANTS, REGARDING CONTRACTORS DEMOLITION AND CONSTRUCTION PROCEDURES, SHORING AND BRACING PROTECTION, STRUCTURAL STABILITY REVIEWS DURING CONSTRUCTION AGAINST DAMAGE COLLAPSE, DISTORTION AND OFF-ALIGNMENT, ETC., AND ON SITE SAFETY RESPONSIBILITIES AND BUILDING DEPARTMENT REPORTING AND FILING REQUIREMENTS.
- THE CONTRACTOR SHALL SAFEGUARD ALL ELECTRICAL AND MECHANICAL CONDUITS AND UTILITIES AND COORDINATE ALL EXISTING LINES, PIPES, DUCTS AND OTHER UTILITIES WHICH ARE TO BE RELOCATED AND REINSTALLED AS A RESULT OF THE NEW WORK.
- EXISTING FOUNDATIONS, WALLS, COLUMNS, SLABS, BEAMS, FLOORS, (CONCRETE, STEEL, TIMBER, ETC.), ARE ASSUMED TO BE IN GOOD CONDITION, THIS MUST BE VERIFIED IN THE FIELD BY THE CONTRACTOR. CONTRACTOR SHALL NOTIFY ENGINEER-OF-RECORD OF ANY CONDITIONS REQUIRING REPAIRS PRIOR TO CONSTRUCTION.
- THE ENGINEER OF RECORD SHALL NOT HAVE CONTROL OR CHARGE OF AND SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURE, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK FOR THE ACTS OR OMISSIONS OF THE CONTRACTORS, SUBCONTRACTOR, OR ANY OTHER PERSON PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

STRUCTURAL CONCRETE NOTES

- MATERIALS SHALL CONFORM WITH THE FOLLOWING STANDARDS:
 - PORTLAND CEMENT AS PER ASTM C-150, TYPE I / II.
 - CONCRETE AGGREGATES AS PER ASTM C-33 AND C-330.
 - WATER SHALL BE CLEAN AND FREE FROM INJURIOUS OILS, ACIDS, ALKALIS, SALT, ORGANIC MATERIALS AND DELETERIOUS MATERIALS.
 - REBARS SHALL BE DEFORMED BARS CONFORMING TO ASTM A-615, GRADE 60. TIES MAY BE A-615, GRADE 40. FOR BARS REQUIRING WELDING CONFORM TO ASTM A-706.
 - WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM 185 WITH ULTIMATE TENSILE STRENGTH OF 70 K.S.I. WIRE FABRIC SHALL CONFORM TO ASTM 475.
- CONCRETE DESIGN STRENGTH: CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES FOR THE VARIOUS COMPONENTS, UNLESS NOTED OTHERWISE

	SPECIFIED MINIMUM			
	28-DAY COMPRESSIVE STRENGT (f _c) (PSI)	ENTRAINED AIR CONTENT	DENSITY (PCF)	MAX. W/C RATIO
GRADE BEAMS, FOUNDATION FOOTINGS & WALLS	5000	6%	145	0.45

THE REQUIRED AVERAGE CONCRETE MIX DESIGN (F'CR) USED AS A BASIS OF SELECTION OF CONCRETE PROPORTIONS SHALL BE DETERMINED FROM A CURVE OF CONCRETE STRENGTH VERSUS WATER-CEMENT RATIO WHERE THE AVERAGE CONCRETE MIX DESIGN SHALL COMPLY WITH ACI 318.
- CONCRETE SHALL BE PRODUCED FROM APPROVED BATCH PLANTS, BASED ON PRELIMINARY TEST DESIGN AND RESULTING INTO MIX STRENGTH SPECIFIED. PRODUCER SHALL CERTIFY CONFORMANCE OF QUALITY AND CONDITION OF MATERIALS TO ACI-318 AND THAT INGREDIENTS ARE THE SAME OR EQUAL TO THOSE USED FOR THE PRELIMINARY TESTS. ATTESTATION OF QUALITY INSPECTION AT THIS BATCH PLANT SHALL APPEAR ON THE TICKET ACCOMPANYING

EACH LOAD OF CONCRETE.

- CONTRACTOR SHALL SUBMIT PRELIMINARY TESTS AND DESIGN MIX REPORTS IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE FOR REVIEW BY THE STRUCTURAL ENGINEER BEFORE COMMENCING WORK. NO CONCRETE SHALL BE PLACED UNTIL THE DESIGN MIXES WITH ACCOMPANYING COMPRESSIVE TEST RESULTS HAVE BEEN APPROVED BY THE ENGINEER OF RECORD.
- COMPRESSION TEST SAMPLES SHALL BE TAKEN FROM THE MIXER IN ACCORDANCE WITH ASTM C172, CURED IN ACCORDANCE WITH ASTM C-31, AND TESTED AT 28-DAYS IN ACCORDANCE WITH ASTM C-39.
- THE APPROVED INSPECTION AGENCY WILL CONDUCT ALL NECESSARY TESTS AT THE LABORATORY, AND WILL HAVE ONE QUALIFIED CONCRETE TECHNICIAN TO BE STATIONED AT THE MIXING PLANT AND A MINIMUM OF ONE QUALIFIED CONCRETE TECHNICIAN AT THE JOB SITE. THE TECHNICIAN AT THE JOB SITE WILL PERFORM THE TESTS AND PREPARE THE SPECIMENS AS REQUIRED UNDER THE CODE.

THE TESTING AGENCY SHALL PROVIDE ACI CERTIFIED TECHNICIANS.
- ALL CONCRETE USED IN THE STRUCTURE SHALL CONFORM IN ALL RESPECTS TO THE MATERIAL AND PROPORTIONS OF THESE MATERIALS IDENTIFIED IN THE APPROVED DESIGN MIX. THE USE OF ANY ADDITIVES NOT PRESENT IN THE APPROVED DESIGN MIX IS PROHIBITED.
- WHEN CHANGES TO BRAND, TYPE, SIZE OR SOURCE OF CEMENTITIOUS MATERIALS, AGGREGATES, WATER, ICE OR ADMIXTURES ARE PROPOSED, SUBMIT NEW FIELD DATA, DATE FROM NEW TRIAL MIXTURES, OR OTHER EVIDENCE THAT THE CHANGE WILL NOT ADVERSELY AFFECT THE RELEVANT PROPERTIES OF THE CONCRETE. DATA SHALL BE SUBMITTED FOR ACCEPTANCE BEFORE CHANGES ARE MADE.
- CONCRETE TEST CYLINDERS AND SLUMP. INSPECTION OF REINFORCING BARS AND PLACING OF CONCRETE TO BE TAKEN AND TESTED IN ACCORDANCE WITH ACI-318 AND AS REQUIRED BY THE NEW YORK STATE BUILDING CODE. (SEE SPECIAL INSPECTION NOTE)
- CONTRACTOR TO PROVIDE A LOCATION WHICH IS LEVEL, SAFE AND FOR THE STORING OF CURING BOXES IN ORDER FOR THE TESTING LABORATORY TO PREPARE TEST CYLINDERS.
- THE TEST CYLINDERS IMMEDIATELY AFTER MOLDING AND FINISHING, SHALL BE STORED IN CURING BOXES (SUPPLIED BY THE TESTING AGENCY) ON-SITE FROM 24-HOURS TO 48-HOURS IN A TEMPERATURE RANGE FROM 60F TO 80F IN AN ENVIRONMENT PREVENTING MOISTURE LOSS FROM THE SPECIMENS BEFORE BEING TRANSPORTED TO THE TESTING LABORATORY.
- NO CHLORIDES SHALL BE INTRODUCED INTO THE CONCRETE OR GROUT, ETC., UNDER ANY CIRCUMSTANCES.
- ALL CONTINUOUS REINFORCING BARS SHALL BE EMBEDDED AT ENDS OF MEMBERS AND LAPPED AT SPLICES AND CORNERS AS PER THE CRSI LATEST EDITION. SPLICES FOR BOTTOM STEEL TO BE LOCATED AT SUPPORTS; SPLICES FOR TOP STEEL TO BE LOCATED AT MID-SPAN. NO SPLICES ARE PERMITTED IN TENSION ZONE, CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT. ALL DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS SHALL COMPLY WITH THE REQUIREMENTS OF ACI 315 AND ACI 318-02, CHAPTERS 7 AND 12, AND THE NEW YORK STATE BUILDING CODE.
 - ALL SPLICES FOR SLABS AND BEAMS SHALL BE TENSION SPLICES, U.O.N.
 - COMPRESSION SPLICES SHALL BE USED IN PIERS AND COLUMNS, U.O.N.
 - EMBEDMENT FOR DOWELS, ETC. SHALL BE TENSION DEVELOPED, U.O.N.
- MECHANICAL SPLICES MAY BE USED. WHERE THESE SPLICES ARE USED, SPLICES SHOULD BE STAGGERED AND SHALL DEVELOP 133 PERCENT OF THE SPECIFIED YIELD STRENGTH IN TENSION OR COMPRESSION.
- MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE

UNFORMED SURFACE IN CONTACT WITH THE GROUND	3 IN.
FORMED SURFACES EXPOSED TO EARTH	
#6 BARS AND LARGER	2 IN.
#5 BARS AND SMALLER	1-1/2 IN.
FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER	
BEAMS, GIRDERS, AND COLUMNS	1-1/2 IN.
SLABS, WALLS, AND JOISTS:	
#11 BARS AND SMALLER	3/4 IN.
#14 BARS #18 BARS	1-1/2 IN.

SURVEY REBAR ELEVATIONS BEFORE PLACING CONCRETE.

- ALL BAR SIZES DESIGNATED ARE BASED ON NUMBER OF EIGHTHS OF AN INCH NOMINAL DIAMETER, NOT METRIC UNITS.
- THE CONTRACTOR SHALL FURNISH AND INSTALL ALL THE NECESSARY CHAIRS, REBARS, TIES, SPACERS, ETC., TO SECURE AND SUPPORT THE REINFORCING. PROPERLY INSTALLED SPACERS SHALL BE UTILIZED TO PROPERLY MAINTAIN APPROPRIATE CONCRETE COVER WHILE PLACING CONCRETE. LIFTING OF BARS OR MESH DURING PLACEMENT OF CONCRETE IS NOT PERMITTED.

NOTE: PROVIDE PLASTIC TIPPED BOLSTERS AND CHAIRS AT THOSE LOCATIONS WHERE THE CONCRETE SURFACE IN CONTACT WITH THE BOLSTERS OR CHAIRS ARE EXPOSED.
- THE CONTRACTOR SHALL SUBMIT CHECKED SHOP DRAWINGS CONSISTING OF COMPLETE PLANS AND DETAILS OF REINFORCEMENT, LOCATIONS OF POUR LINES, CONSTRUCTION JOINTS, ETC., FOR APPROVAL BEFORE PROCEEDING WITH THE WORK. SUCH DRAWINGS SHALL BE OF SUFFICIENT DETAIL TO PERMIT PLACING OF REINFORCEMENT WITHOUT REFERENCE TO THE STRUCTURAL DRAWINGS.
- NO COLD WEATHER CONSTRUCTION OR HOT WEATHER CONSTRUCTION IS PERMITTED WITHOUT WRITTEN APPROVAL OF THE ARCHITECT/ENGINEER. SUBMIT PROCEDURES FOR REVIEW.
- LOCATION OF ALL CONSTRUCTION AND CONTRACTION JOINTS, NOT SHOWN IN DRAWINGS, SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL PRIOR TO DETAILING OF REINFORCEMENT. ALL CONSTRUCTION JOINTS TO BE CLEARLY INDICATED ON REBAR DETAIL DRAWING. MAXIMUM CONTRACTION JOINT SPACING = 40" O.C.
- ALL STRUCTURAL MEMBERS SHALL BE POURED FOR THEIR FULL DEPTHS IN ONE OPERATION. CONSTRUCTION JOINTS SUCH AS DAY'S END POUR SHALL BE LOCATED IN THE MIDDLE THIRD OF THE SPAN. MAIN REINFORCING TO RUN THROUGH THE JOINT. KEY AND ROUGHEN JOINTS TO EXPOSE AGGREGATE FOR CHEMICAL BOND. NO HORIZONTAL CONSTRUCTION JOINTS PERMITTED IN BEAMS OR WALLS WITHOUT THE EXPRESS CONSENT OF ENGINEER-OF-RECORD. NO CONSTRUCTION JOINTS ARE PERMITTED IN BEAMS CARRYING COLUMN LOADS OR OTHER CONCENTRATED LOADS. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS. ALL REINFORCING IS TO BE CONTINUOUS THROUGH JOINT.

ROUGHENED SURFACE AT INTERFACE OF SEPARATE CONCRETE POURS (JOINTS) SHALL BE PREPARED AS FOLLOWS:

 - ROUGHEN SURFACE TO A FULL AMPLITUDE OF APPROXIMATELY 1/4" WITH STIFF BROOM AFTER INITIAL SET.
 - BEFORE PLACING FRESH CONCRETE, CLEAN SURFACE AND REMOVE LAITANCE WITH WIRE BRUSH.
 - IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, WET SURFACE AND REMOVE STANDING WATER.
- ALL OPENINGS IN WALLS, UNLESS OTHERWISE NOTED, SHALL HAVE TWO #6 BARS ON SIDES AND SHALL EXTEND 2'-0" BEYOND EDGES OF OPENINGS, U.O.N.
- ANCHOR BOLTS TO BE ASTM F1554 (WELDABLE) FY=55KSI UNLESS OTHERWISE NOTED.
- NO PIPES OR CONDUITS EXCEEDING 1/3 SLAB THICKNESS IN OUTSIDE DIAMETER SHALL BE EMBEDDED IN THE STRUCTURAL CONCRETE FLOOR OR ROOF SLAB. WHERE CONDUITS OR PIPES ARE PERMITTED, THEY SHALL BE PLACED NOT CLOSER THAN 3 DIAMETER O.C. AND SHALL BE LOCATED SO AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE. ALUMINUM CONDUITS ARE NOT PERMITTED.
- PROVIDE 4-#6 DOWELS X 3'-0" LONG BETWEEN ALL PIERS, FOOTINGS, GRADE BEAMS, ETC., U.N.
- CONCRETE DIMENSIONAL TOLERANCE SHALL CONFORM TO ACI 117.
- ALL SLAB-ON-GROUND SHALL HAVE THICKENING, DEPRESSIONS, OPENINGS, ETC., AS REQUIRED OR AS SHOWN HEREIN, OR ON ARCHITECTURAL, ELECTRICAL OR MECHANICAL DRAWINGS.
- THE METHOD FOR CONVEYING CONCRETE TO THE PLACE OF DEPOSIT SHALL COMPLY WITH THE NEW YORK STATE BUILDING CODE. CONCRETE MAY BE CONVEYED BY PUMPING. PUMPING METHODS SHALL COMPLY WITH DEPARTMENT OF BUILDING REGULATIONS. ALUMINUM OR ALUMINUM ALLOY PIPE SHALL NOT BE USED.
- THE MEANS AND METHODS OF CONCRETE PLACEMENT, INCLUDING POUR HEIGHTS, TEMPERATURE CONTROL, LATERAL PRESSURE AND UPLIFT PRESSURE CONTROL, FORMWORK DESIGN, AND OTHER REQUIRED CONSTRUCTION METHODS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR DOING THE WORK. THE CONTRACTOR SHALL EMPLOY ALL NECESSARY PRECAUTIONS IN ORDER TO AVOID ANY MOVEMENT OR

DISLOCATION OF ALL M/E SYSTEMS, PIPING PENETRATIONS, ETC., INCLUDING THE CONCRETE PLACEMENT. ALL M/E EQUIPMENT SHALL BE INSTALLED AS INDICATED ON THE M/E DRAWINGS.

- SEE ARCHITECTURAL DRAWINGS FOR FINISH OF ALL EXPOSED CONCRETE.

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APPROVED BY : P. BRUNO

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SCALE : AS SHOWN

DRAWING TITLE :

NOTES

DWG NUMBER :

S-001

2 OF 60

LOADING SCHEDULE

SUPERIMPOSED DEAD LOADING: SELF WEIGHT OF CONCRETE SLAB AND METAL DECK IS NOT INCLUDED IN THE FOLLOWING VALUES.

SNOW LOADING:

- GROUND SNOW LOAD PG= 30 PSF
- SNOW EXPOSURE FACTOR CE= .9
- IMPORTANCE FACTOR IS= 1
- THERMAL FACTOR CT= 1
- FLAT ROOF SNOW LOAD PF= 20 PSF
- SLOPE ROOF FACTOR CS= 1.0
- SLOPE ROOF SNOW LOAD PS= 20 PSF
- DRIFT
 - SNOW DENSITY = 17.9 PCF
 - DRIFT HEIGHT HD = 3.52 FT
 - MAXIMUM DRIFT LOAD = 83 PSF
 - DRIFT LOAD AT END = 20 PSF
 - DRIFT WIDTH W= 14 FT
 - FROST DEPTH = 4'-0" BELOW GRADE

WIND LOADING:

- BASIC WIND VELOCITY V= 120 MPH
- EXPOSURE FACTOR B
- IMPORTANCE FACTOR IW= 1
- ASCE 7-10 SIMPLIFIED PROCEDURE WIND LOADS
 - BUILDING OCCUPANCY II
 - a=4.0
 - TERRAIN FACTOR KZT= 1.0
 - ROOF MEAN HEIGHT HR= 50 FT

- WIND FORCE MWFRS HORIZONTAL PRESSURE.
 - ZONE = 28 PSF

- WIND FORCE MWFRS VERTICAL PRESSURE.
 - ZONE = -18 PSF

- TOTAL BASE SHEAR PARKING BUILDING.
 - N-S = 207 KIPS
 - E-W = 85 KIPS

- TOTAL BASE SHEAR ASRS BUILDING.
 - N-S = 215 KIPS
 - E-W = 260 KIPS

COMPONENTS AND CLADDING

- DISTANCE a= 21.0 FT
- EFFECTIVE WIND AREA OF C&C = 50 SF.
- WIND FORCE C&C HORIZONTAL PRESSURE.
 - WALL INTERIOR=31 PSF
 - WALL CORNERS=37 PSF

- WIND FORCE C&C VERTICAL PRESSURE.
 - ROOF EDGES = -43 PSF
 - ROOF CORNERS = -56 PSF

SEISMIC LOADING PARKING BUILDING:

- SEISMIC LOADING CALCULATED AS PER ASCE 7-10.
- SEISMIC OCCUPANCY II
- SS= 0.26g
- S1= 0.07g
- SPECTRAL RESPONSE COEFFICIENT SDS= 0.27 g
- SPECTRAL RESPONSE COEFFICIENT SD1= 0.114 g
- SITE CLASS D

- SEISMIC DESIGN CATEGORY B
- BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL ORDINARY REINFORCED CONCRETE SHEAR WALLS
- RESPONSE MODIFICATION COEFFICIENT 'R' = 5
- SYSTEM OVER-STRENGTH FACTOR 'Wo' = 2.50
- DEFLECTION AMPLIFICATION FACTOR 'Cd' = 4.5
- CS= 0.055%g
- TOTAL BASE SHEAR = 629 KIPS

SEISMIC LOADING ASRS BUILDING:

- BASIC SEISMIC RESISTING FROM SYSTEM: ORDINARY CONCENTRIC BRACED FRAMES
- R = 3.25
- = 2.0
- Cc = 3.25
- CS = 0.084g
- TOTAL BASE SHEAR = 284 KIPS

NEW YORK STATE SPECIAL INSPECTION NOTES

THE DESIGNATED INSPECTING AGENCY FOR SPECIAL INSPECTION APPROVED BY THE CODE ENFORCEMENT OFFICIAL AND ACCEPTABLE TO THE STRUCTURAL ENGINEER RETAINED BY THE OWNER/CONTRACTOR SHALL PERFORM ON SITE INSPECTION AND TESTING IN ACCORDANCE WITH NEW YORK STATE BUILDING CODE REGULATIONS UNDER THE SUPERVISION OF A NEW YORK STATE LICENSED PROFESSIONAL. SPECIAL INSPECTION ITEMS INCLUDE THE FOLLOWING LISTED BELOW.

THE SPECIAL INSPECTION ENGINEERS ARE RESPONSIBLE FOR FILING AND OBTAINING APPROVAL OF ALL STATEMENTS, TEST AND INSPECTION REPORTS, INCLUDING STEEL, CONCRETE, MASONRY PRODUCER'S CERTIFICATES. COPIES TO BE SENT TO ENGINEER OF RECORD.

ALL TESTING AGENCY REPORTS SHALL BE SIGNED AND SEALED BY A NEW YORK STATE LICENSED PROFESSIONAL ENGINEER. TECHNICAL REPORTS SHALL BE FILED WITH THE BUILDING DEPARTMENT FOR APPROVAL OF SAID REPORTS.

CONTRACTOR TO NOTIFY THE SPECIAL INSPECTION ENGINEERS AT LEAST 72 HOURS BEFORE THE SPECIFIC WORK COMMENCES. THE CONTRACTOR SHALL PROVIDE AMPLE TIME FOR THE TESTING AGENCY TO COMPLETE THEIR WORK.

CONTRACTOR SHALL NOT CONCEAL ANY WORK REQUIRING SPECIAL INSPECTION UNTIL SUCH WORK IS INSPECTED OR TESTED AND DEEMED TO MEET PROJECT CRITERIA AND CODE REQUIREMENTS.

SPECIAL INSPECTION	CODE SECTION	EMPLOYED BY
CONCRETE - CAST IN PLACE	TABLE 1704.4	OWNER
CONCRETE DESIGN MIX	1904 & 1905	CONTRACTOR
CONCRETE TEST CYLINDERS *	1905.6	OWNER
STRUCTURAL STEEL - ERECTION & BOLTING	1704.3.3 & TABLE 1704.3	OWNER
STRUCTURAL STEEL - WELDING	1704.3.1	OWNER
STRUCTURAL SAFETY - STRUCTURAL STABILITY	1704.19	CONTRACTOR
ADHESIVE ANCHOR AS REQ'D BY MANUFACTURER	N/A	OWNER

SEISMIC SPECIAL INSPECTION

INSPECTION AND TESTING	REFERENCE STANDARD	BCNYS STANDARD
A. SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE		
STRUCTURAL STEEL	AISC 341	1707.2
MECHANICAL AND ELECTRICAL COMPONENTS		1707.8
B. STRUCTURAL TESTING FOR SEISMIC RESISTANCE		
TESTING FOR SEISMIC RESISTANCE		1708.2
STRUCTURAL STEEL	AISC 341, AWS D1.1	1708.4
SEISMIC QUALIFICATION OF MECHANICAL AND ELECTRICAL EQUIPMENT		1708.5

LOADING SCHEDULE									
SUPERIMPOSED DEAD LOADING (PSF)	PARKING AREA	ADMIN BUILDING OFFICE	ADMIN BUILDING CORRIDOR UPPER FLOORS	ADMIN BUILDING TERRACE	ASRS ROOF	OP'S BUILDING OFFICE	OP'S BUILDING CORRIDOR UPPER FLOORS	OP'S BUILDING FIRST FLOOR AND LOBBIES	OP'S BUILDING ROOF
CEILING	1	2	2	2		2	2	2	2
FIREPROOFING	1	2	2	2		2	2	2	2
MEP	1	2	2	2	2	2	2	2	2
WATERPROOFING				5	6				
INSULATION				3	6				
DRAINAGE MAT				4					
SLAB TOPPING 4"	50								
FLOORING		3	9	25		3	3	3	3
	68	9	15	43	14	9	9	9	9
LIVE LOADING (PSF)									
LIVE LOADING	100	50	80	100	20	50	80	100	50
PARTITION		15				15			15

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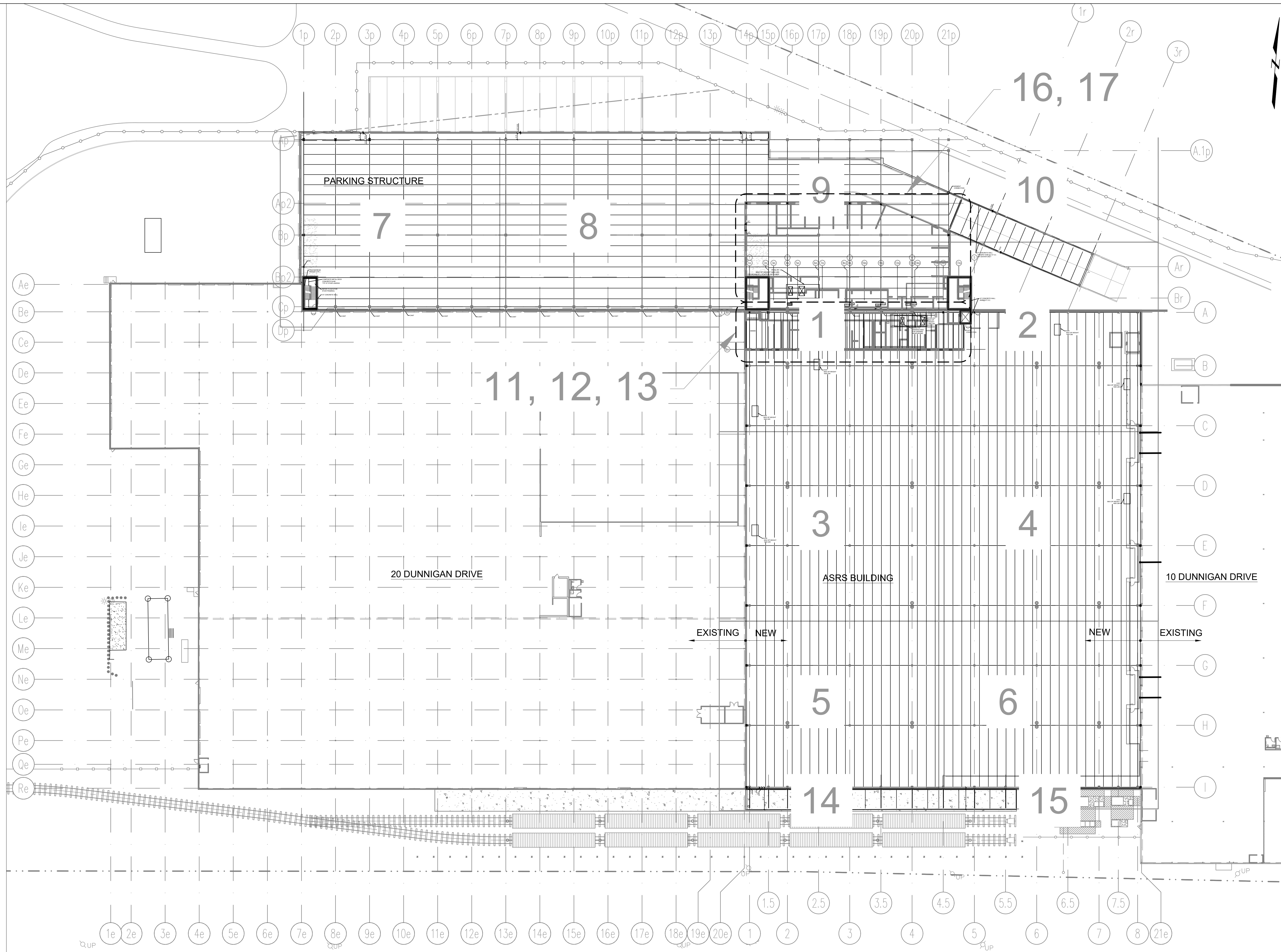
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NOTES

DWG NUMBER :

S-002

3 OF 60



FRAMING KEY PLAN
SCALE: 1/32" = 1'-0"

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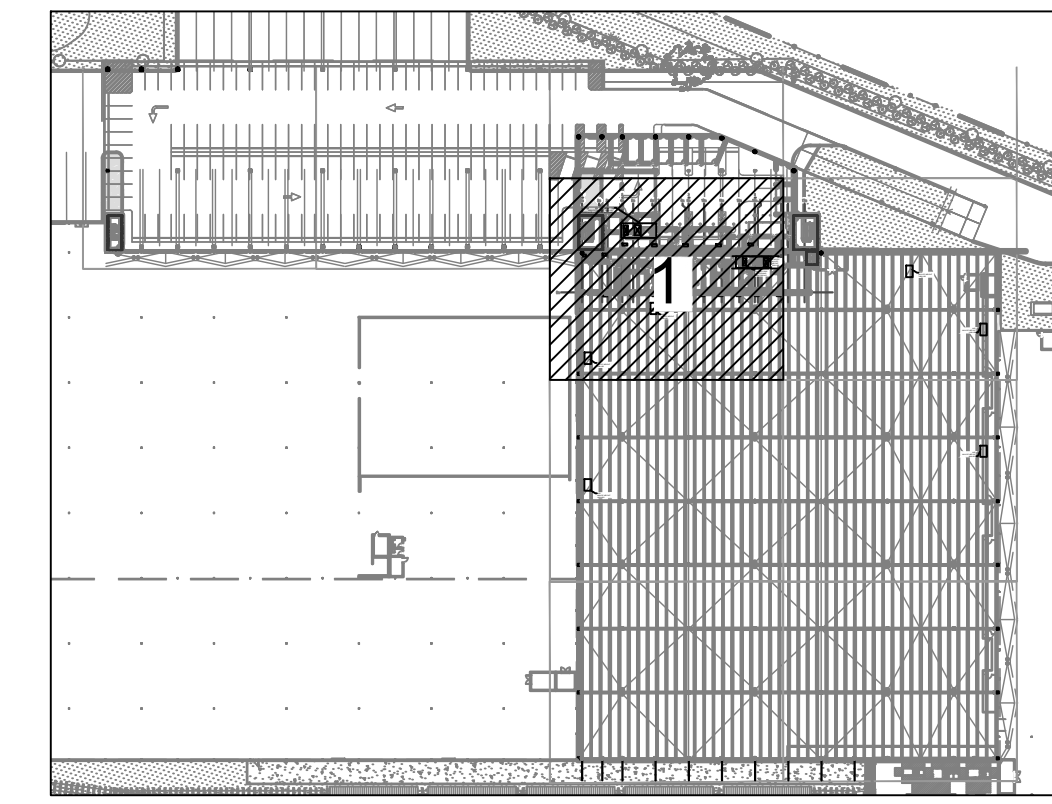
DWG NUMBER:
S-100
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KEY PLAN
 SCALE: N.T.S.

- LEGEND
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #

NOTE:
 ASRS ROOF DECK WELDING
 SHALL BE 3/4" PUDDLE WELD
 W/36/7 WELD PATTERN
 W10-#12 SIDELAP FASTENERS



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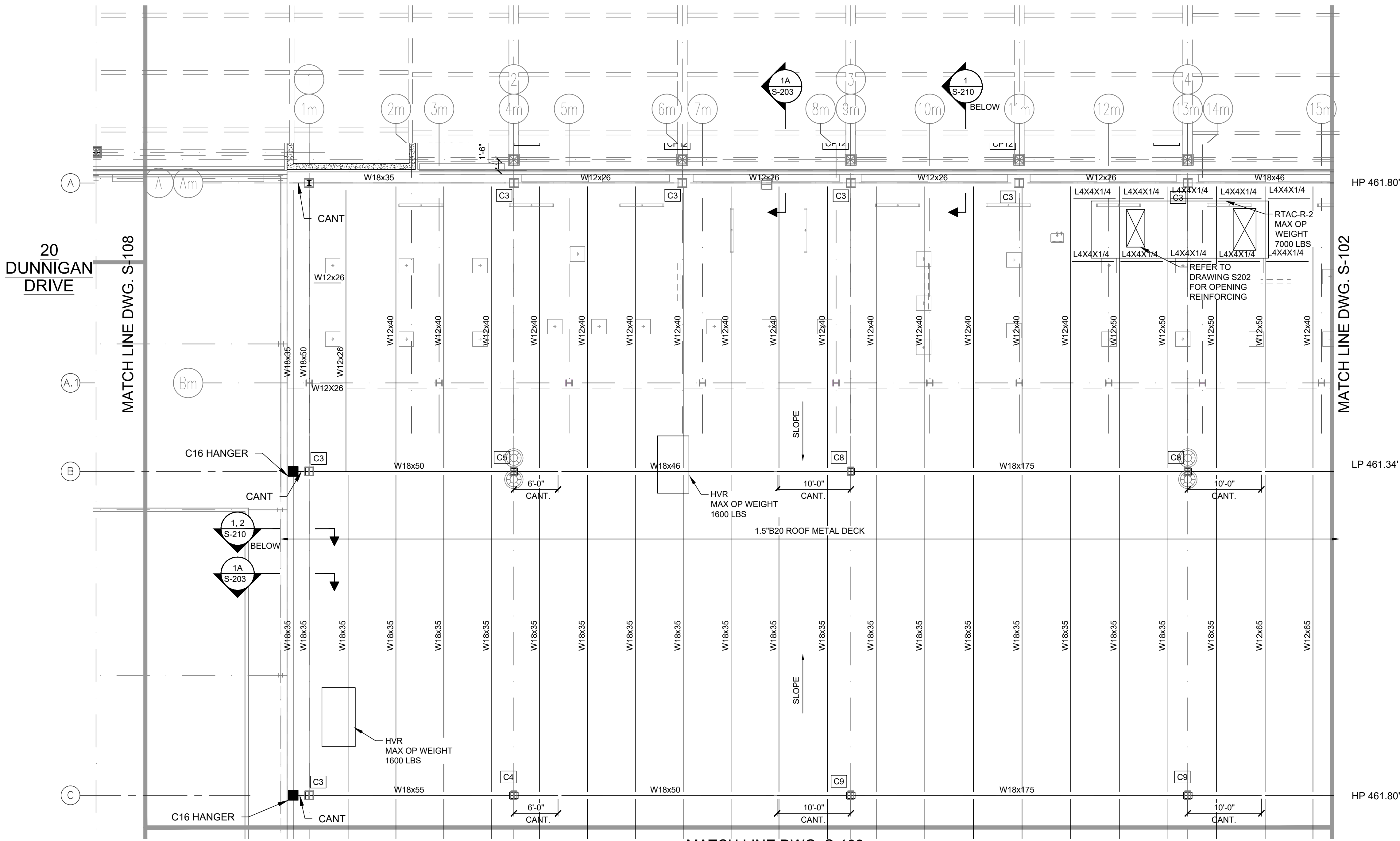
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AS/RS FRAMING PART PLAN

DWG NUMBER :
S-101
 6 OF 60

MATCH LINE DWG. S-109



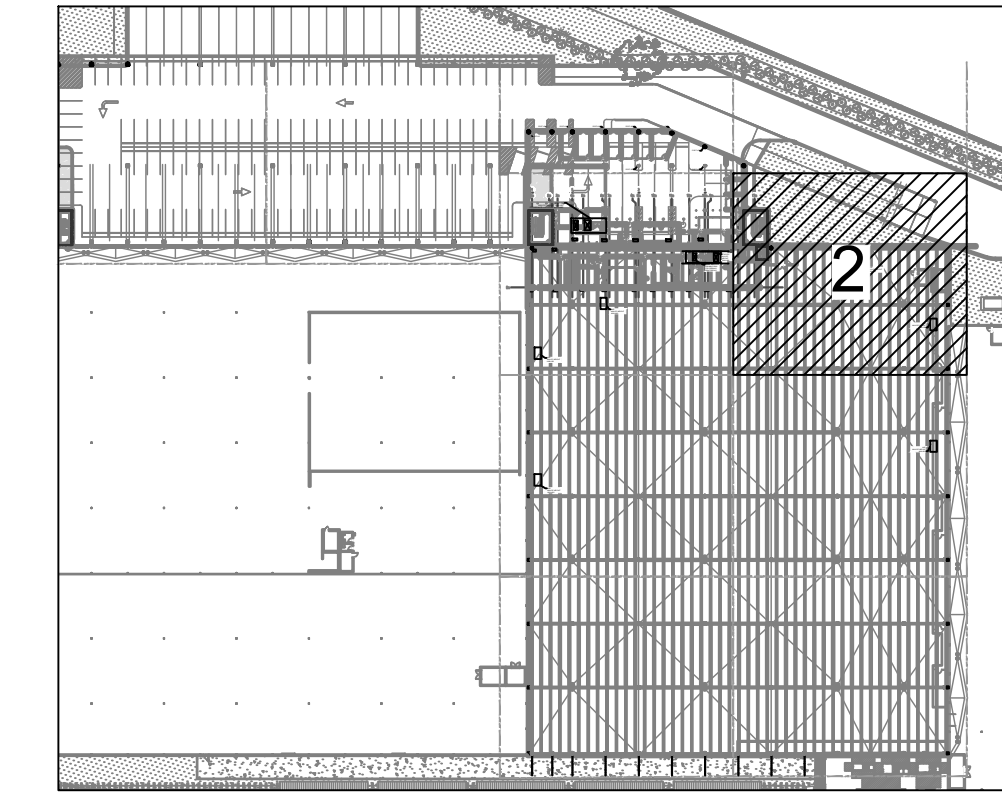
MATCH LINE DWG. S-103

1 PART PLAN
 SCALE: 1/8" = 1'-0"
 TOS REFER TO PLAN

20 DUNNIGAN DRIVE

MATCH LINE DWG. S-108

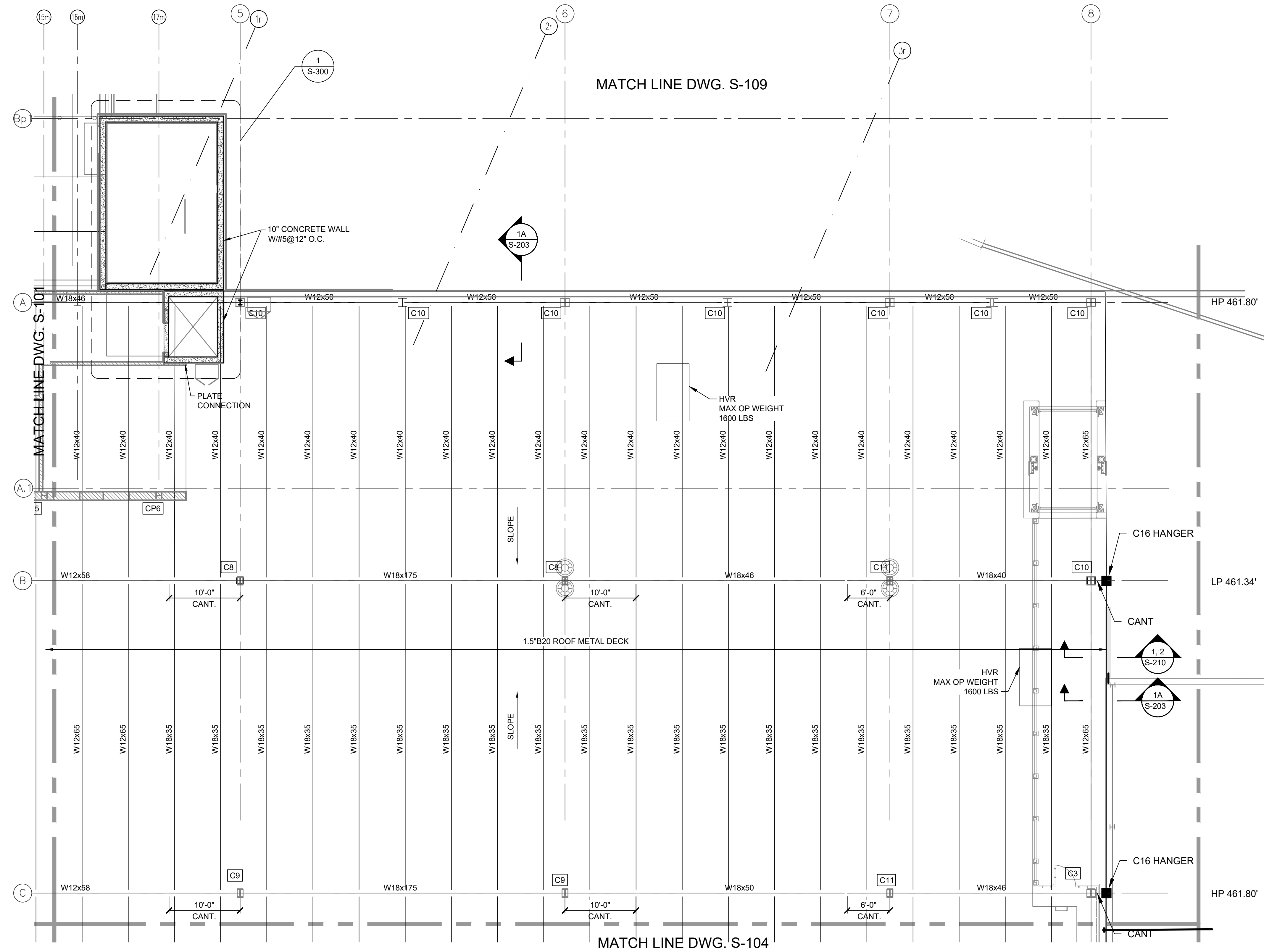
MATCH LINE DWG. S-102



KEY PLAN
SCALE: N.T.S.

LEGEND

- TOS - DENOTES TOP OF STEEL ELEVATION
- HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
- LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
- C1 - COLUMN #



2 PART PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN

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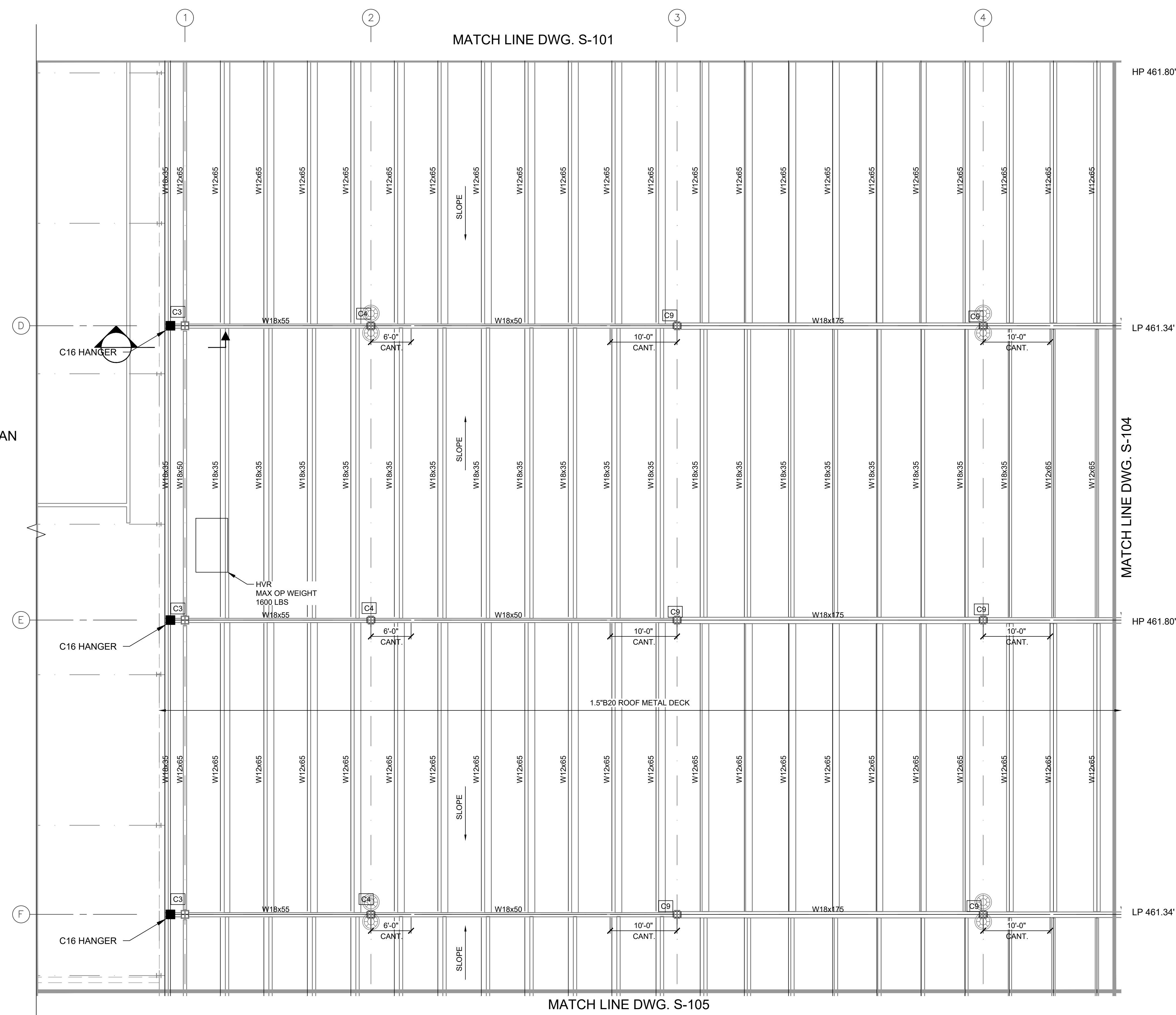
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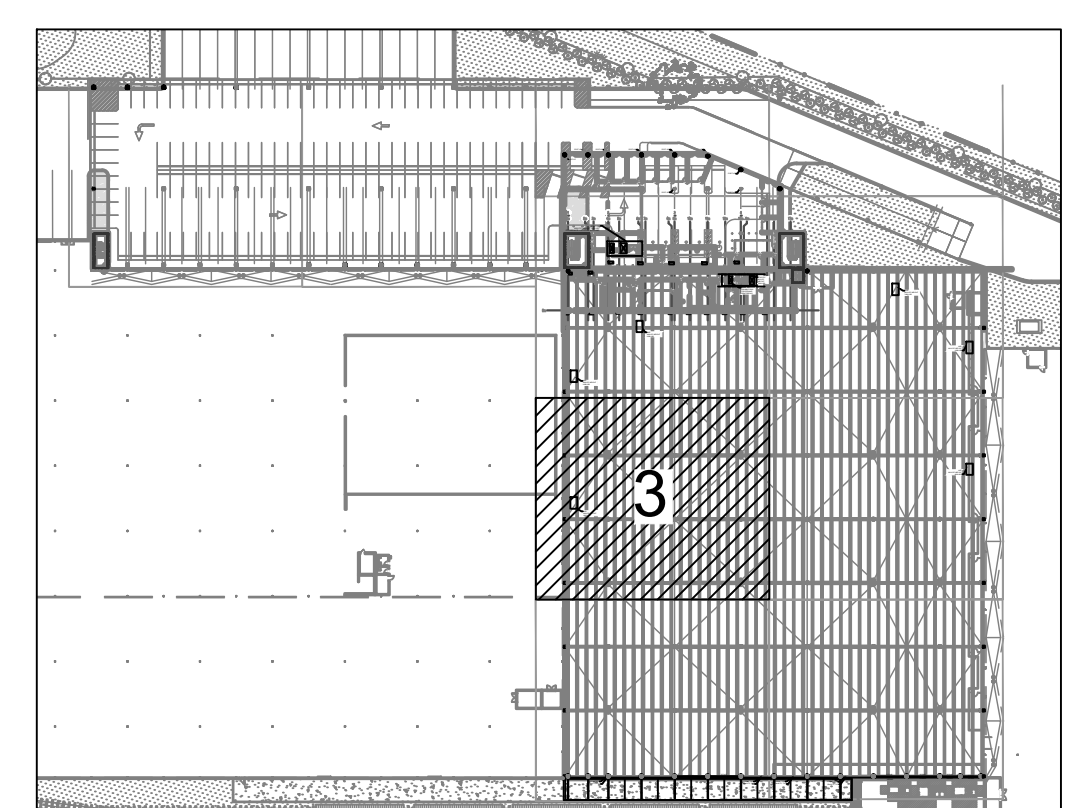
AS/RS FRAMING PART PLAN

DWG NUMBER :
S-102
7 OF 60

20 DUNNIGAN DRIVE



3 PART PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN



KEY PLAN
SCALE: N.T.S.

- LEGEND**
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #

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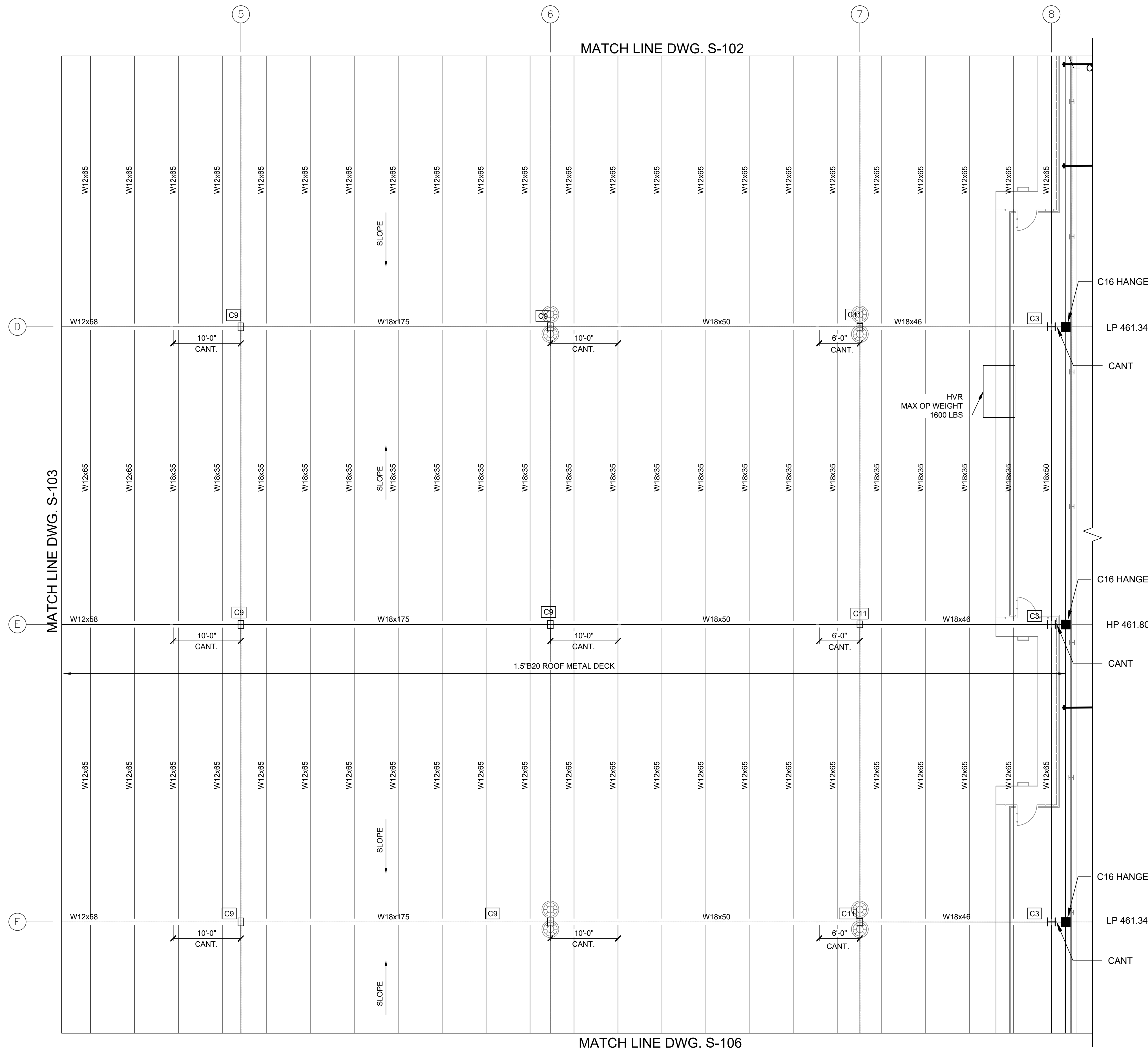
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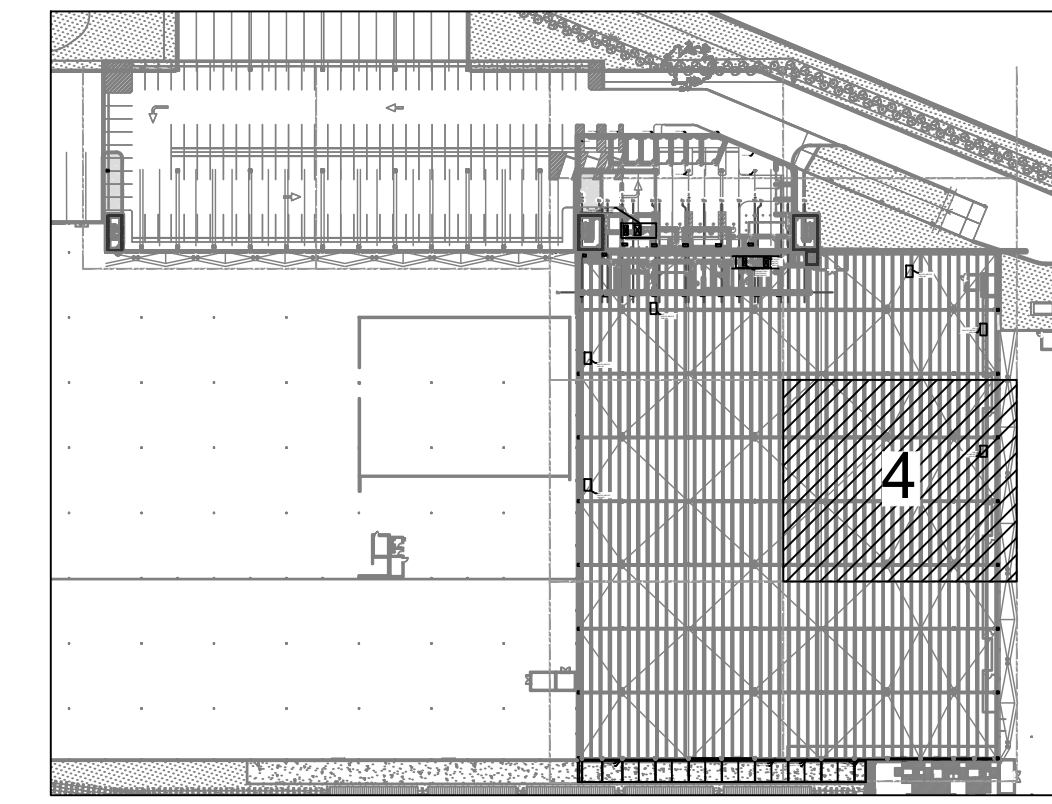
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AS/RS FRAMING PART PLAN

DWG NUMBER :
S-103
8 OF 60



4 PART PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN



KEY PLAN
SCALE: N.T.S.

- LEGEND**
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 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #

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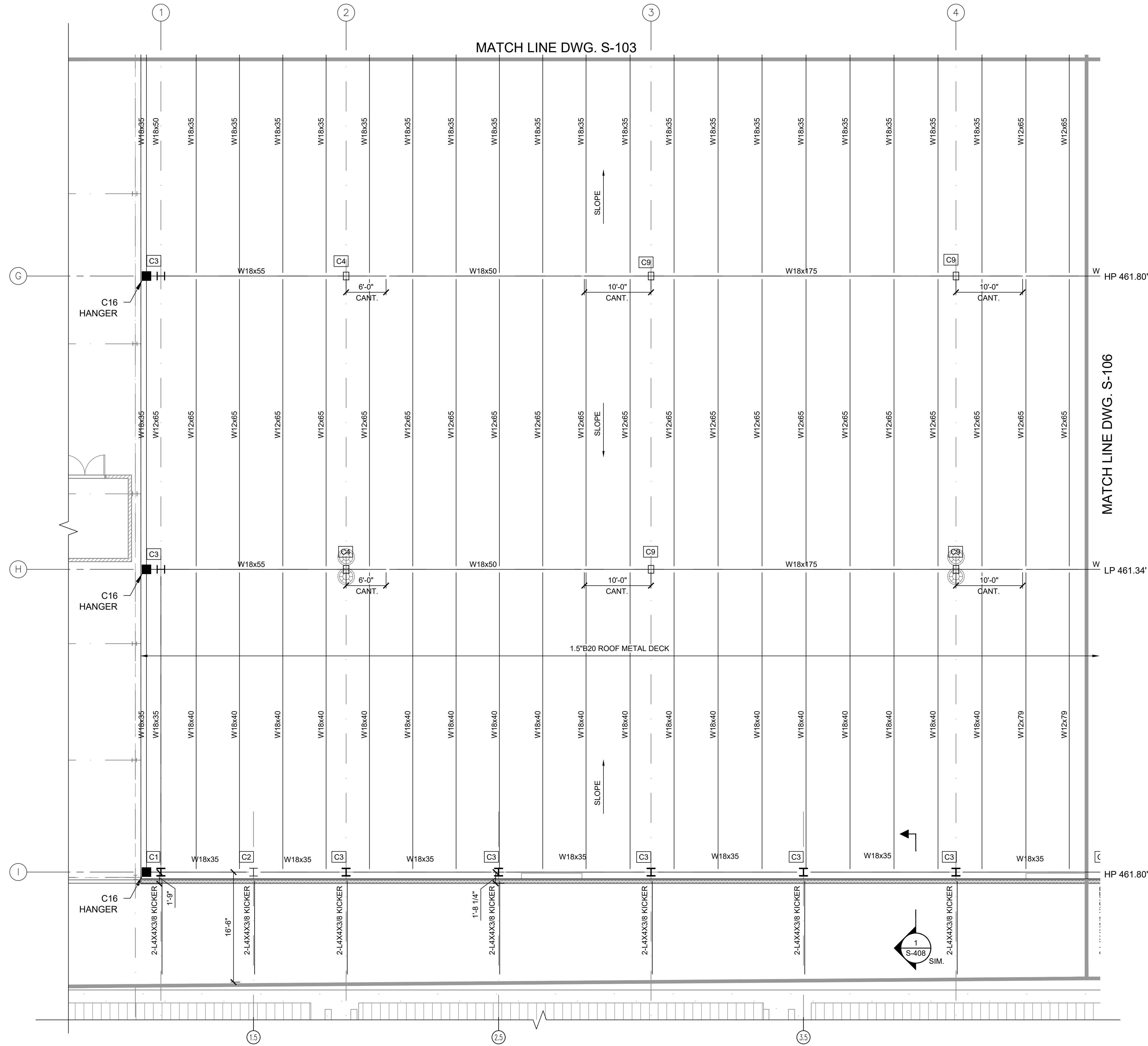
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AS/RS FRAMING PART PLAN

DWG NUMBER :
S-104
9 OF 60

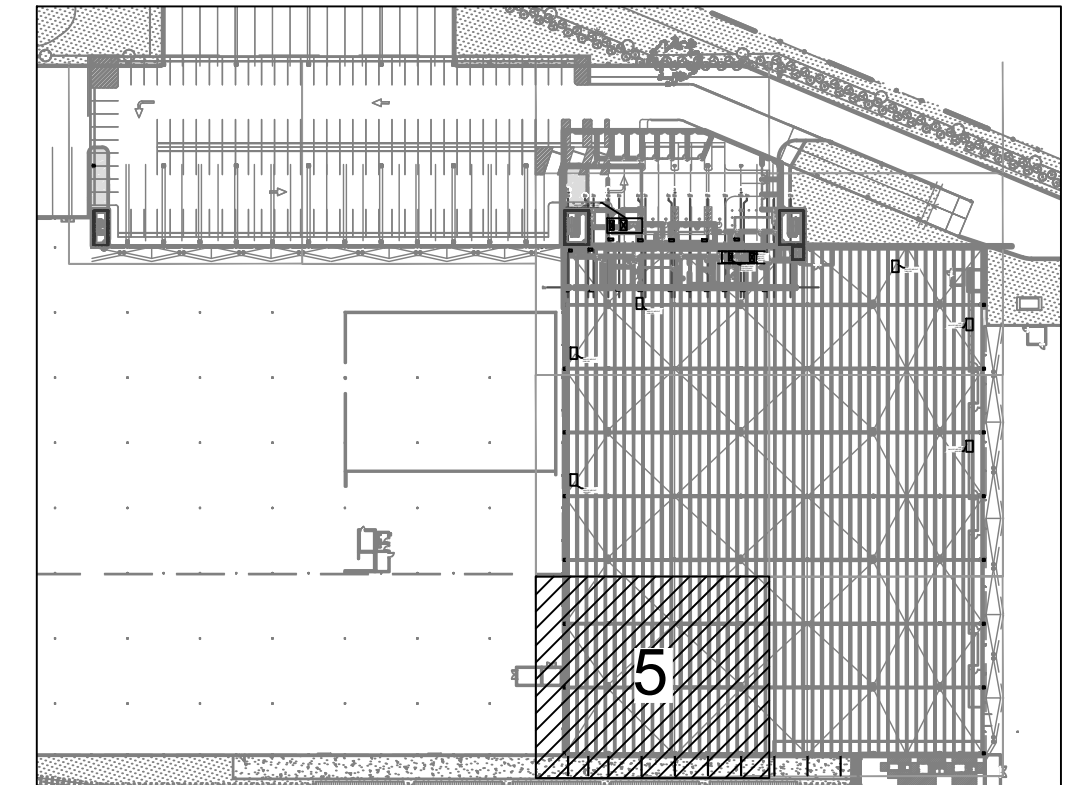
20 DUNNIGAN DRIVE



MATCH LINE DWG. S-103

MATCH LINE DWG. S-106

5 PART PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN



KEY PLAN
SCALE: N.T.S.

- LEGEND
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #

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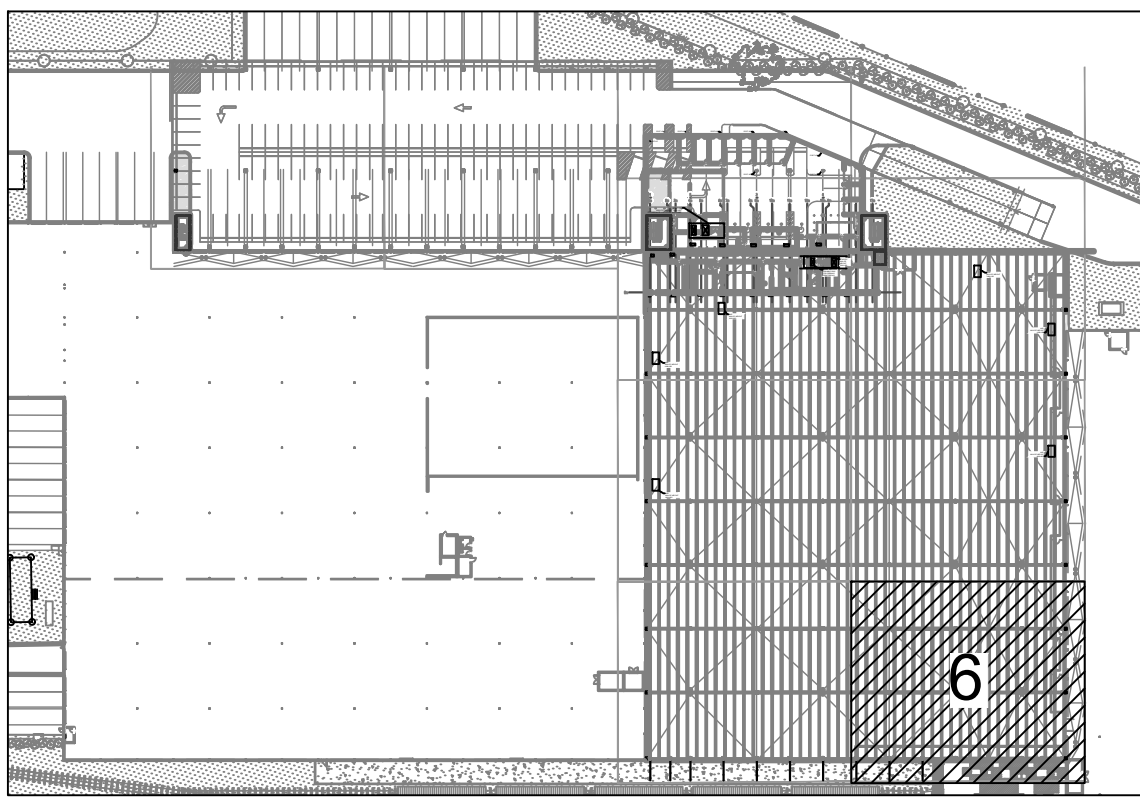
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AS/RS FRAMING PART PLAN

DWG NUMBER :
S-105
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KEY PLAN
SCALE: N.T.S.

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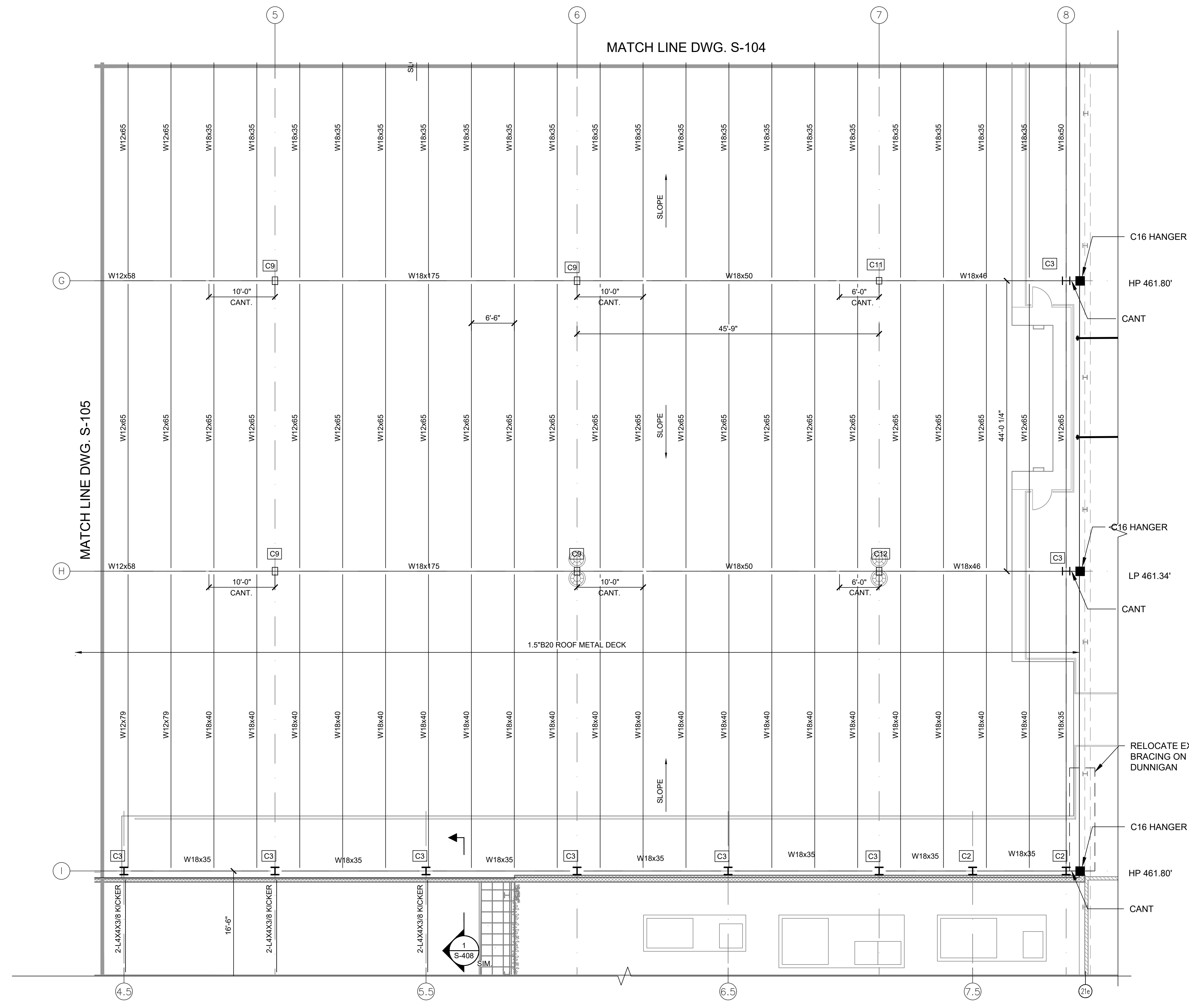
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AS/RS FRAMING PART PLAN

DWG NUMBER:
S-106
11 OF 60



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HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
C1 - COLUMN #

6 PART PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN

MATCH LINE DWG. S-105

MATCH LINE DWG. S-104

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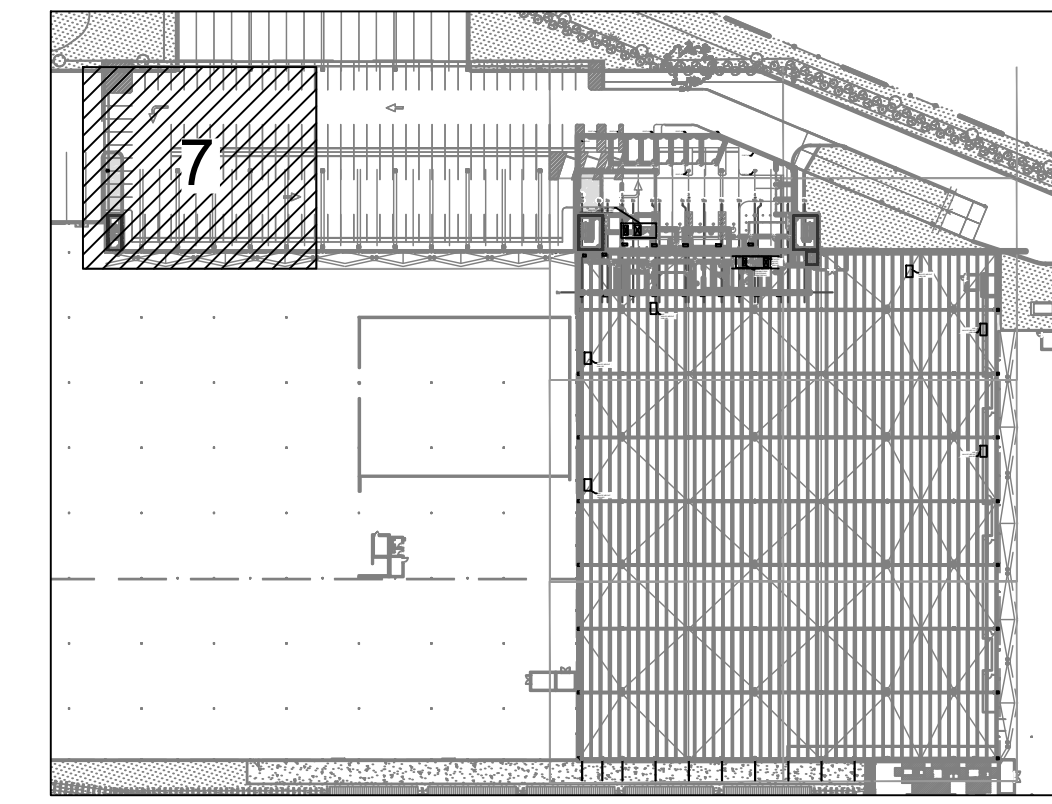
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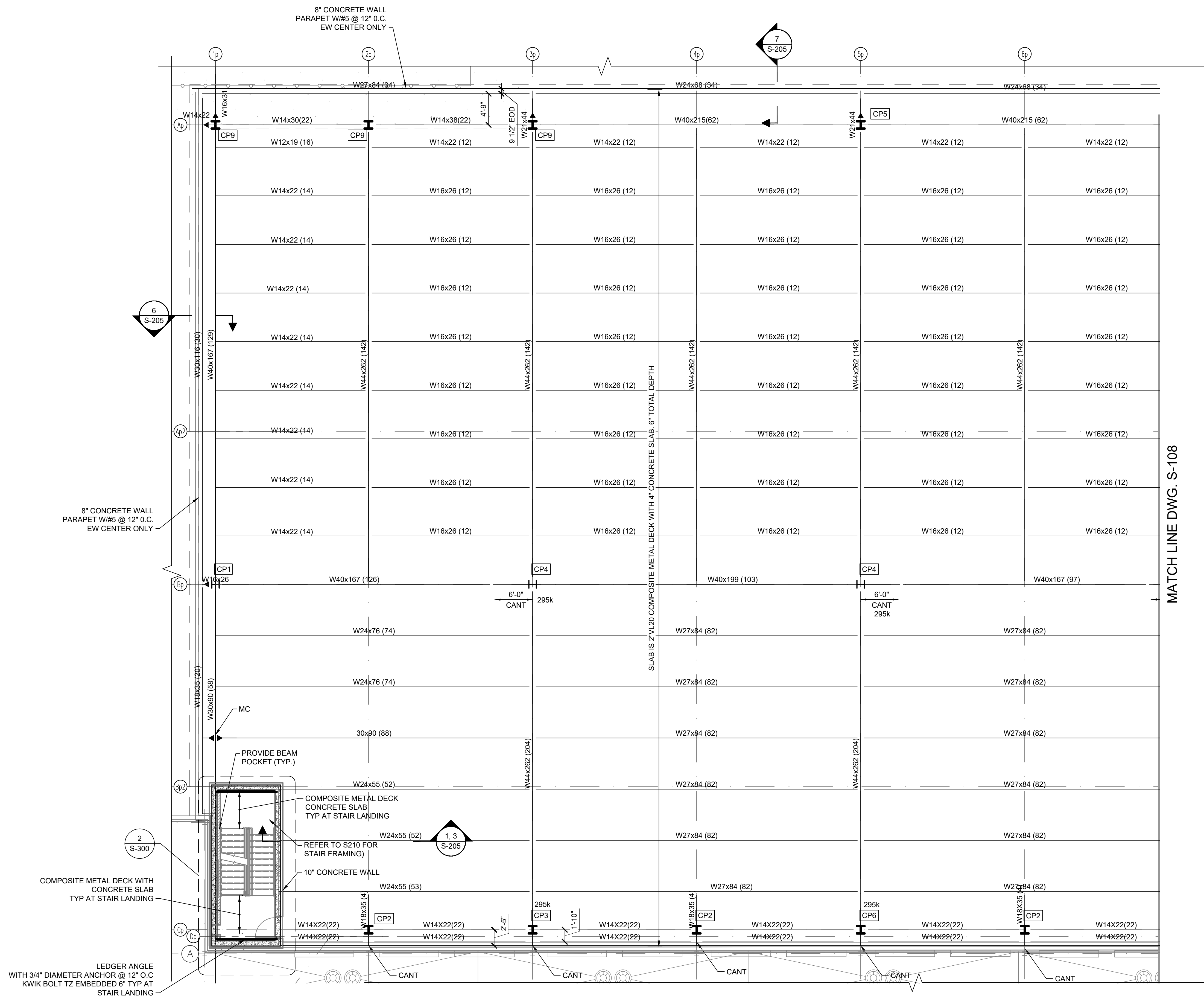
KEY PLAN
 SCALE: N.T.S.

LEGEND

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- HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
- LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
- [C1] - COLUMN #
- ◀ - DENOTES MOMENT CONNECTION TO DEVELOP 80% MP

NOTE
 SHEAR STUDS SHALL BE 3/4"-5" LONG (TYP) FOR 6" SLAB.
 INSTALL SHEAR STUD AT 1' O.C. MAXIMUM AT EVERY BEAM MINIMUM (UNO)

PARKING SLAB SHALL BE REINFORCED WITH
 6X6-W2.9XW2.9 WWF



7 PART PLAN
 SCALE: 1/8" = 1'-0"
 TOS 429.58

MATCH LINE DWG. S-108

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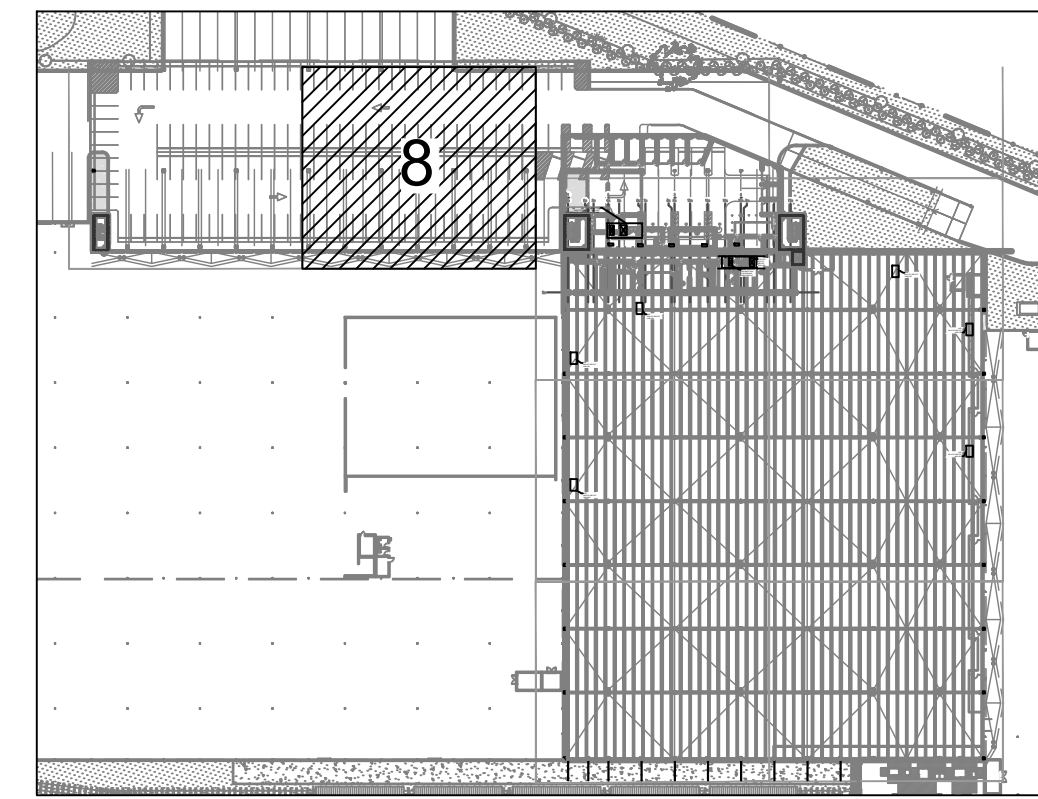
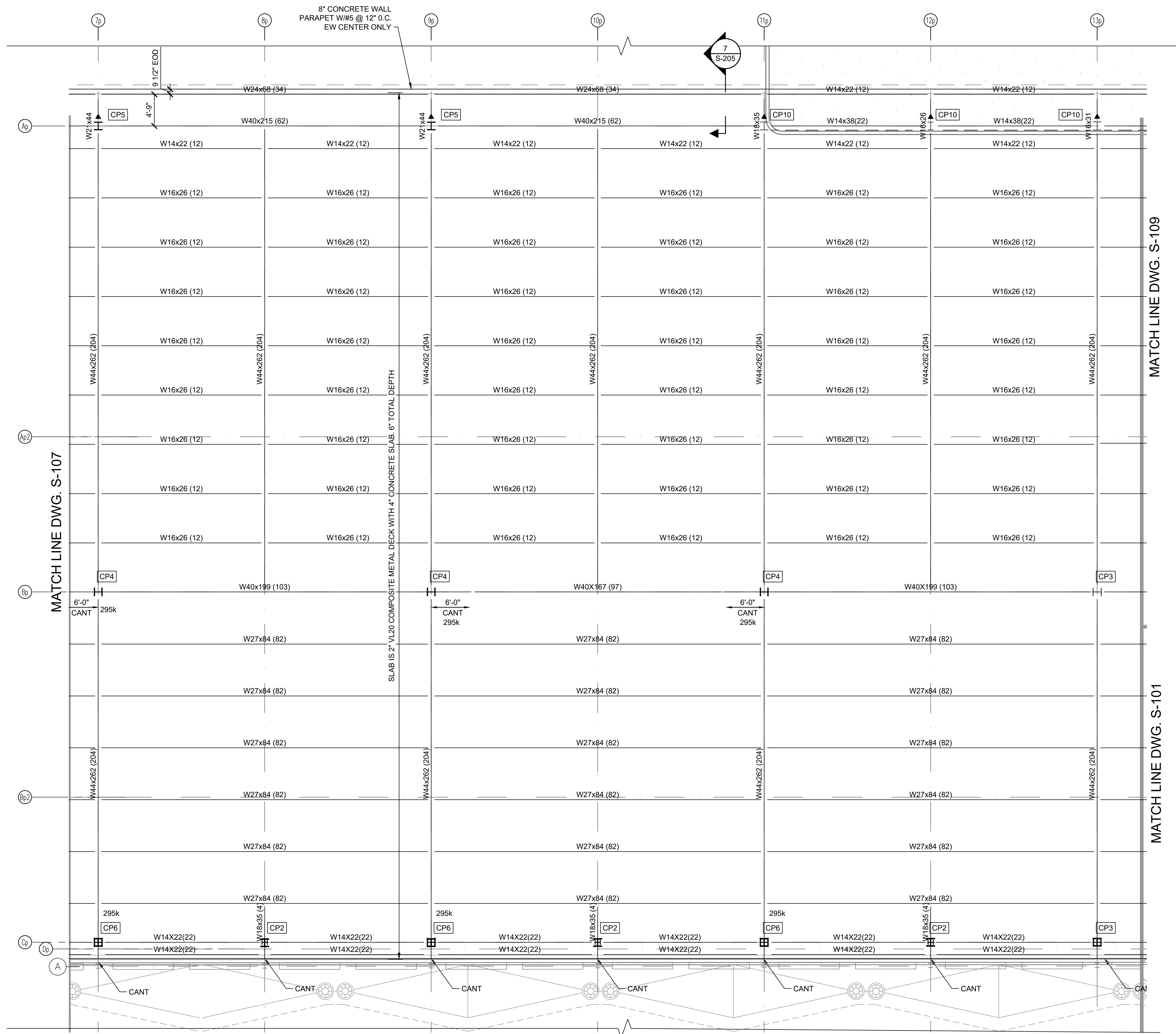
DRAWING TITLE :

**PARKING FRAMING
 PART PLAN**

DWG NUMBER :

S-107

12 OF 60



KEY PLAN
SCALE: N.T.S.

LEGEND

- TOS - DENOTES TOP OF STEEL ELEVATION
- HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
- LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
- [C1] - COLUMN #

8 PART PLAN
SCALE: 1/8" = 1'-0"
TOS 429.58

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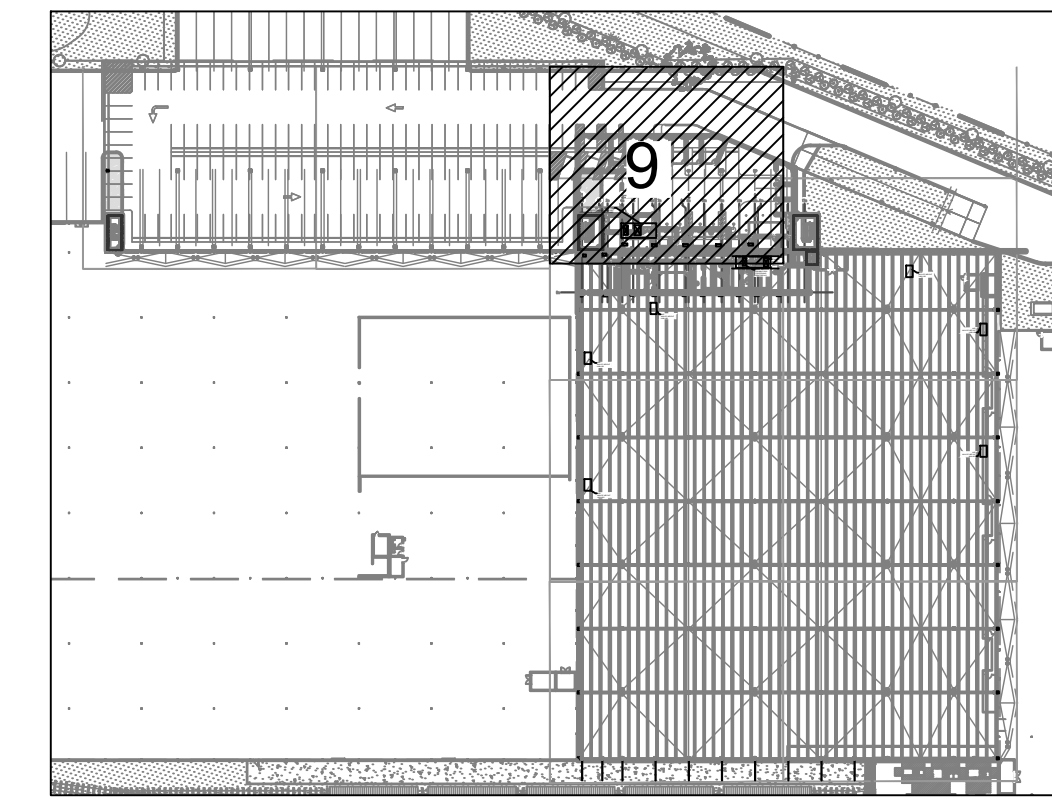
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PARKING FRAMING
PART PLAN

DWG NUMBER :

S-108

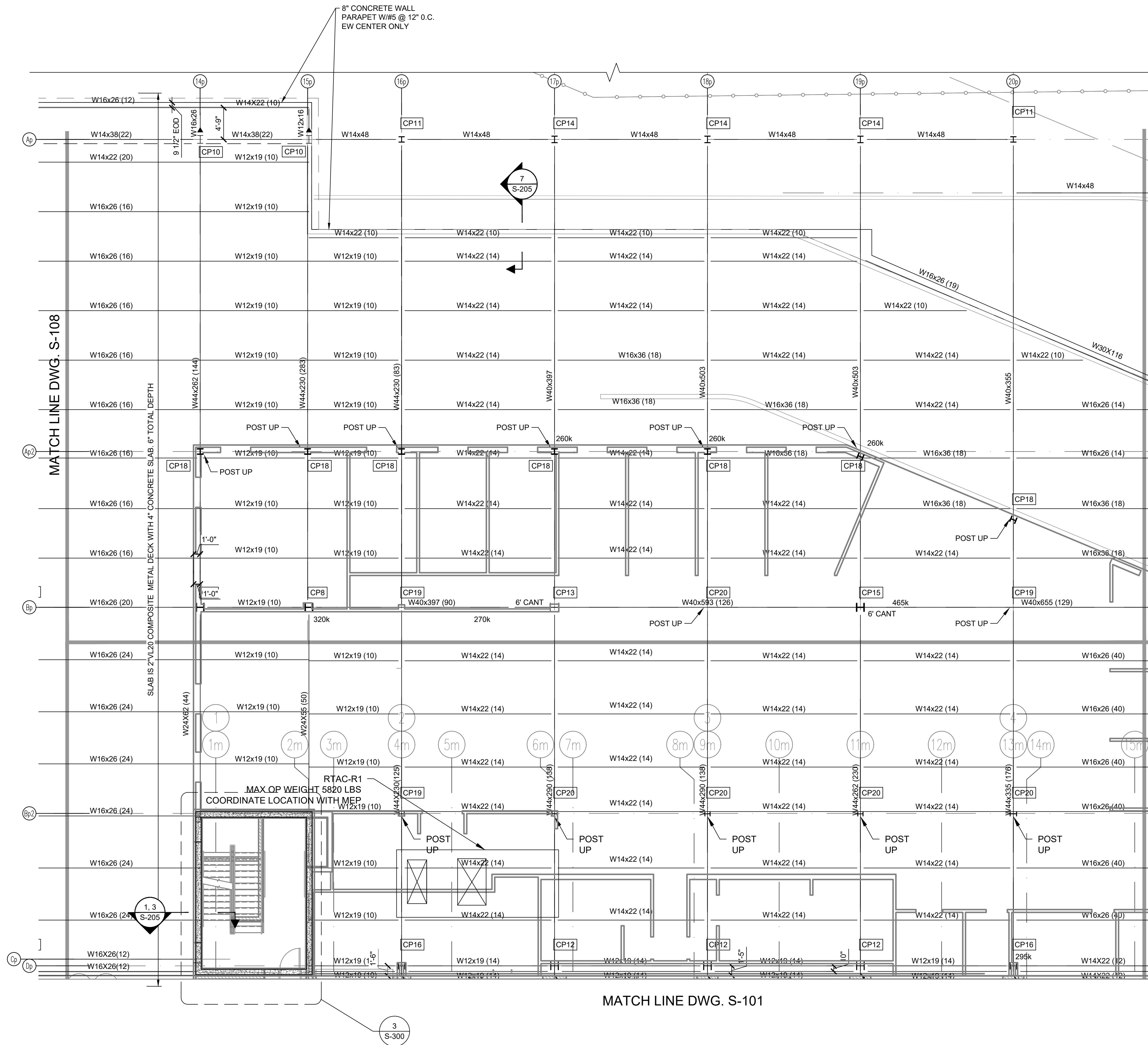
13 OF 60



KEY PLAN
SCALE: N.T.S.

LEGEND

- TOS - DENOTES TOP OF STEEL ELEVATION
- HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
- LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
- CP1 - COLUMN #



MATCH LINE DWG. S-101

9 PART PLAN
SCALE: 1/8" = 1'-0"
TOS 429.58

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SCALE : AS SHOWN

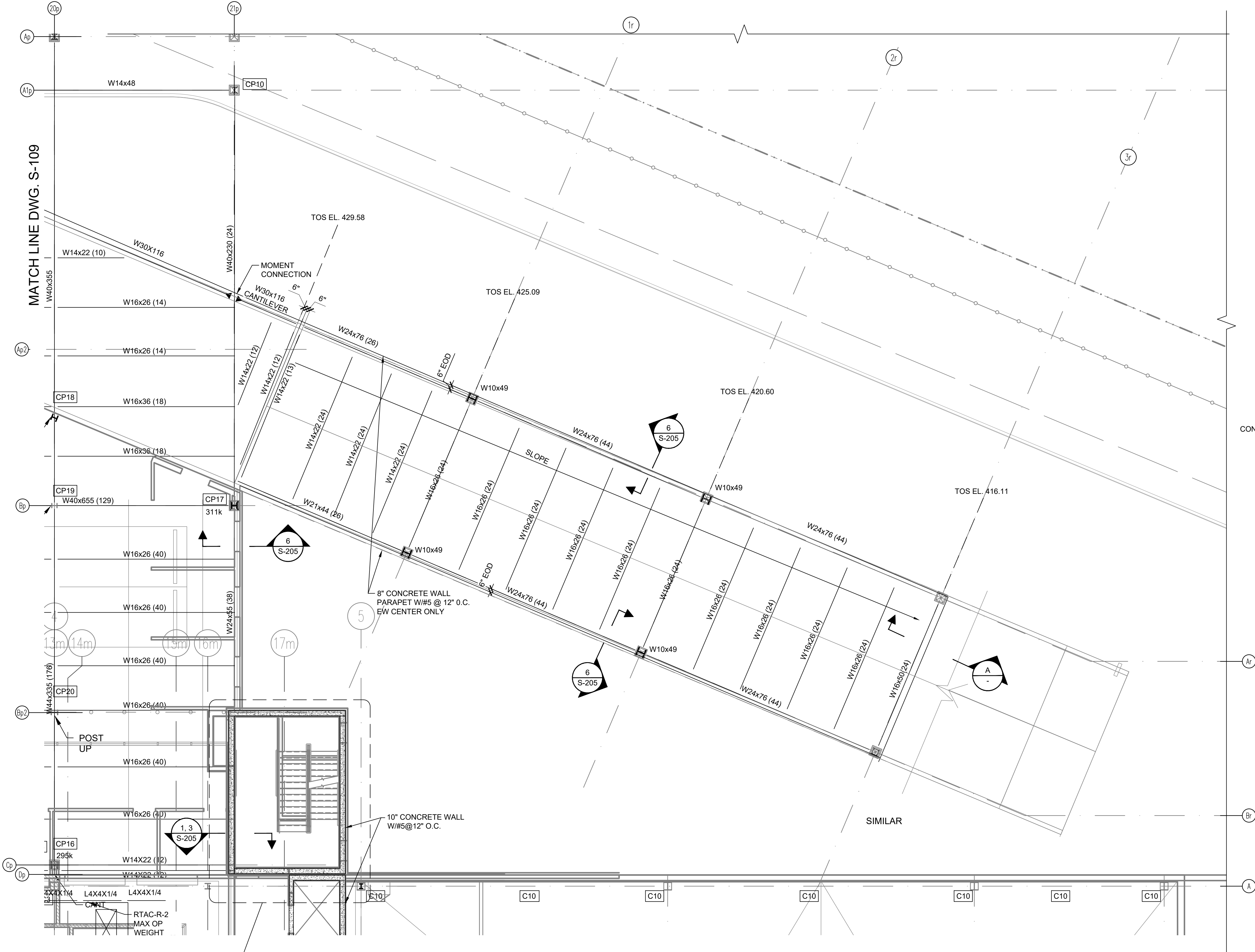
DRAWING TITLE :

**PARKING FRAMING
PART PLAN**

DWG NUMBER :

S-109

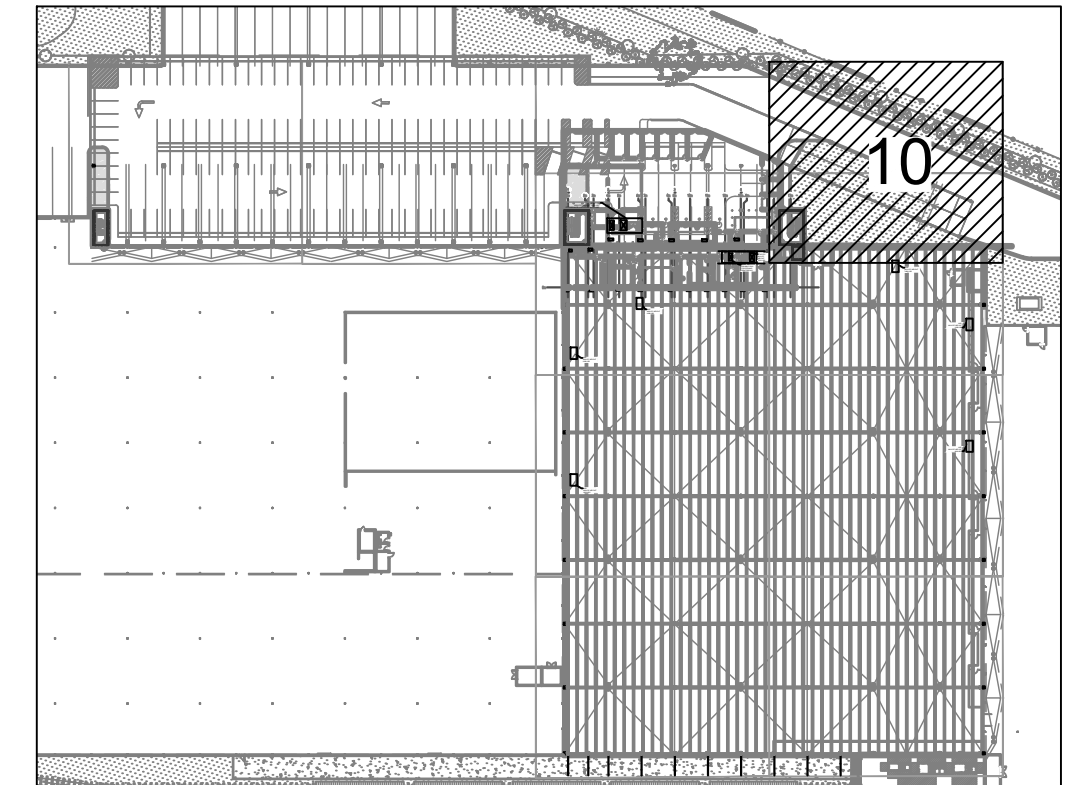
14 OF 60



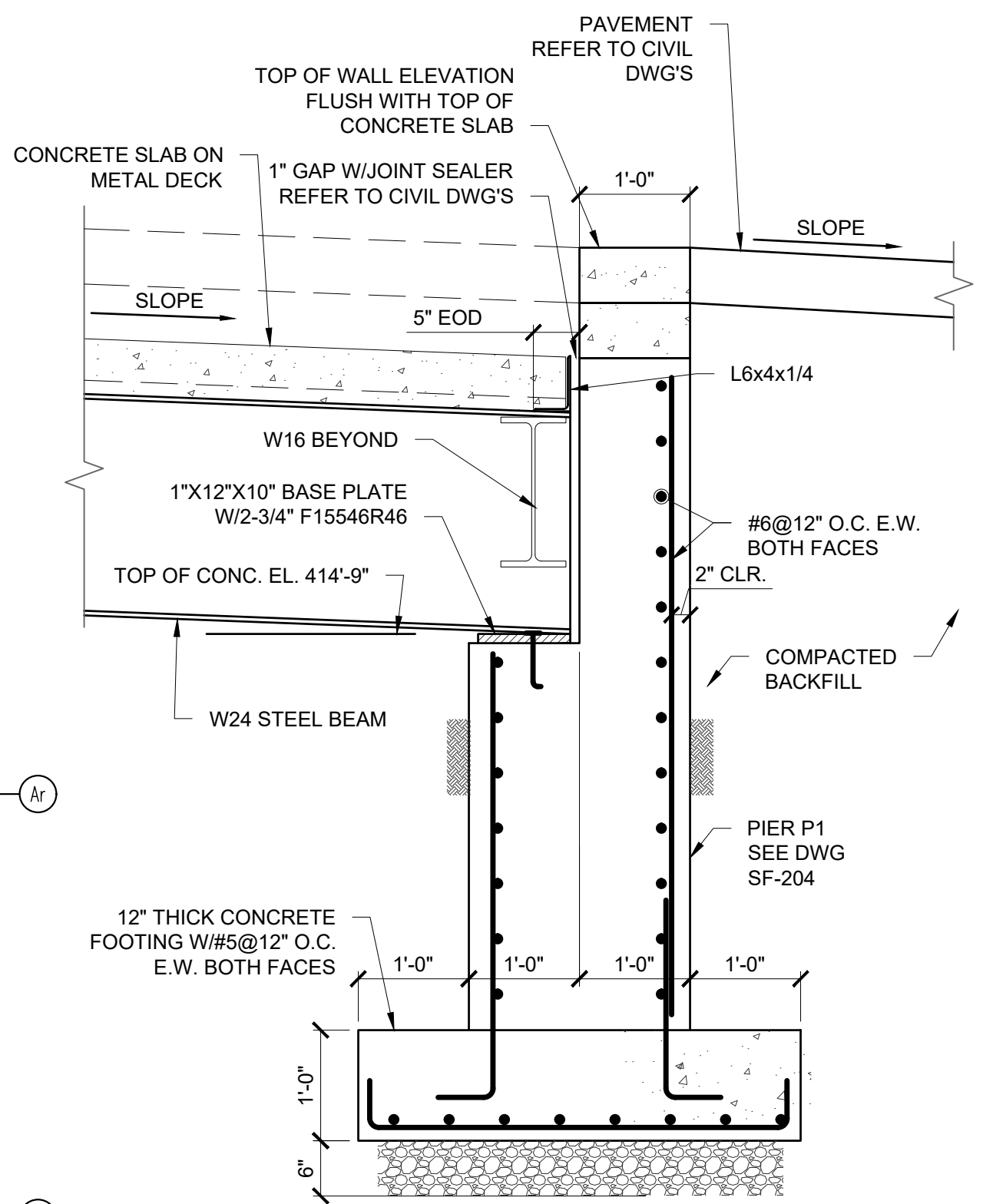
MATCH LINE DWG. S-109

10 PART PLAN
SCALE: 1/8" = 1'-0"

TOS 429.58 UNO.
REFER TO PLAN FOR
RAMP ELEVATIONS



KEY PLAN
SCALE: N.T.S.



A BUTTRESS SECTION
SCALE: 3/4" = 1'-0"

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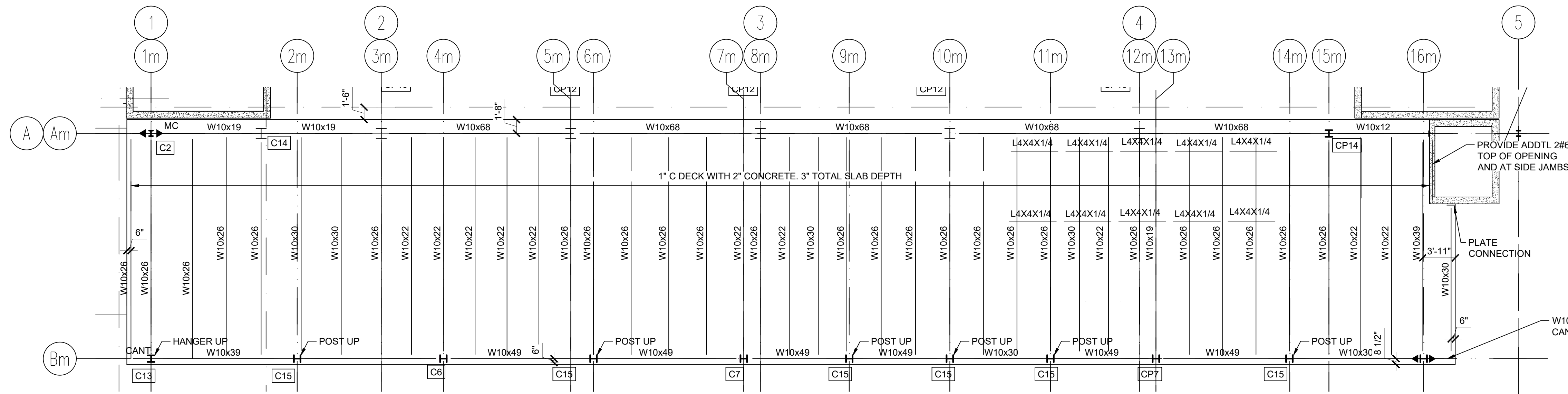
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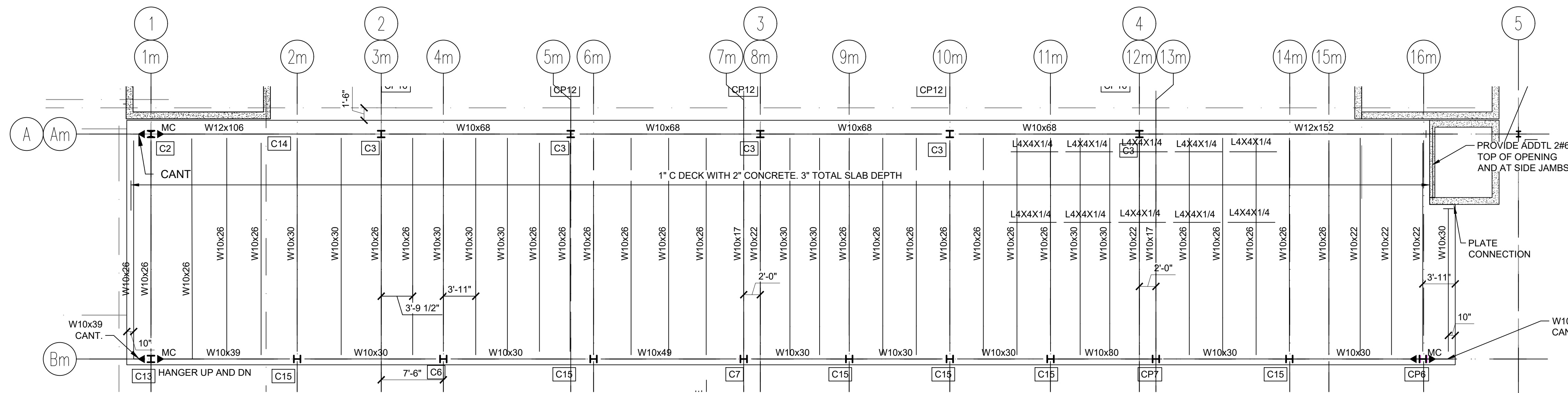
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**PARKING FRAMING
PART PLAN**

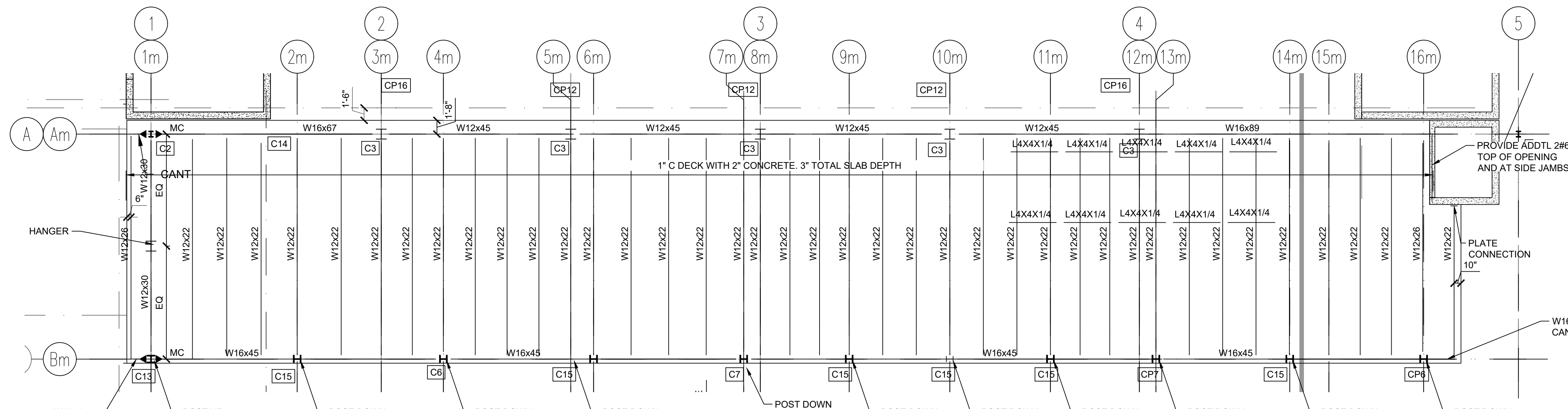
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S-110
15 OF 60



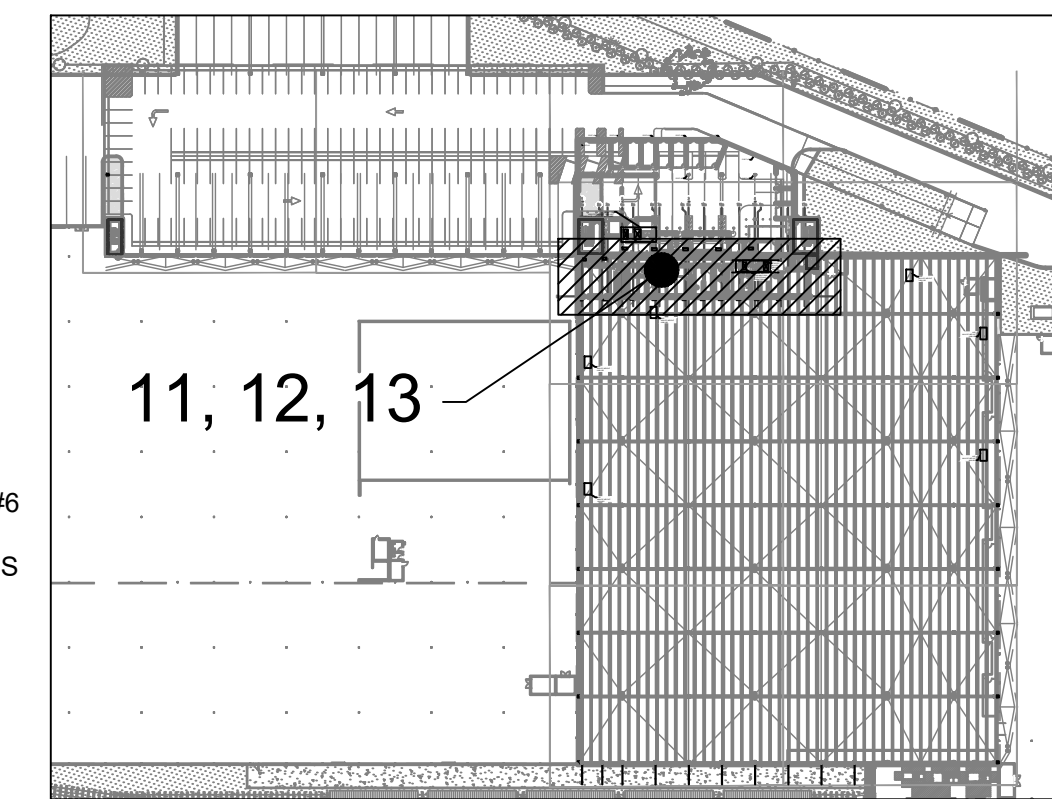
11 MEZZANINE FRAMING PLAN
 SCALE: 1/8" = 1'-0"
 TOS 421.00'



12 UPPER MEZZANINE FRAMING PLAN
 SCALE: 1/8" = 1'-0"
 TOS 430.75'



13 OP BUILDING ROOF PLAN
 SCALE: 1/8" = 1'-0"
 TOS 441.95'



KEY PLAN
 SCALE: N.T.S.

LEGEND

- TOS - DENOTES TOP OF STEEL ELEVATION
- HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
- LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
- C1 - COLUMN #
- - DENOTES MOMENT CONNECTION TO DEVELOP 80% MP

NOTE

- METAL DECK WELDING SHALL BE 5/8" PUDDLE WELD 36/4 WELD PATTERN
- W/4-#12 SIDELAP FASTENERS
- SLAB SHALL BE REINFORCED WITH 6X6-2.9XW2.9 WWF

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DRAWING TITLE :

MEZZANINE FRAMING PLAN

DWG NUMBER :

S-111

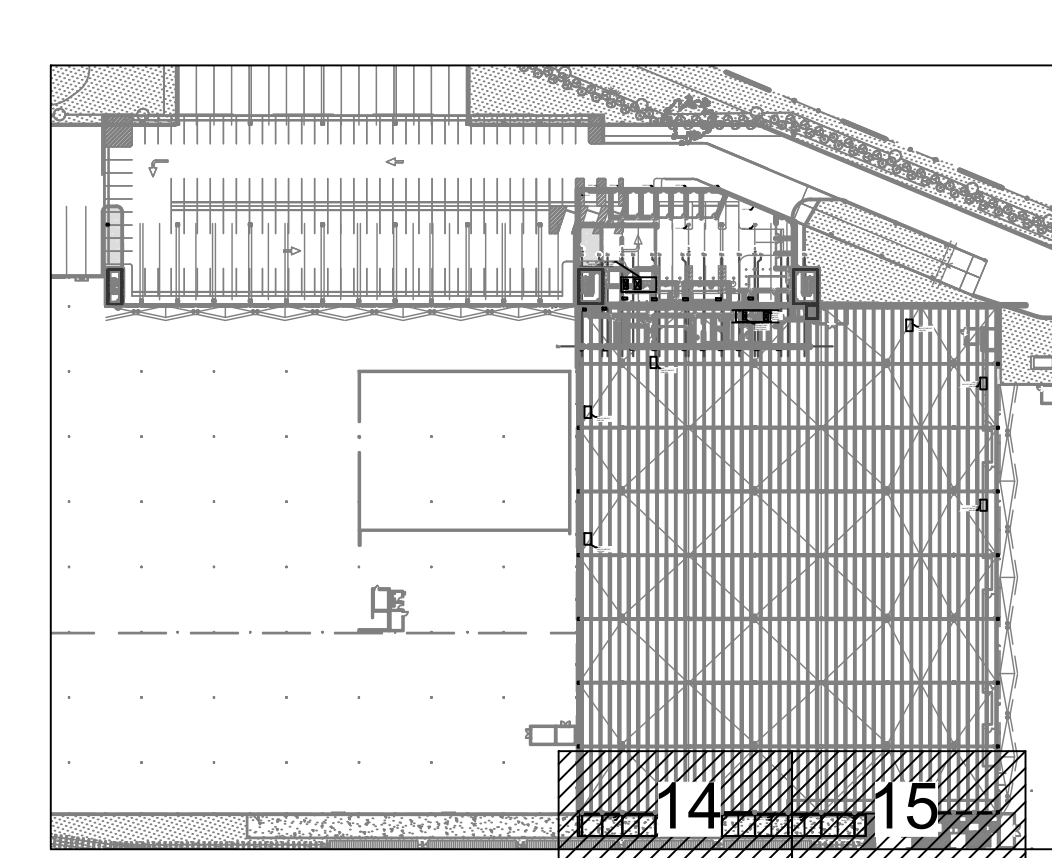
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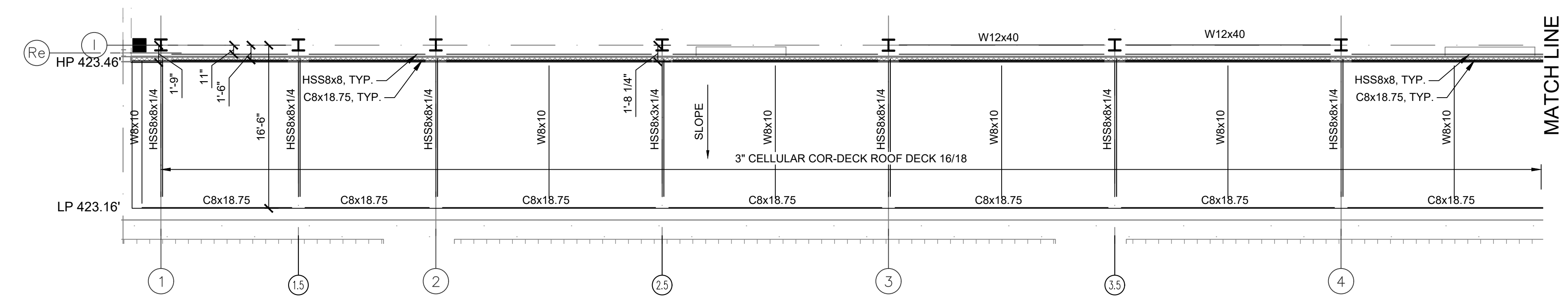
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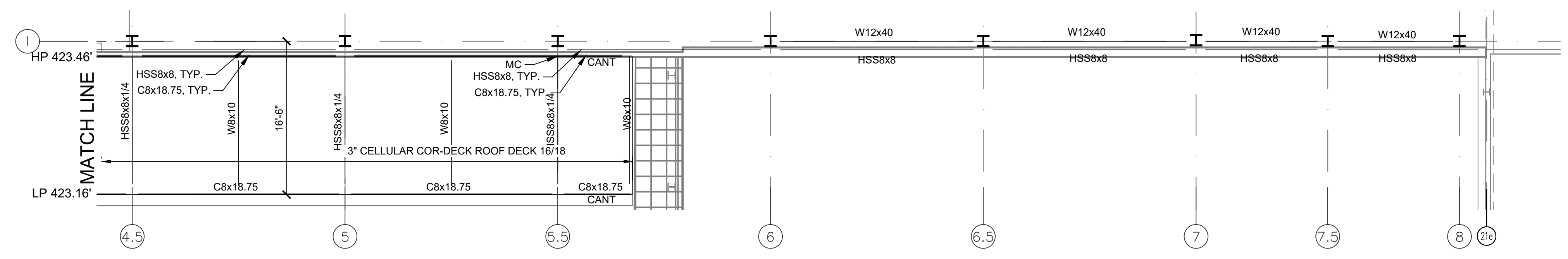


KEY PLAN
 SCALE: N.T.S.

- LEGEND
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #



14 CANOPY FRAMING
 SCALE: 1/8" = 1'-0"



15 CANOPY FRAMING
 SCALE: 1/8" = 1'-0"

TOS REFER TO PLAN

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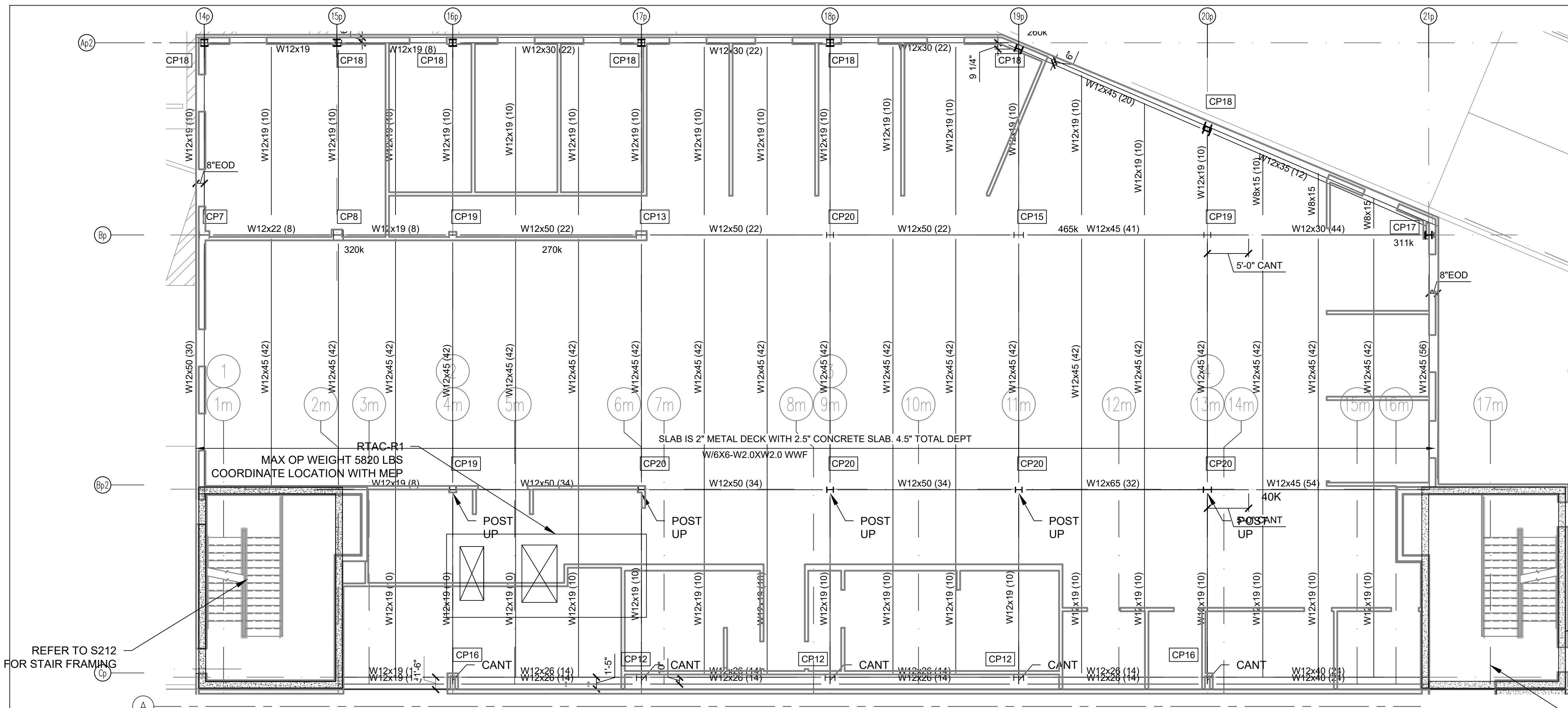
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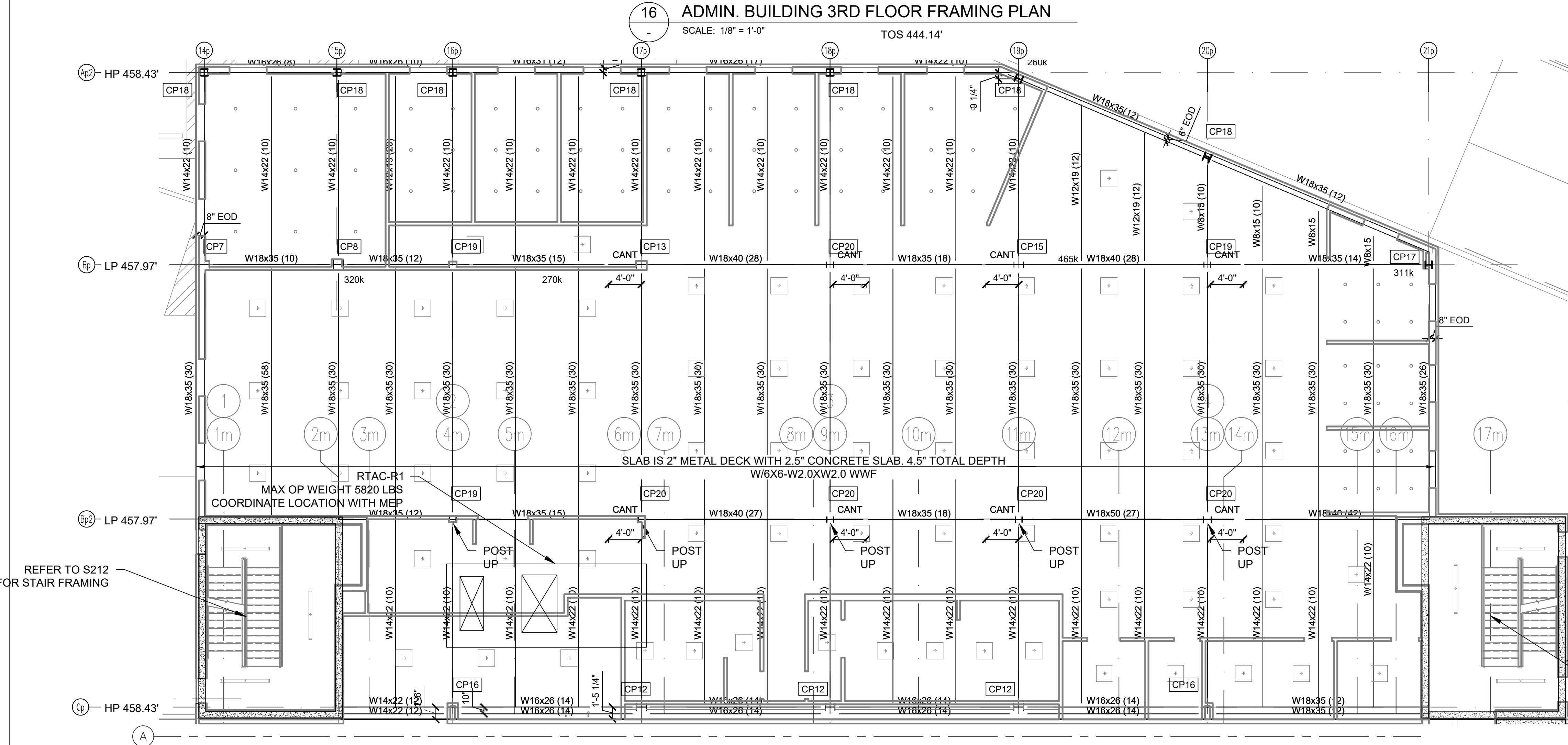
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CANOPY FRAMING

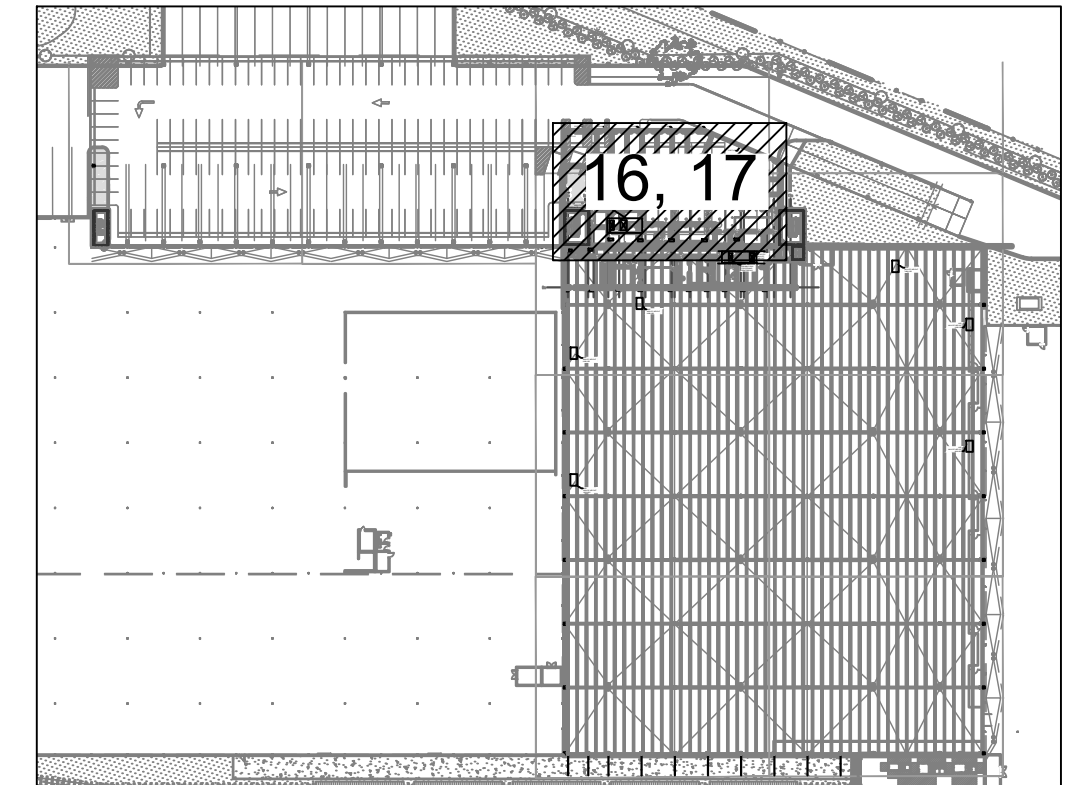
DWG NUMBER :
S-120
 17 OF 60



16 ADMIN. BUILDING 3RD FLOOR FRAMING PLAN
SCALE: 1/8" = 1'-0"
TOS 444.14'



17 ADMIN. BUILDING ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN



KEY PLAN
SCALE: N.T.S.

- LEGEND
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #
- REFER TO DETAILS 1 AND 3 S205 FOR STEEL BEAM POCKET DETAIL AND METAL DECK CONNECTION TO CONCRETE SLAB AT STAIR CORES

REFER TO S212 FOR STAIR FRAMING

REFER TO S214 FOR STAIR FRAMING

REFER TO S212 FOR STAIR FRAMING

REFER TO S214 FOR STAIR FRAMING

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DRAWING TITLE :
ADMIN. BUILDING 3RD FLOOR AND ROOF FRAMING PLAN

DWG NUMBER :
S-130

ROOF HIGH	TOP OF STEEL EL:																	ROOF HIGH
	HIGH POINT 461.80 (LINES A,C,E,G,I) LOW POINT 461.34 (LINES B,D,F,H)																	
		W14X90	W14X90	W14X109	HSS14X10X5/16 (-1.51, SEE NOTE 8)	HSS12X10X5/16 (-1.50, SEE NOTE 8)	HSS12X10X1/2 (-1.67, SEE NOTE 8)	HSS14X10X1/2 (-1.67, SEE NOTE 8)	HSS12X10X3/8 (-1.50 SEE NOTE 8)	HSS14X10X3/8	W10X39	W10X39	W10X33	L6X4X5/16				
ROOF MEZZ	(TOP OF STEEL FRAMING EL 441.95)	W14X90	W14X90	W14X109	HSS14X10X5/16	HSS12X10X5/16	HSS12X10X1/2	HSS14X10X1/2	HSS12X10X3/8	HSS14X10X3/8	W10X39	W10X39	W10X33	L6X4X5/16			roof mezz	
UPPER MEZZ	(TOP OF STEEL FRAMING EL 430.75)	W14X90	W14X90	W14X109	HSS14X10X5/16	HSS12X10X5/16	HSS12X10X1/2	HSS14X10X1/2	HSS12X10X3/8	HSS14X10X3/8	W10X39	W10X39	W10X33	L6X4X5/16			UPPER MEZZ	
MEZZANINE	(TOP OF STEEL FRAMING EL 421.00)	W14X90	W14X90	W14X109	HSS14X10X5/16	HSS12X10X5/16	HSS12X10X1/2	HSS14X10X1/2	HSS12X10X3/8	HSS14X10X3/8	W10X39	W10X39	W10X33	L6X4X5/16			MEZZANINE	
BASE SLAB	(TOP OF SLAB ELEVATION 408.5)	W14X90	W14X90	W14X109	HSS14X10X5/16	HSS12X10X5/16	HSS12X10X1/2	HSS14X10X1/2	HSS12X10X3/8	HSS14X10X3/8	W10X39	W10X39	W10X33	L6X4X5/16			BASE	
BASEPLATE (EL. 407.00 - BOTTOM OF PLATE)		24"x24"x1"	24"x24"x1"	20"x32"x1.5" (P2) 20"x48"x2" (P3)	22"x22"x0.75"	20"x20"x0.75"	18"x18"x1.25"	18"x18"x1.25"	20"x20"x0.875"	22"x22"x0.875"	20"x32"x1.5" (P2) 20"x48"x2" (P3)	20"x20"x0.75"	22"x22"x0.875"	20"x20"x0.75"	20"x20"x0.75"	20"x20"x0.75"		
COLUMN MARK		C1	C2	C3	C4	C5	C6	C7	C8	C9	C10	C11	C12	C13	C14	C15	C16	

ASRS BUILDING COLUMN SCHEDULE

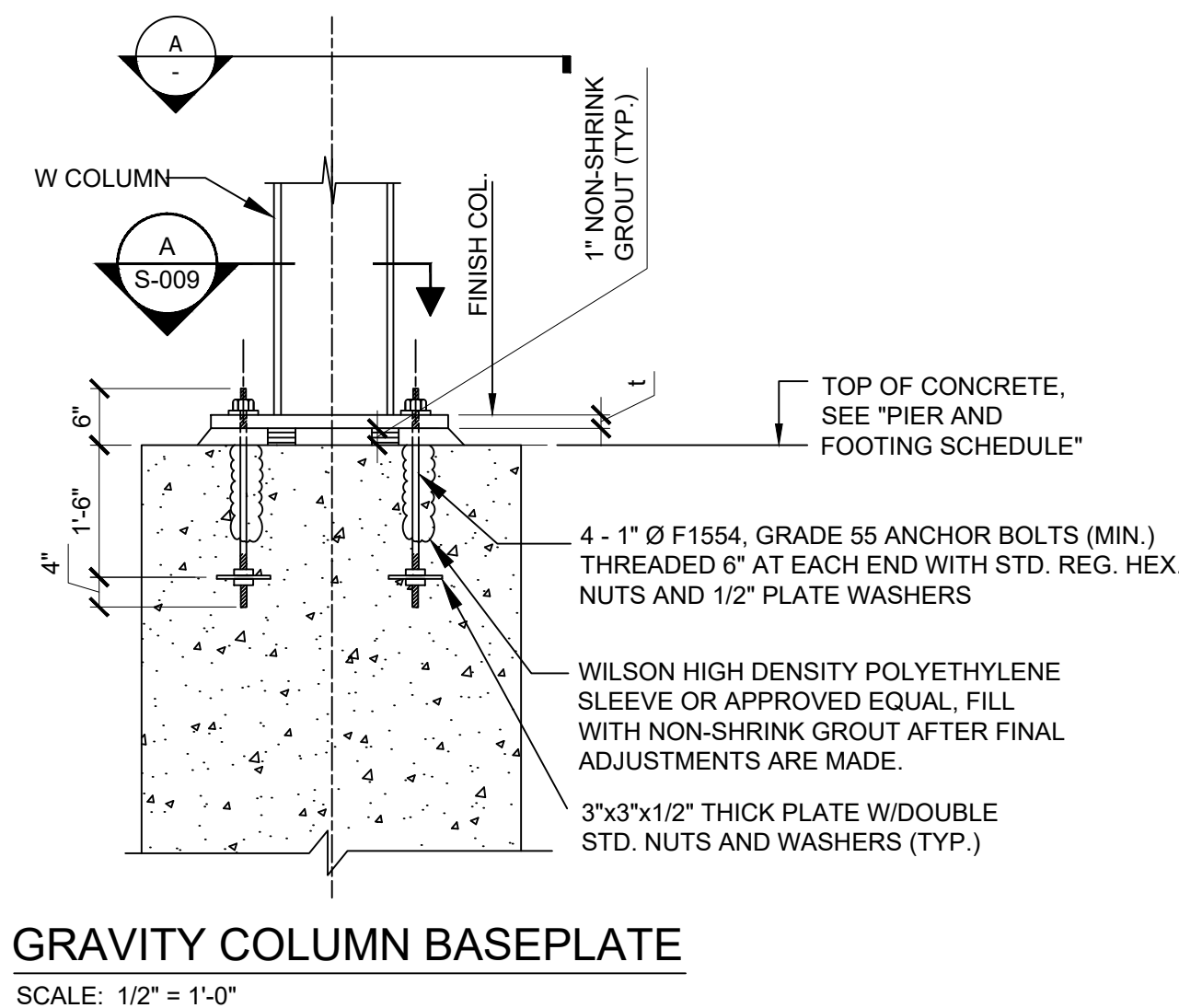
SCALE: NTS

ROOF	TOP OF STEEL EL:																				
	HIGH POINT 458.43 (LINES Ap2, Cp) LOW POINT 457.97 (LINES Bp, Bp2)																				
3RD FLOOR	(TOP OF STEEL FRAMING EL 444.14)	W14X61	W14X90	W14X82	W14X82	W14X43	W14X90	W14X90	W10X33	W10X39	W12X50										
PARKING	(TOP OF STEEL FRAMING EL 429.50)	W14X90	W14X43	W14X145	W14X145	W14X120	W14X159	W14X99	W14X145	W14X132	W14X145	W12X65	W14X99	W14X233	W12X72	W14X257	W14X120	W14X159	W10X33	W10X39	W12X50
BASE		21"x21"x1.5"	18x18"x1"	24"x24"x1.5"	24"x24"x2"	24"x24"x1.625"	24"x24"x1.25"	24"x24"x1.5"	24"x24"x2"	20"x32"x1.5" (P5) 20"x48"x2" (P6)	20"x32"x1.5" (P2) 20"x48"x2" (P3)	20"x20"x1.25"	24"x24"x1.375"	24"x24"x2.5"	20"x20"x1.25"	24"x24"x2.75"	24"x24"x1.25"	24"x24"x1.75"	CP18	CP19	CP20

*SEE FOUNDATION PLANS FOR TOP OF FOOTING ELEVATIONS AT THE PARKING STRUCTURE. BOTTOM OF BASE PLATE ELEVATIONS ARE 1" ABOVE THE TOP OF FOOTING ELEVATIONS.

PARKING AREA COLUMN SCHEDULE

SCALE: NTS



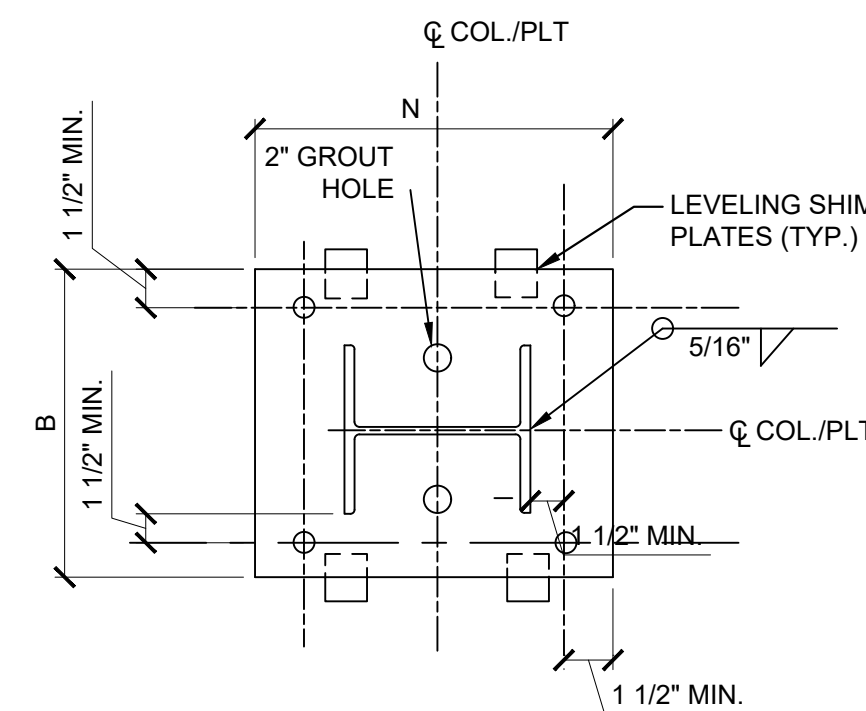
GRAVITY COLUMN BASEPLATE

SCALE: 1/2" = 1'-0"

- NOTES:
- FOR BASE PLATE DIMENSIONS B, N AND L, SEE COLUMN SCHEDULE.
 - FOR SIZE OF COLUMN, SEE COLUMN SCHEDULE; FOR ORIENTATION, SEE FRAMING PLANS.
 - WASHER PLATES SHALL HAVE HOLES WHICH ARE 1/16" LARGER THAN THE BOLT DIAMETER.
 - HOLE DIAMETER SHALL BE EQUAL TO "BOLT DIAMETER + 5/16"
 - GROUT SHALL COMPLETELY FILL THE SPACE UNDERNEATH THE PLATE.
 - WASHER MATERIAL TO MEET ASTM A36.
 - FOR CLARITY, FIREPROOFING IS NOT SHOWN.
 - TOP OF COLUMNS NOTED ARE LOWER THAN THE TYPICAL TOP OF STEEL BY THE DIMENSION SHOWN (-XX.X FEET)

1/2" PLATE WASHER

SCALE: 1/2" = 1'-0"



PLAN VIEW

SCALE: 1/2" = 1'-0"

- NOTES:
- HSS POST BASEPLATE DETAIL SIMILAR.

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DRAWING TITLE :

COLUMN SCHEDULE

DWG NUMBER :

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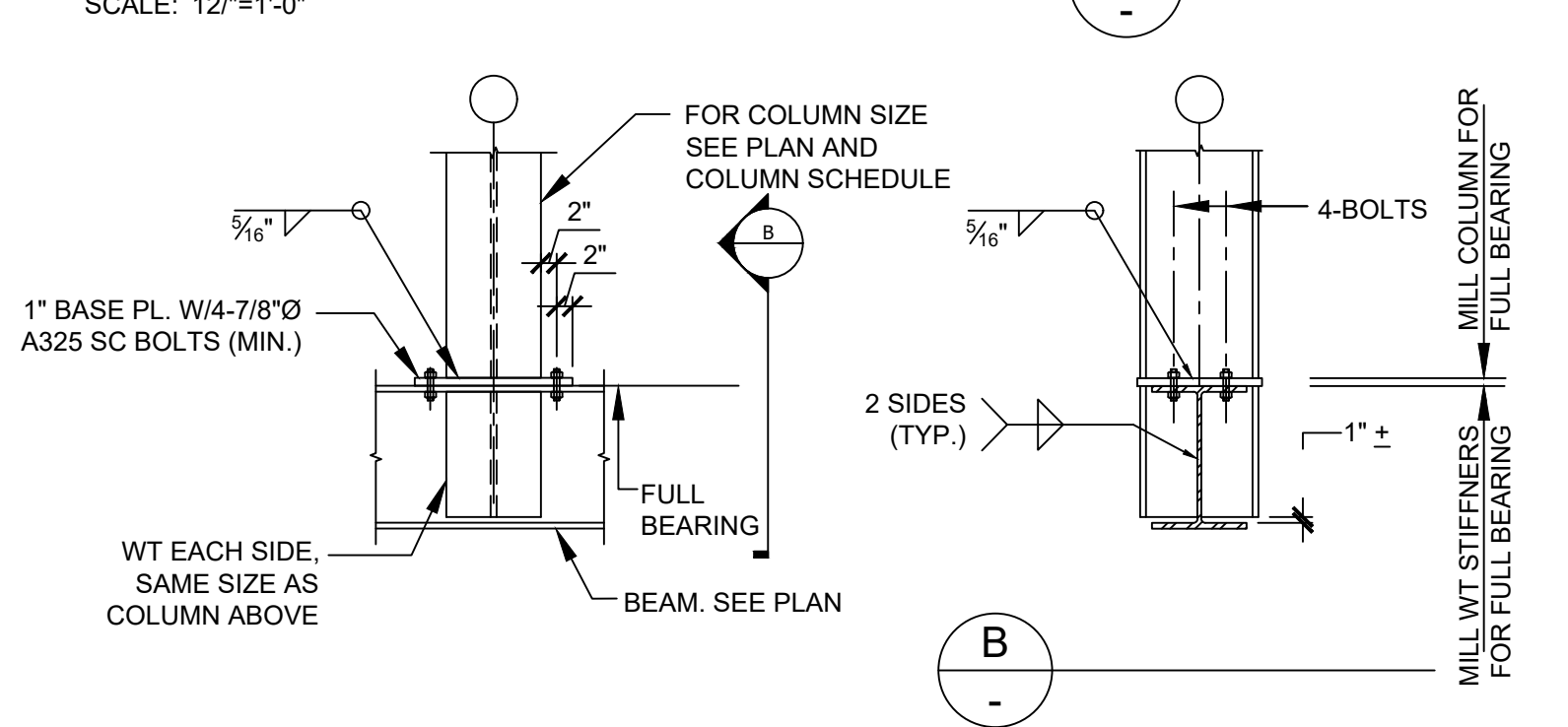
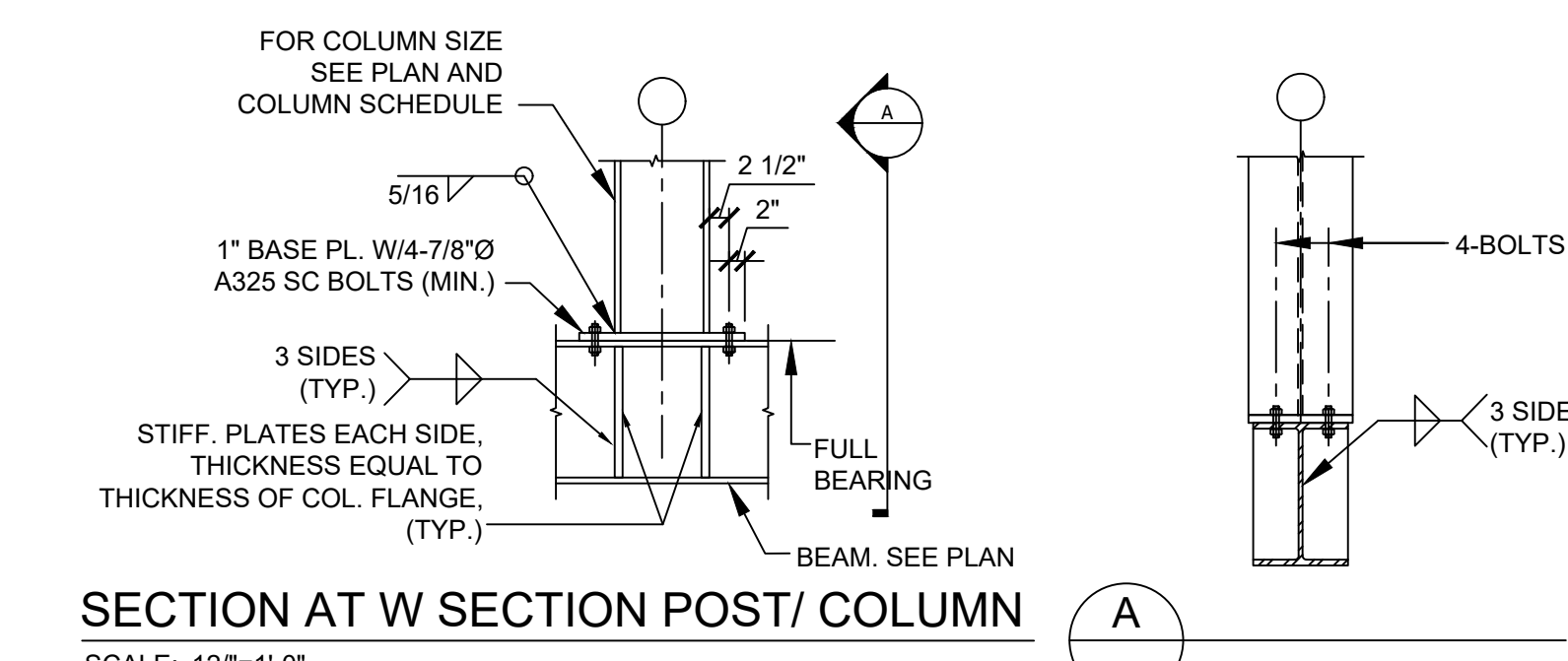
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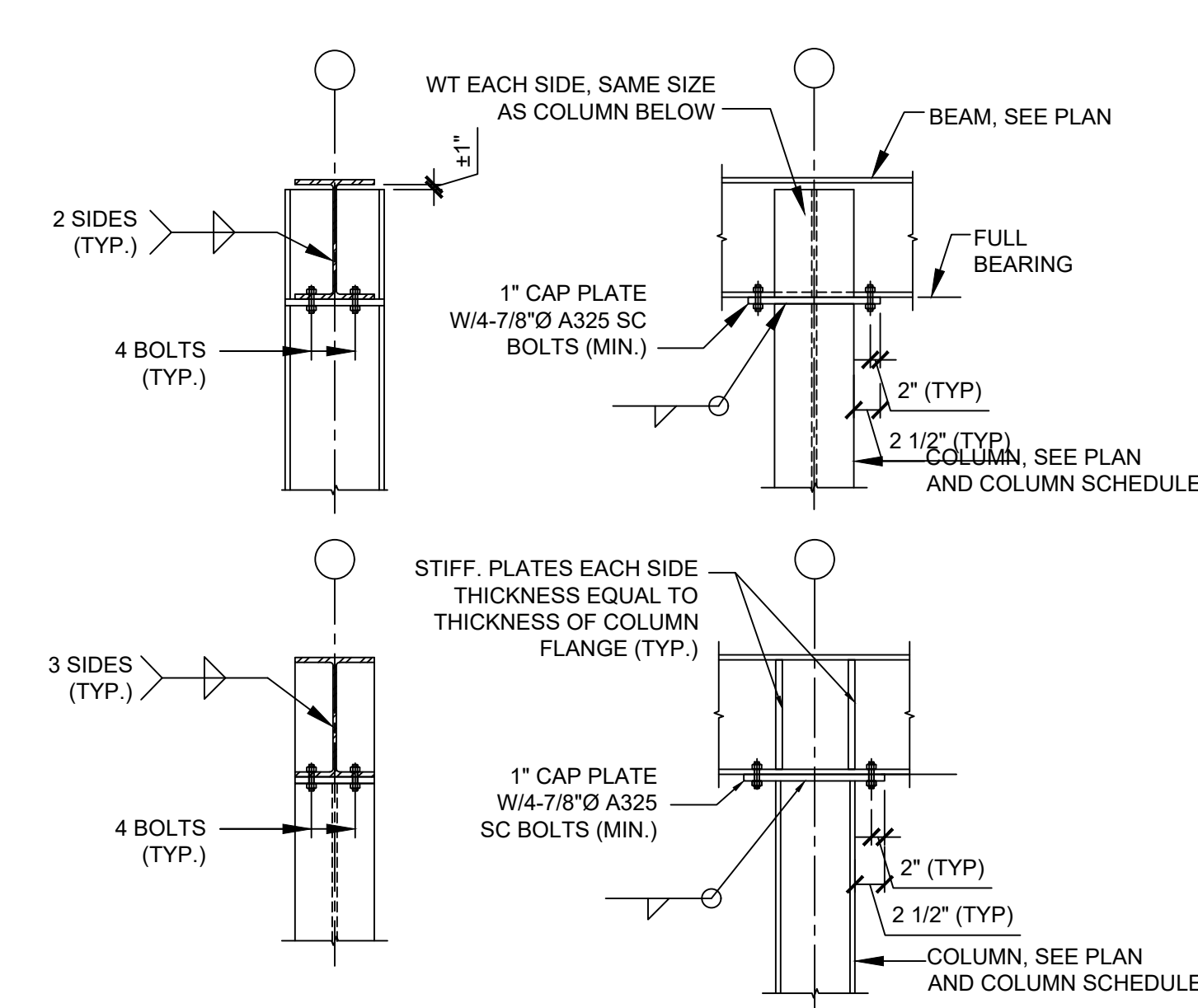
STEEL BEAM AND COLUMN
DETAILS

DWG NUMBER :

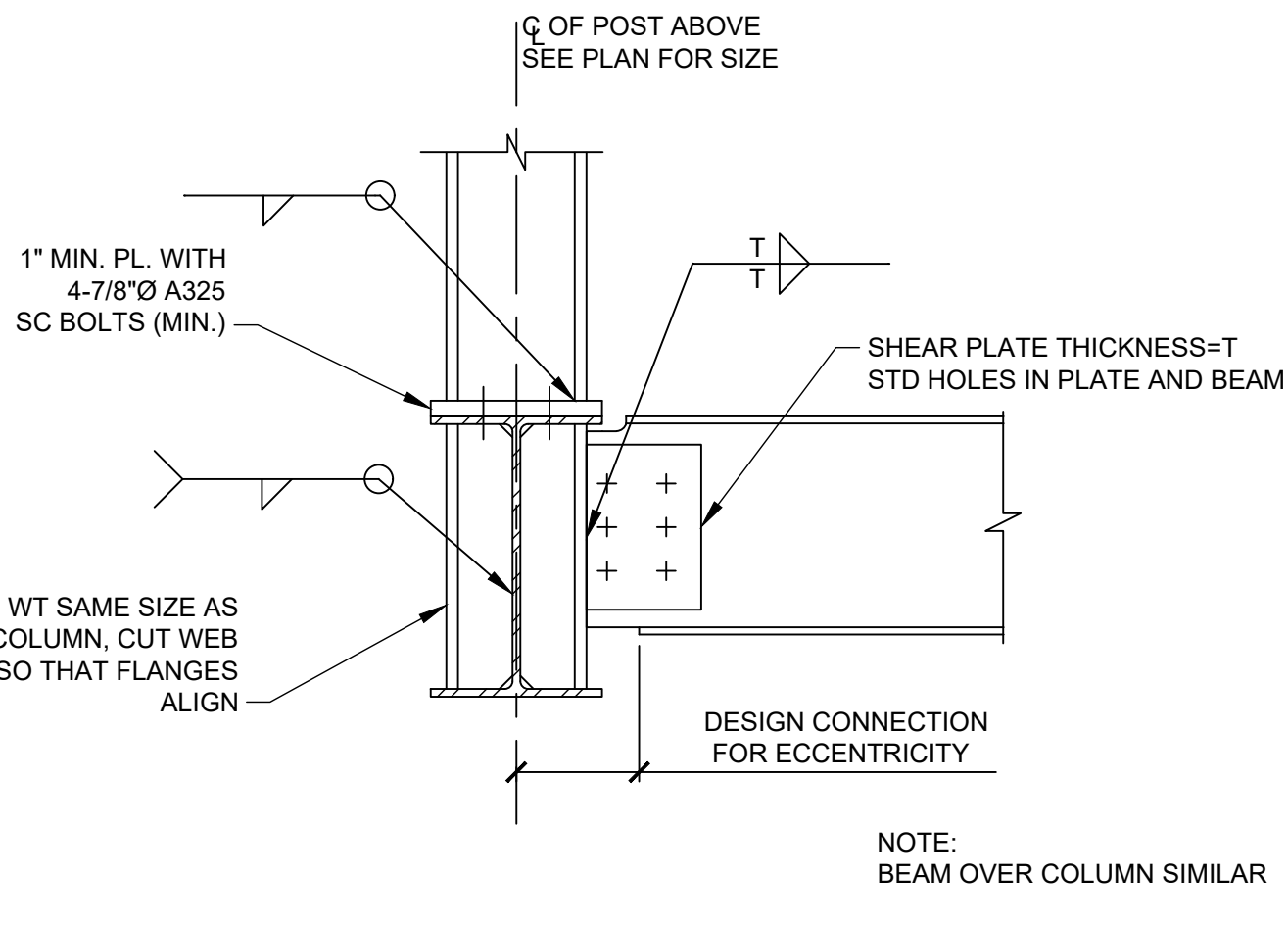
S-201



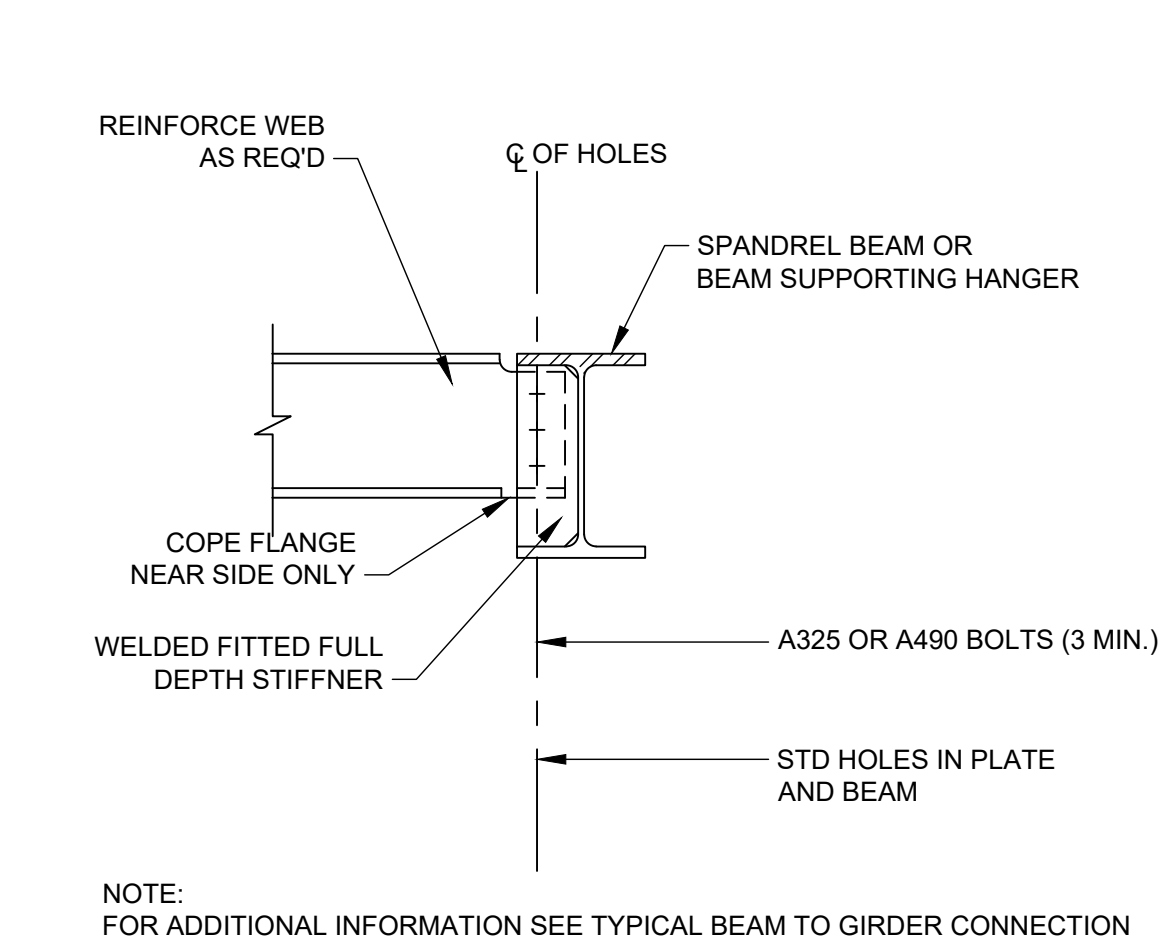
TYP. COLUMN OR POST SUPPORTED ON BEAM
SCALE: 1 1/2" = 1'-0"



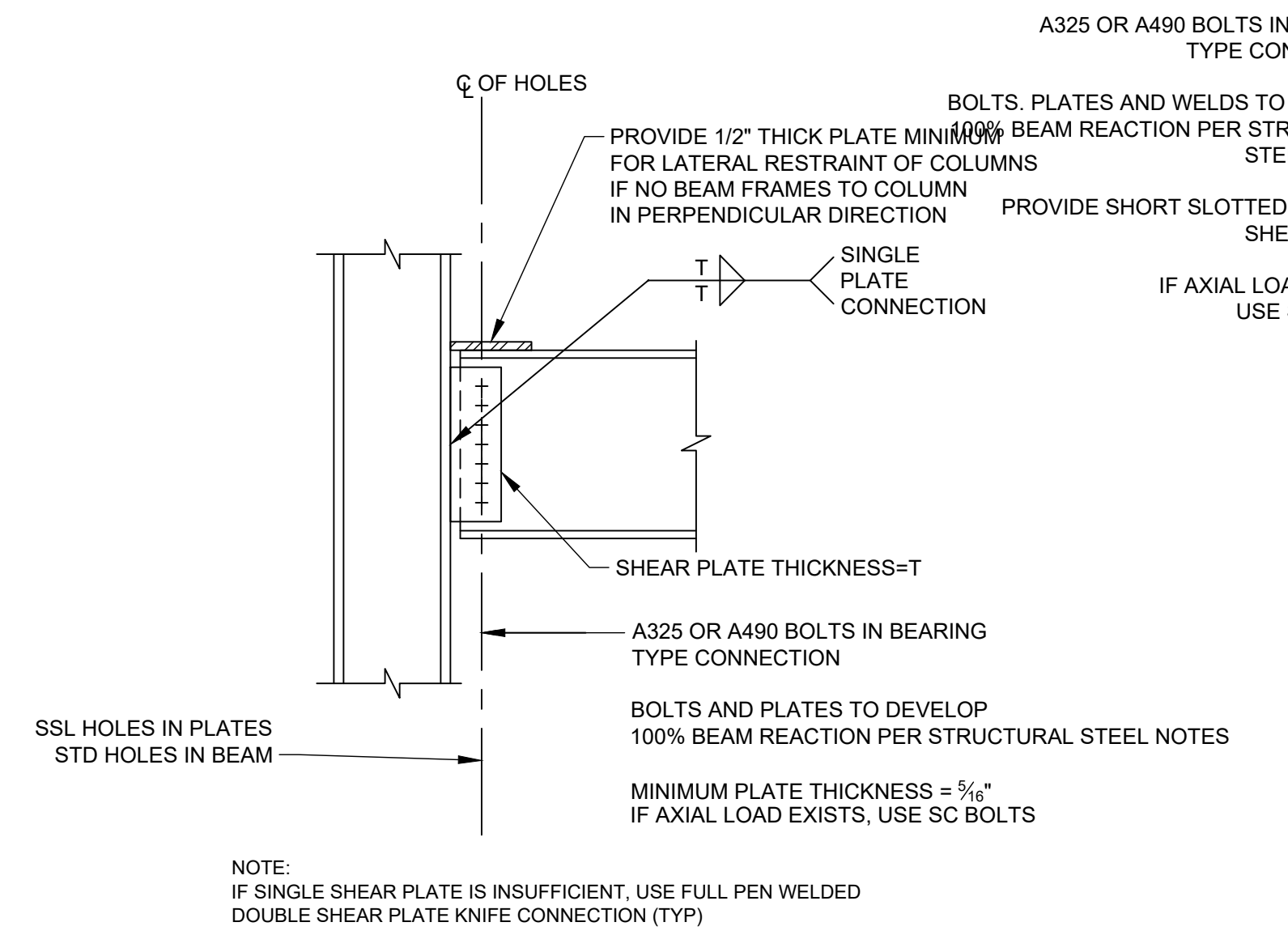
TYP. BEAM SUPPORTED OVER COLUMN
SCALE: 1 1/2" = 1'-0"



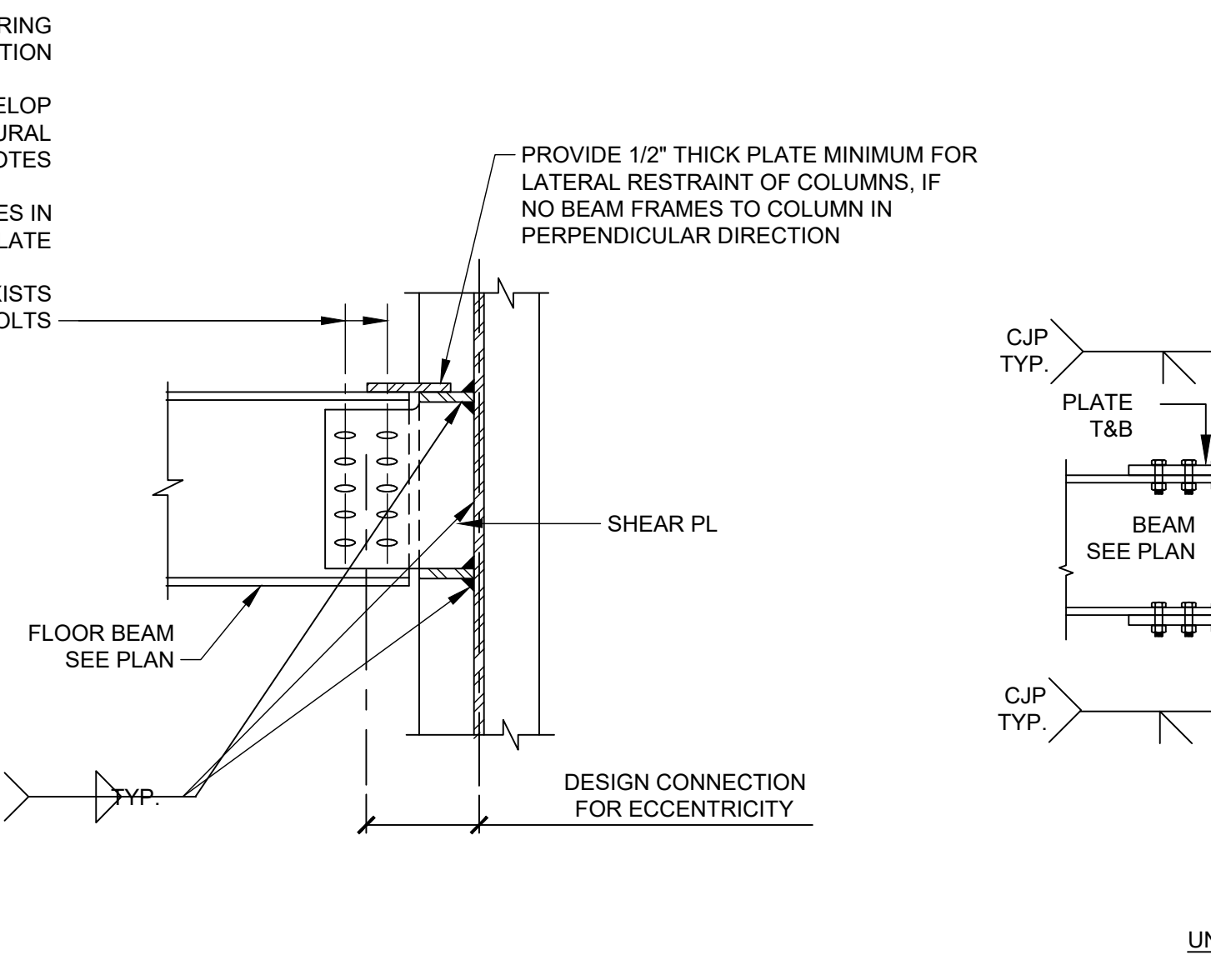
TYP. COLUMN SUPPORT WITH WEB PERPENDICULAR TO SUPPORT BEAM
SCALE: 3/4" = 1'-0"



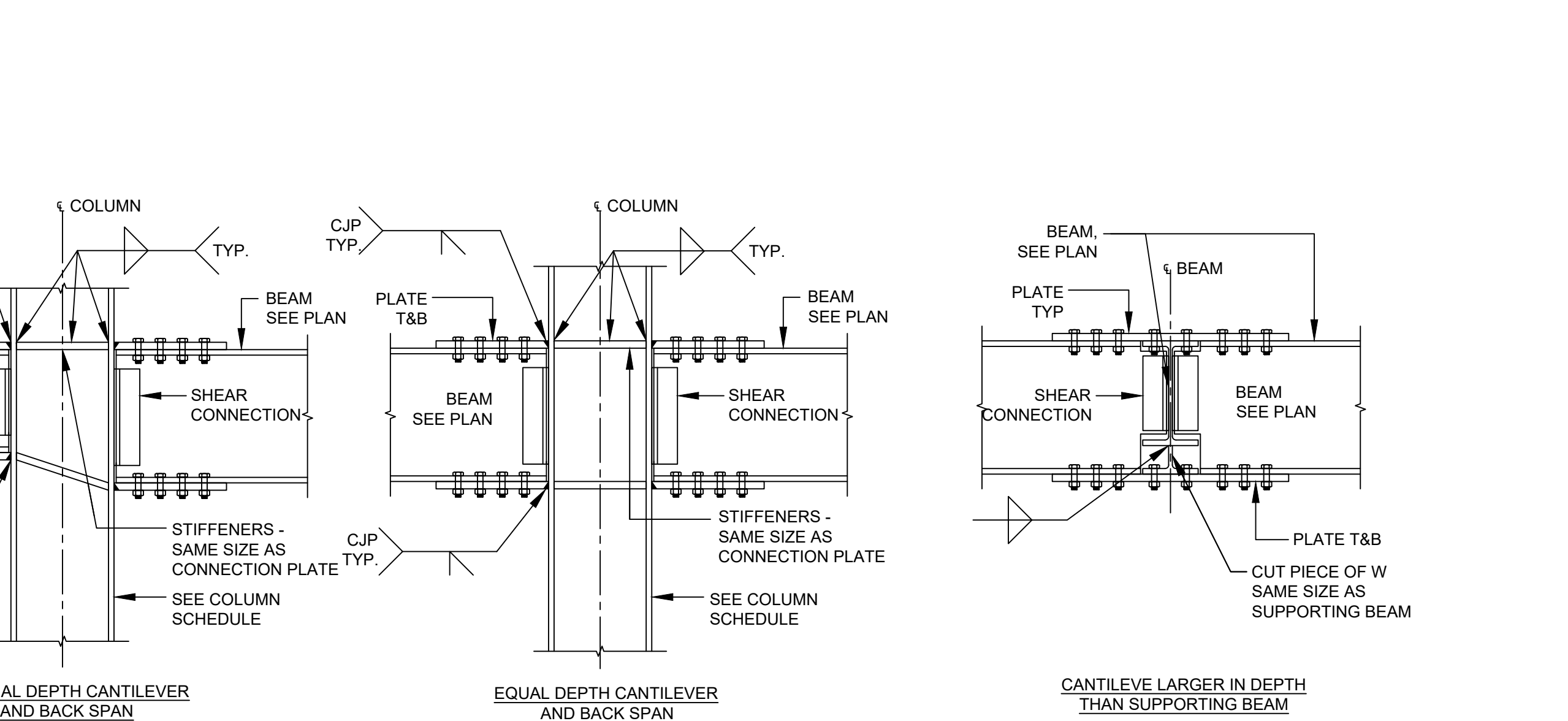
TYP. END CONNECTION AT BEAMS SUPPORTING STAIR HANGERS AT SPANDRELS
SCALE: 3/4" = 1'-0"



TYP. BEAM TO COLUMN FLANGE SHEAR CONNECTION
SCALE: 3/4" = 1'-0"

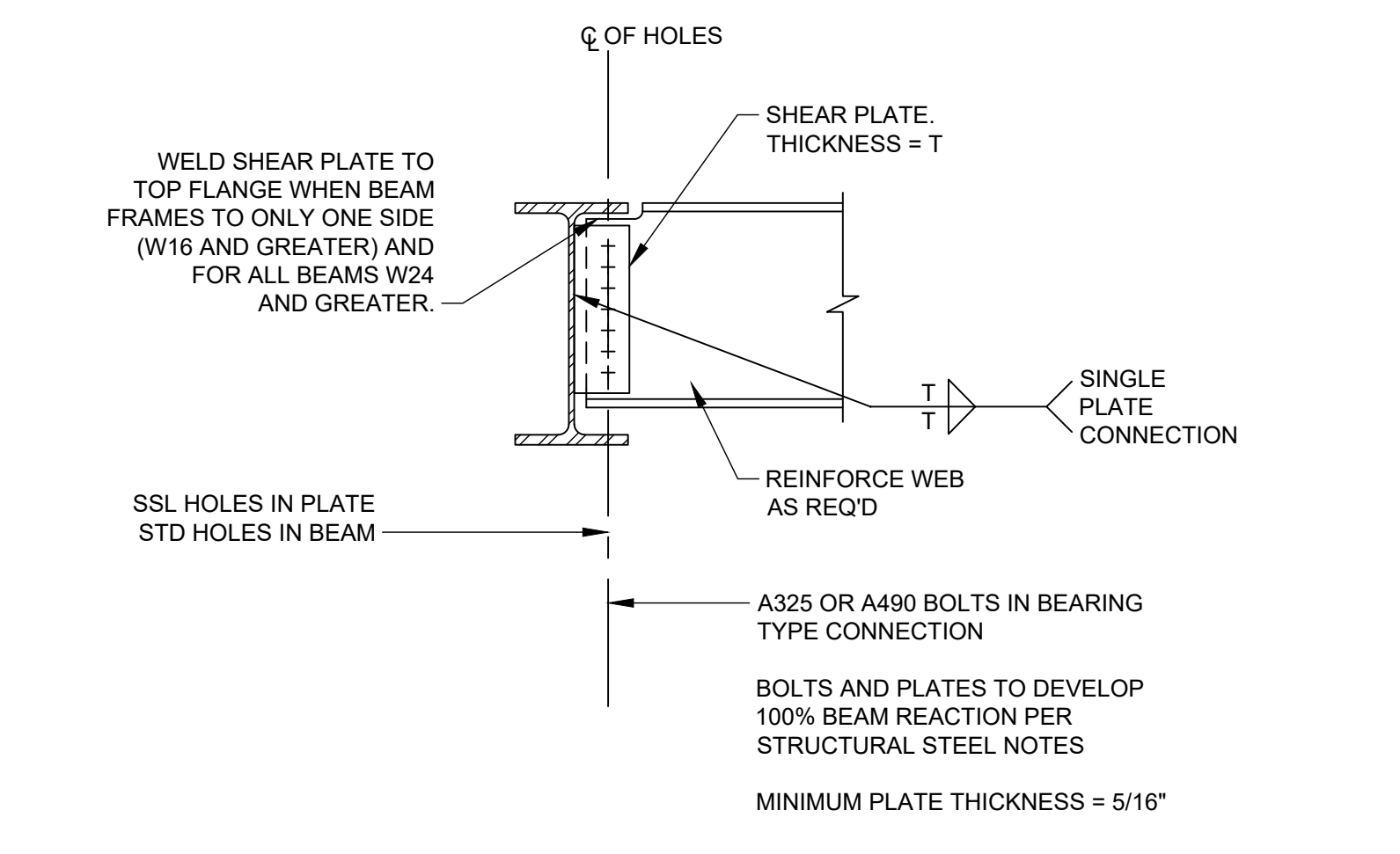


TYP. BEAM TO COLUMN WEB SHEAR CONNECTION
SCALE: 3/4" = 1'-0"

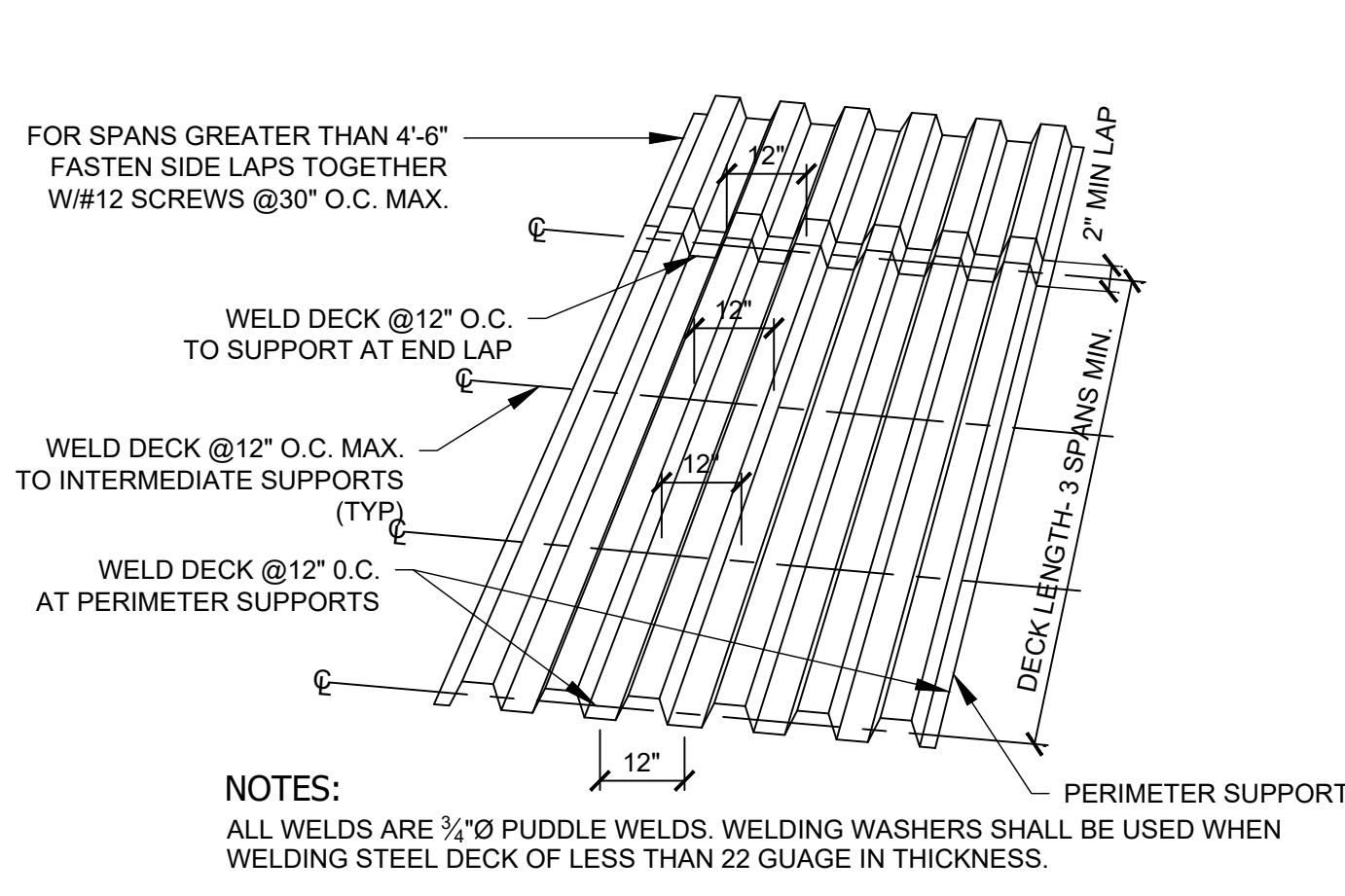


TYPICAL CANTILEVERED BEAM CONNECTION
SCALE: N.T.S.

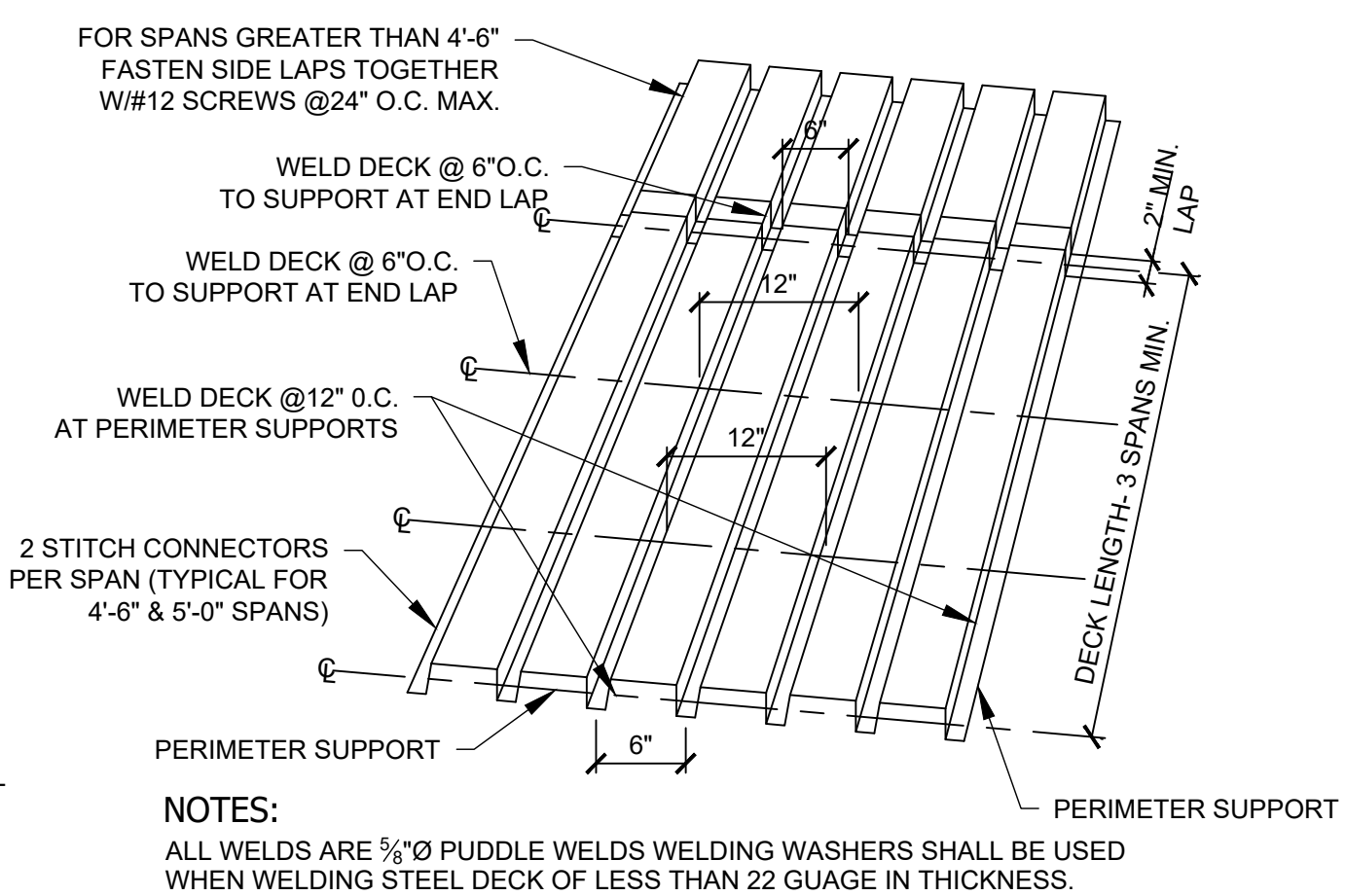
- NOTES:
- MOMENT CONNECTIONS ARE TO DEVELOP FULL MOMENT CAPACITY OF CANTILEVER BEAMS UNLESS SHOWN OTHERWISE ON PLAN.
 - WELD, BOLT AND CONNECTION PLATE CAPACITIES SHALL BE DESIGNED AS PER AISC SPECIFICATIONS.
 - CONNECTION PLATES AND BOLTS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN NY STATE. CALCULATIONS SHALL BE SUBMITTED FOR THE ENGINEER OF RECORD'S REVIEW.
 - FIELD WELDED CONNECTIONS MAY BE USED IF REQUESTED BY CONTRACTOR AND APPROVED BY ENGINEER OF RECORD.
 - CJP INDICATES COMPLETE JOINT PENETRATION GROOVE WELDING.
 - FOR CLARITY, FIREPROOFING IS NOT INDICATED.
 - REFER TO PLANS FOR LOCATION OF CANTILEVERED BEAMS.
 - PROVIDE DOUBLE PLATE @ COLUMN B-1 & C-1 AT SECOND FLOOR AND THIRD FLOOR LEVEL.



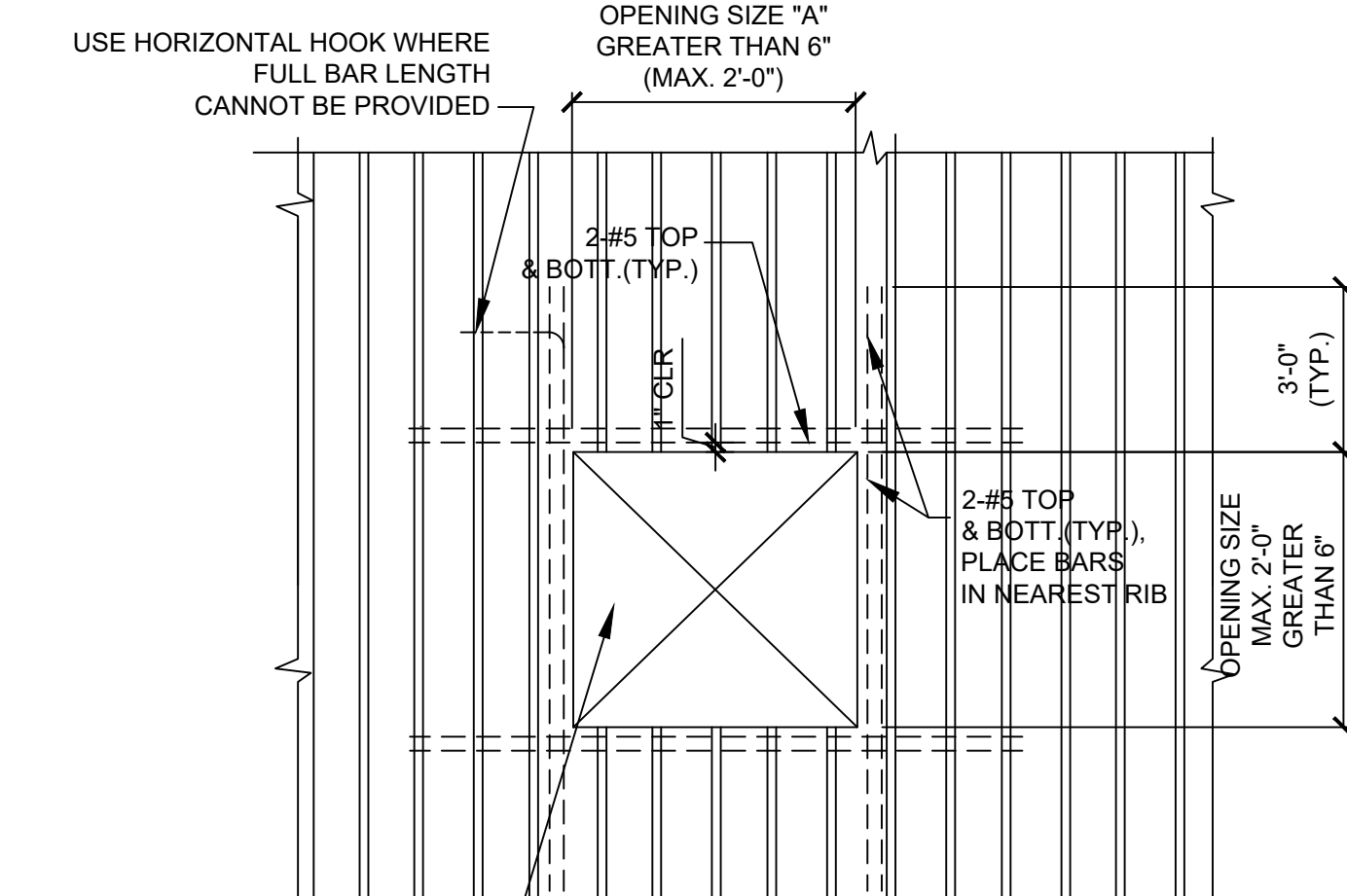
TYP. BEAM TO GIRDER CONNECTION
SCALE: 3/4" = 1'-0"



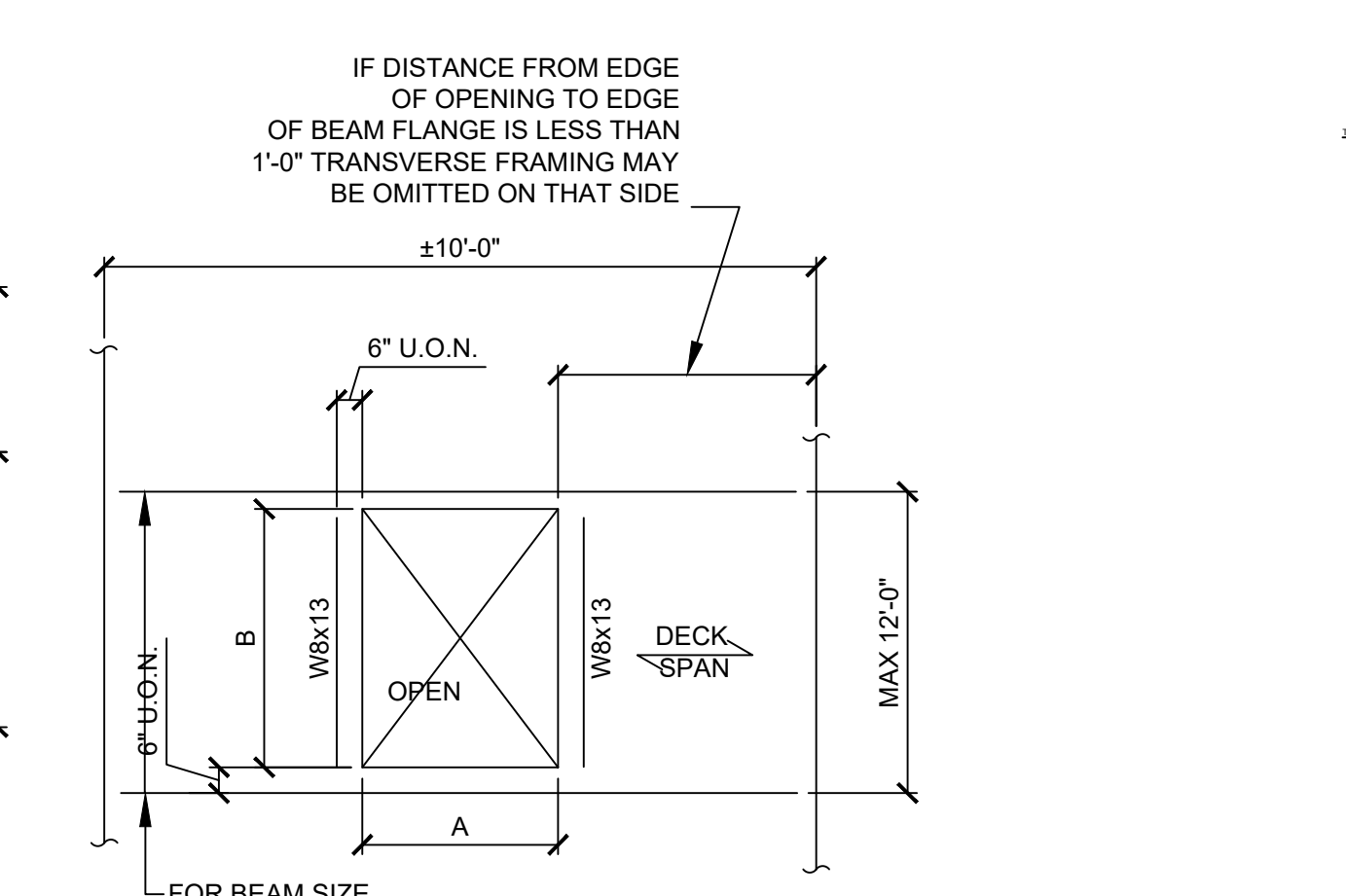
1 TYP. SLAB DECK WELDING
SCALE: 3/4"=1'-0"



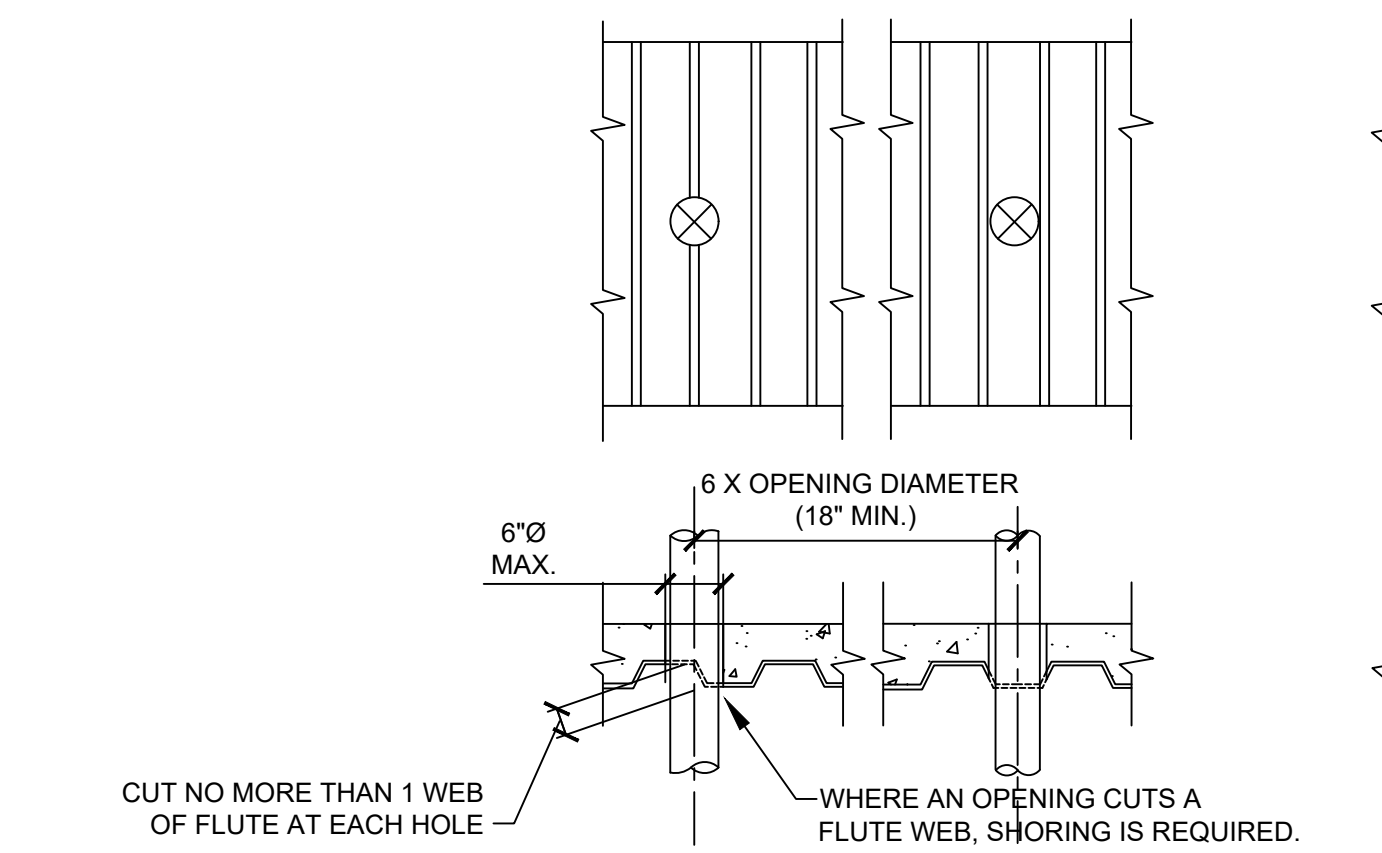
2 TYP. SLAB ROOF WELDING
SCALE: 3/4"=1'-0"



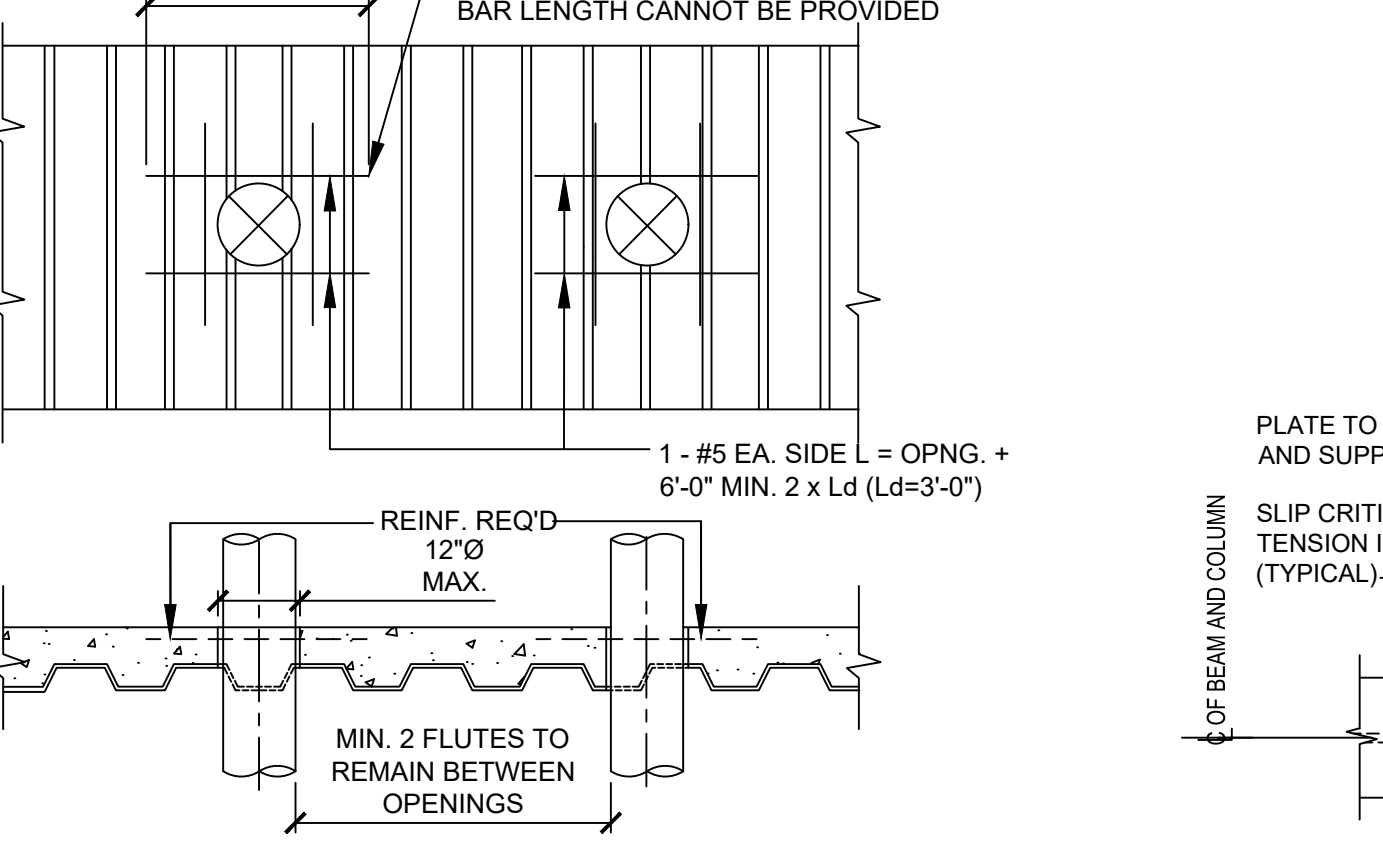
4 DECK OPENING
SCALE: 1/2"=1'-0"



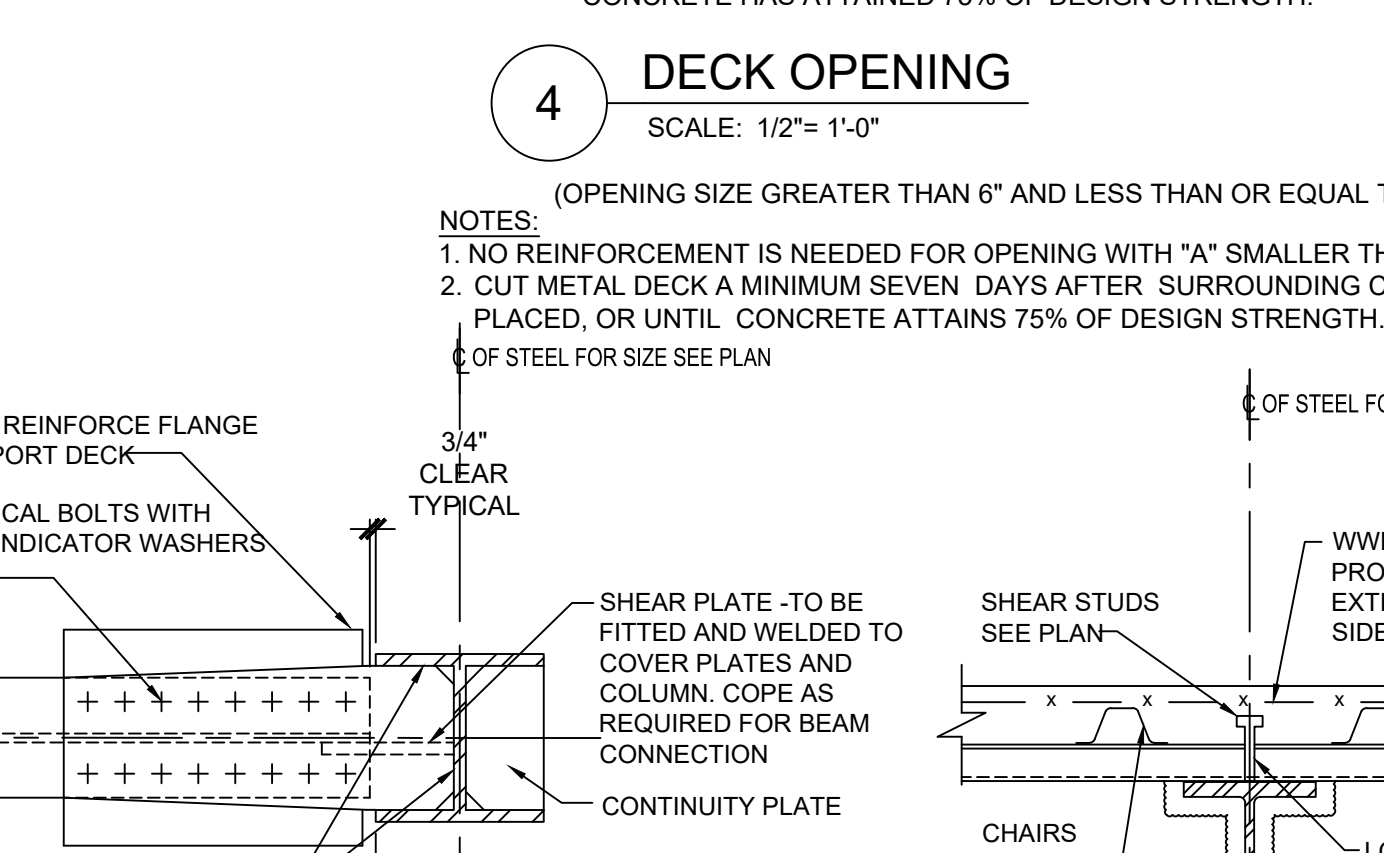
5 DECK OPENING
SCALE: 1/2"=1'-0"



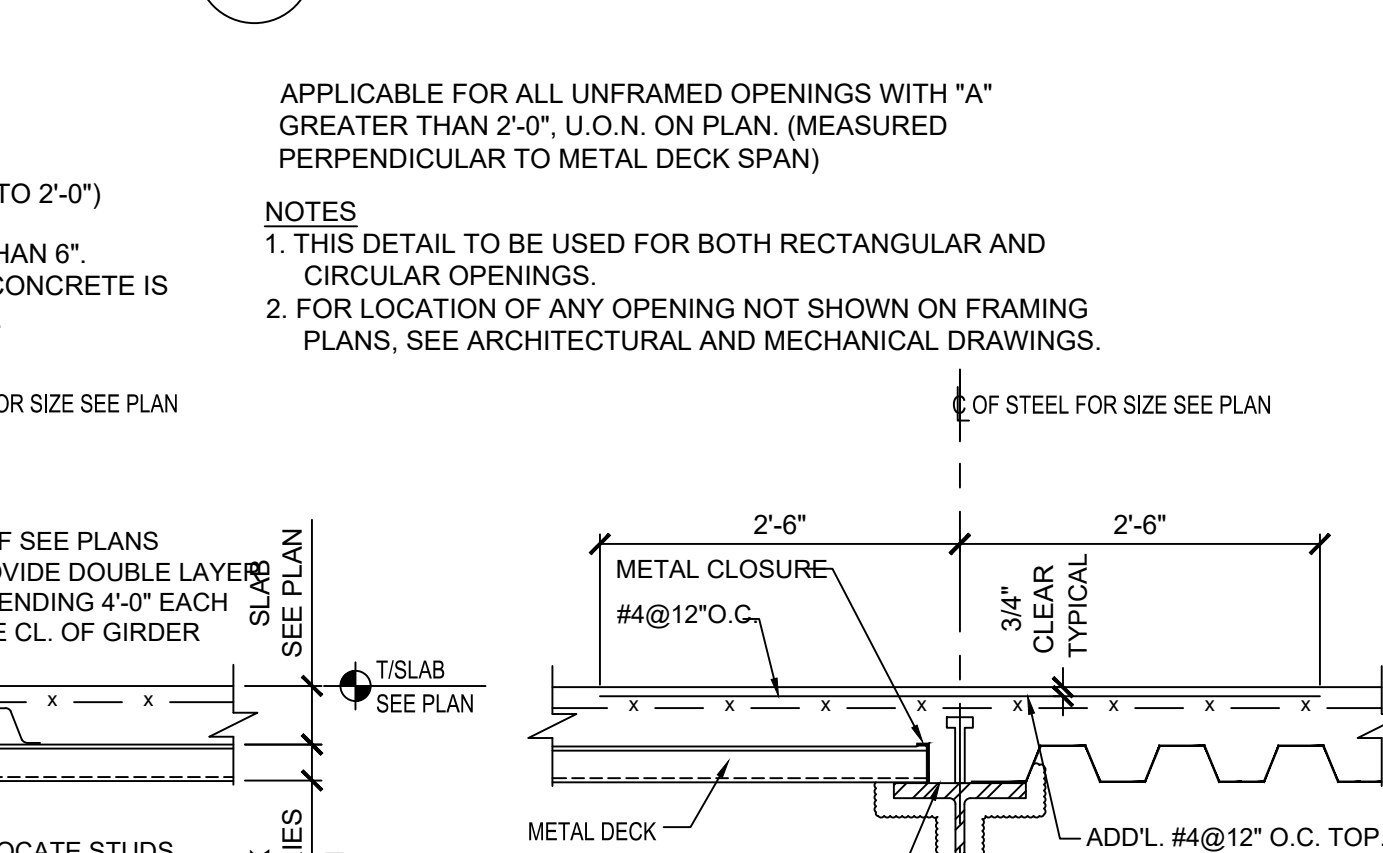
3 TYP. METAL DECK OPENING
SCALE: 3/4"=1'-0"



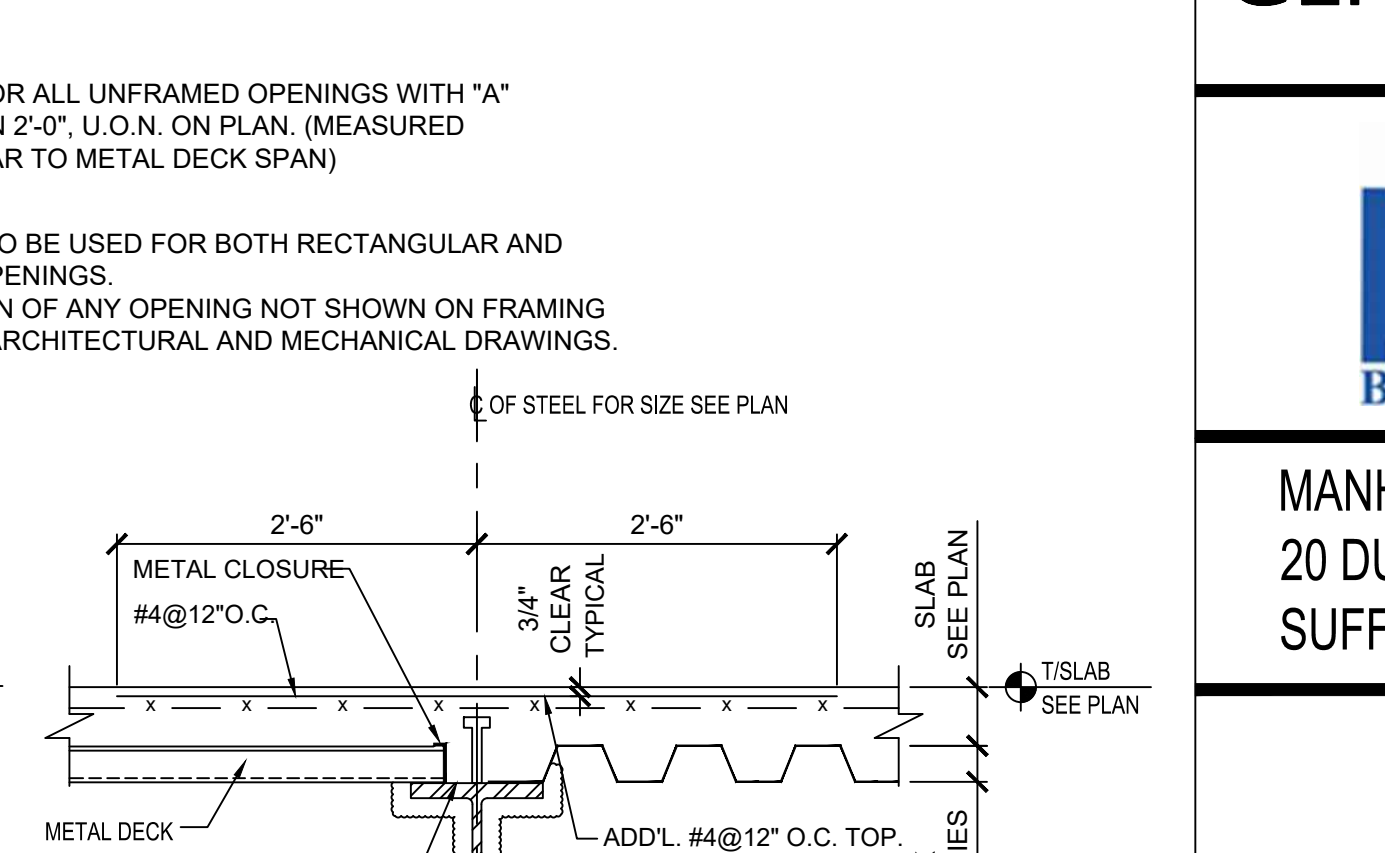
4 DECK OPENING
SCALE: 3/4"=1'-0"



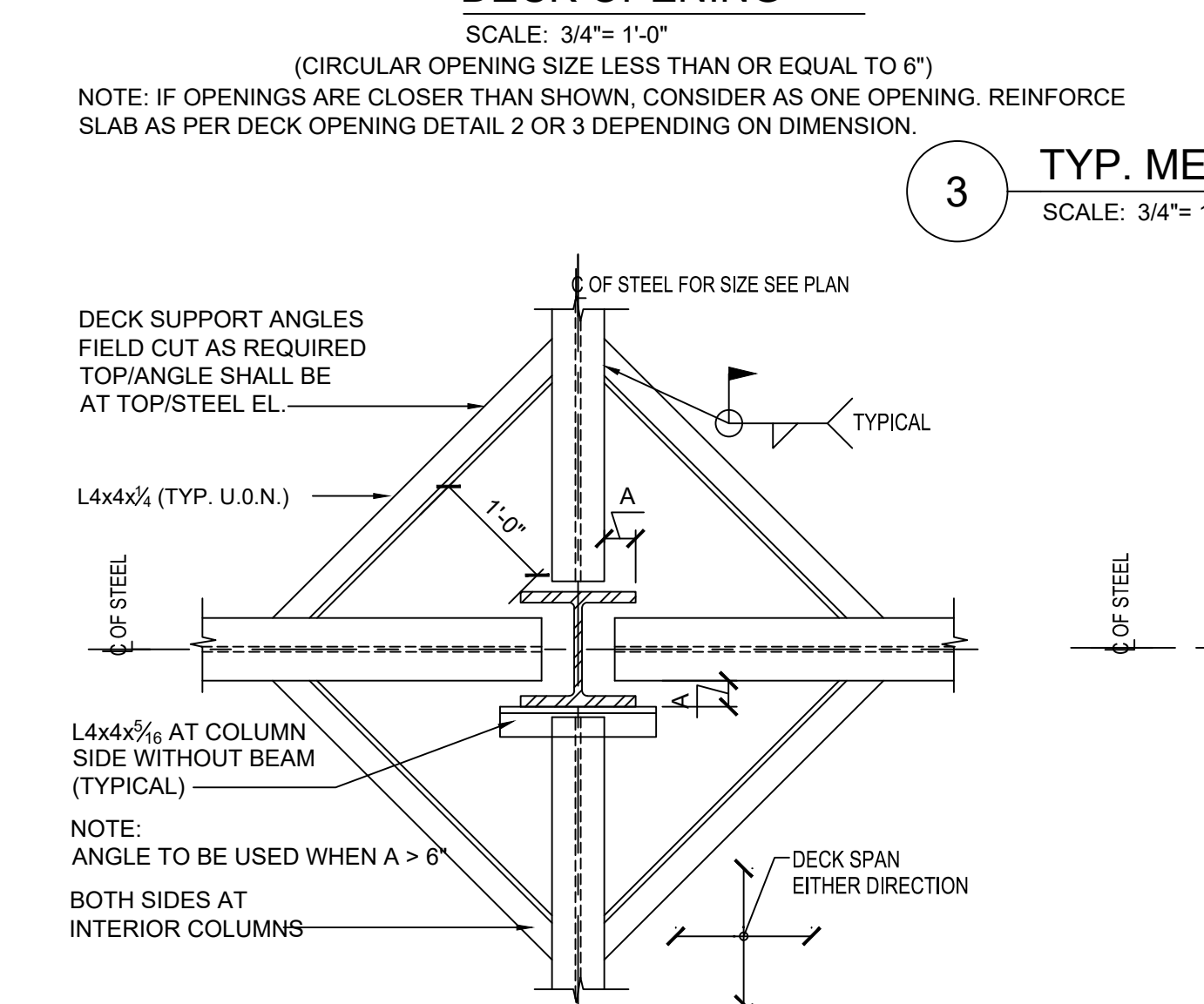
5 TYPICAL DECK SUPPORT AT BEAM MOMENT CONNECTION TO COLUMN WEB
SCALE: 3/4"=1'-0"



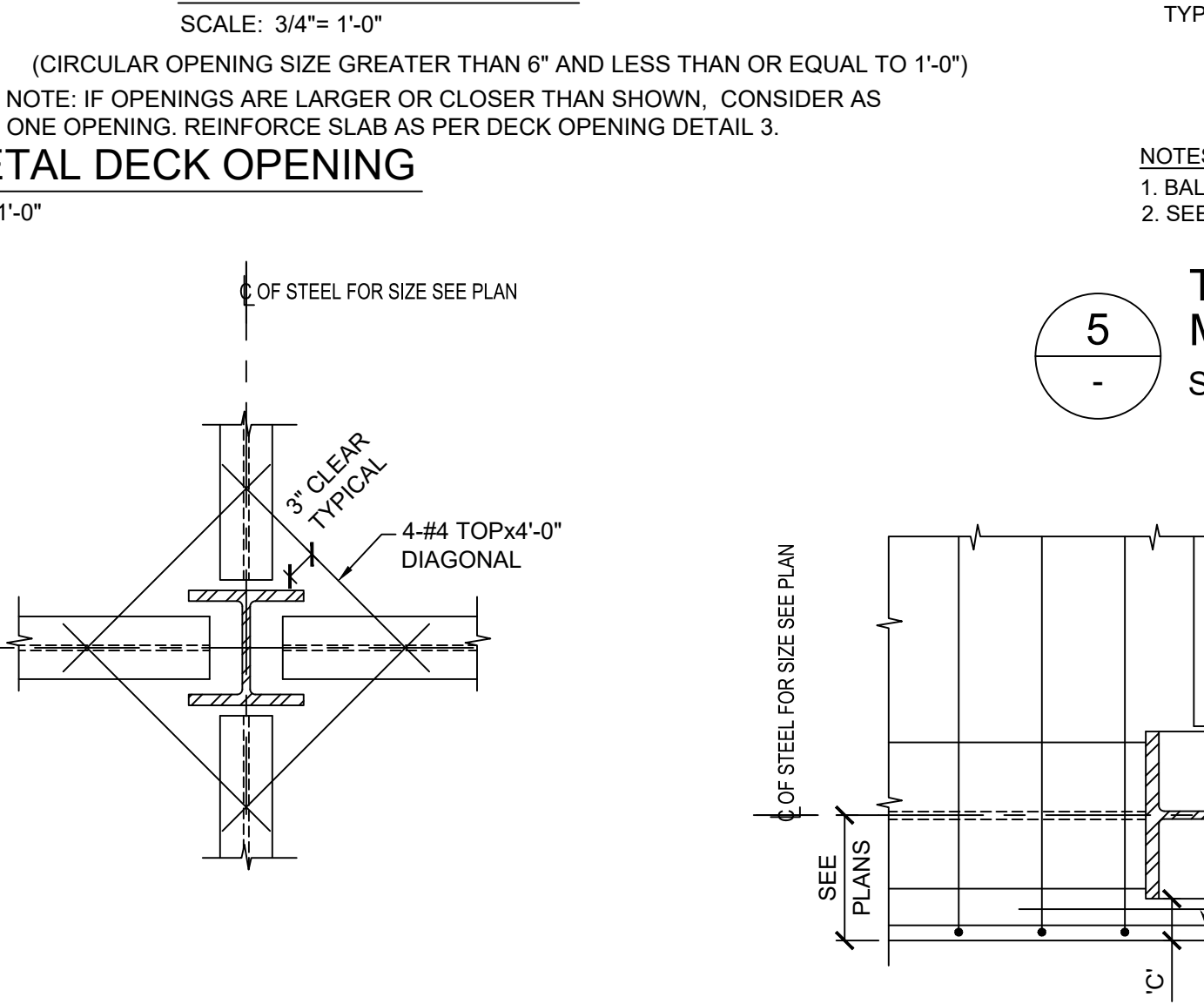
10 TYPICAL FLOOR FRAMING
SCALE: 3/4"=1'-0"



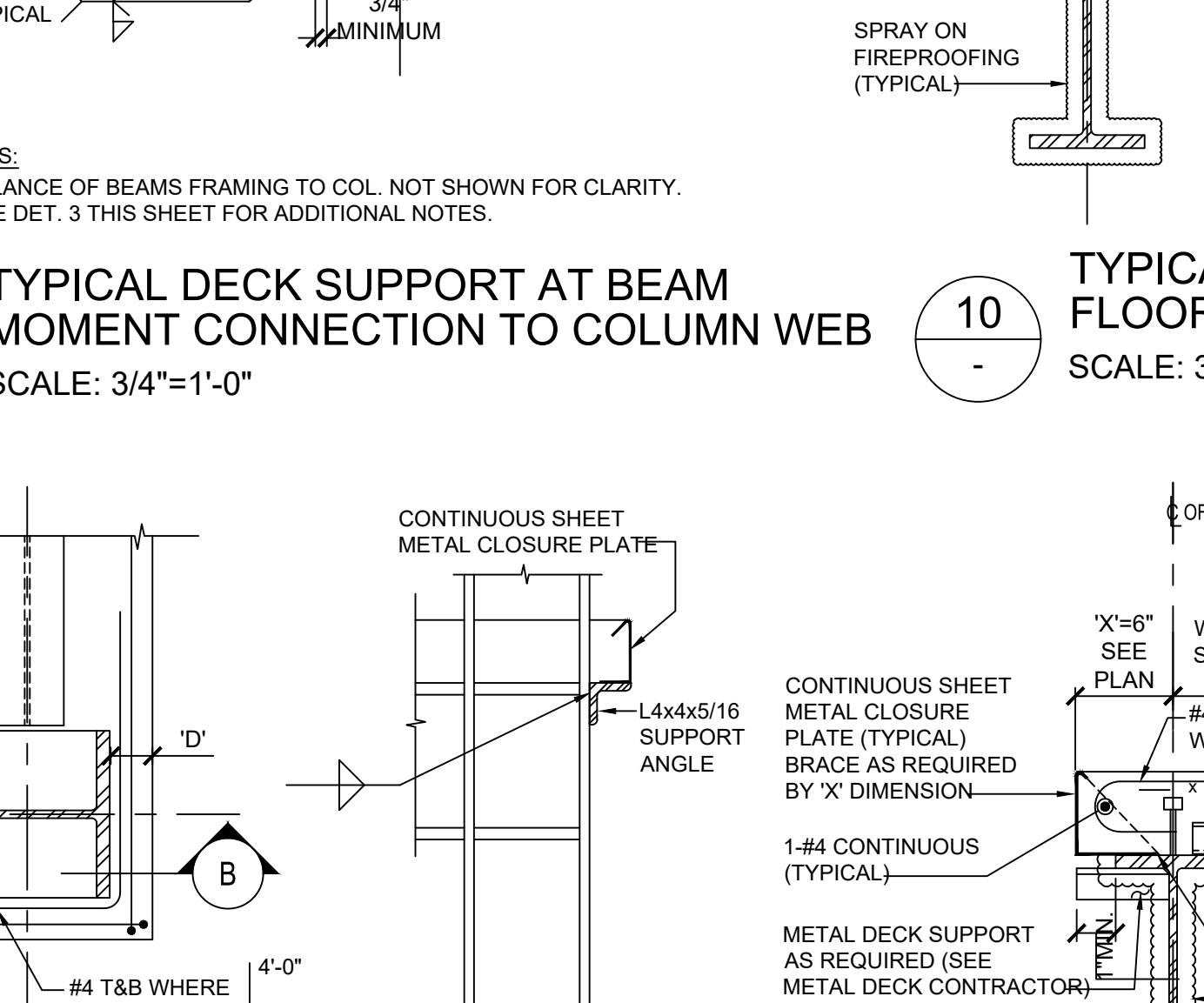
11 TYPICAL REINFORCING OVER GIRDERS OR DIRECTION CHANGE OF METAL DECK
SCALE: 3/4"=1'-0"



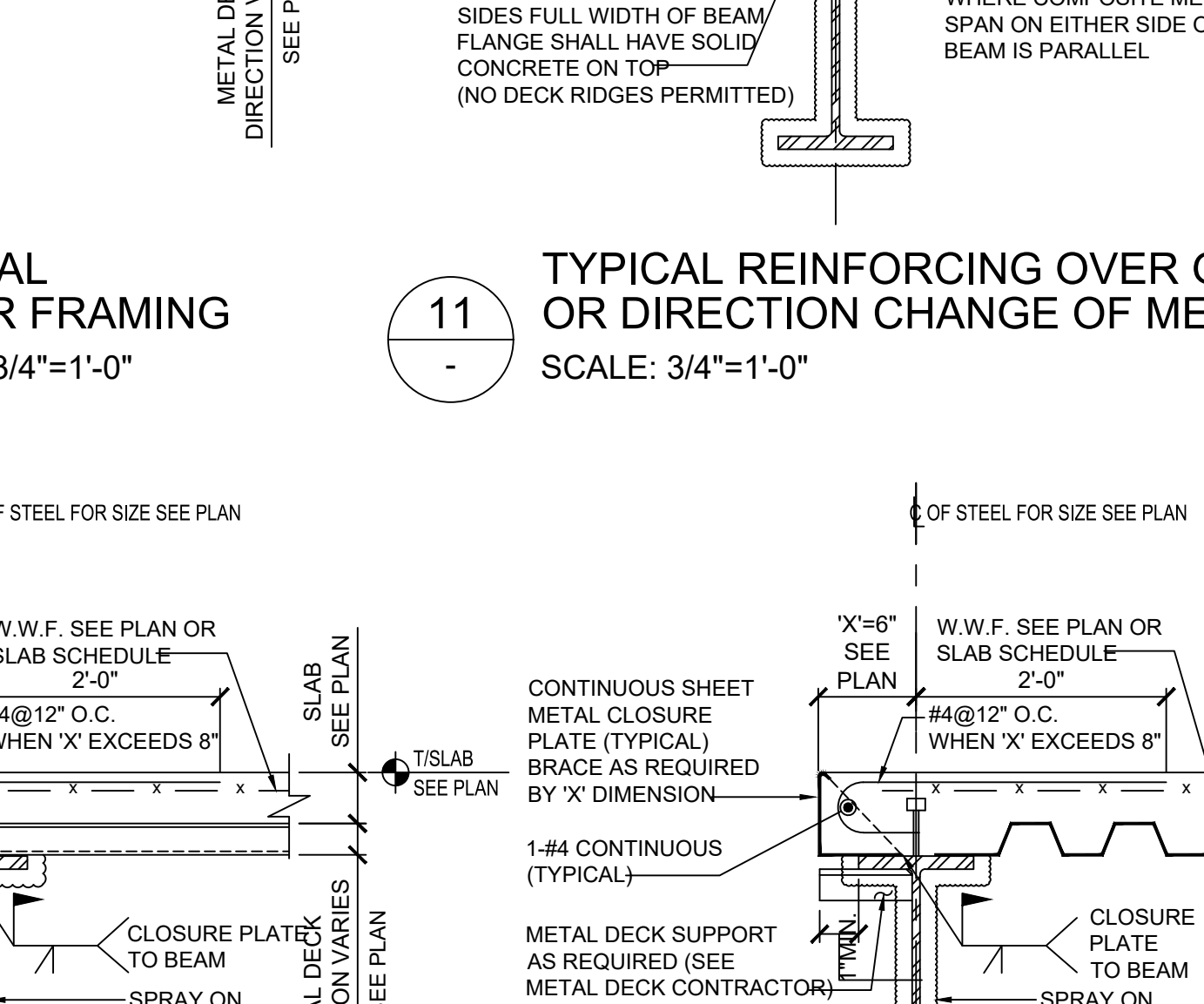
3 TYPICAL DECK SUPPORT AT INTERIOR OR EXTERIOR COLUMNS
SCALE: 3/4"=1'-0"



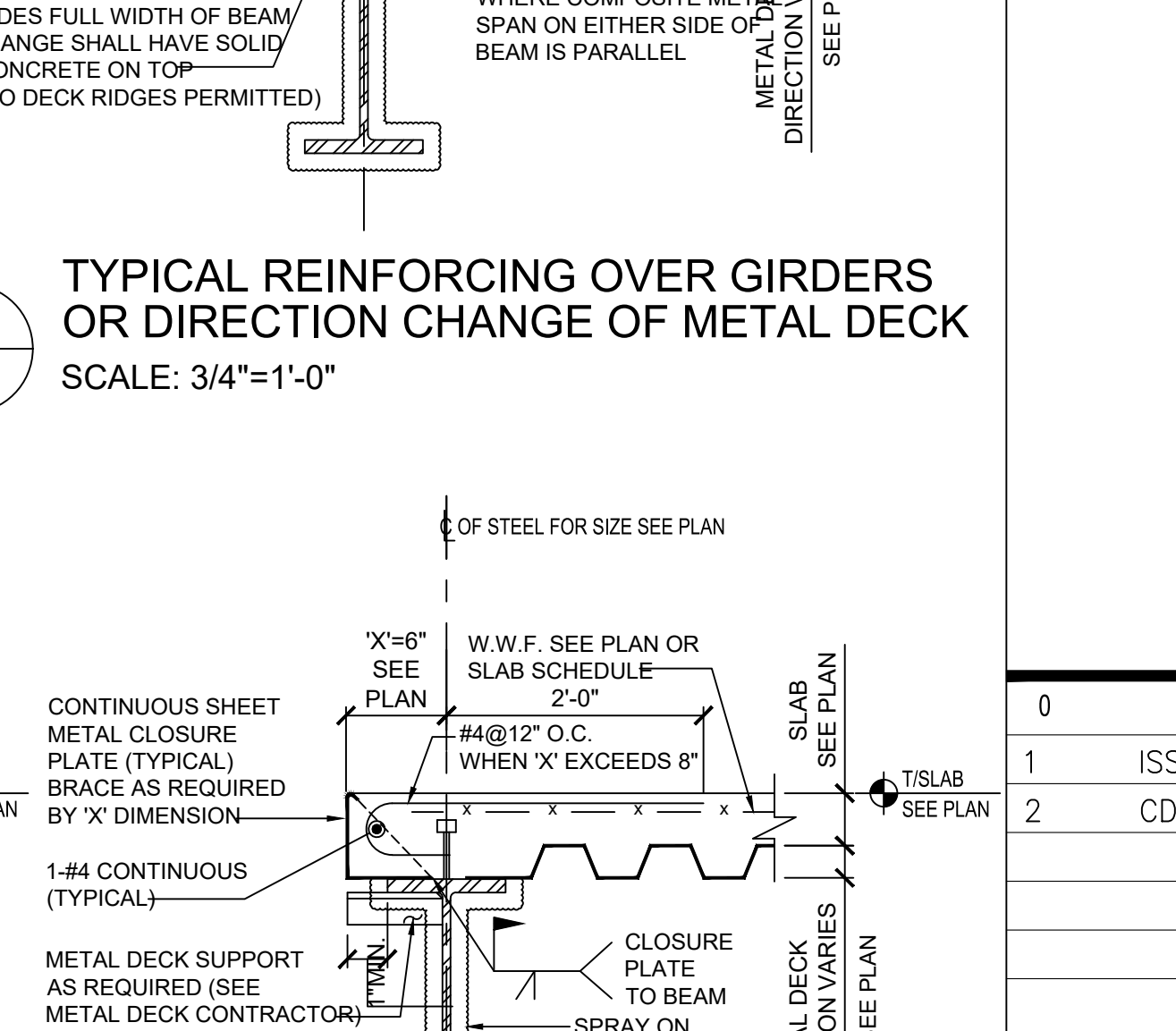
4 TYPICAL SLAB REINFORCING AT INTERIOR COLUMNS
SCALE: 3/4"=1'-0"



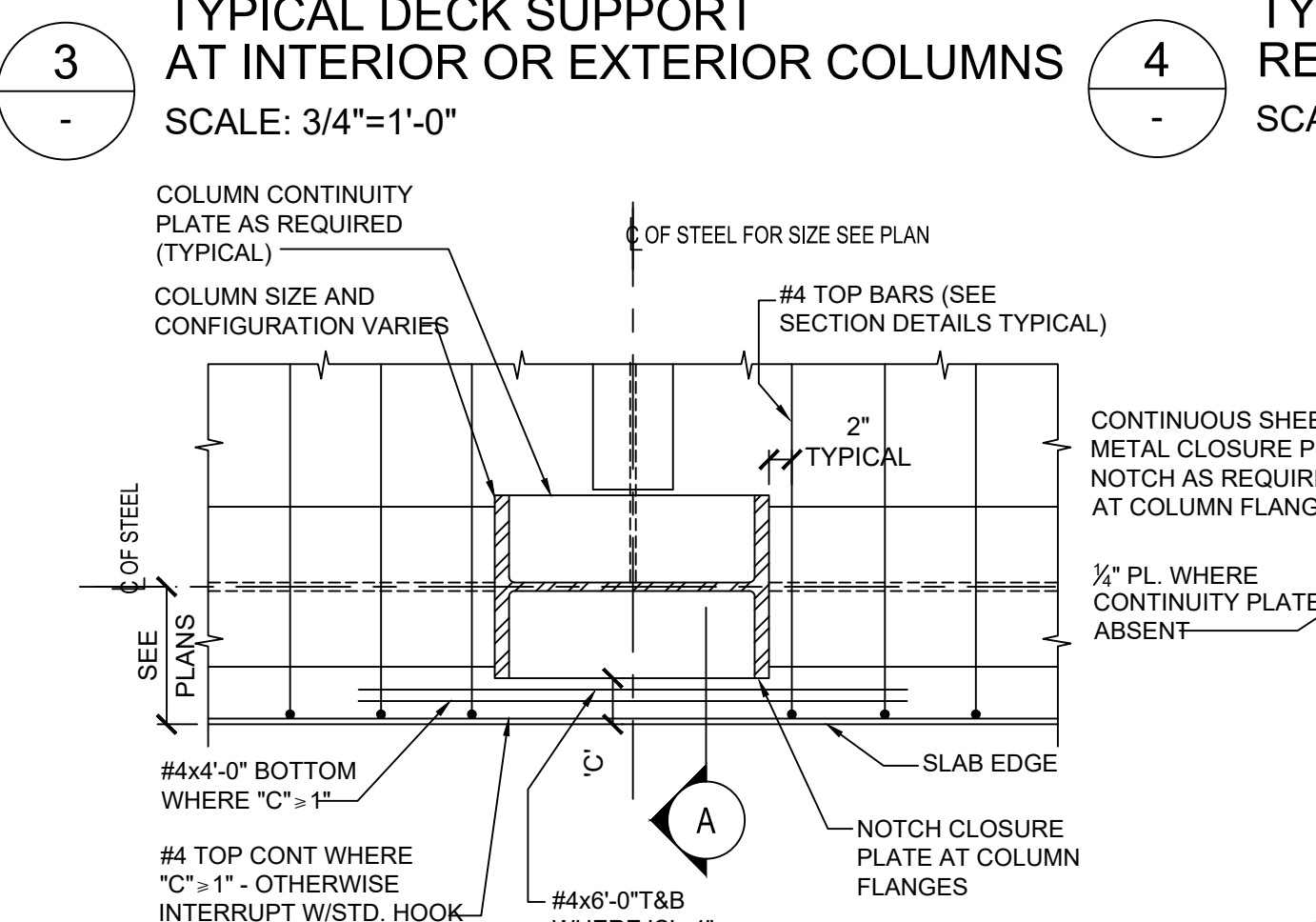
8 TYPICAL SLAB REINFORCING AT CORNER COLUMNS
SCALE: 3/4"=1'-0"



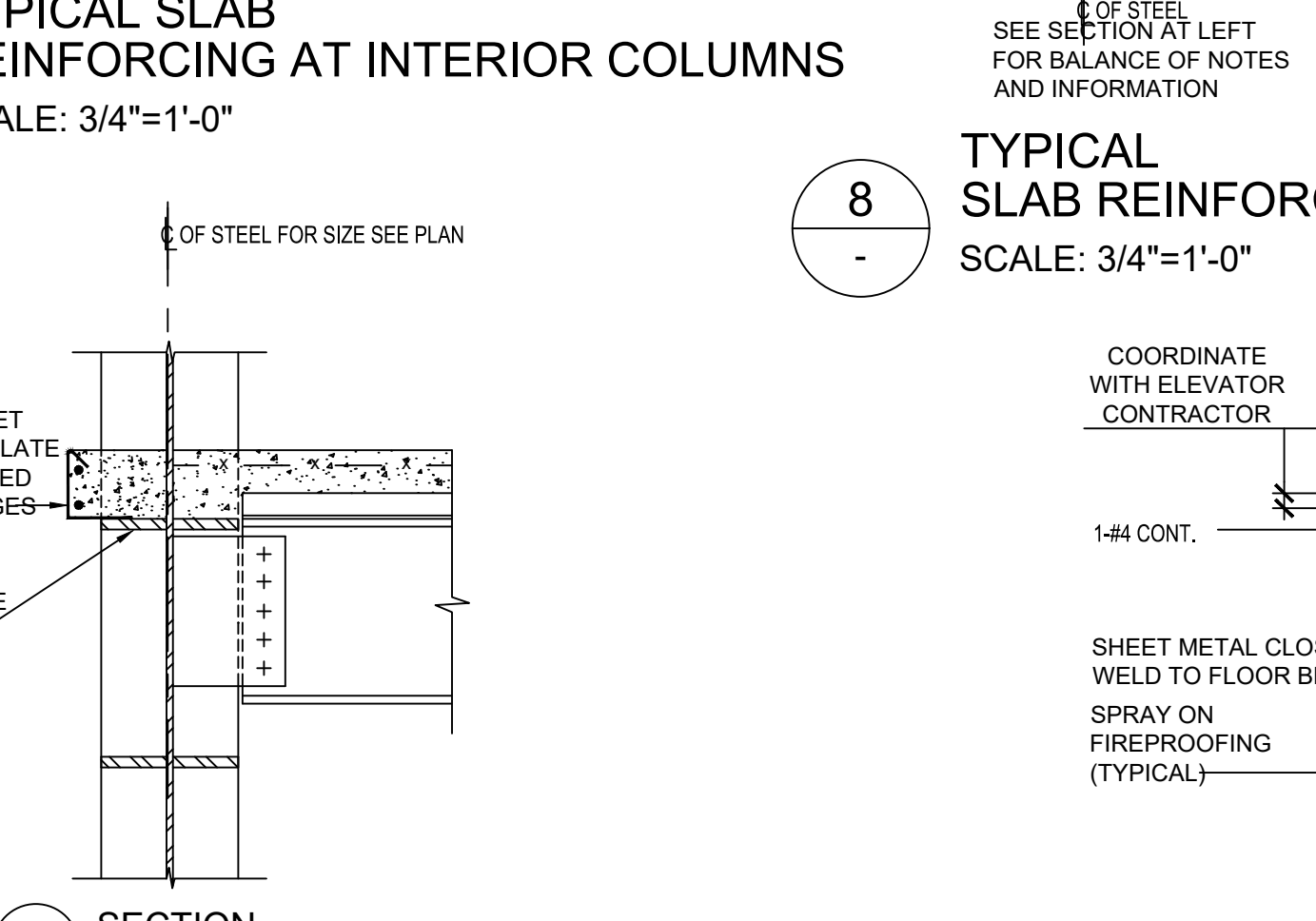
15 TYPICAL SLAB EDGE AT INTERIOR OPENINGS
SCALE: 3/4"=1'-0"



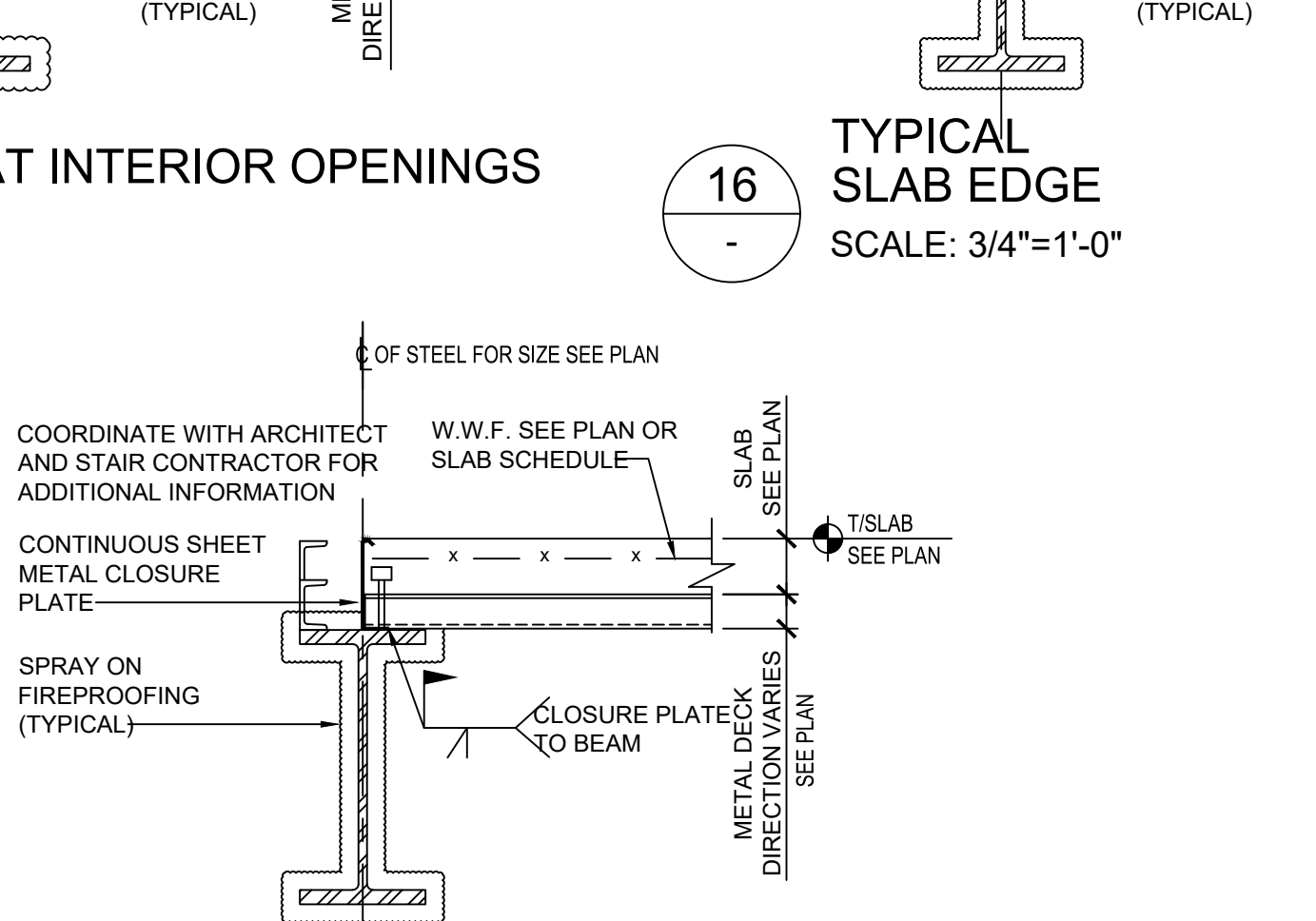
16 TYPICAL SLAB EDGE
SCALE: 3/4"=1'-0"



8 TYPICAL SLAB REINFORCING AT EXTERIOR COLUMNS
SCALE: 3/4"=1'-0"



12 TYPICAL SLAB EDGE AT ELEVATOR SILL
SCALE: 3/4"=1'-0"



17 TYPICAL SLAB EDGE AT STAIR LANDING
SCALE: 3/4"=1'-0"

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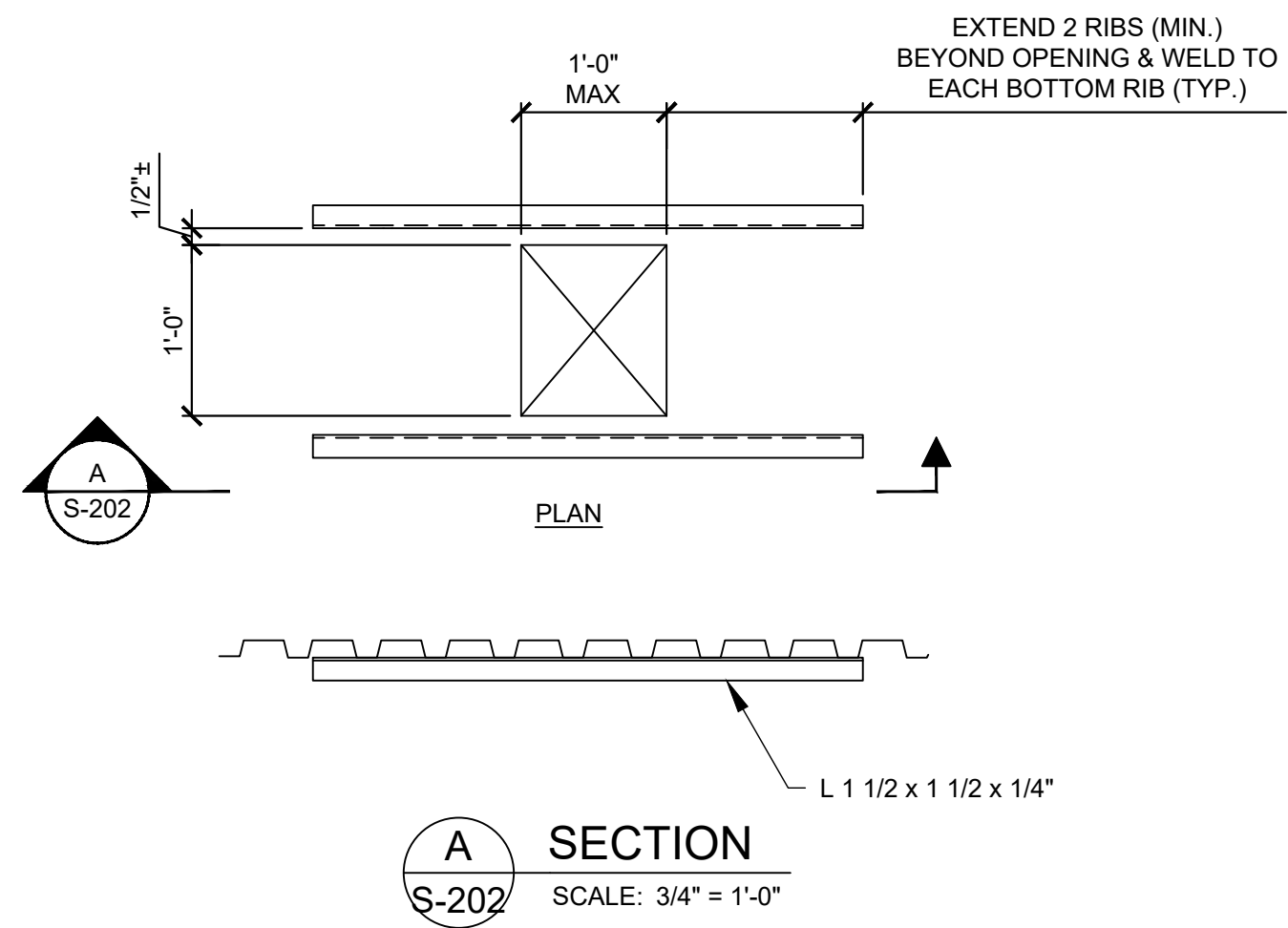
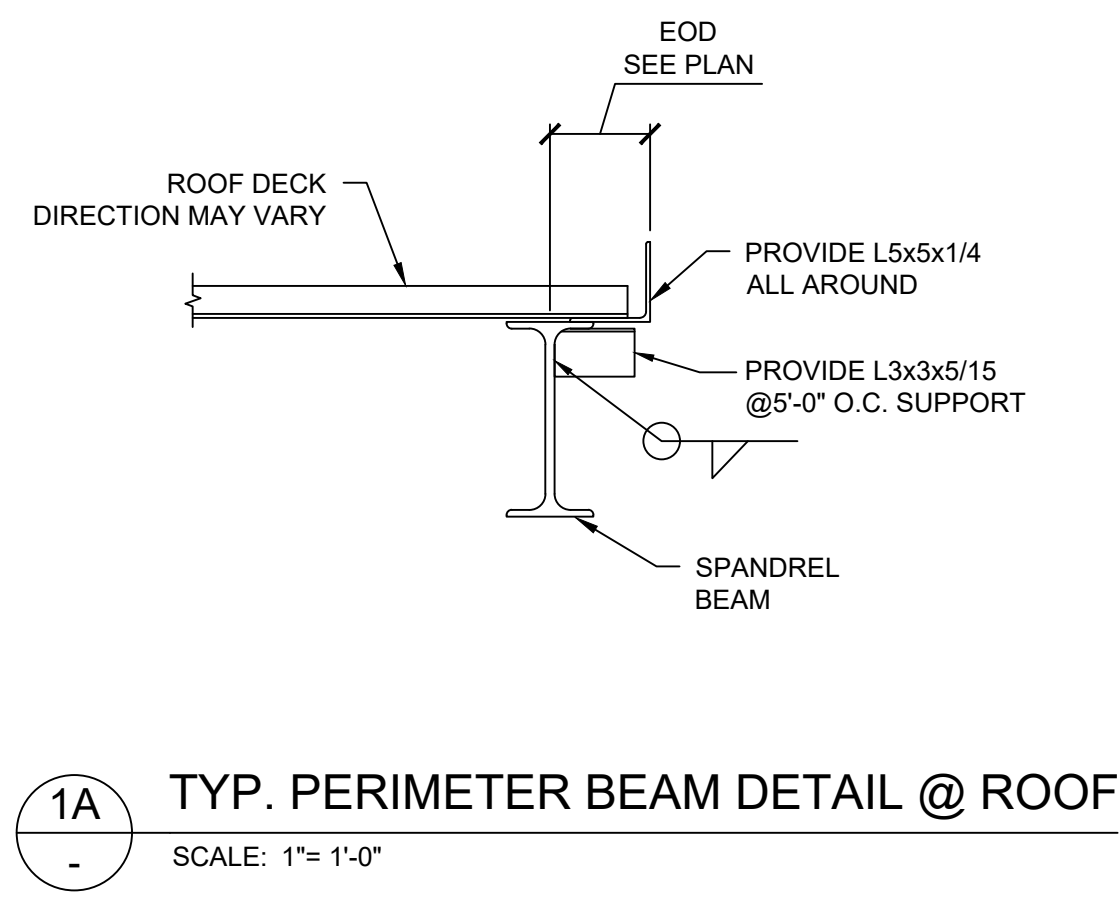
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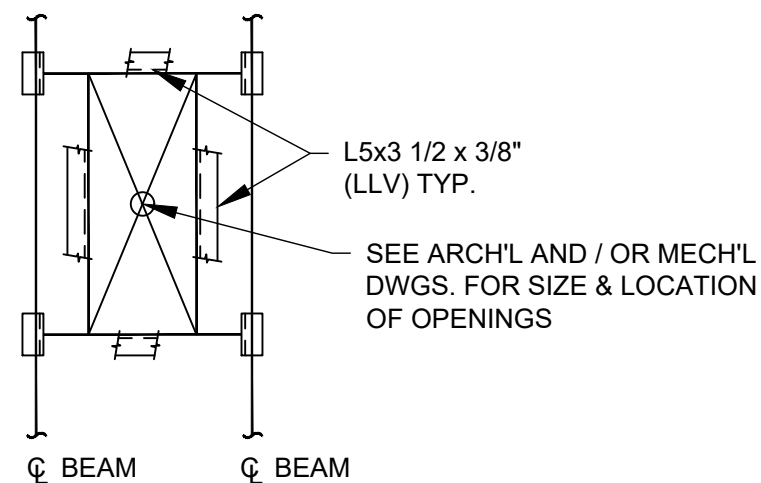
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METAL DECK DETAILS

DWG NUMBER :
S-202

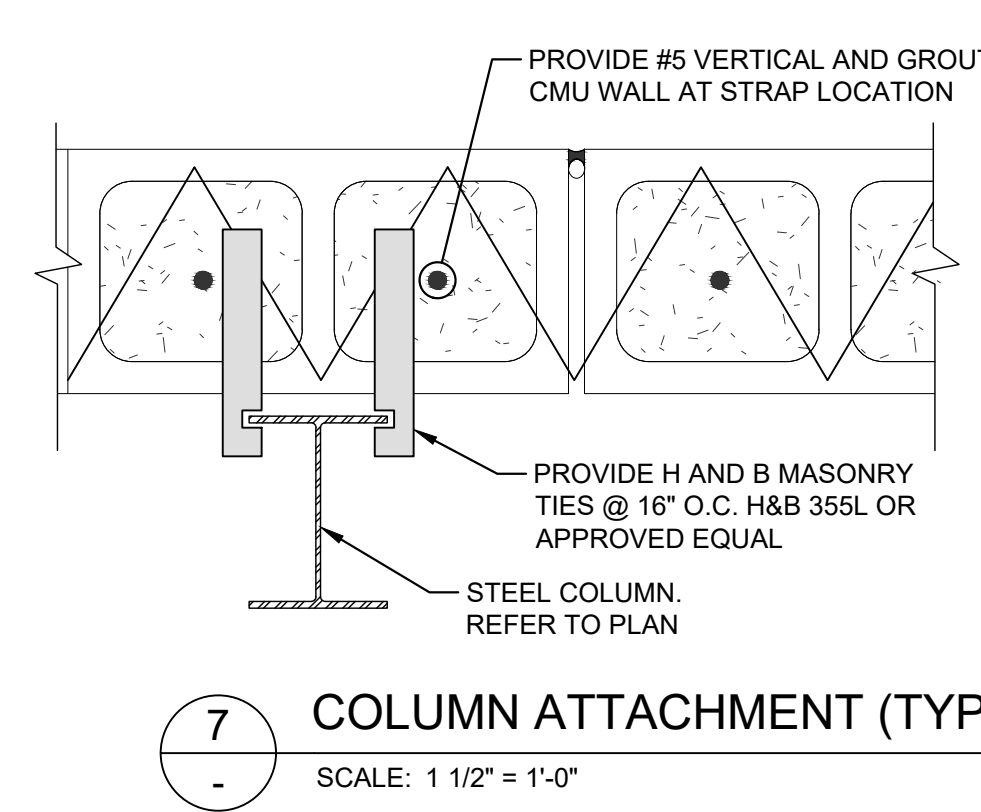
21 OF 60



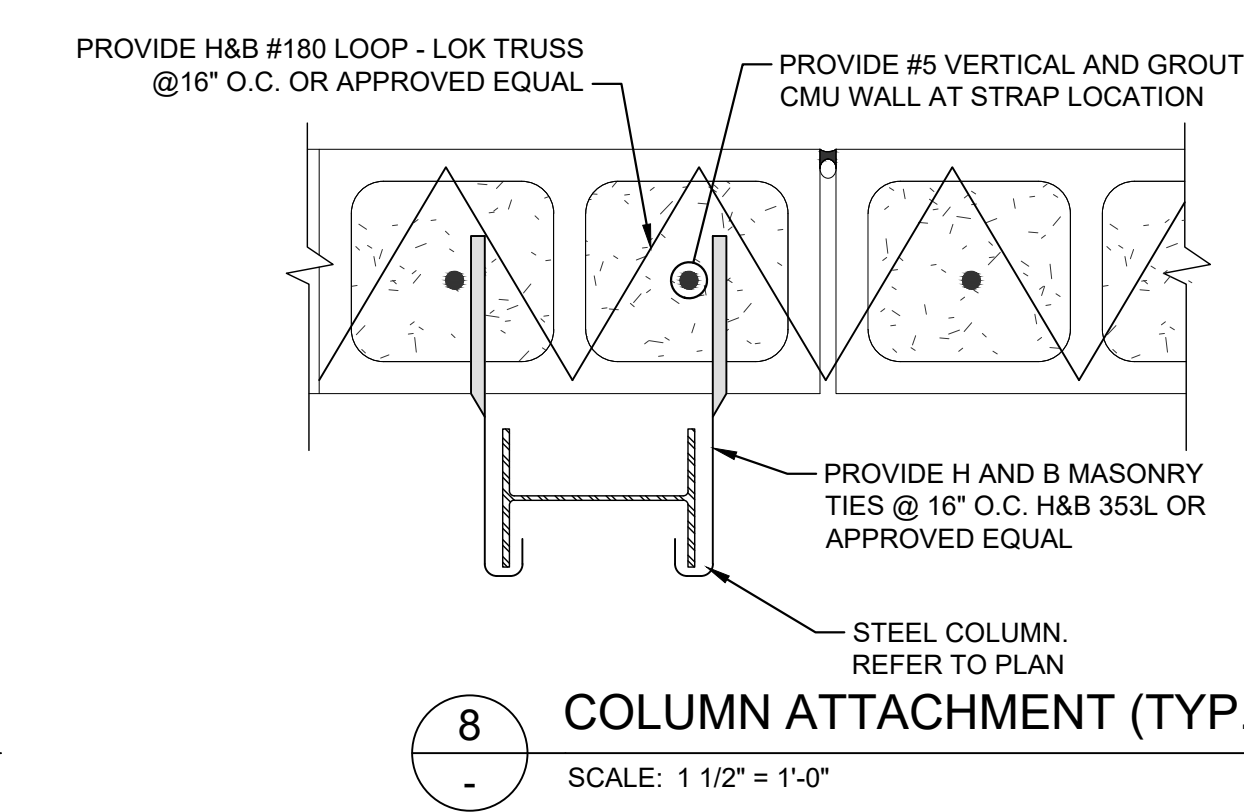
ROOF DECK PENETRATION AT SMALL OPENINGS
SCALE: 3/4" = 1'-0"



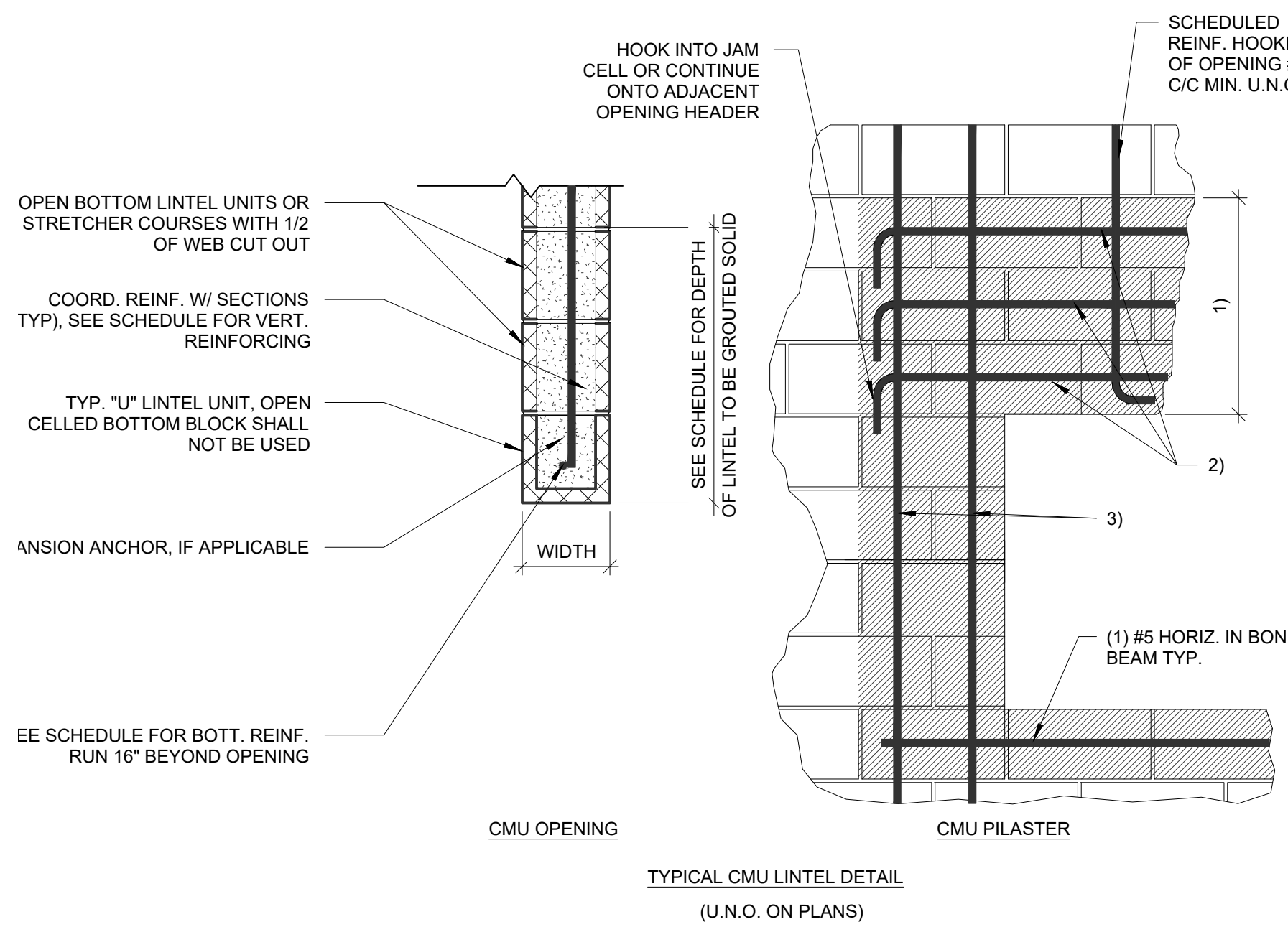
TYP. FRAMING AT ROOF OPENINGS
SCALE: 3/4" = 1'-0"



7 COLUMN ATTACHMENT (TYP.)
SCALE: 1 1/2" = 1'-0"



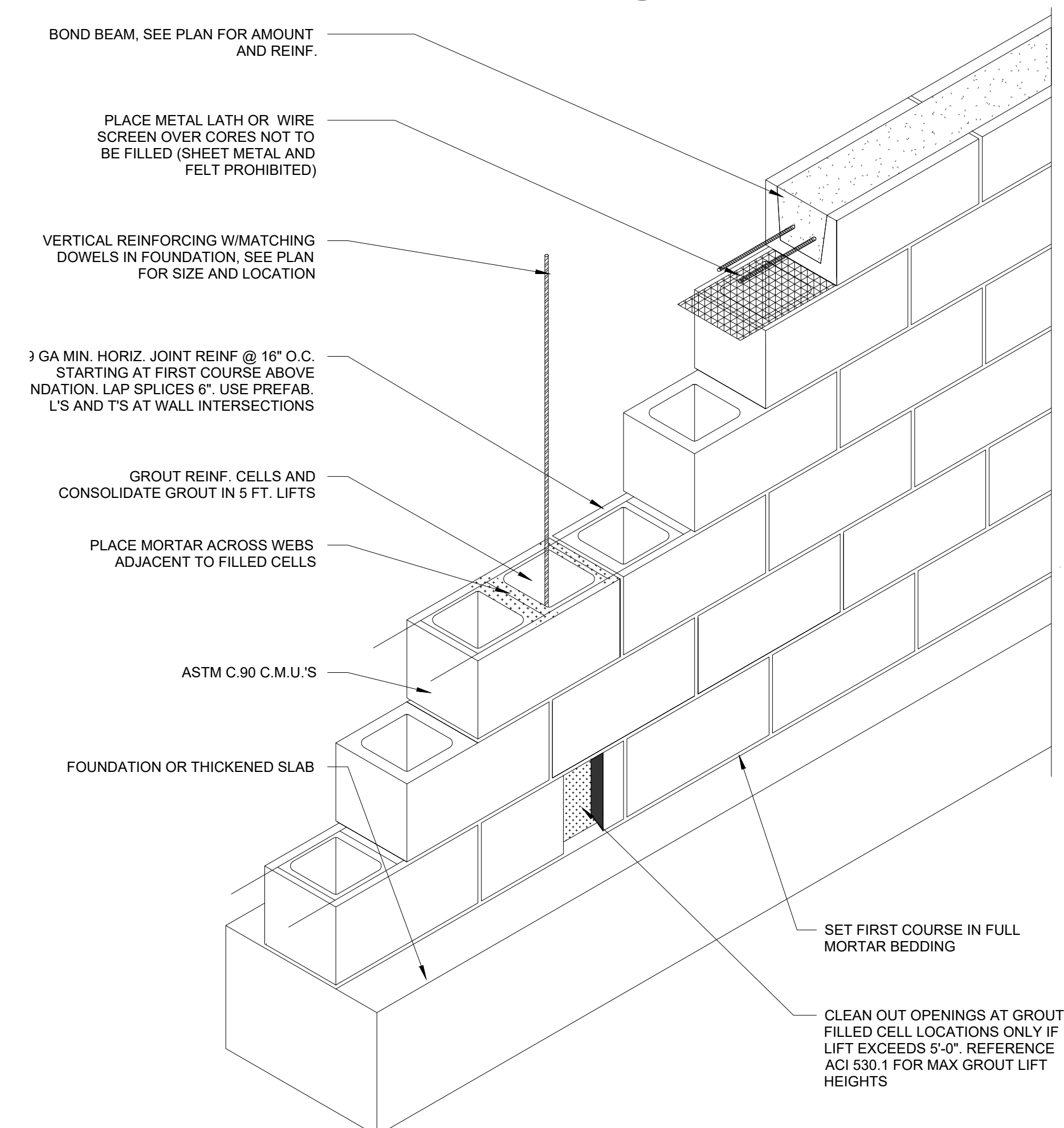
8 COLUMN ATTACHMENT (TYP.)
SCALE: 1 1/2" = 1'-0"



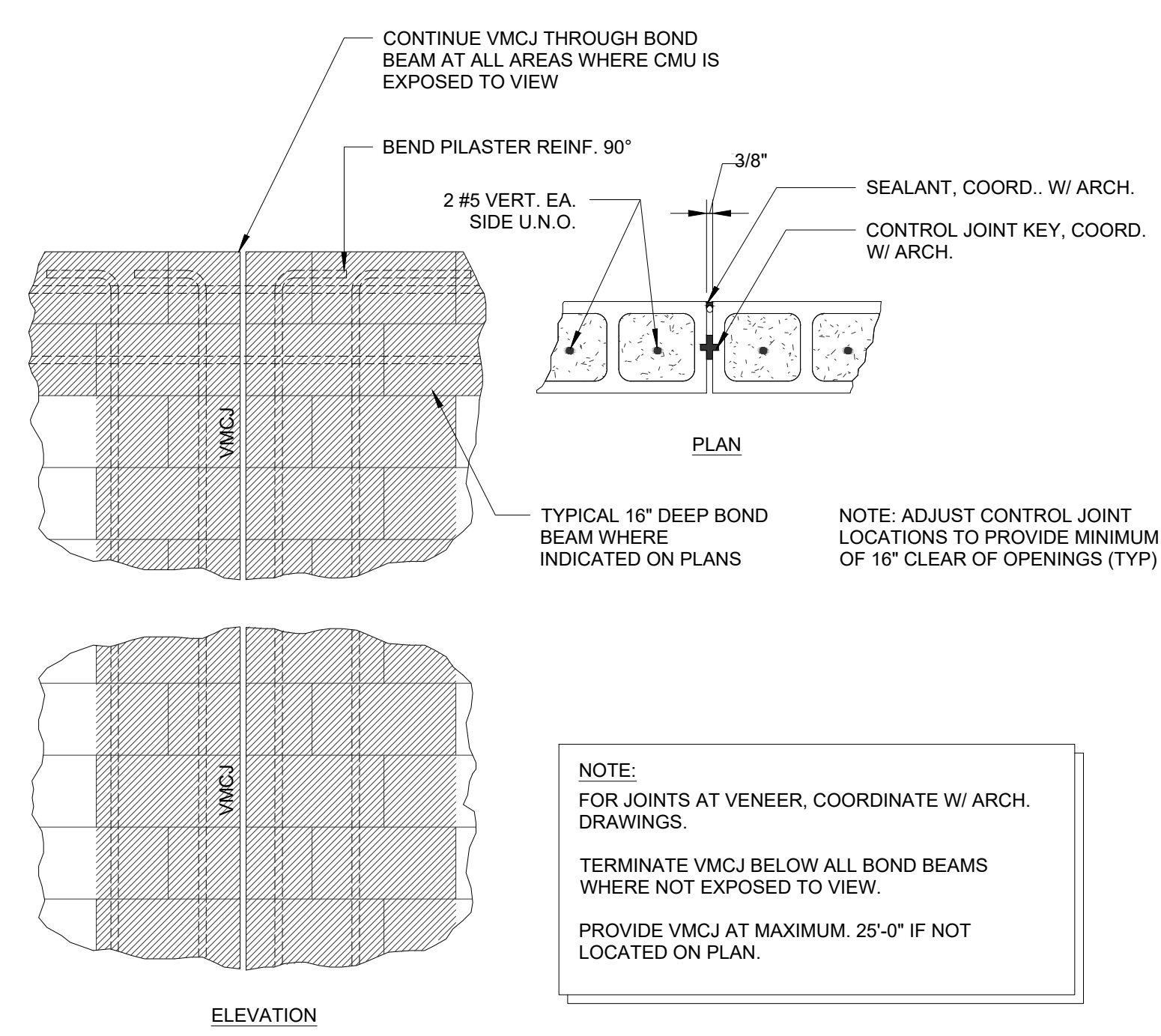
3 CMU LINTEL SCHEDULE
1" = 1'-0"

CMU LINTEL/JAMB SCHEDULE					
MAX OPENING SPAN	LOCATION	WIDTH	1) MIN. DEPTH OF FULL GROUDED CELLS	2) HORIZONTAL BOTT. REINF. U.N.O. (1 HORIZ. PER COURSE)	3) MIN. ADJACENT FULLY GROUDED CELLS AND REINF. (1 VERT. PER CELL U.N.O.)
≤ 4'-4"	TYP.	8"	8"	(1) #5 BOTT.	8" #5
4'-4" < OPNG. ≤ 6'-4"	TYP.	8"	16"	(1) #5 BOTT.	16" (2) #5
6'-4" < OPNG. ≤ 8'-4"	TYP.	8"	24"	(1) #5 BOTT.	16" (2) #6
8'-4" < OPNG. ≤ 10'-8"	TYP.	8"	32"	(2) #5 BOTT.	16" (2) #6

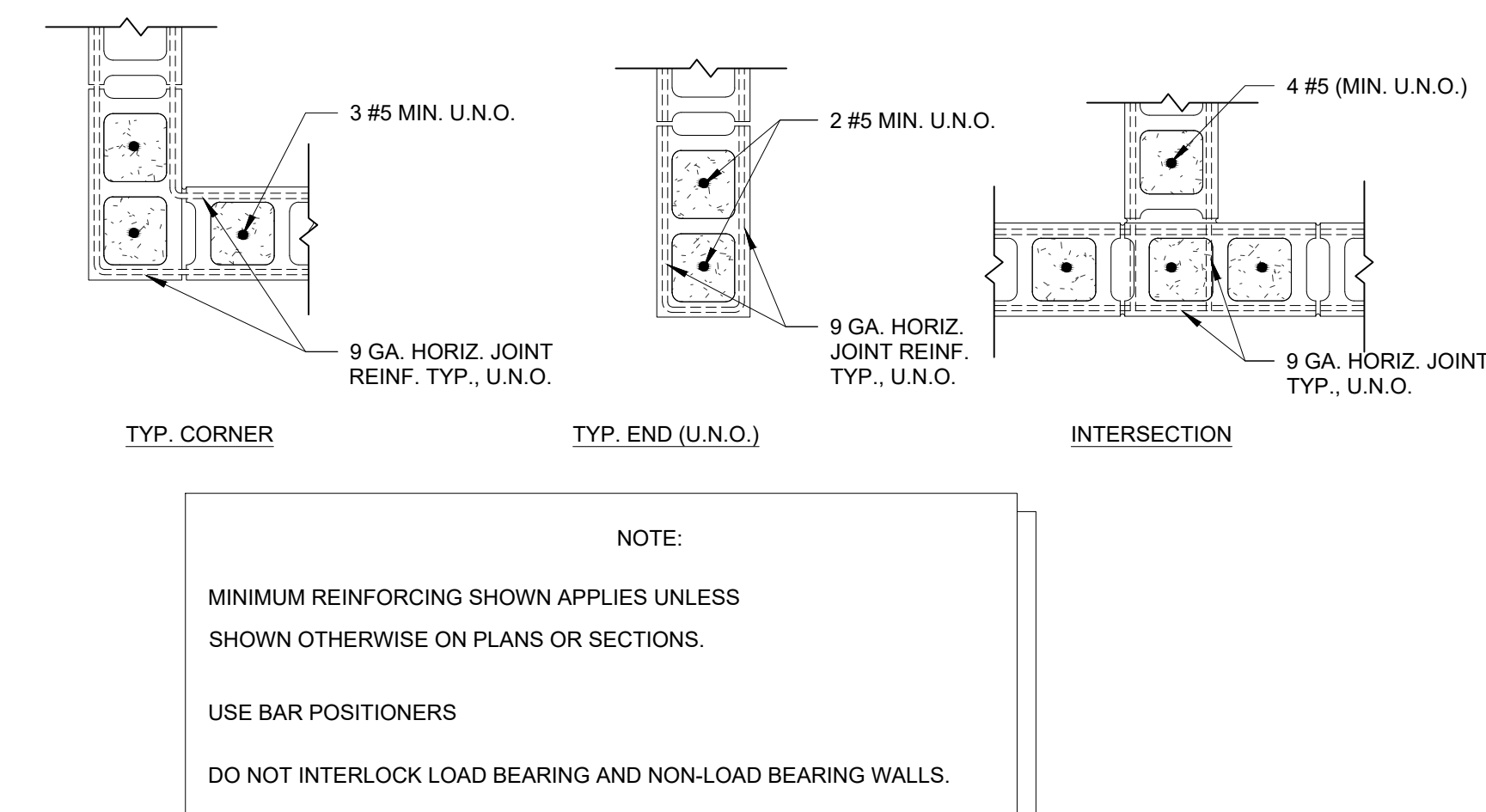
- NOTES:**
- #6 VERT. SHALL START ABOVE FOOTING WITH A MINIMUM LAP LENGTH OF 30".
 - VERIFY ALL CMU OPENINGS W/ ARCHITECT DWG'S.
 - ALL CMU LINTEL BEAMS ABOVE BOTTOM COURSE SHALL BE CONSTRUCTED OF OPEN BOTTOM LINTEL UNITS, OR STRETCHER COURSES WITH 1/2 WEB CUT OUT.
 - COORDINATE W/ SECTIONS, DETAILS, & GENERAL NOTES.
 - PRE-CAST CONCRETE LINTEL MAY BE USED IN LIEU OF LINTEL BLOCK.
 - REFER TO PLANS FOR FULLY GROUDED AREAS.



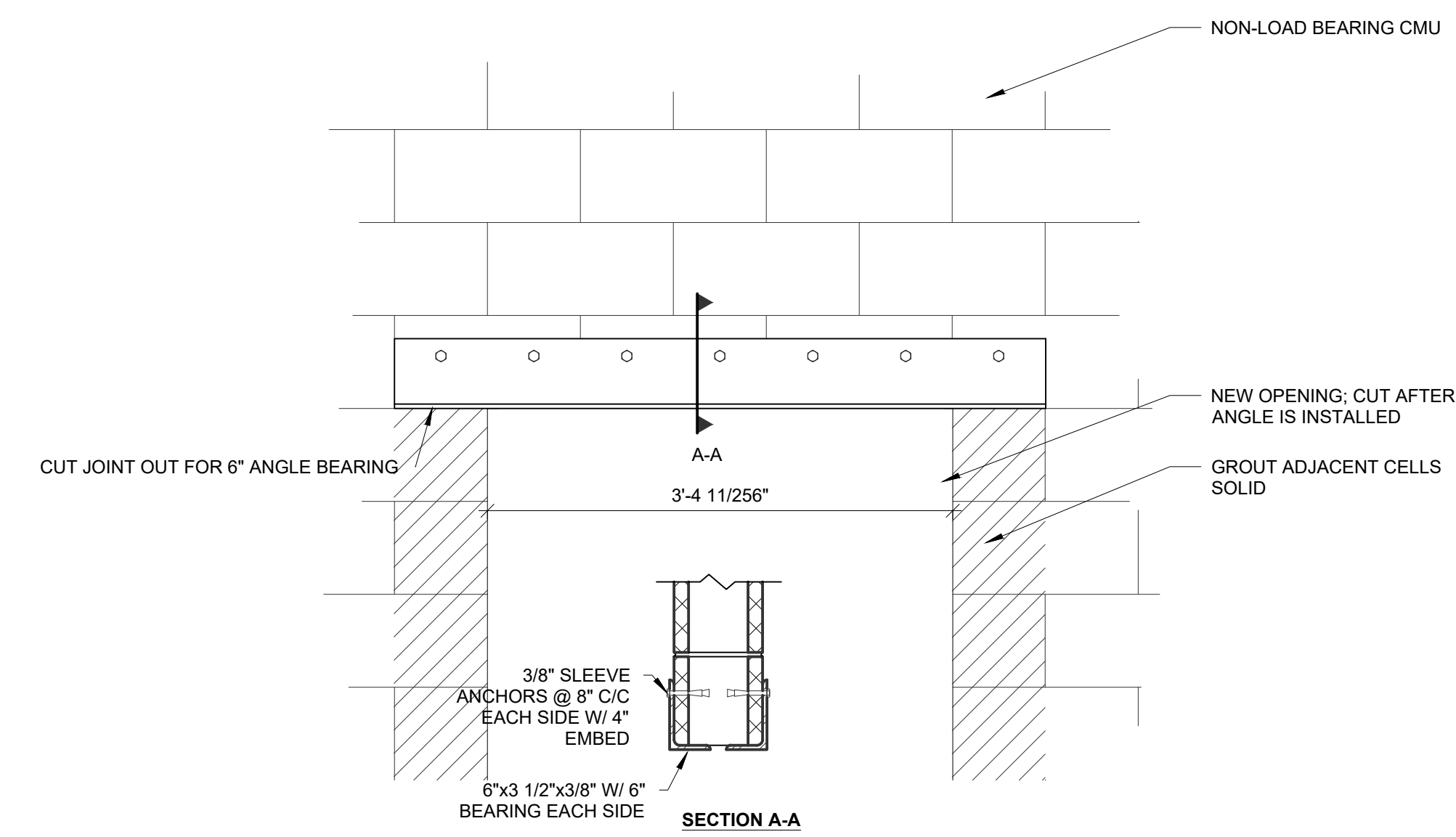
2 TYPICAL MASONRY WALL CONSTRUCTION
3/16" = 1'-0"



4 TYPICAL VMCJ DETAILS



5 TYPICAL CMU DETAILS



6 CMU OPENING

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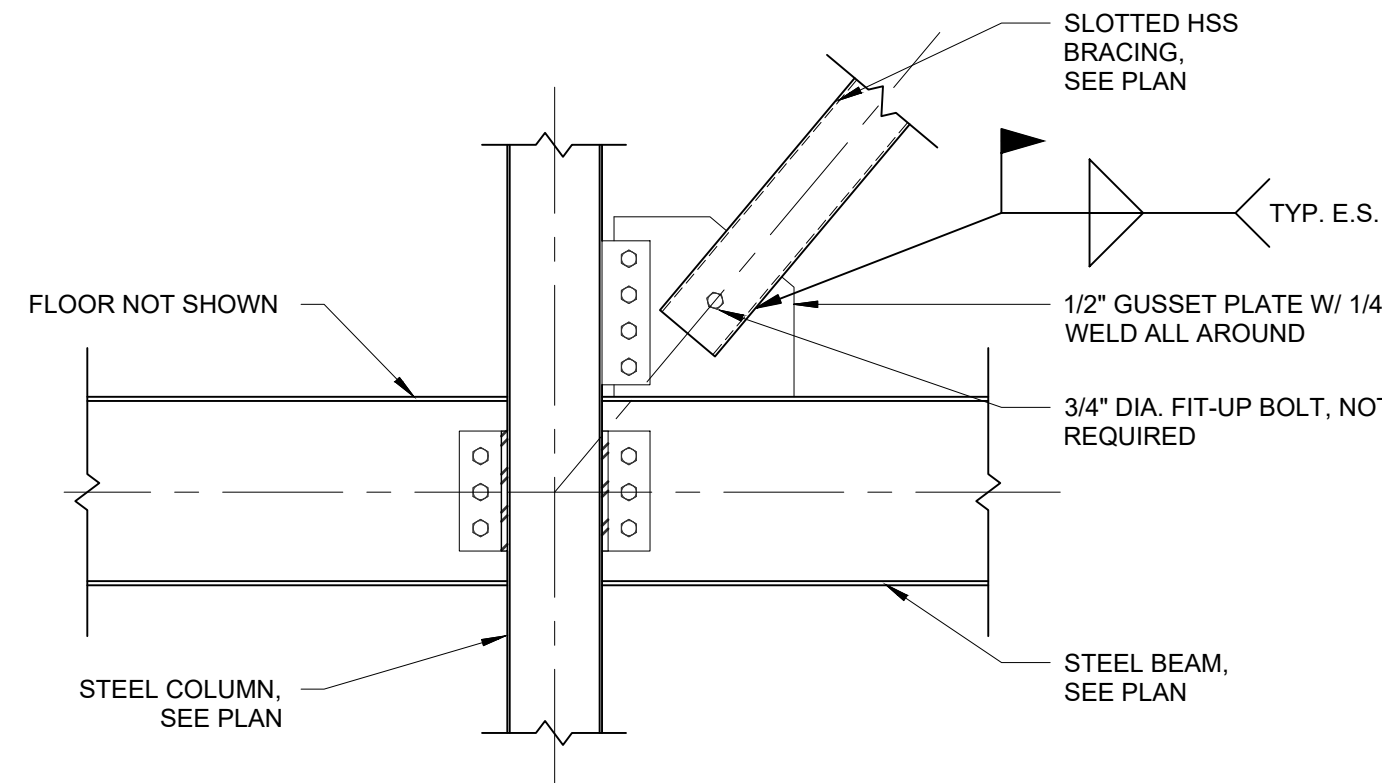
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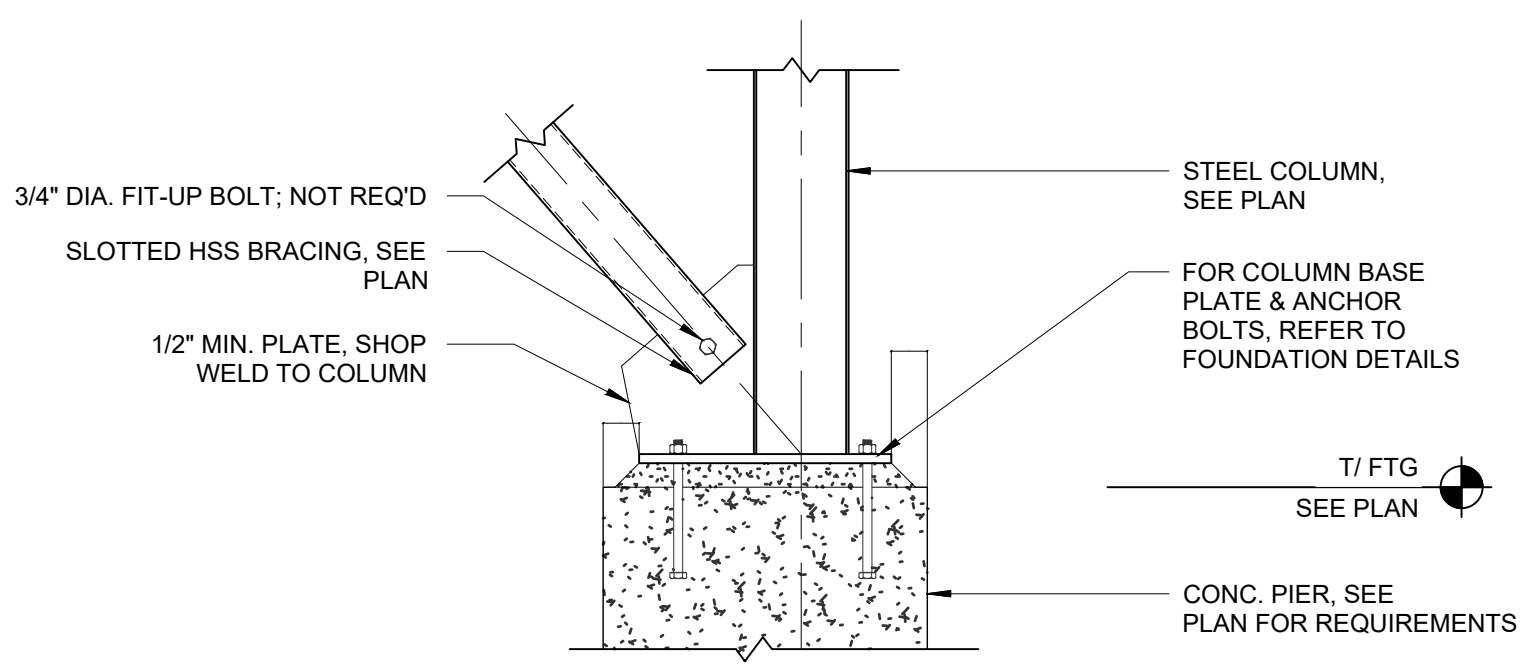
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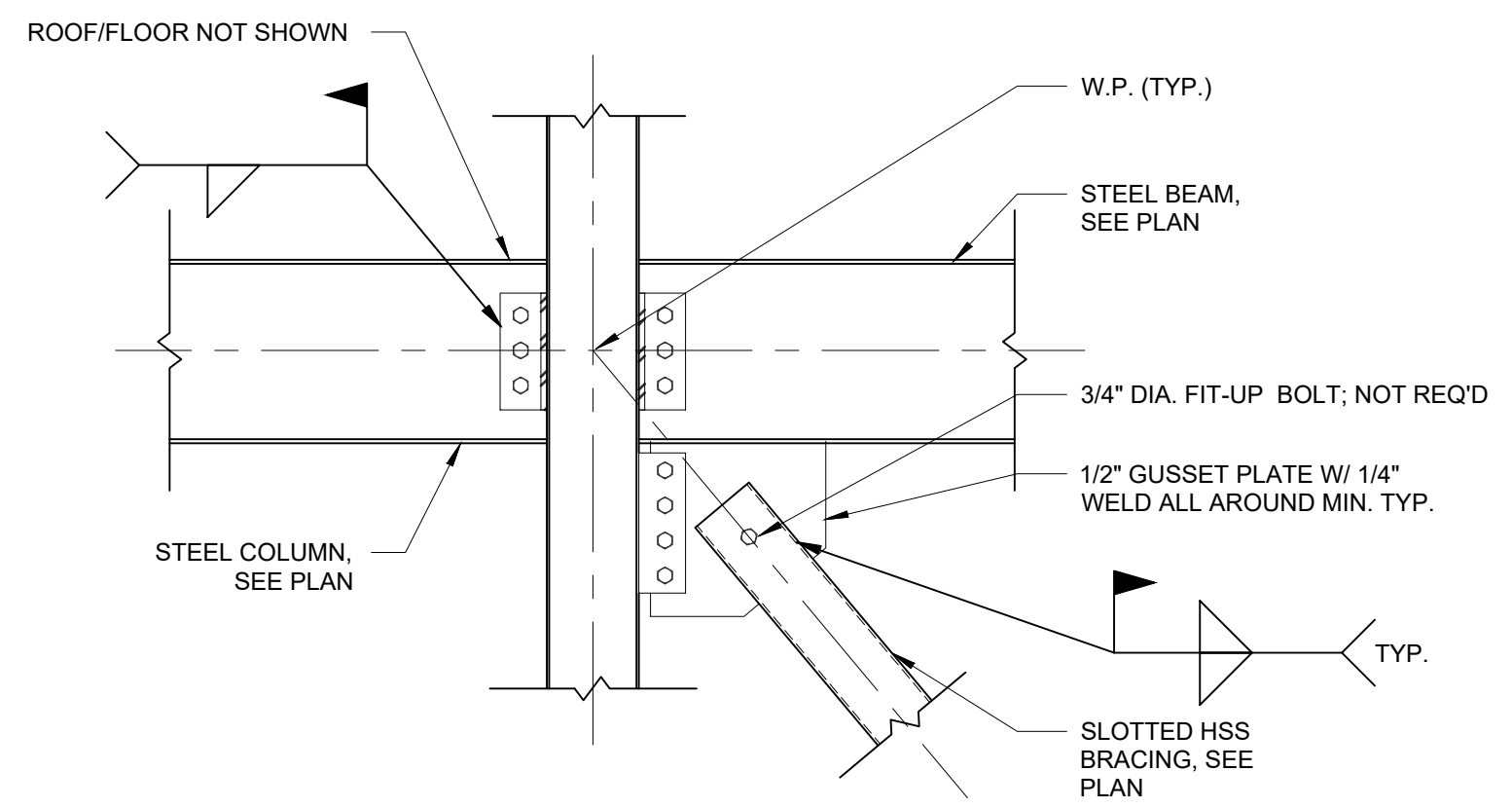
DRAWING TITLE :
ROOF STRUCTURAL AND MASONRY DETAILS
DWG NUMBER :
S-203
22 OF 60



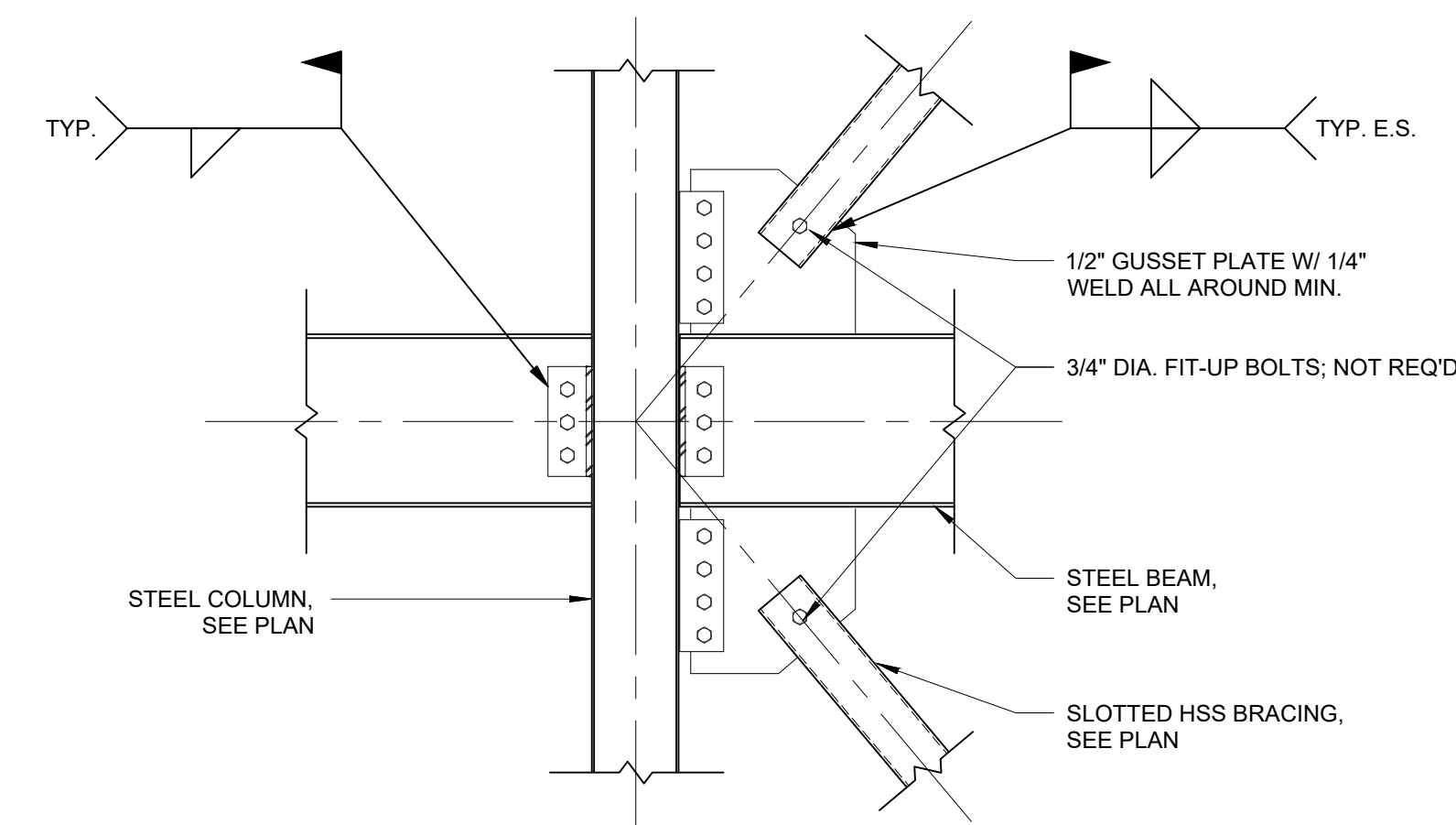
1 HSS BRACE ABOVE BEAM
3/4" = 1'-0"



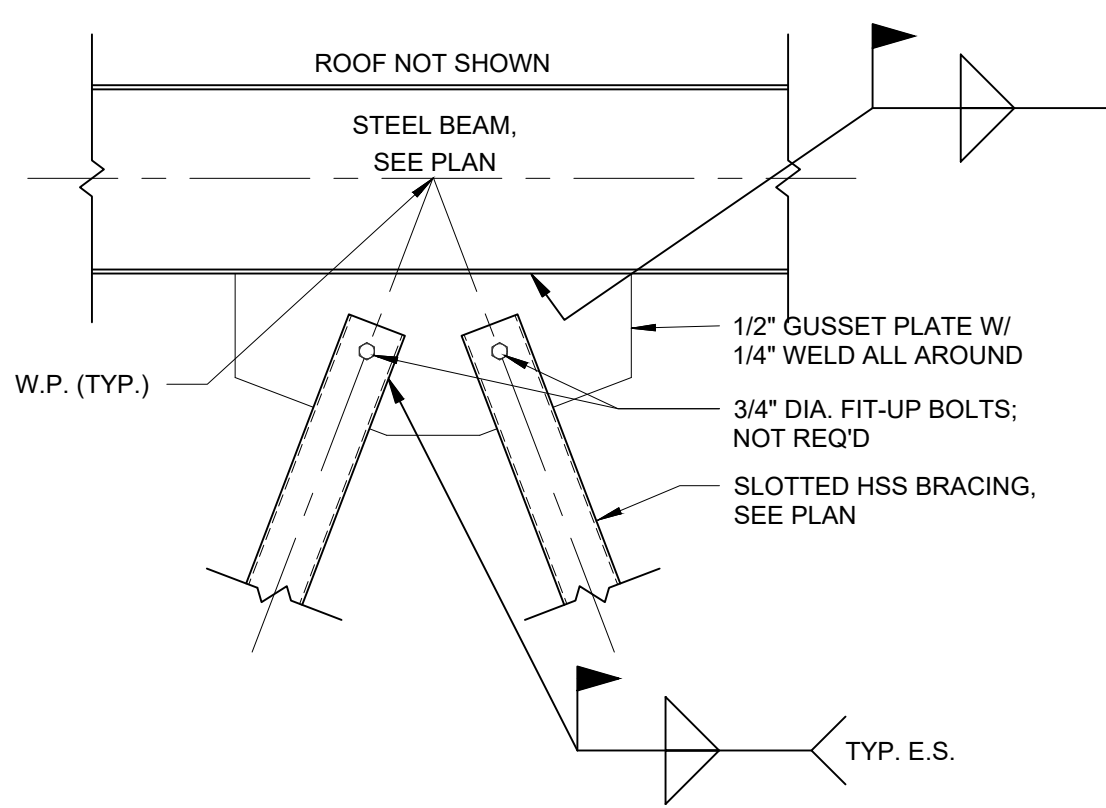
2 HSS BRACE AT BASE
3/4" = 1'-0"



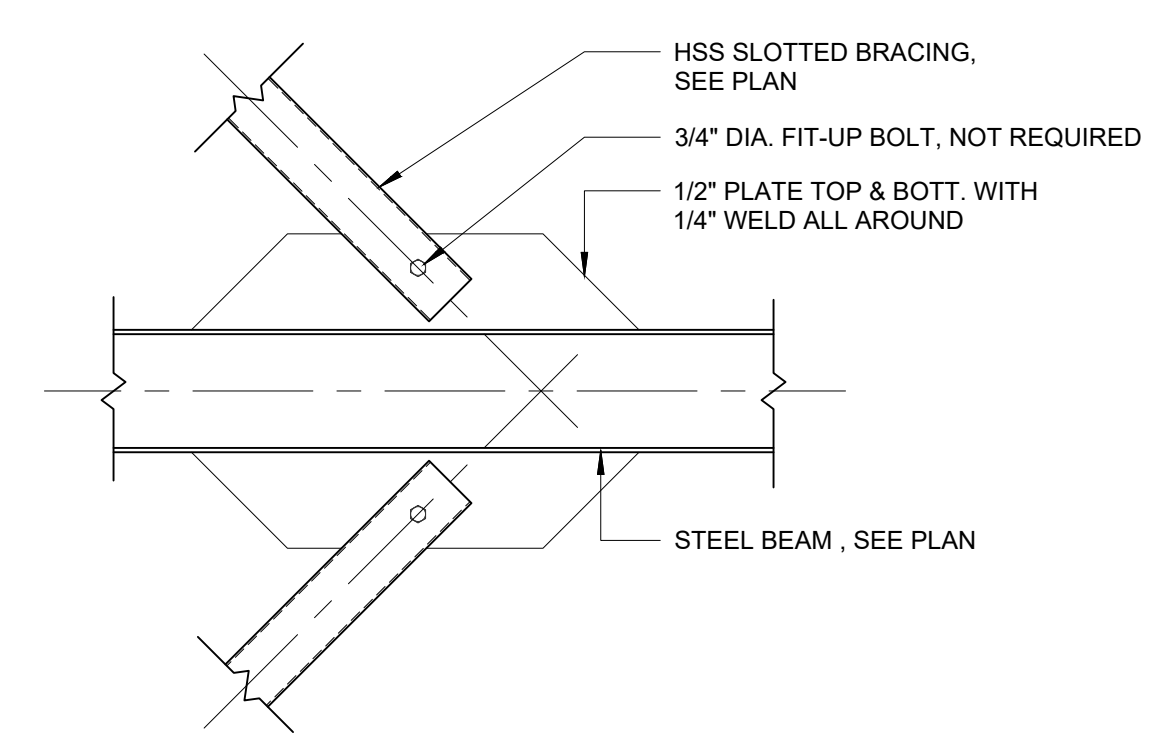
5 HSS BRACE BELOW BEAM
3/4" = 1'-0"



3 HSS BRACE AT BEAM END
3/4" = 1'-0"



4 HSS BRACE AT MID-SPAN
3/4" = 1'-0"



7 HSS BRACED FRAME AT MID-BEAM
3/4" = 1'-0"

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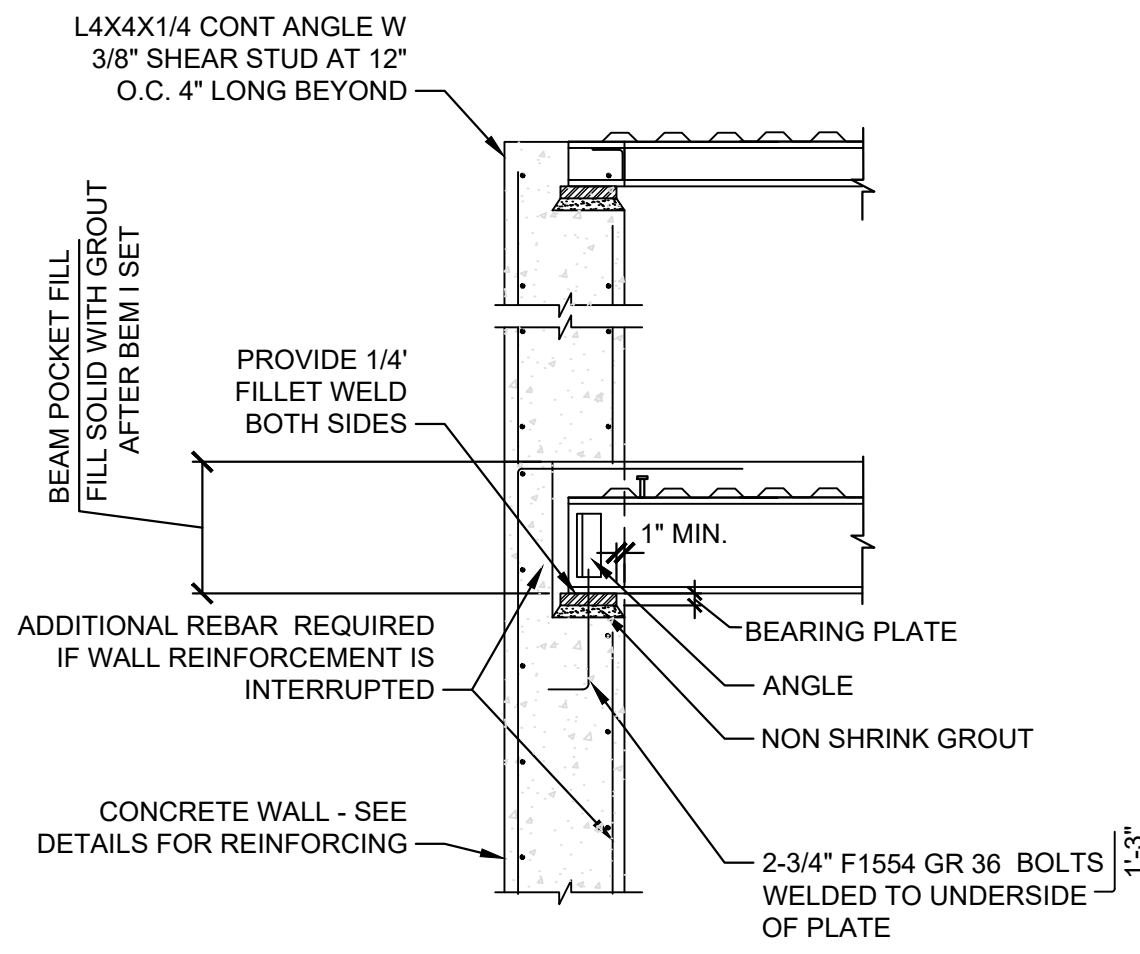
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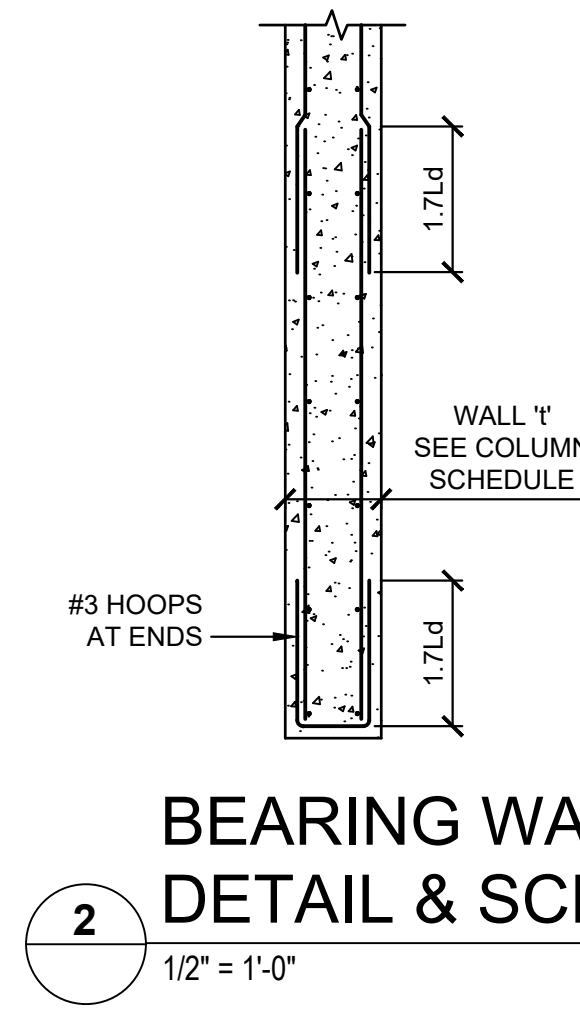
BRACING DETAILS

DWG NUMBER :
S-204
23 OF 60

ALL DIMENSIONS SHALL BE VERIFIED IN FIELD (V.I.F.).
COORDINATE LOCATION OF STEEL BEAMS TO MEET THE
EQUIPMENT SUPPORT REQUIREMENTS.



1 WALL POCKET DETAIL
1/2" = 1'-0"

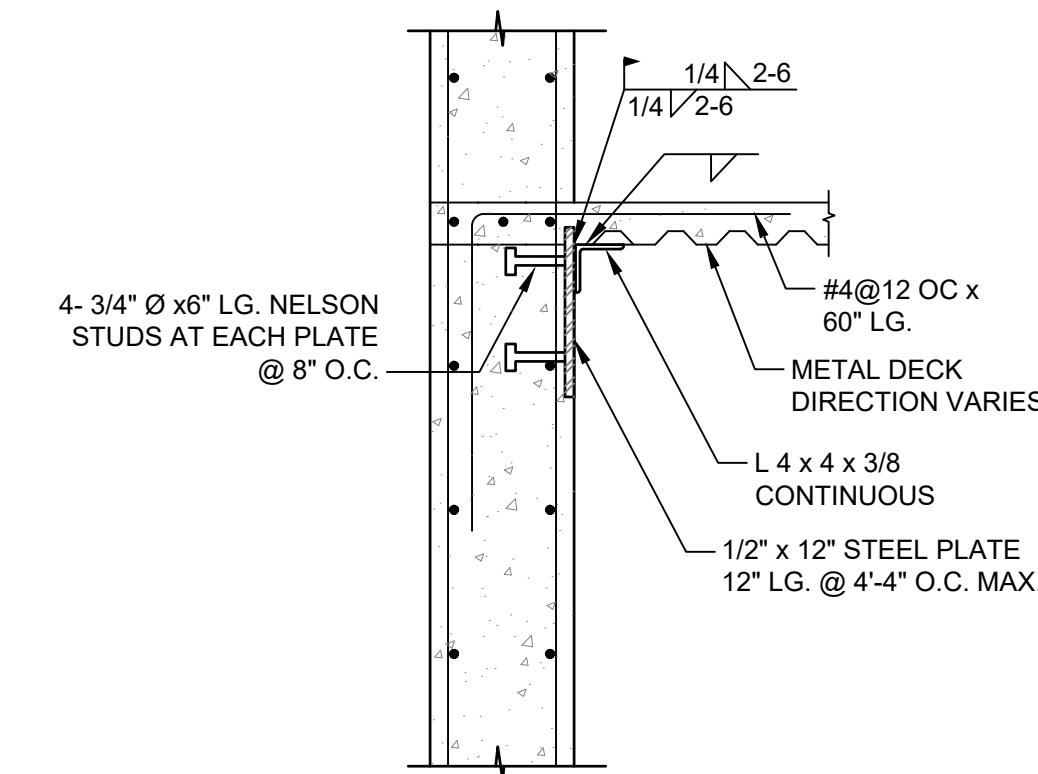


2 BEARING WALL DETAIL & SCHEDULE
1/2" = 1'-0"

BEARING WALL REINFORCING SCHEDULE		
WALL '1'	VERTICAL REINF.	HORIZONTAL REINF.
8"	#5@18	#3@11
9"	#5@18	#3@9
10"	#5@18	#4@16
12"	#5@18	#4@13

SEE PLAN FOR WALL REINFORCING AT ELEVATOR AND STAIR.

BASE PLATE SCHEDULE		
BEAM	BASE PLATE	STIFFENER
W10	8X8X1"	NO STIFFENER
W12	8X8X1"	NO STIFFENER
W14	8X8X1"	NO STIFFENER
W16	8X8X1"	NO STIFFENER
W18	8X8X1"	NO STIFFENER
W24	10X8X1"	2-1/2" STIFFENER
W30	12X12X1"	2-1/2" STIFFENER NOTCH PLATE AS REQUIRED AT CORNER CONDITION



3 TYPICAL METAL DECK SUPPORT
3/4" = 1'-0"

REINFORCEMENT ANCHORAGE SCHEDULE																
TABULATED VALUES ARE GIVEN IN INCHES & ARE CALCULATED FOR f'c = 60,000 P.S.I.																
STONE CONCRETE																
BAR SIZE	f'c	3000 P.S.I.			4000 P.S.I.			5000 P.S.I.			6000 P.S.I.					
		ld	ldt	ldc	ld	ldt	ldc	ld	ldt	ldc	ld	ldt	ldc			
#3	12	13	6/8	8	12	13	6/7									
#4	12	17	8/11	11	12	17	7/9	9	12	17	6/8	9	12	17	6/8	9
#5	15	21	10/14	14	15	21	8/12	12	15	21	7/11	11	15	21	7/10	11
#6	19	27	11/16	16	18	25	10/14	14	18	25	9/13	14	18	25	8/12	14
#7	26	37	13/19	19	23	32	12/17	17	21	29	10/15	16	21	29	10/14	16
#8	35	48	15/22	22	30	42	13/19	19	27	38	12/17	18	24	34	11/16	18
#9	44	61	17/25	25	38	53	15/21	21	34	48	13/19	20	31	43	13/18	20
#10	56	78	19/28	28	48	67	17/24	24	43	60	15/22	23	39	55	14/20	23
#11	68	96	22/31	31	59	83	19/27	27	53	74	17/24	25	48	68	15/22	25

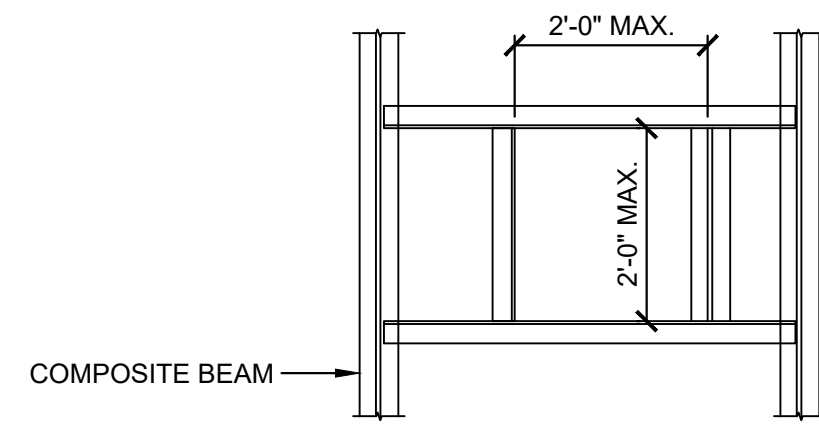
NOTE A: USE ld TABULATED VALUES WHEN BARS HAVE LESS THAN 12" OF CONCRETE BELOW BOTTOM OF BARS (ie: FOR BOTTOM REINFORCING AND FOR TOP REINFORCING IN SHALLOW BEAMS AND SLABS). USE ldt WHEN BARS HAVE MORE THAN 12" OF FRESHLY PLACED CONCRETE BELOW BOTTOM OF BARS.

NOTE B: TABULATED VALUES MAY BE REDUCED BY 20% IF BARS ARE SPACED MORE THAN 6" AND ARE AT LEAST 3" FROM PARALLEL CONCRETE EDGES.

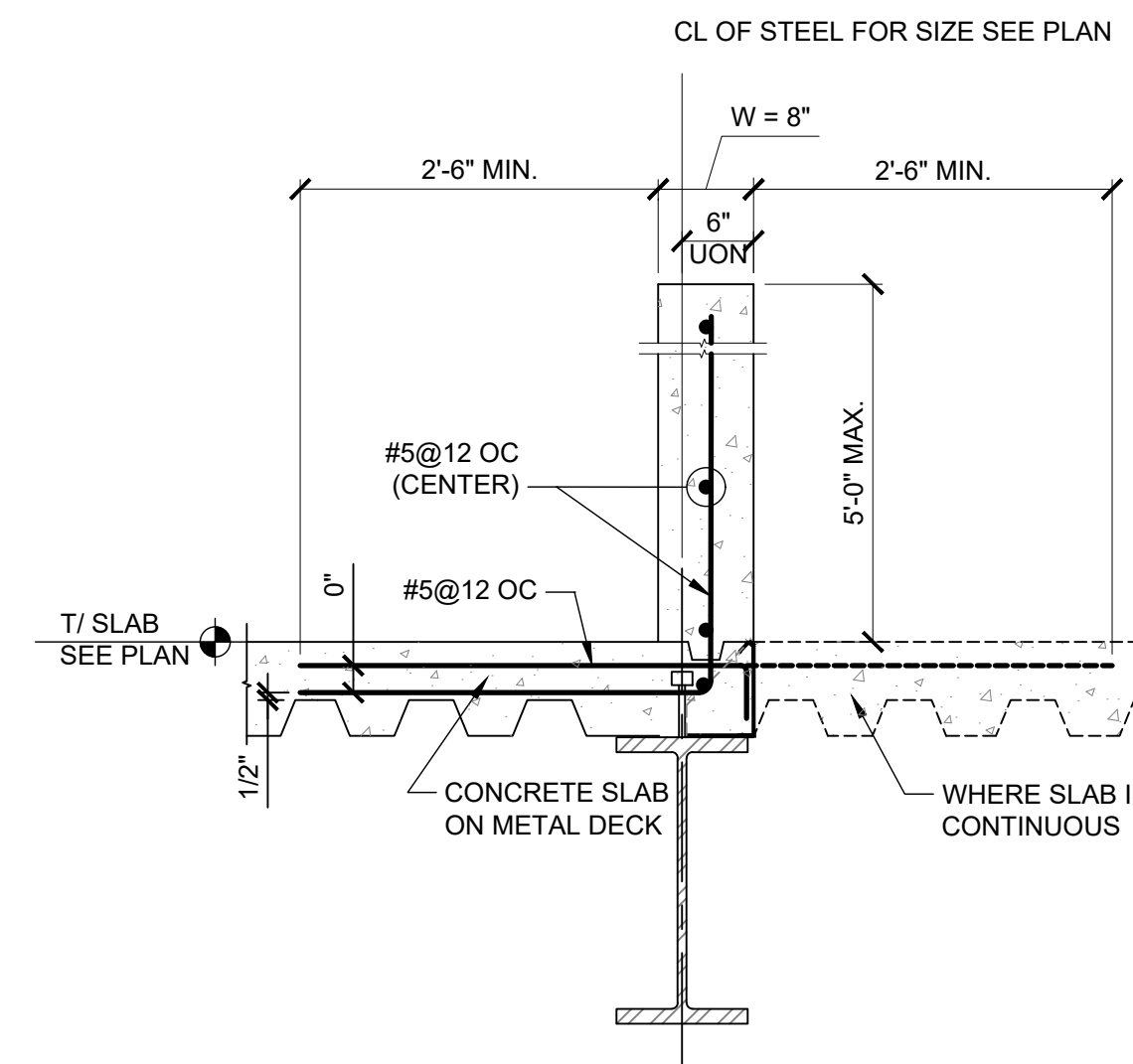
NOTE C: LOWER VALUES OF DEVELOPMENT LENGTH OF HOOKED BARS, ldt, MAY BE USED IF TAIL EXTENSION OF THE HOOK HAS AT LEAST 2" COVER AND CONCRETE COVER TO SIDE OF HOOK IS AT LEAST 2 1/2" THICK. OTHERWISE USE HIGHER ldt VALUES TABULATED.

NOTE D: HOOKS SHALL NOT BE CONSIDERED EFFECTIVE IN REDUCED ldc VALUES (COMPRESSION DEVELOPMENT LENGTH).

GLOSSARY:
ld = DEVELOPMENT LENGTH (IN.) = LENGTH REQUIRED TO ANCHOR REINFORCING AT EACH END.
ldt = DEVELOPMENT LENGTH (IN.) = LENGTH REQUIRED TO ANCHOR TOP BARS HAVING MORE THAN 12" OF FRESH CONCRETE PLACED UNDER THEM.
ldh = DEVELOPMENT LENGTH (IN.) OF HOOKED BARS (TO PERIMETER OF HOOK).
ldc = DEVELOPMENT LENGTH (IN.) FOR BARS IN COMPRESSION.

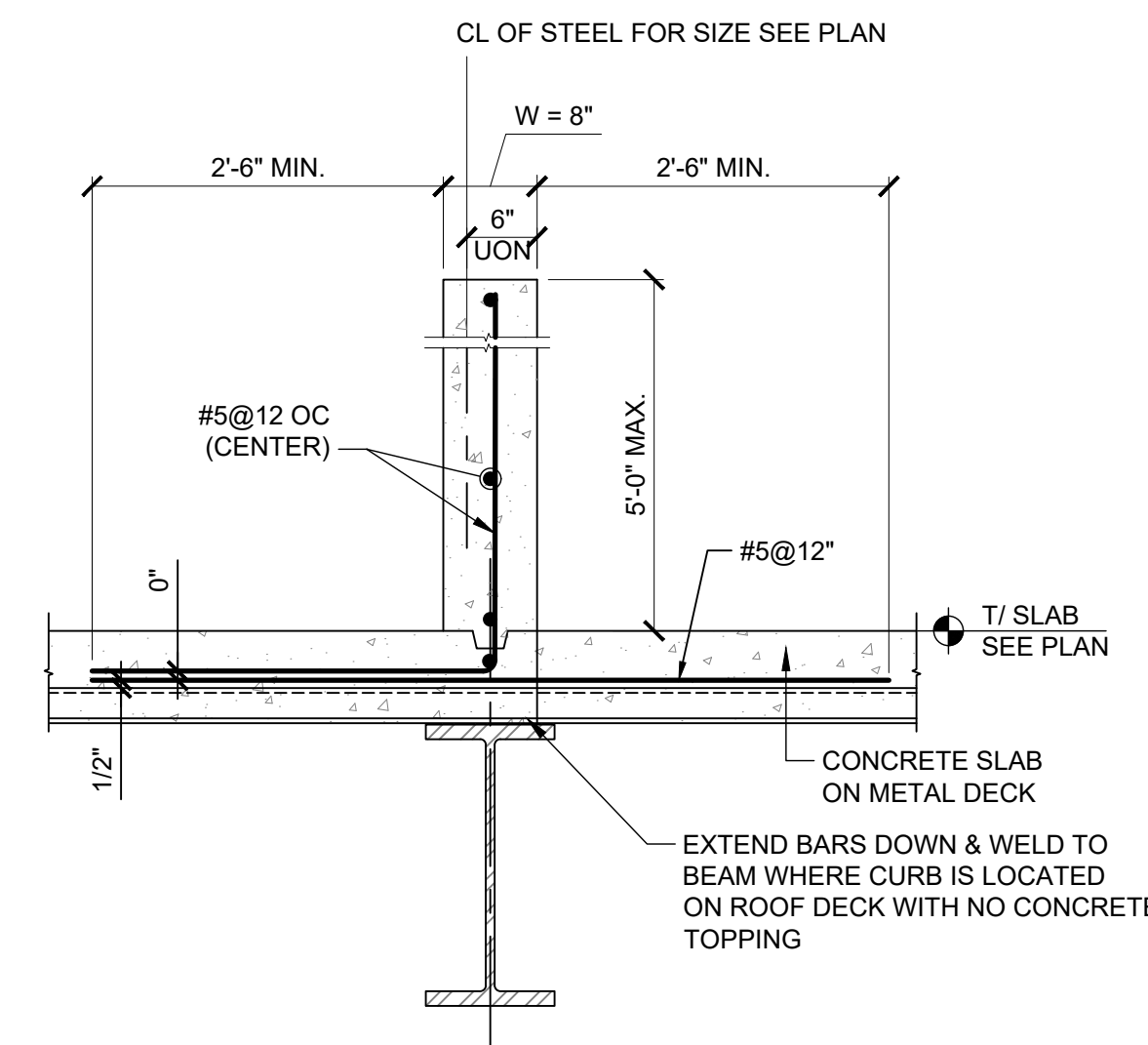


5 TYP. FRAMES AT OPENINGS
1/2" = 1'-0"



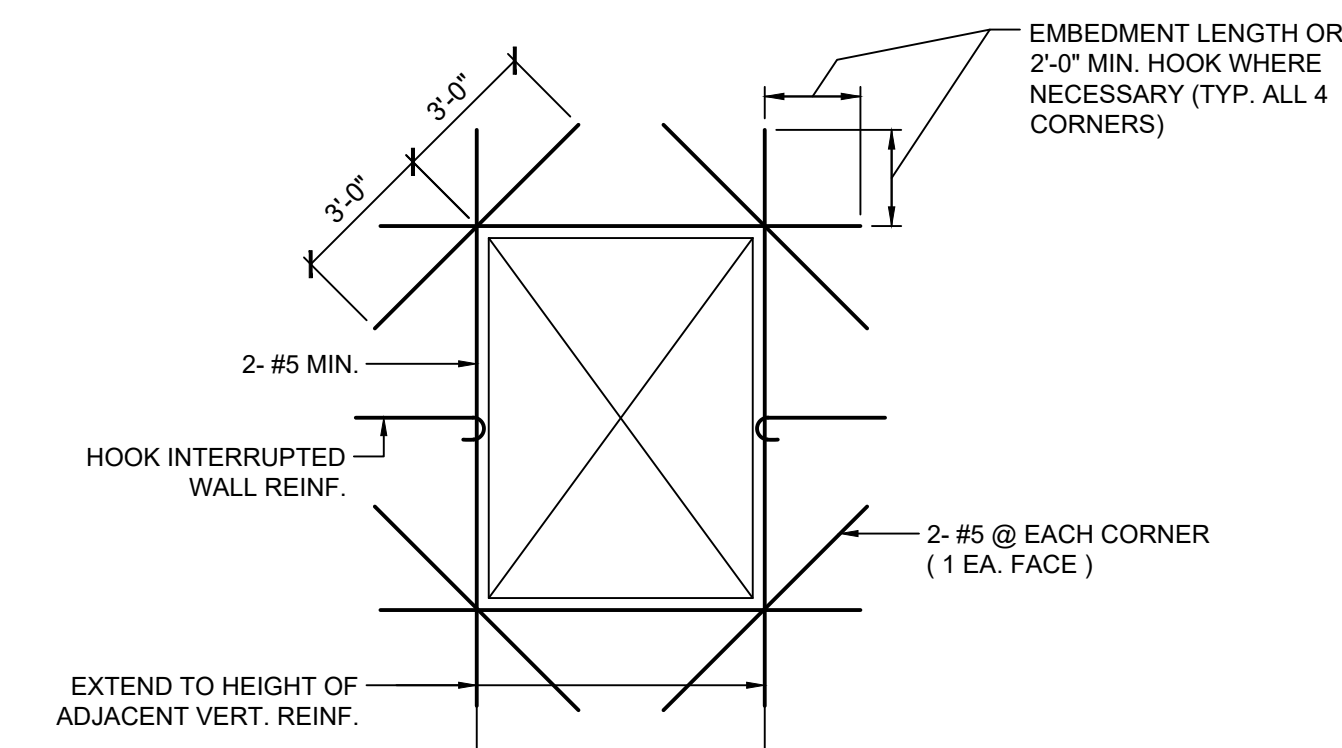
NOTES:
FOR CURB SIZE & LOCATION SEE ARCHITECTURAL DRAWINGS

6 TYPICAL CONCRETE CURB PARALLEL TO DECK
3/4" = 1'-0"



NOTES:
FOR CURB SIZE & LOCATION SEE ARCHITECTURAL DRAWINGS

7 TYPICAL CONCRETE CURB PERPENDICULAR TO DECK
3/4" = 1'-0"



TYPICAL WALL OPENING DETAILS
SCALE: N.T.S.

NOTES:
1. HOOK ALL INTERRUPTED CONTINUOUS BARS.
2. ONE HALF OF REINFORCING BAR AREA INTERRUPTED BY OPENING SHALL BE PROVIDED EACH SIDE OF OPENINGS ON THE SAME FACE, MIN. 2- #5 EACH SIDE.

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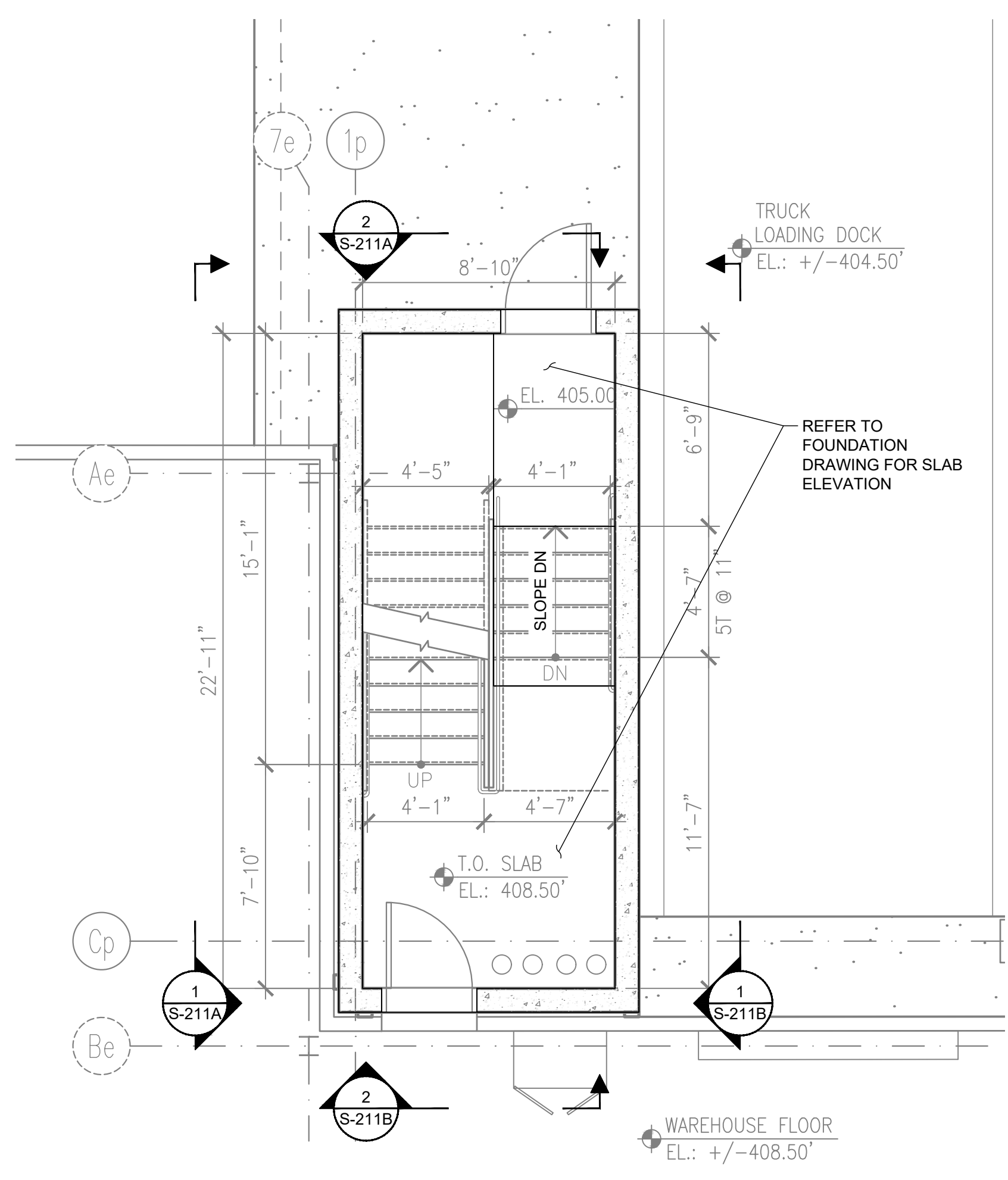
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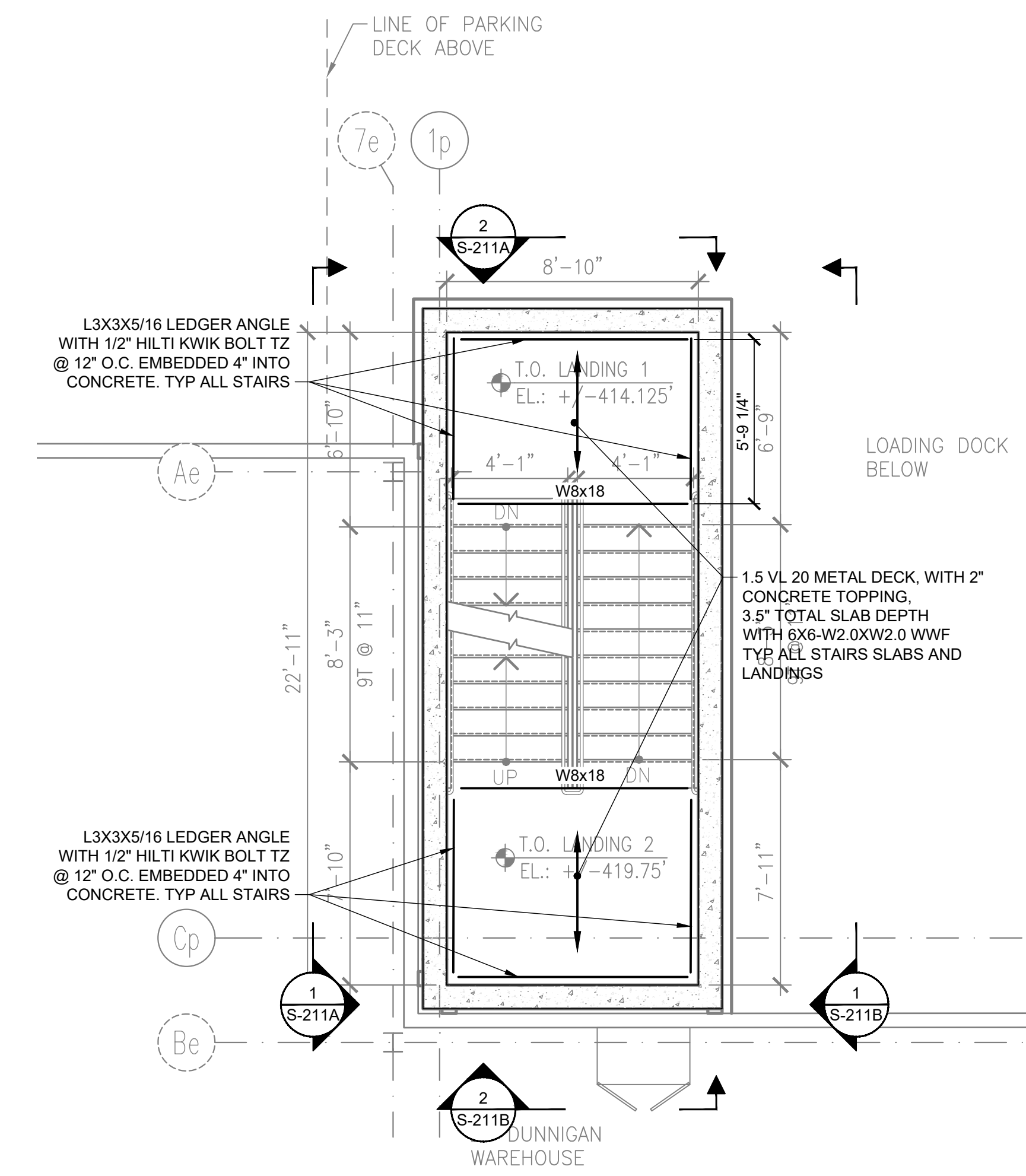
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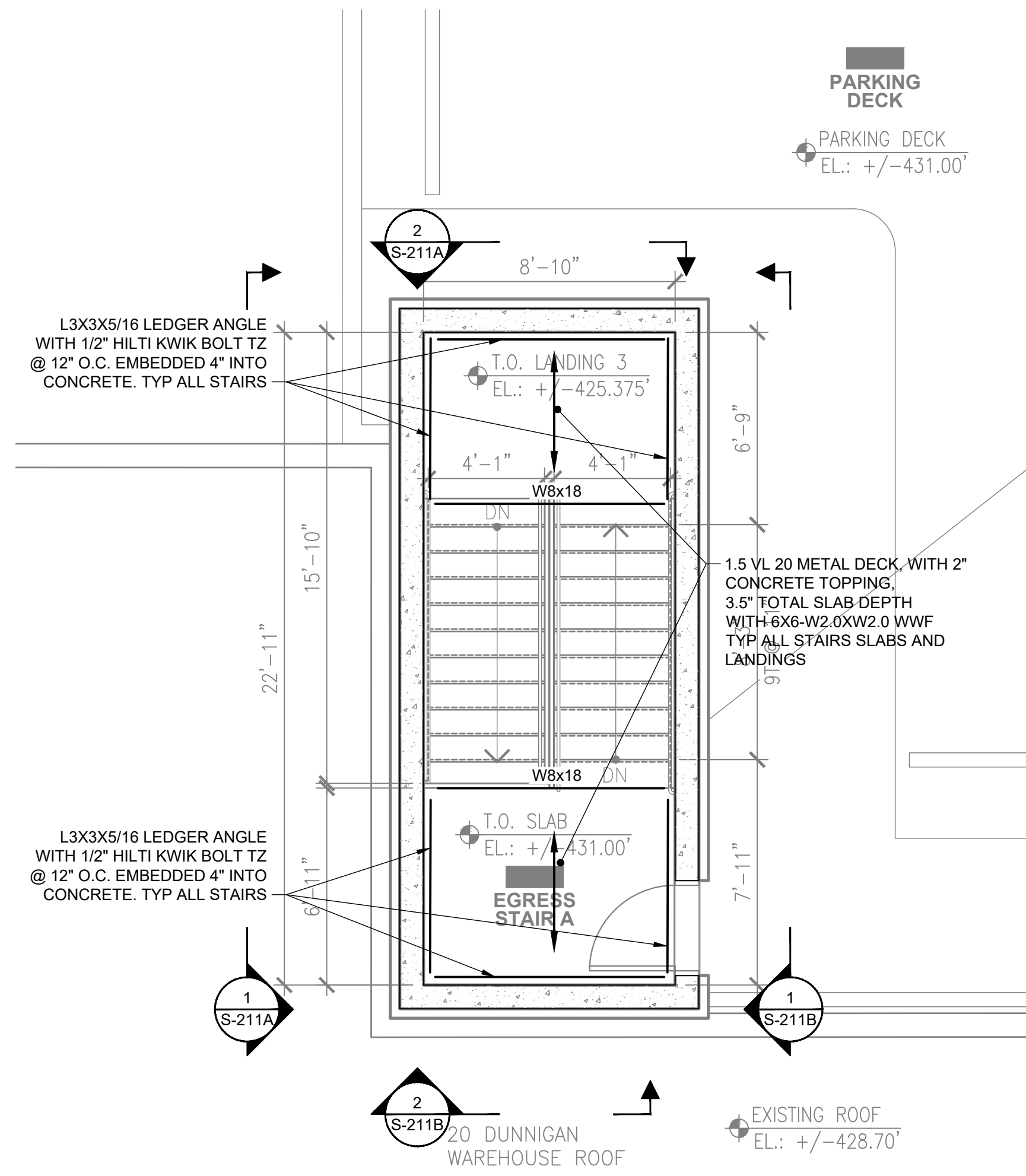
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1
 STAIR A - ENLARGED PLAN
 WAREHOUSE LEVEL
 SCALE: 1/4" = 1'-0"



2
 STAIR A - ENLARGED PLAN
 INTERMEDIATE LEVEL
 SCALE: 1/4" = 1'-0"



3
 STAIR A - ENLARGED PLAN
 PARKING DECK LEVEL
 SCALE: 1/4" = 1'-0"

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STAIR A - PLANS

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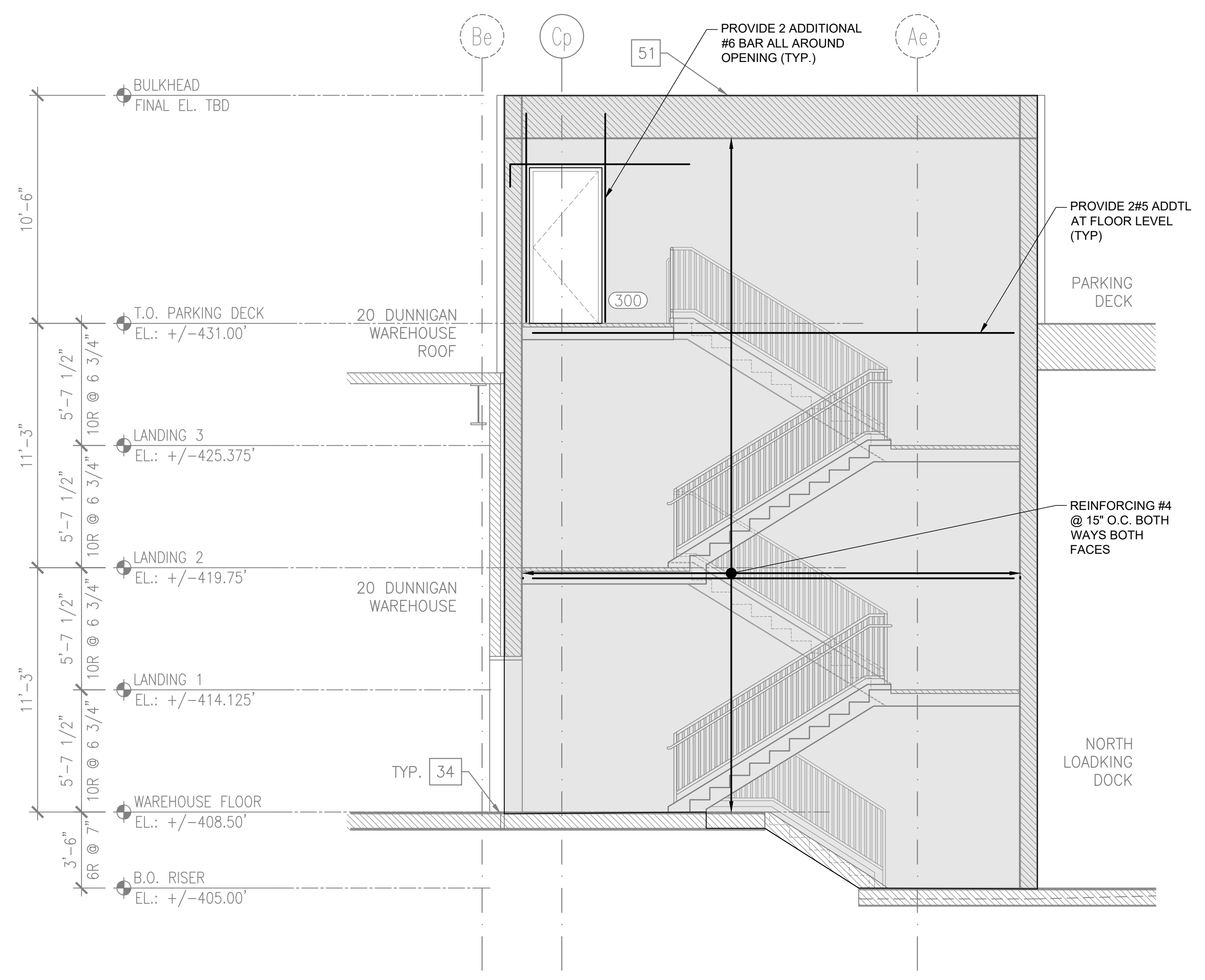
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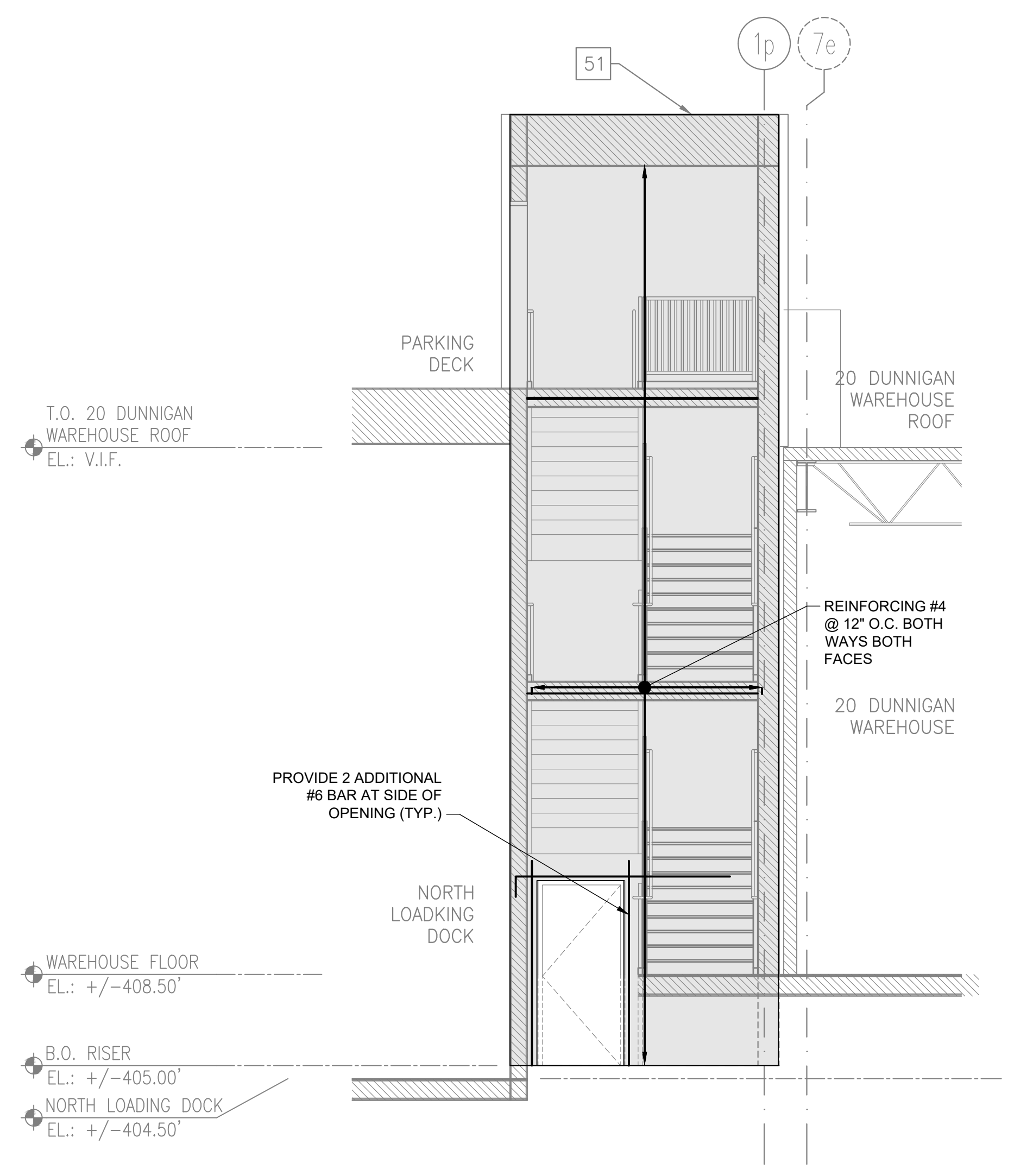
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1
 210 STAIR A - ELEVATION (EAST)
 SCALE: 1/4" = 1'-0"



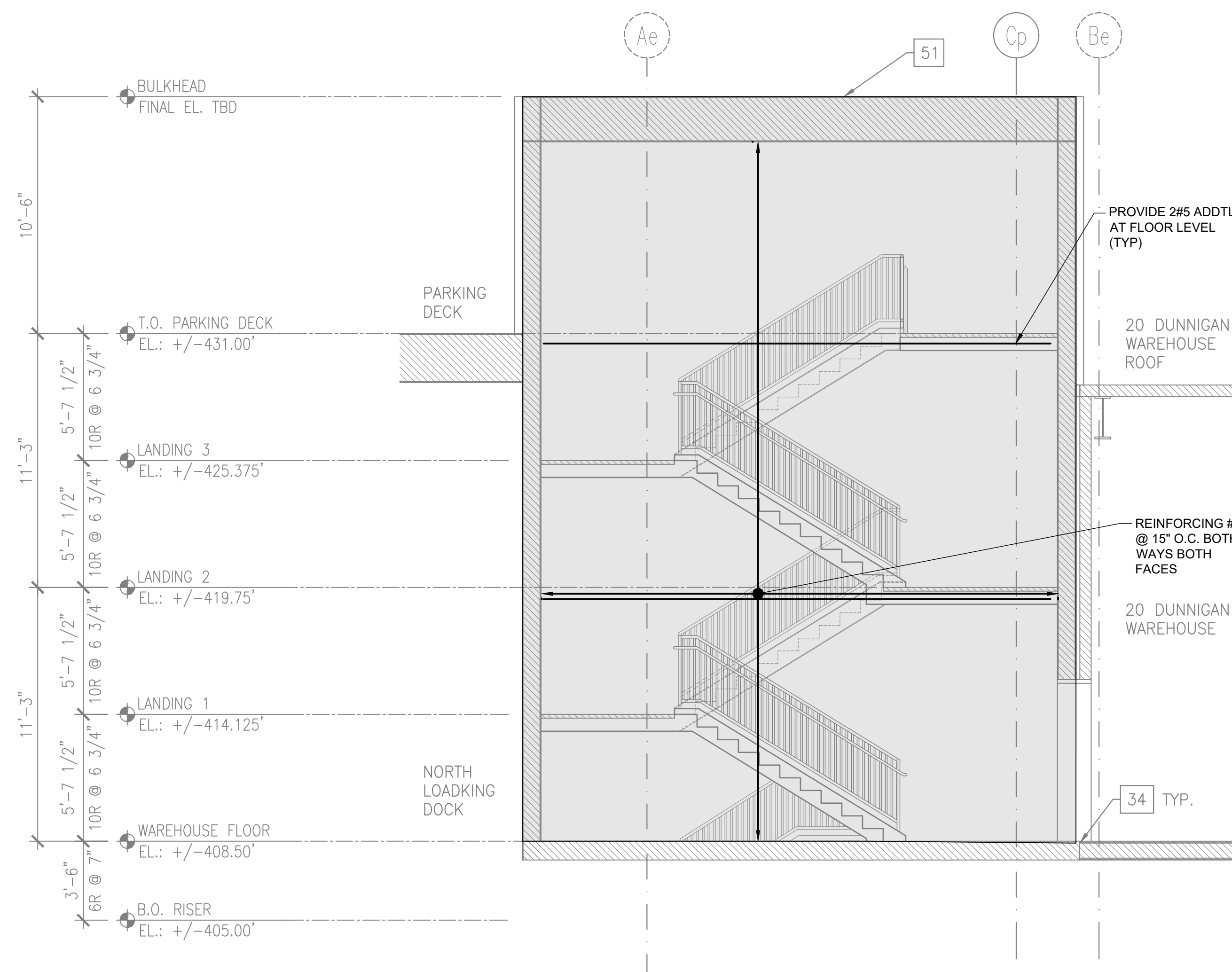
2
 210 STAIR A - ELEVATION (SOUTH)
 SCALE: 1/4" = 1'-0"

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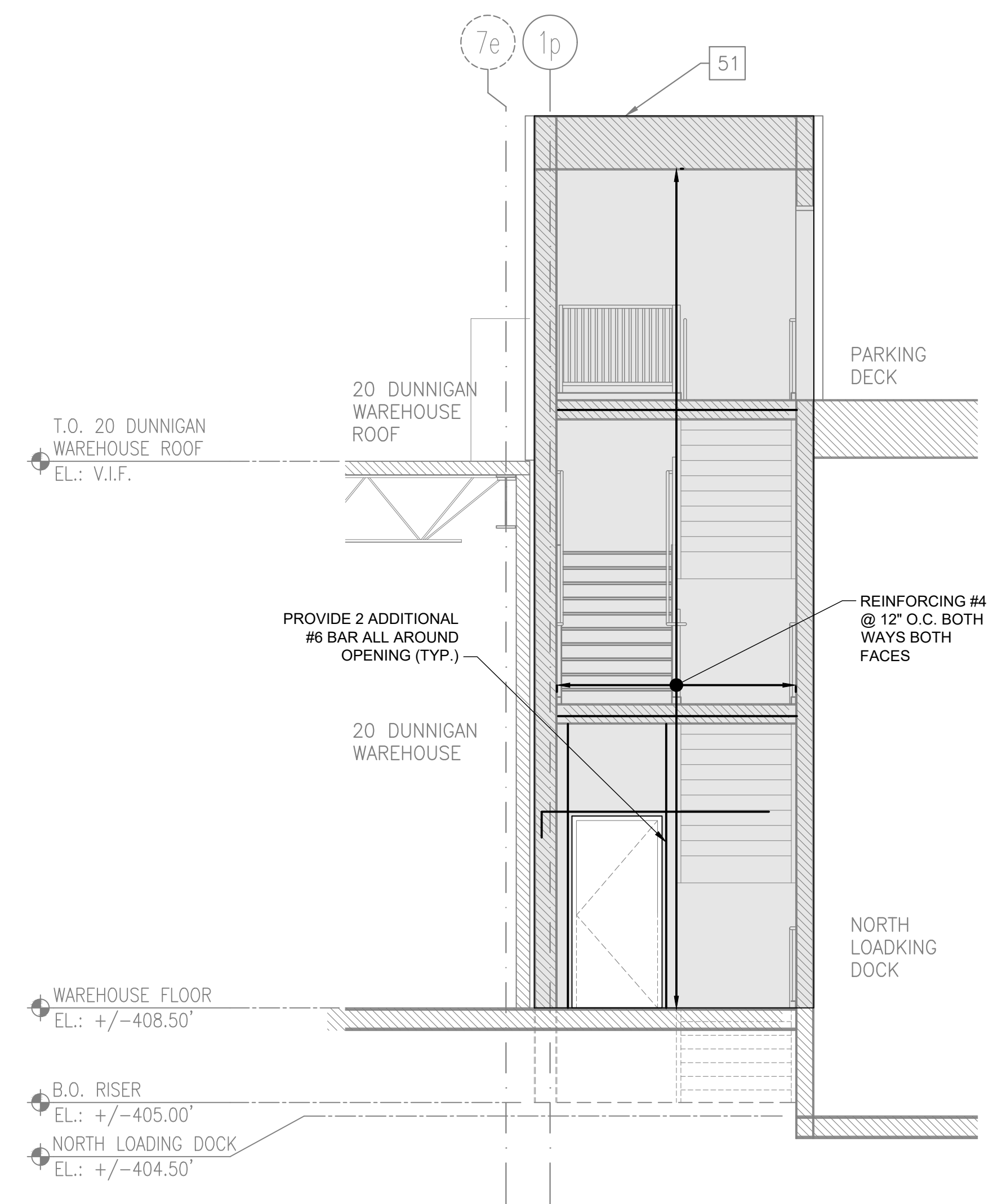
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 SCALE : AS SHOWN

DRAWING TITLE :
 STAIR A - ELEVATIONS

DWG NUMBER :
S-211B
 27 OF 60



1 STAIR A - ELEVATION (WEST)
210 SCALE: 1/4" = 1'-0"



2 STAIR A - ELEVATION (NORTH)
210 SCALE: 1/4" = 1'-0"

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DRAWING TITLE :

STAIR A - ELEVATIONS

DWG NUMBER :
S-211A

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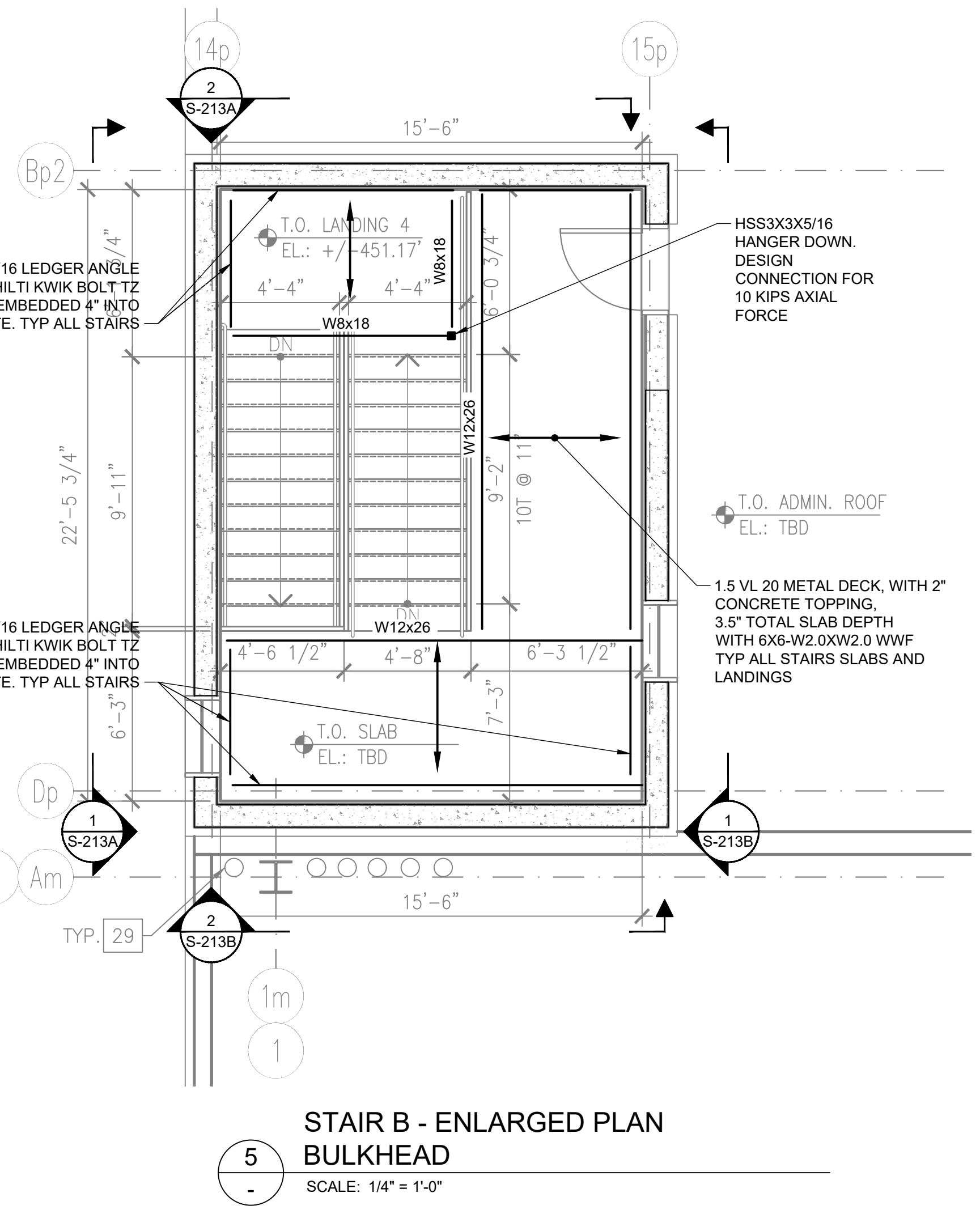
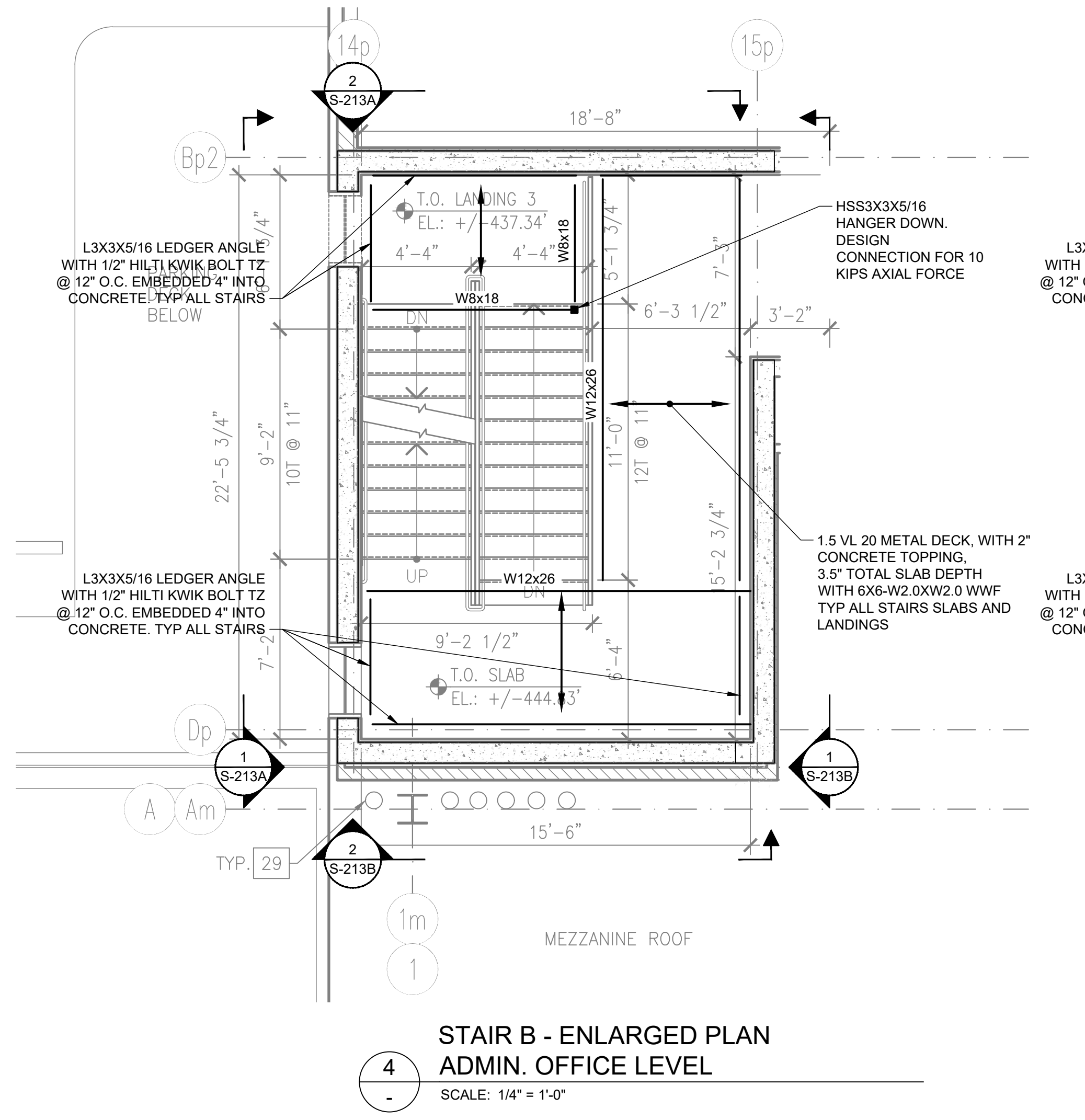
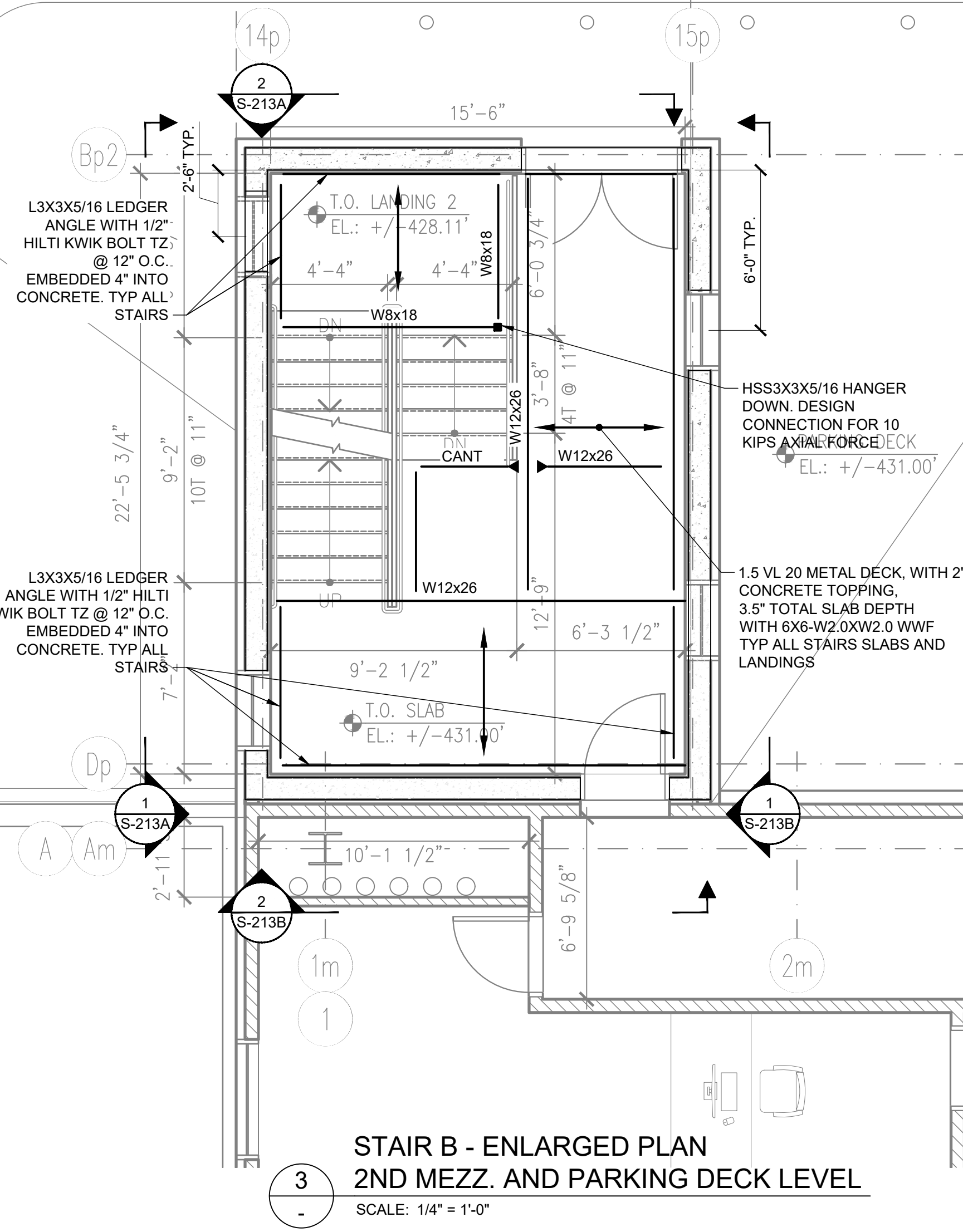
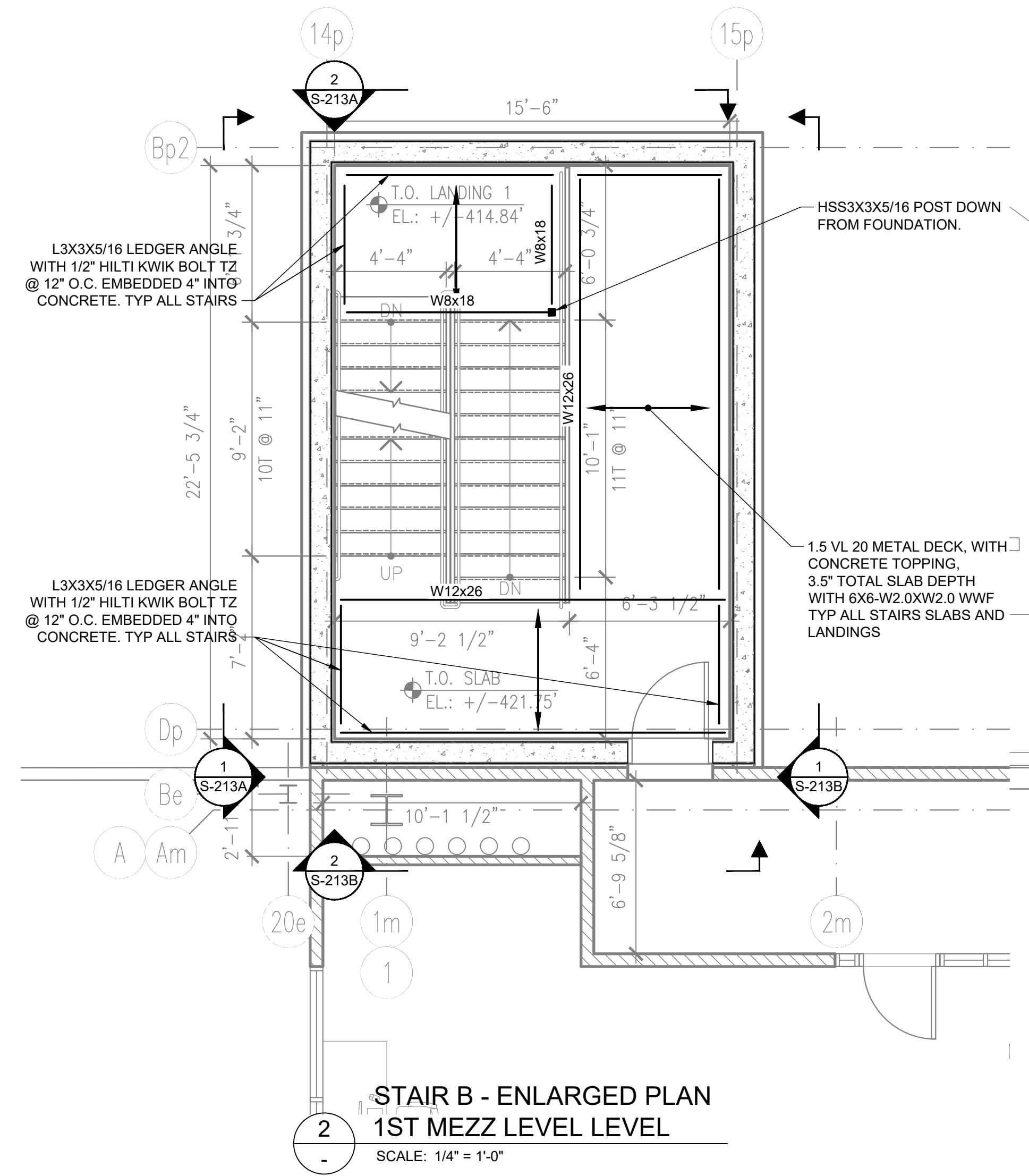
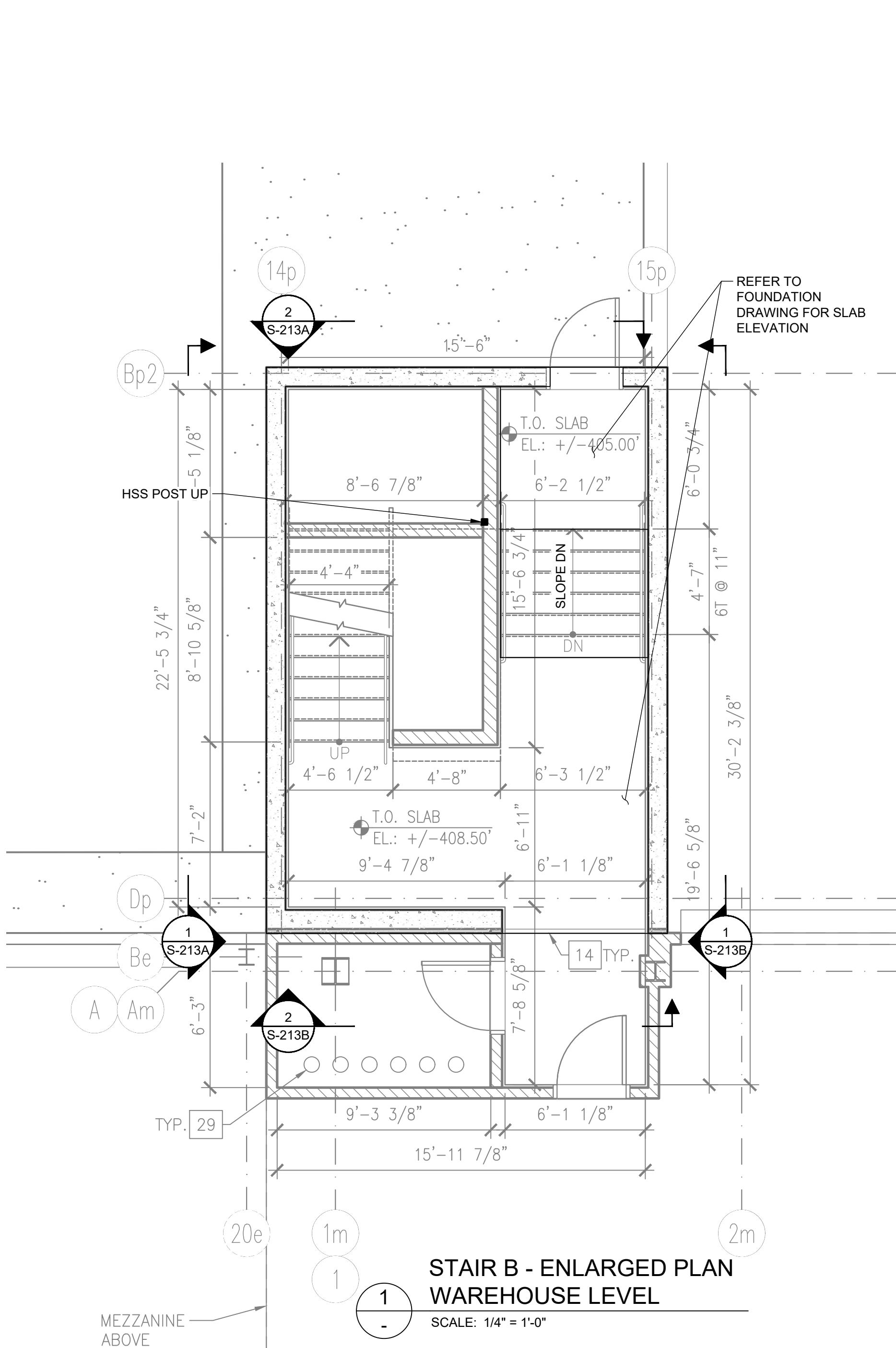
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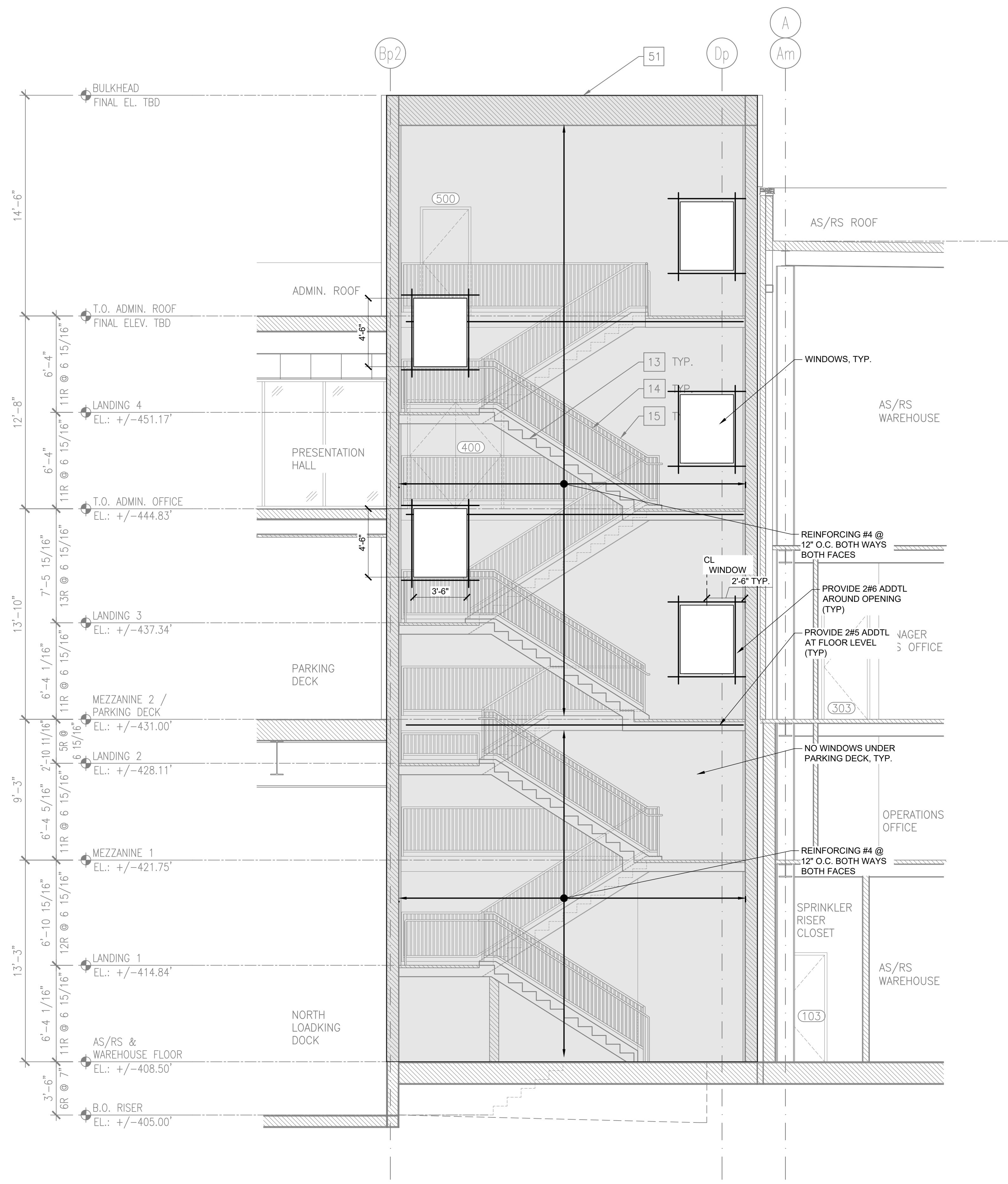
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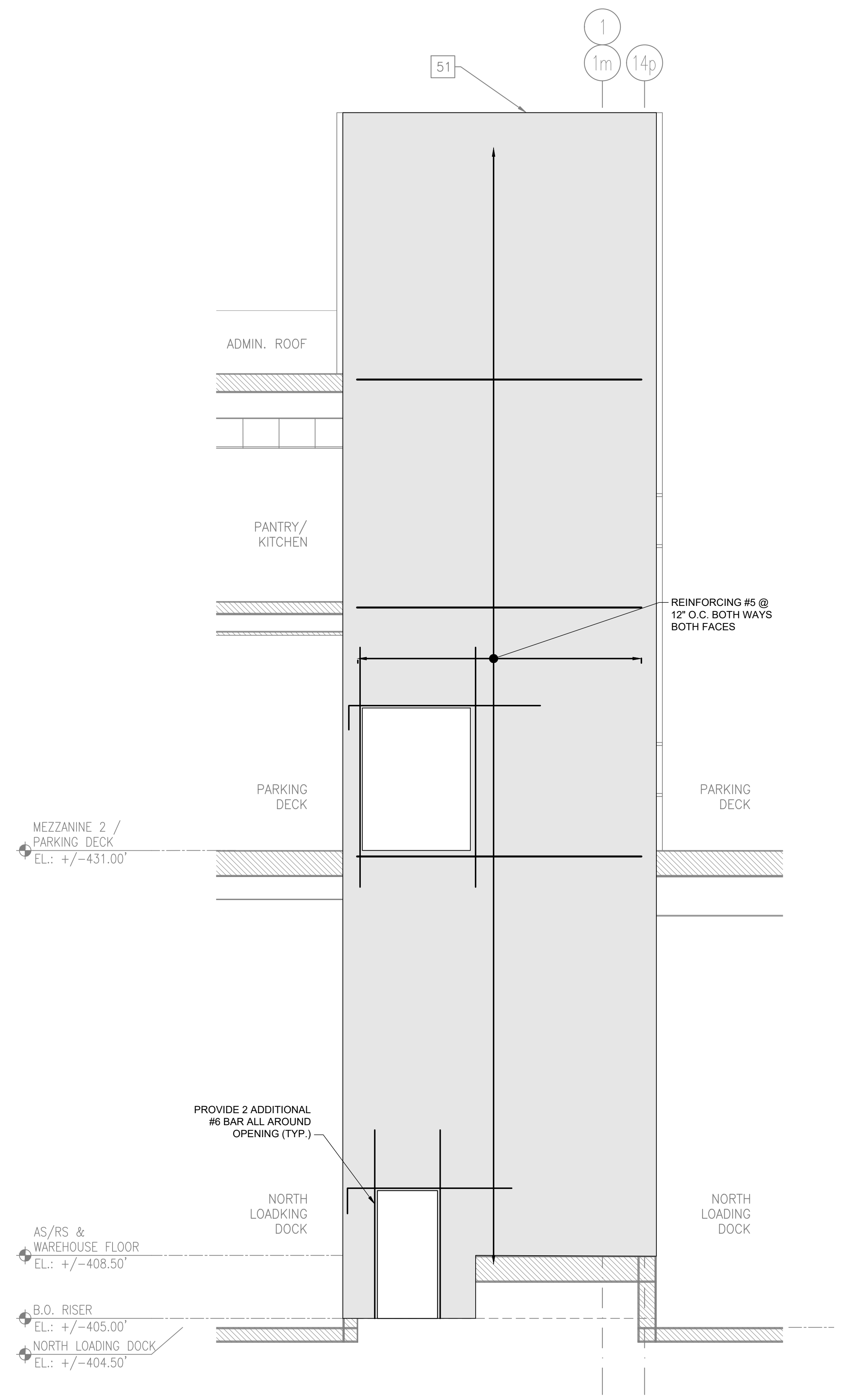
STAIR B - PLANS

DWG NUMBER:
S-212
 28 OF 60





1 STAIR B - ELEVATION (WEST)
212 SCALE: 1/4" = 1'-0"



2 STAIR B - ELEVATION (NORTH)
212 SCALE: 1/4" = 1'-0"

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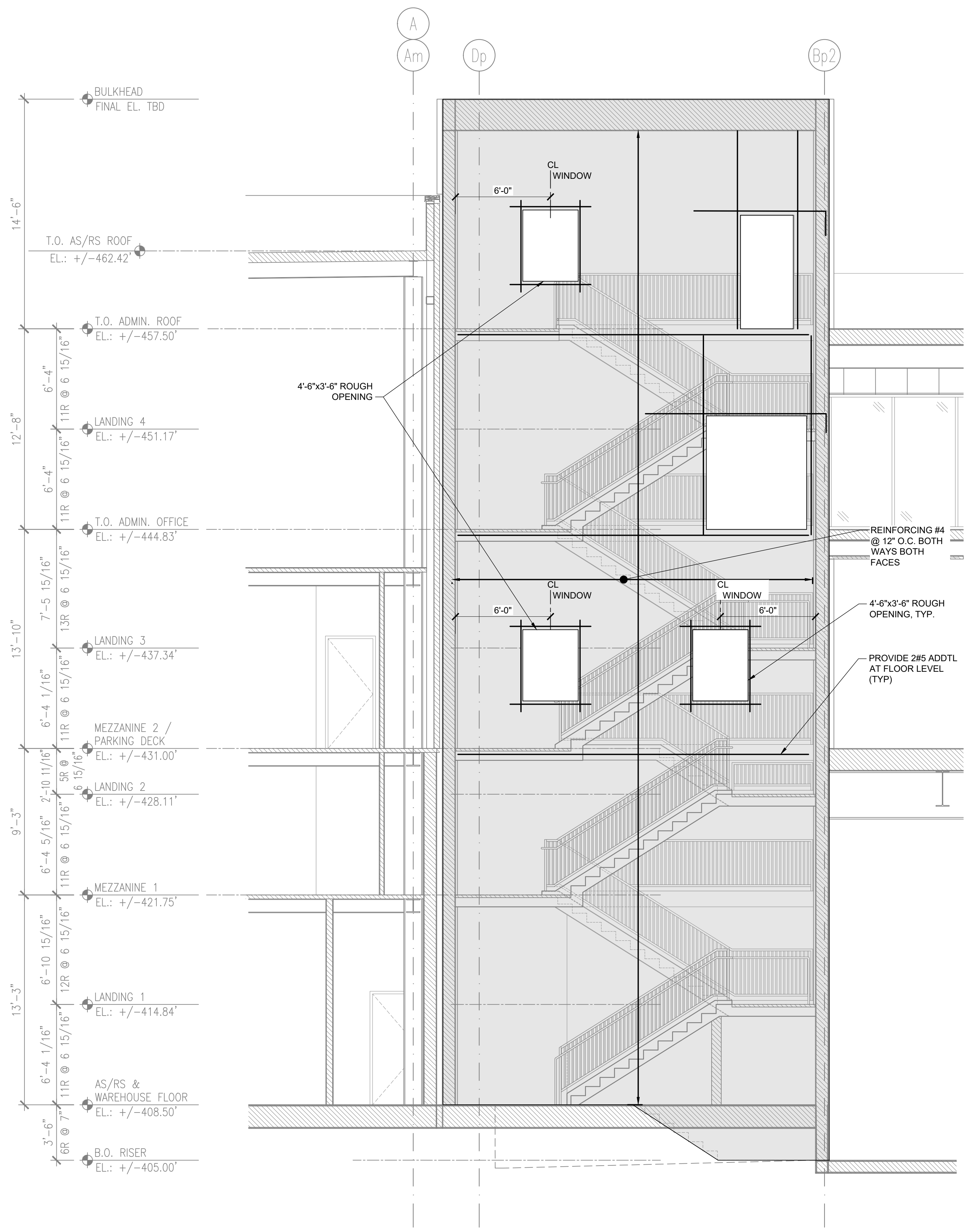
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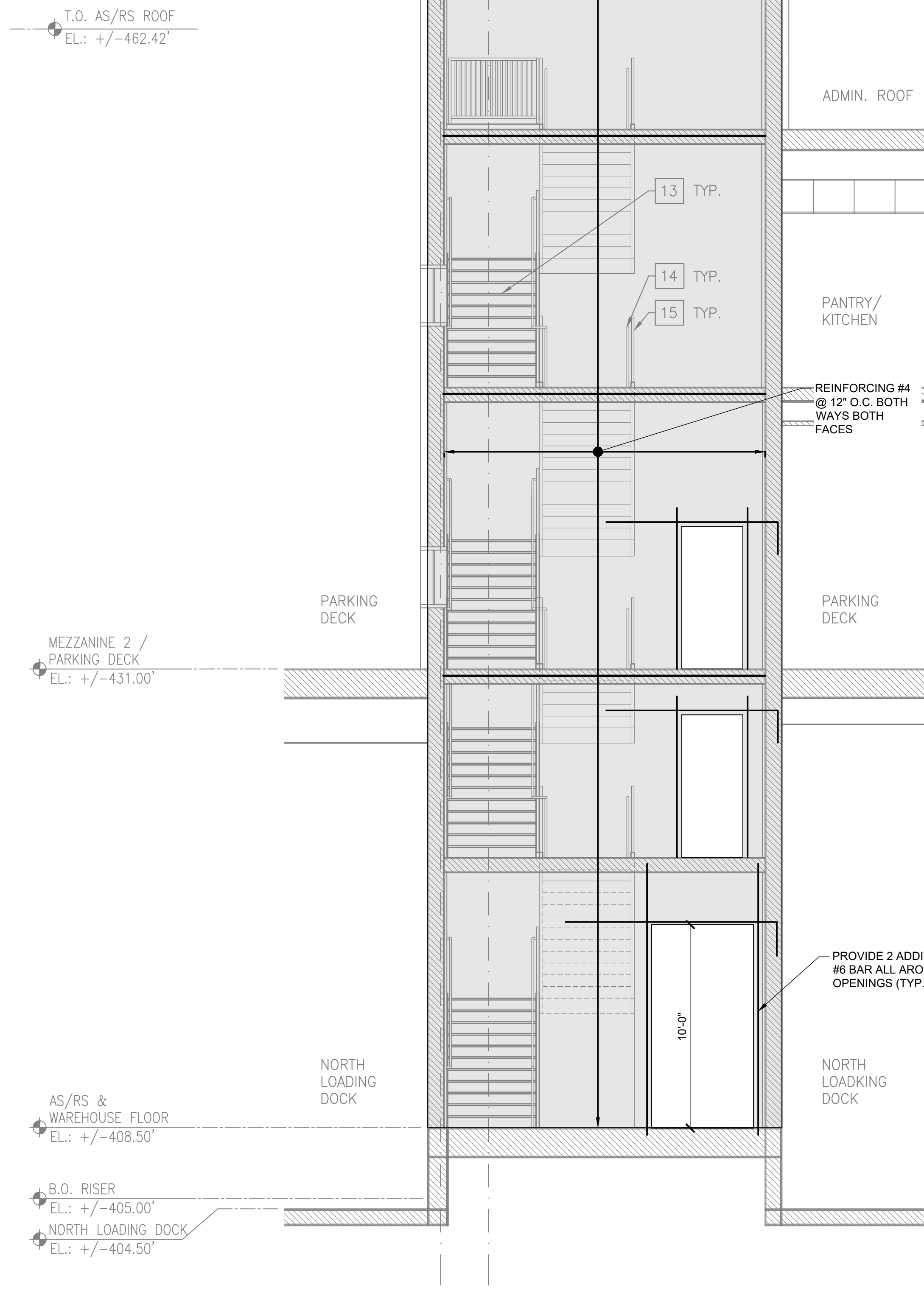
DRAWING TITLE :

STAIR B - ELEVATIONS

DWG NUMBER :
S-213A
29 OF 60



1 STAIR B - ELEVATION (EAST)
SCALE: 1/4" = 1'-0"



2 STAIR B - ELEVATION (SOUTH)
SCALE: 1/4" = 1'-0"

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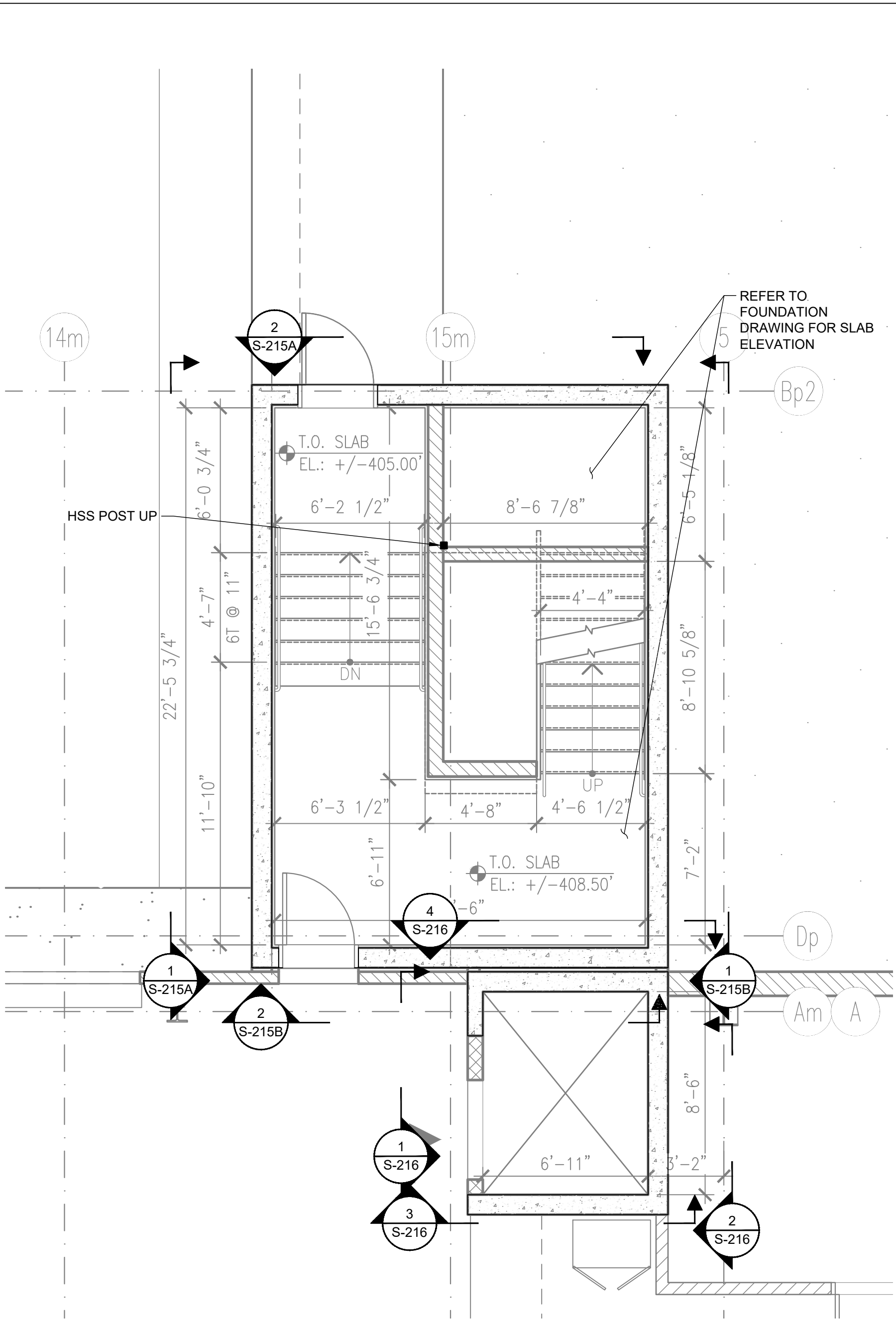
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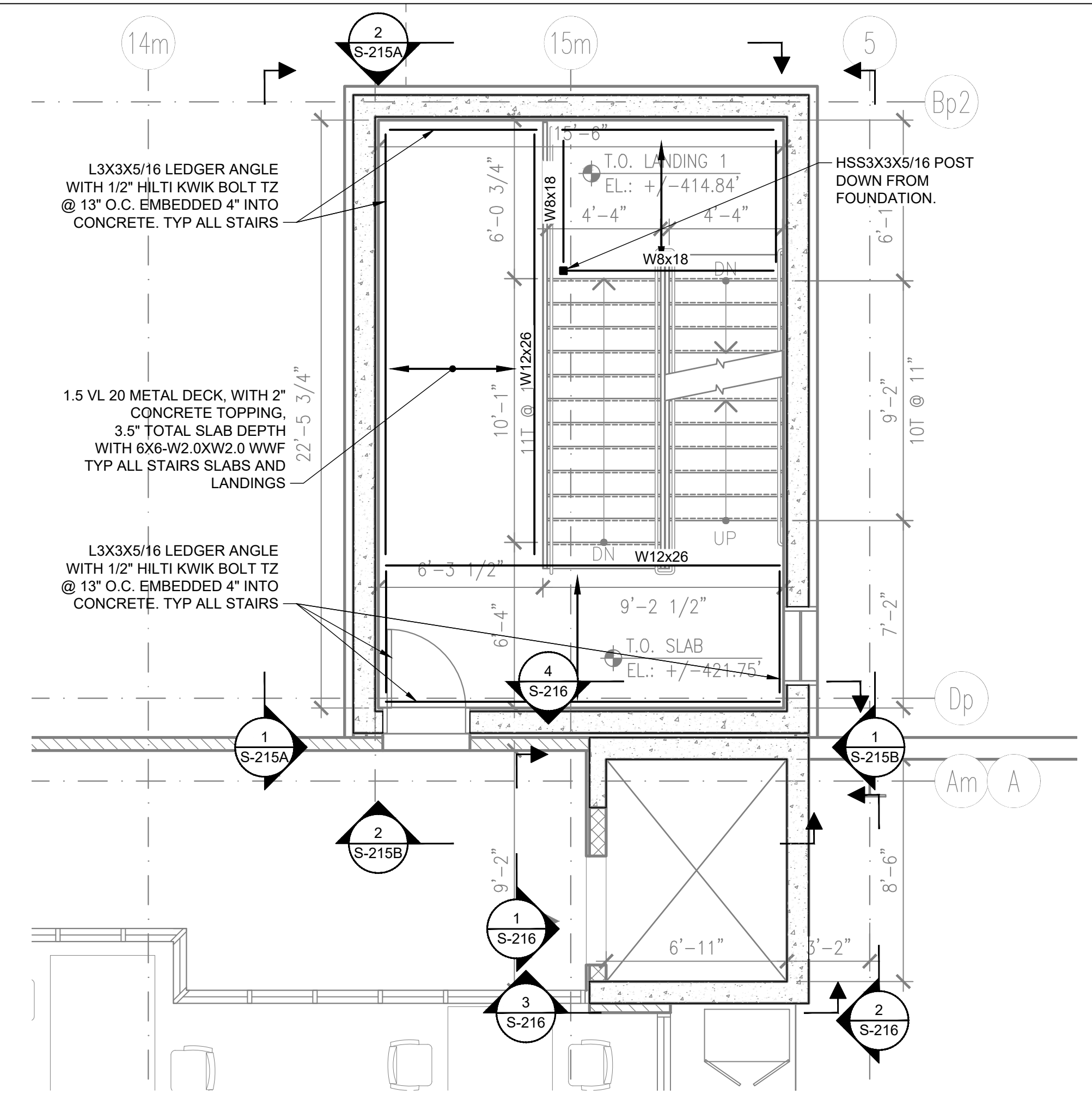
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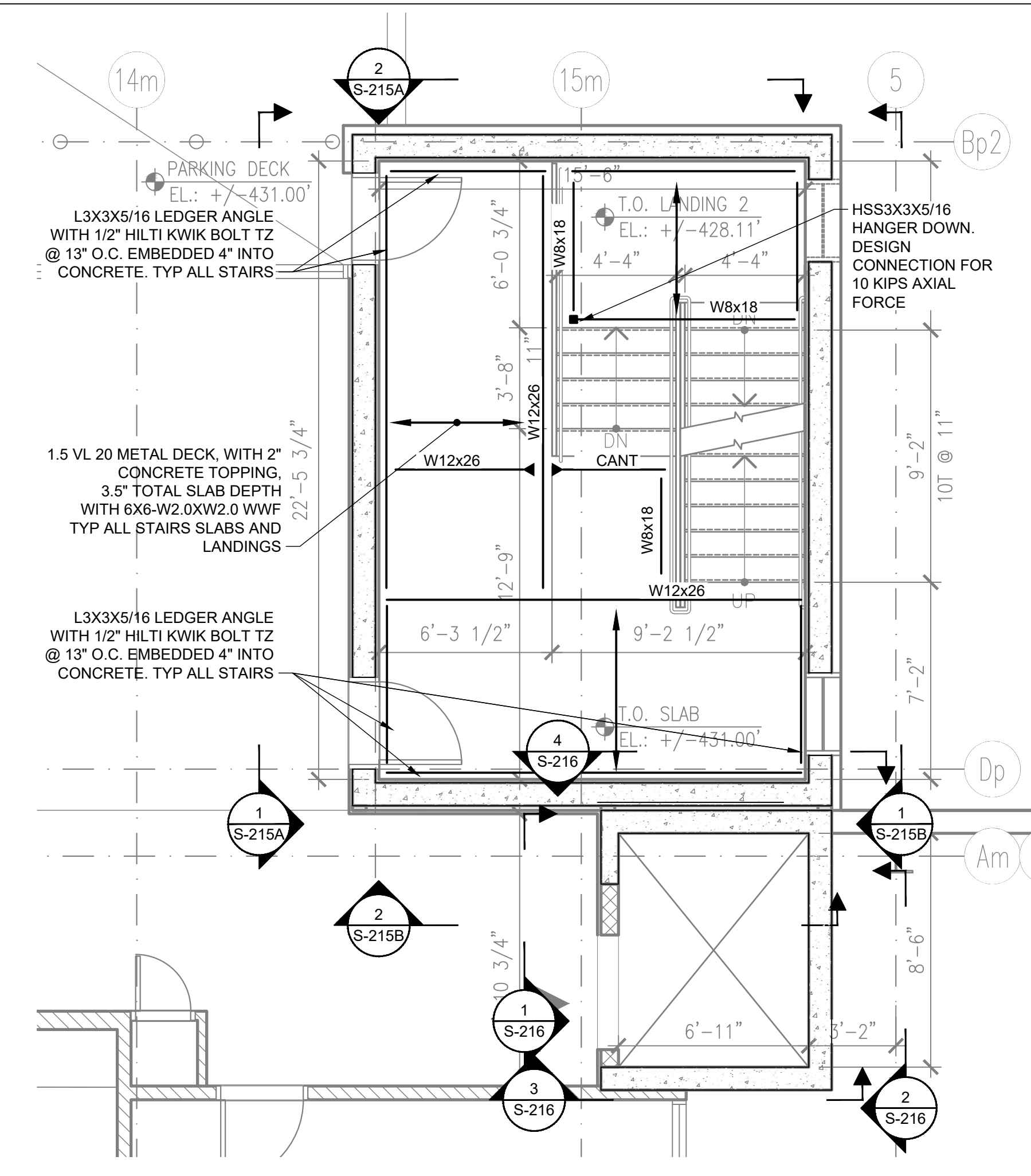
STAIR B - ELEVATIONS
DWG NUMBER :
S-213B
30 OF 60



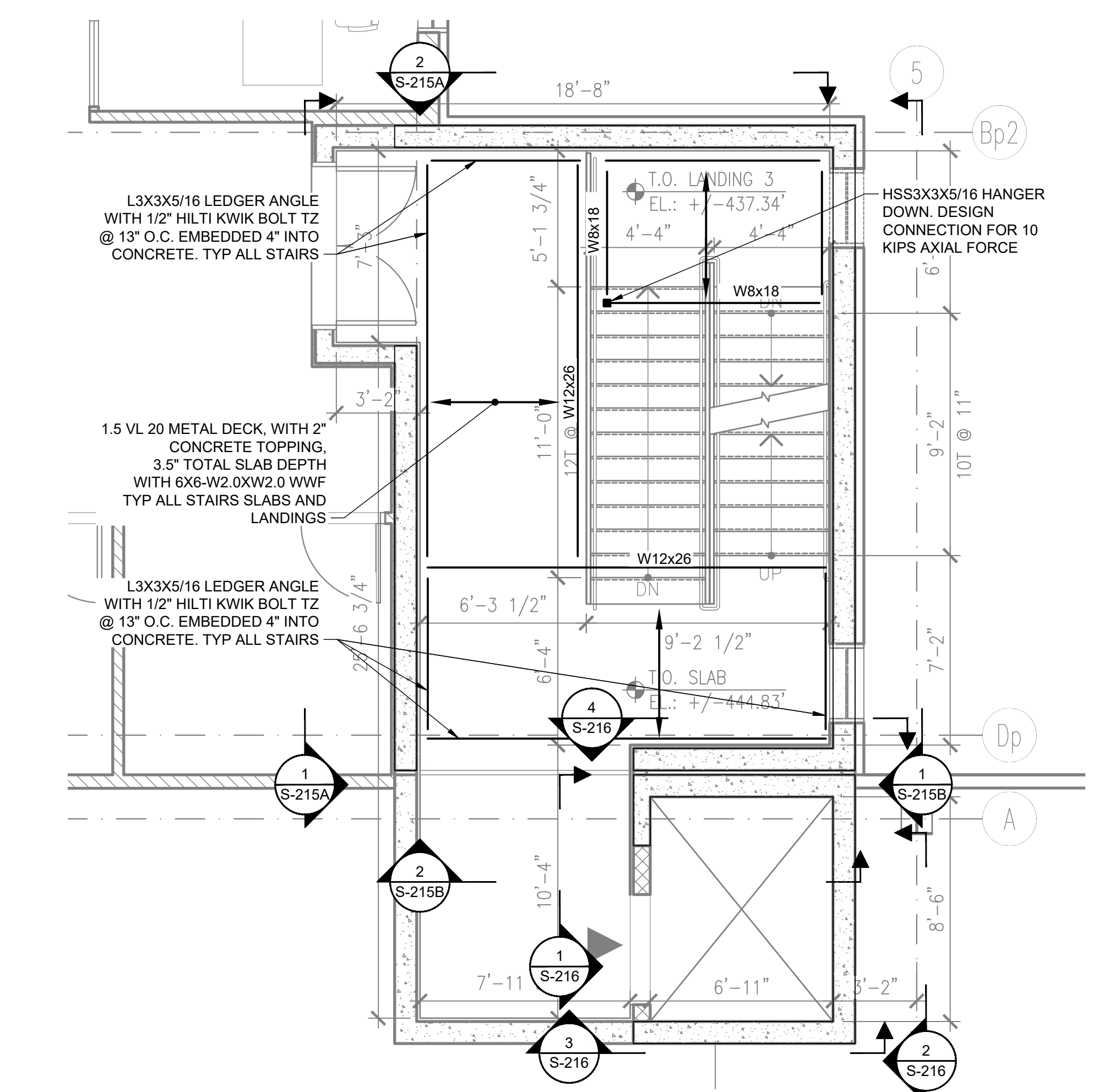
1 STAIR C - ENLARGED PLAN - WAREHOUSE LEVEL
SCALE: 1/4" = 1'-0"



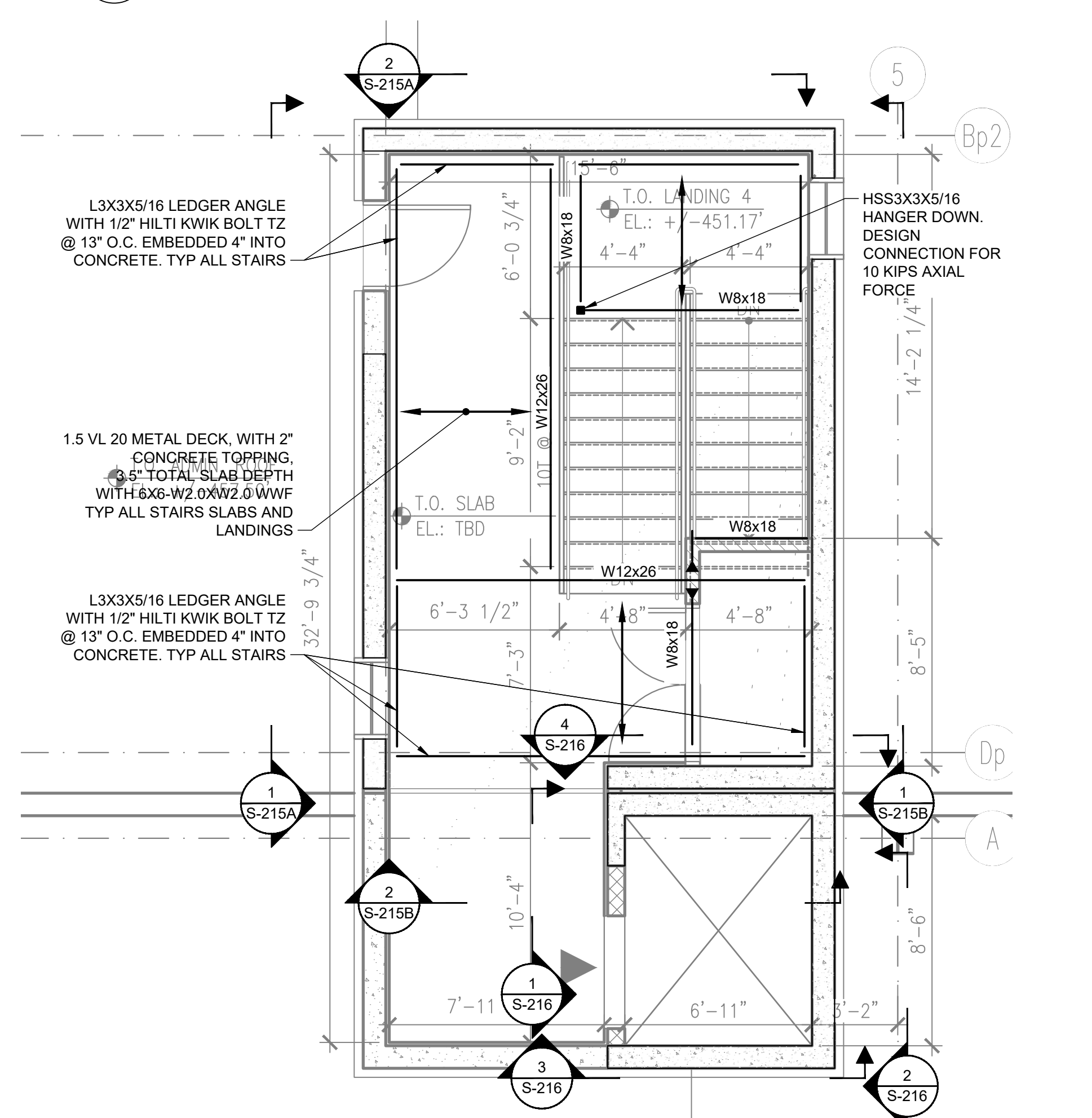
2 STAIR C - ENLARGED PLAN - 1ST MEZZ LEVEL LEVEL
SCALE: 1/4" = 1'-0"



3 STAIR C - ENLARGED PLAN - 2ND MEZZ. AND PARKING DECK LEVEL
SCALE: 1/4" = 1'-0"



4 STAIR C - ENLARGED PLAN - ADMIN. OFFICE LEVEL
SCALE: 1/4" = 1'-0"



5 STAIR C - ENLARGED PLAN - BULKHEAD
SCALE: 1/4" = 1'-0"

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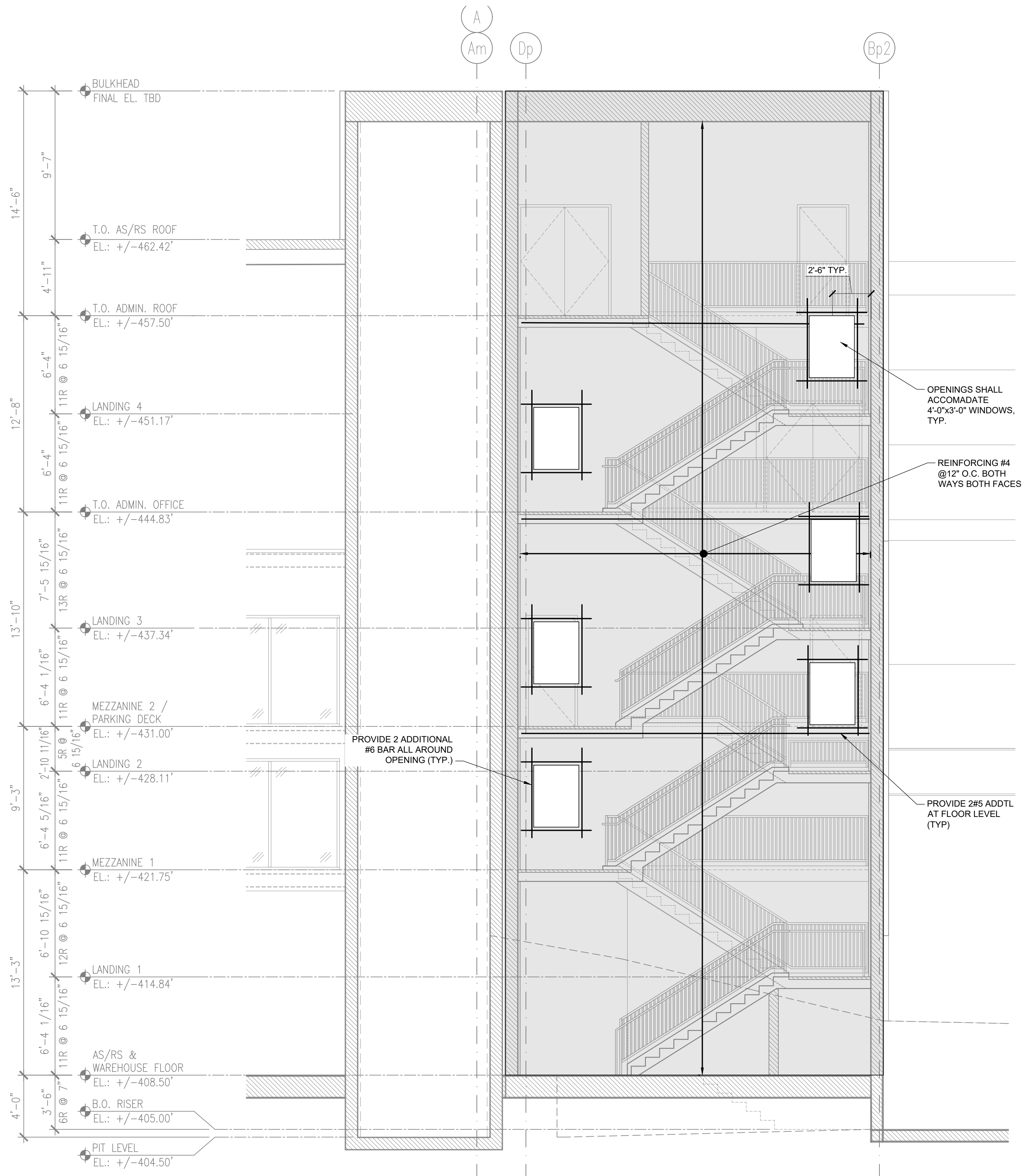
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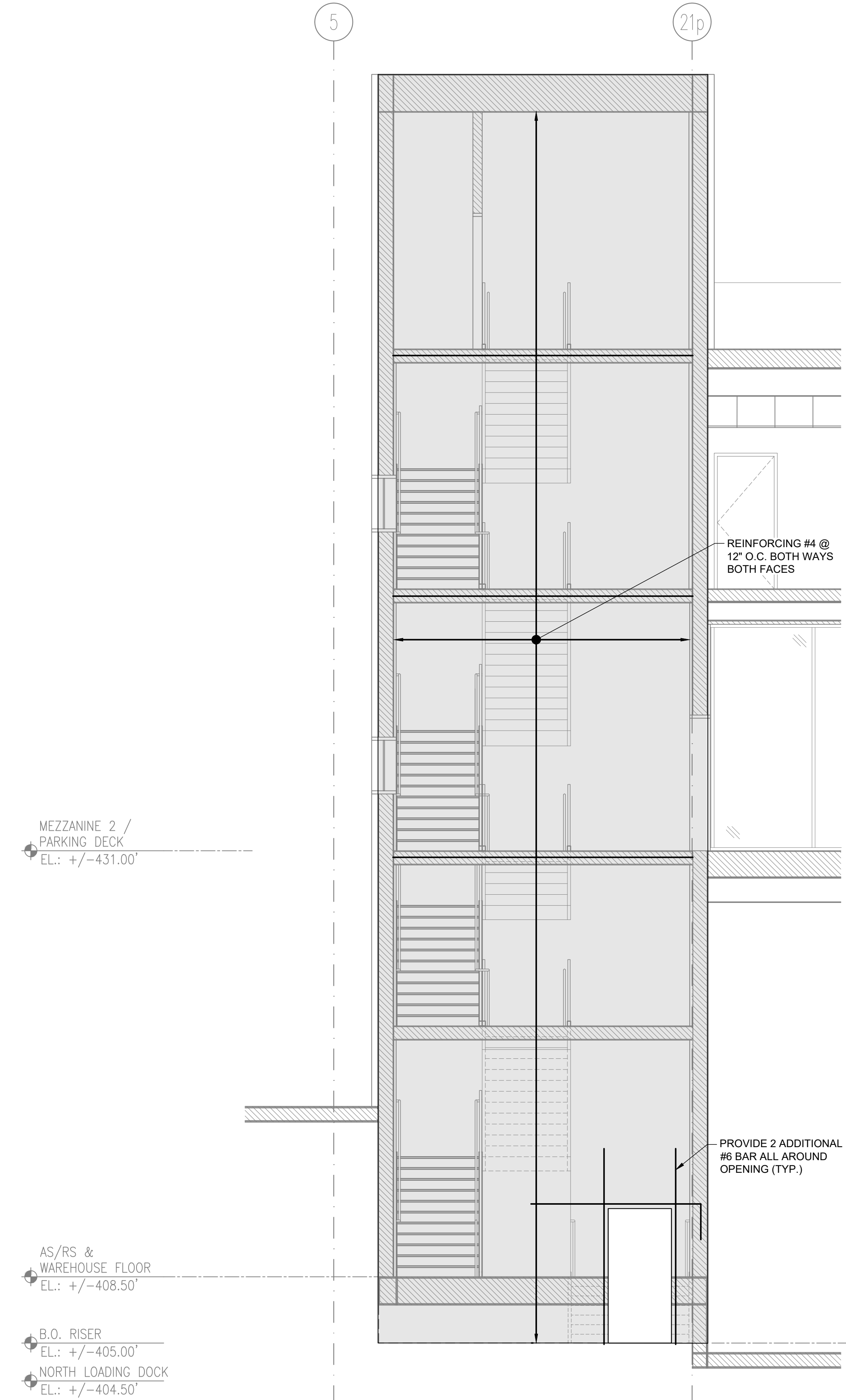
DRAWING TITLE:

STAIR C - PLANS

DWG NUMBER:
S-214
31 OF 60



1 STAIR C - ELEVATION (EAST)
214 SCALE: 1/4" = 1'-0"



2 STAIR C - ELEVATION (NORTH)
214 SCALE: 1/4" = 1'-0"

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DRAWING TITLE :

STAIR C - ELEVATIONS

DWG NUMBER :
S-215A
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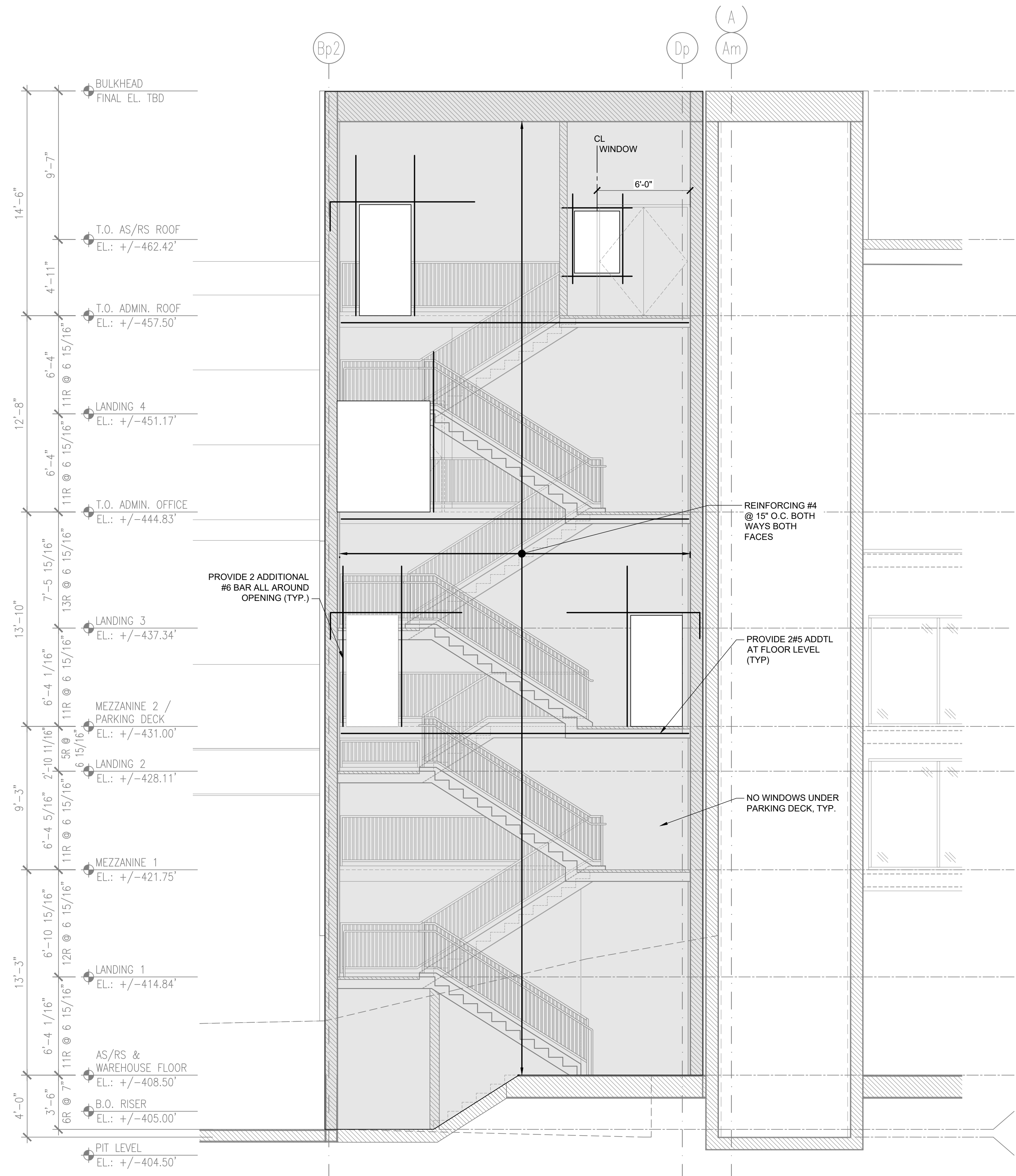
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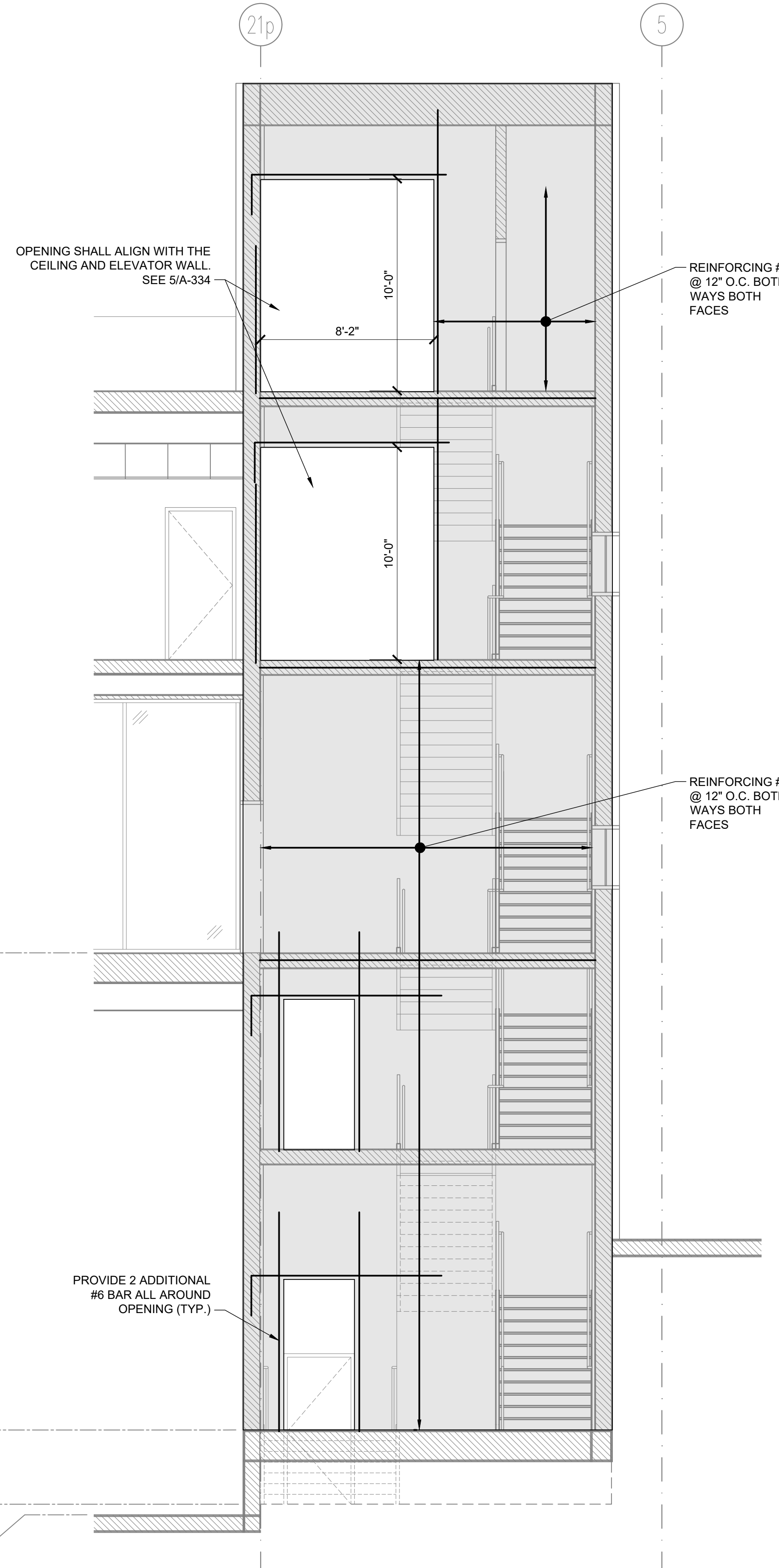
DRAWING TITLE :

STAIR C - ELEVATIONS

DWG NUMBER :
S-215B



1 STAIR C - ELEVATION (WEST)
 214 SCALE: 1/4" = 1'-0"



2 STAIR C - ELEVATION (SOUTH)
 214 SCALE: 1/4" = 1'-0"



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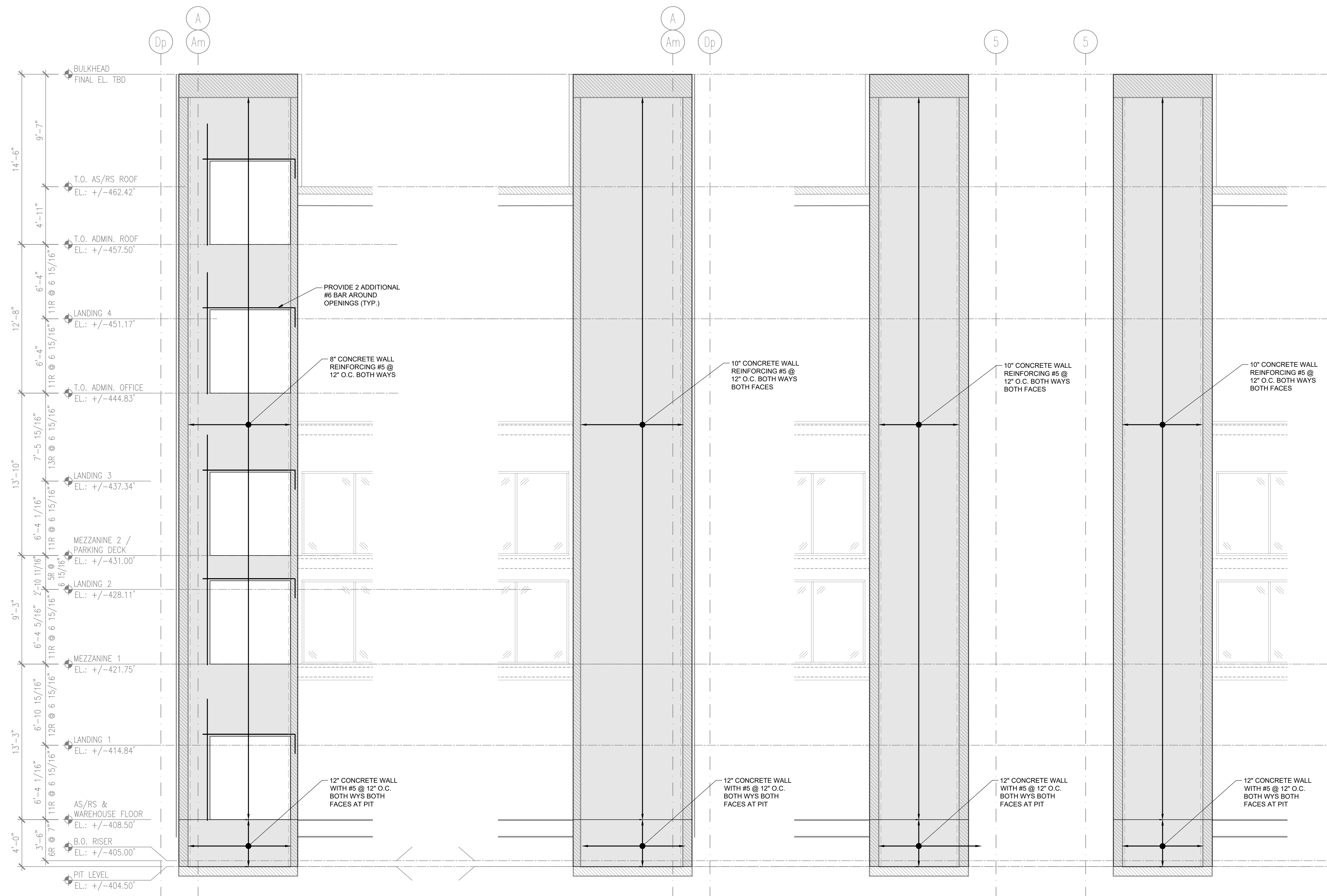
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DRAWING TITLE :

ELEVATOR ELEVATIONS

DWG NUMBER :
S-216
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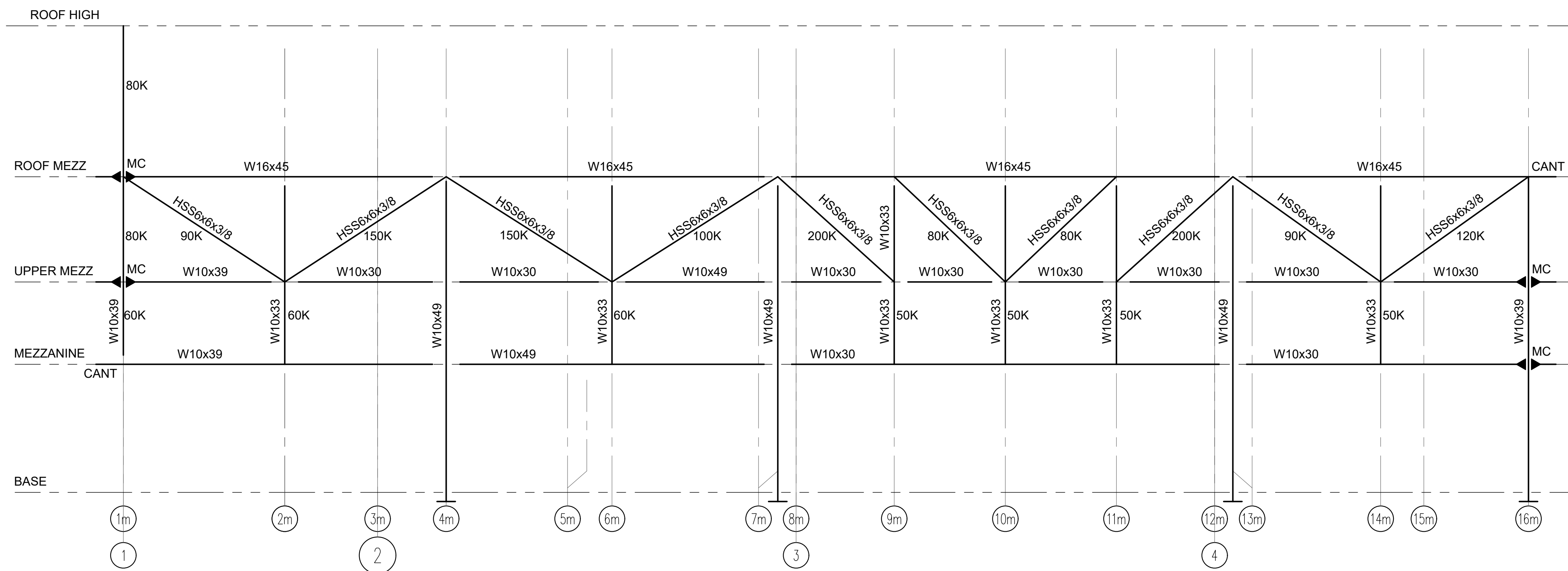


1 ELEVATOR - ELEVATION (WEST)
 214 SCALE: 1/4" = 1'-0"

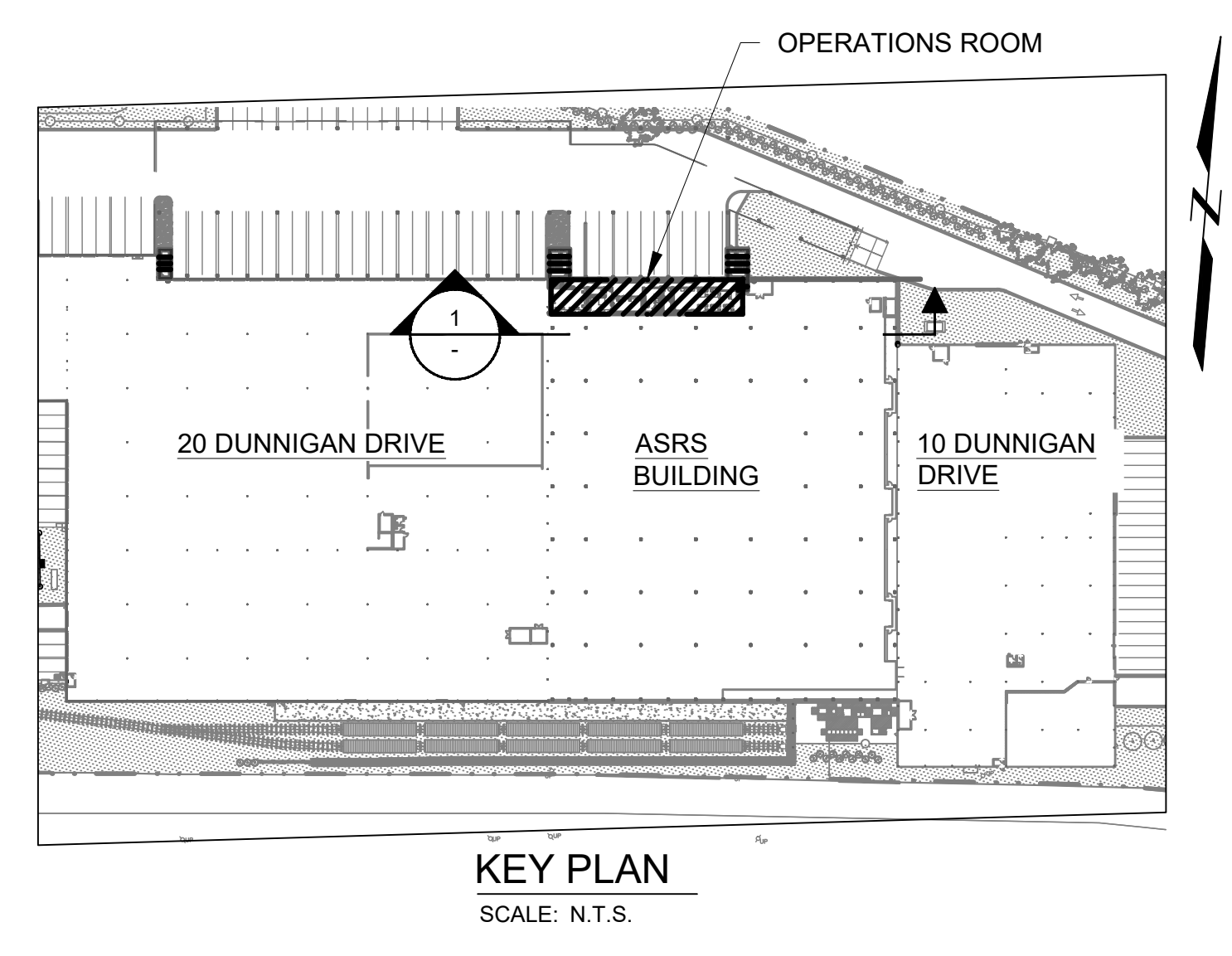
2 ELEVATOR - ELEVATION (EAST)
 214 SCALE: 1/4" = 1'-0"

3 ELEVATOR - ELEVATION (SOUTH)
 - SCALE: 1/4" = 1'-0"

4 ELEVATOR - ELEVATION (NORTH)
 - SCALE: 1/4" = 1'-0"



ASRS OPERATIONS BUILDING SOUTH ELEVATION
ELEVATION Bm
SCALE: 1/8" = 1'-0"



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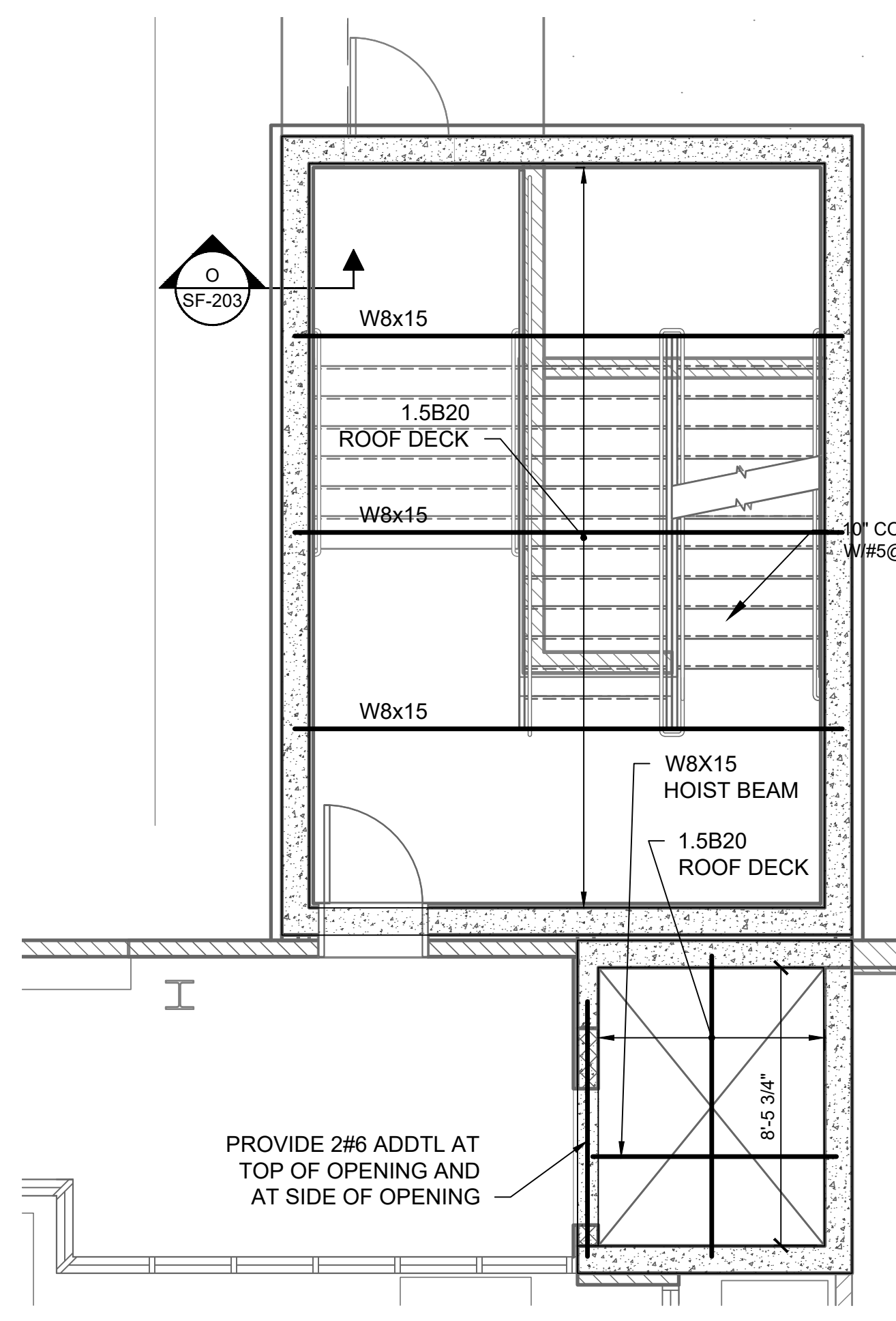
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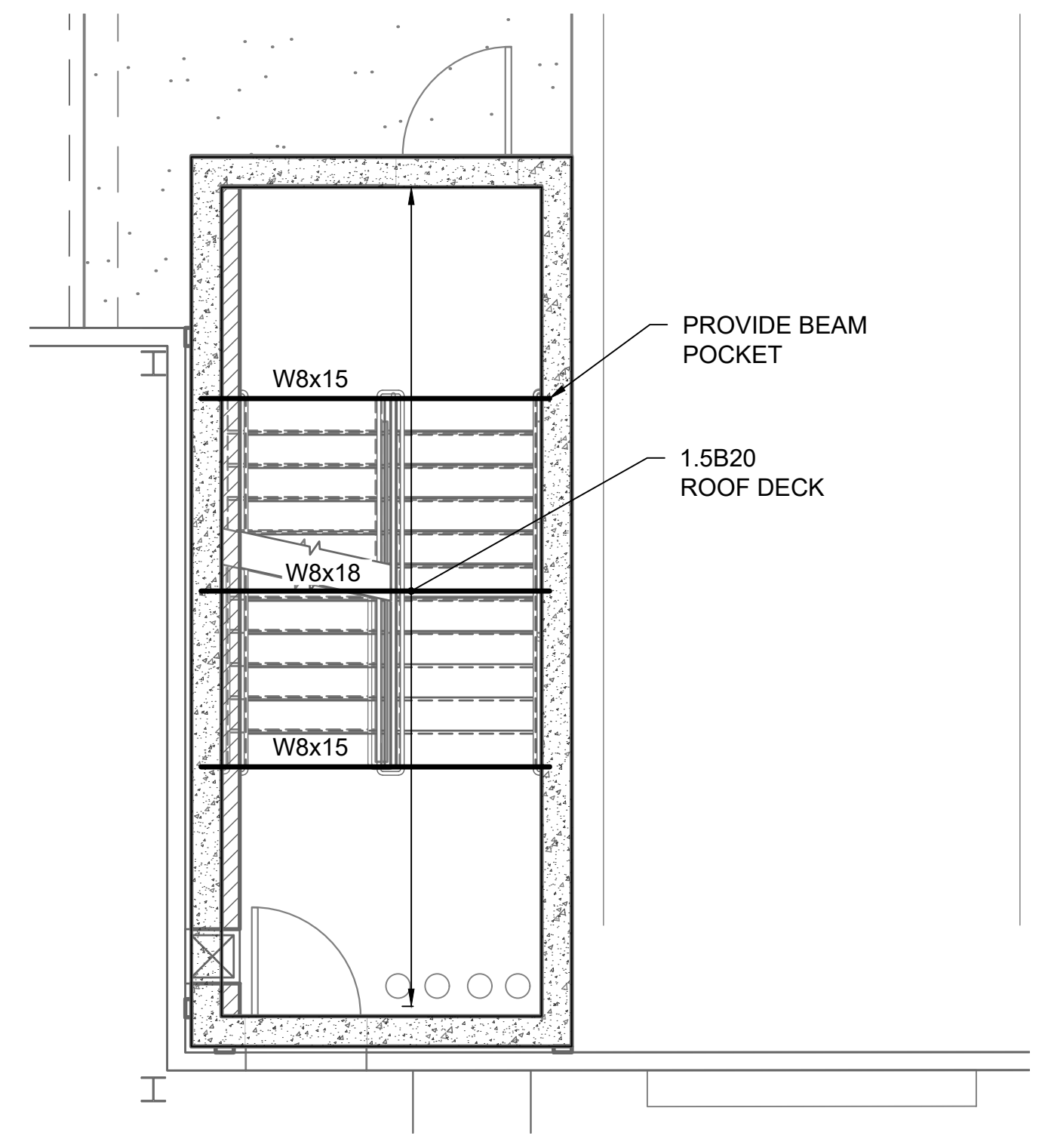
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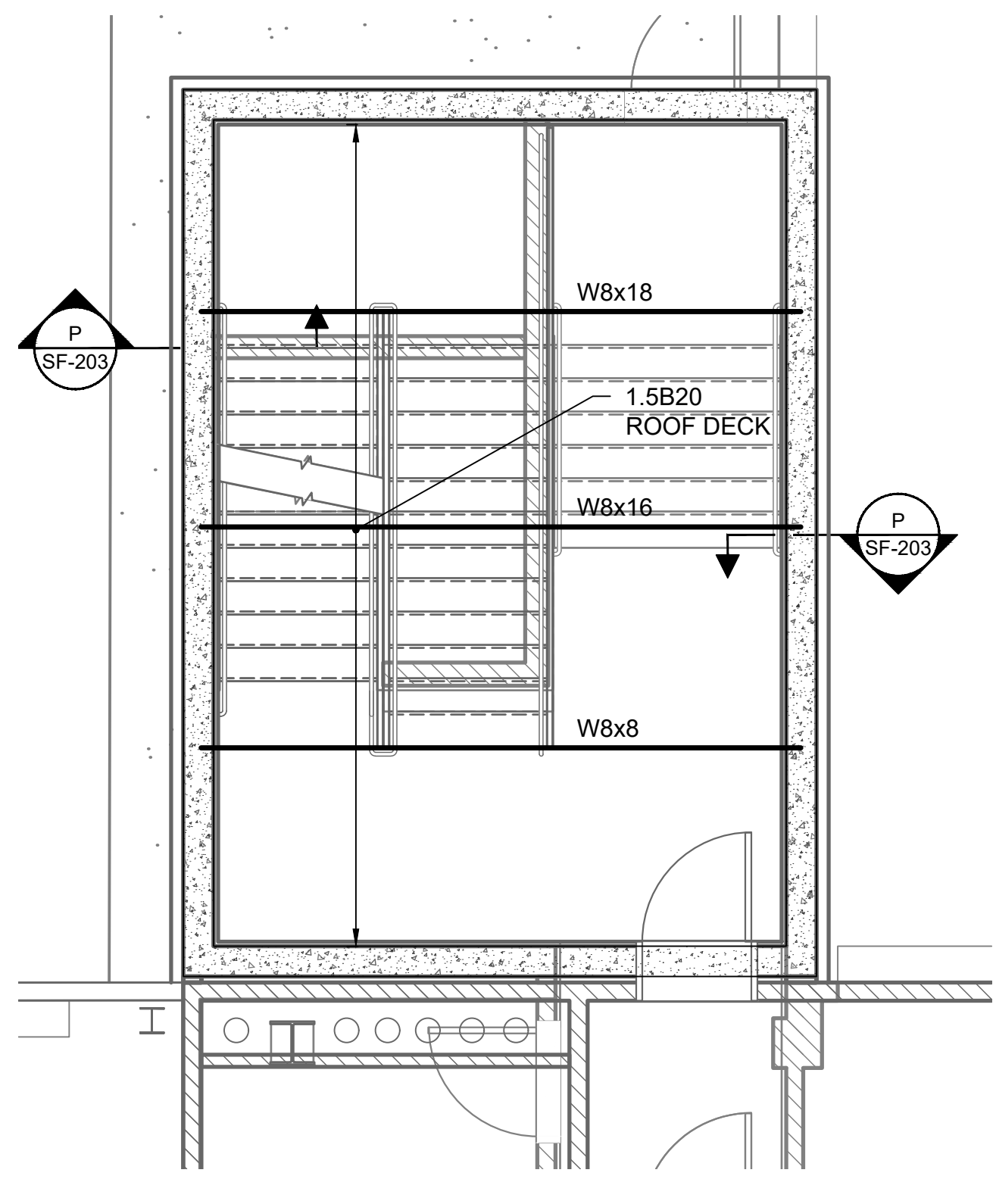
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1 STAIR C BULKHEAD PART PLAN
SCALE: 1/4" = 1'-0"



2 STAIR A BULKHEAD PART PLAN
SCALE: 1/8" = 1'-0"



3 STAIR B BULKHEAD PART PLAN
SCALE: 1/8" = 1'-0"

- NOTES:
- DESIGN ALL BRACING CONNECTIONS FOR THE AXIAL FORCES SHOWN IN ELEVATION AND THE SHEAR FORCES NOTED ON DRAWING S-001.
 - REFER TO DRAWING S-401 FOR CONNECTION DETAILS.
 - ALL FORCES SHOWN ARE ASD.
 - MC INDICATES A MOMENT CONNECTION
 - CANT INDICATES A CANTILEVERED BEAM

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DRAWING TITLE :

LATERAL FRAMING
ELEVATIONS

DWG NUMBER :
S-300
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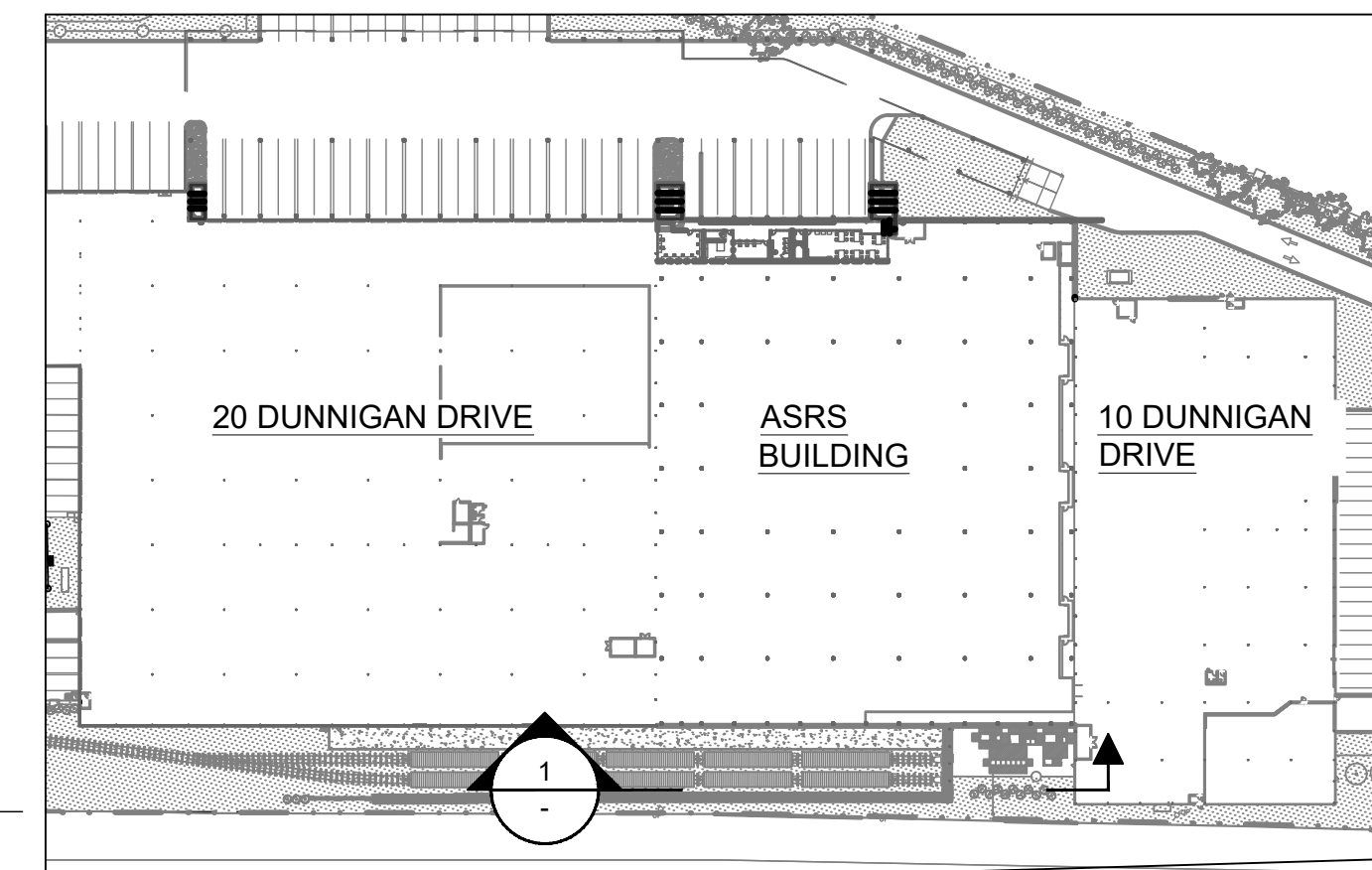
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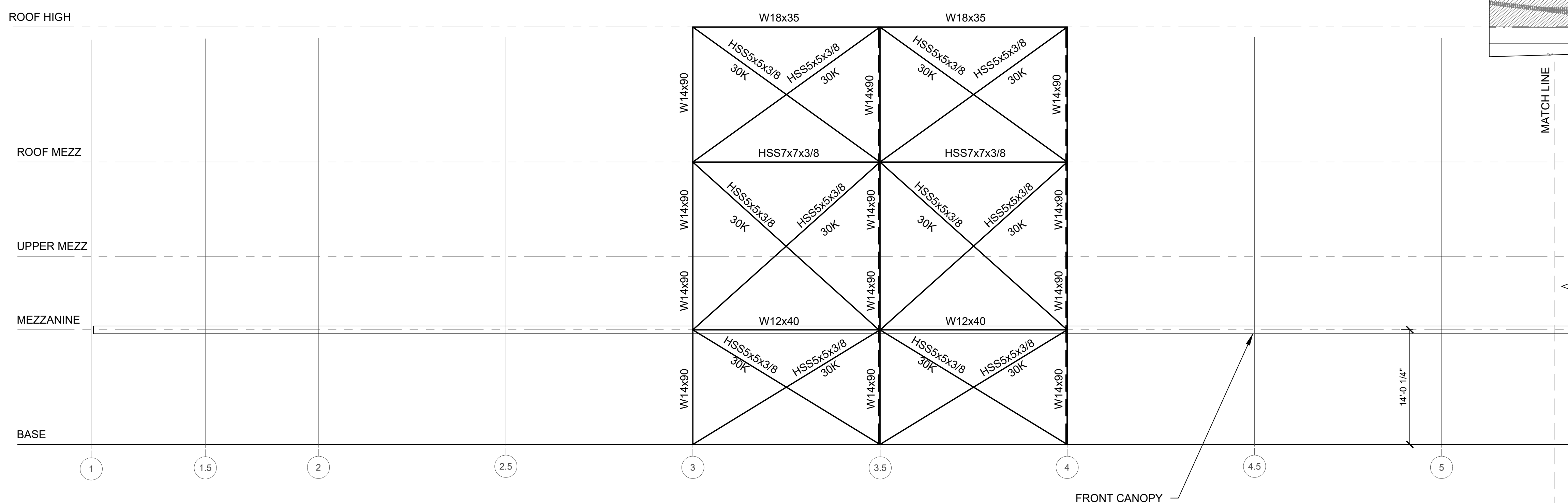
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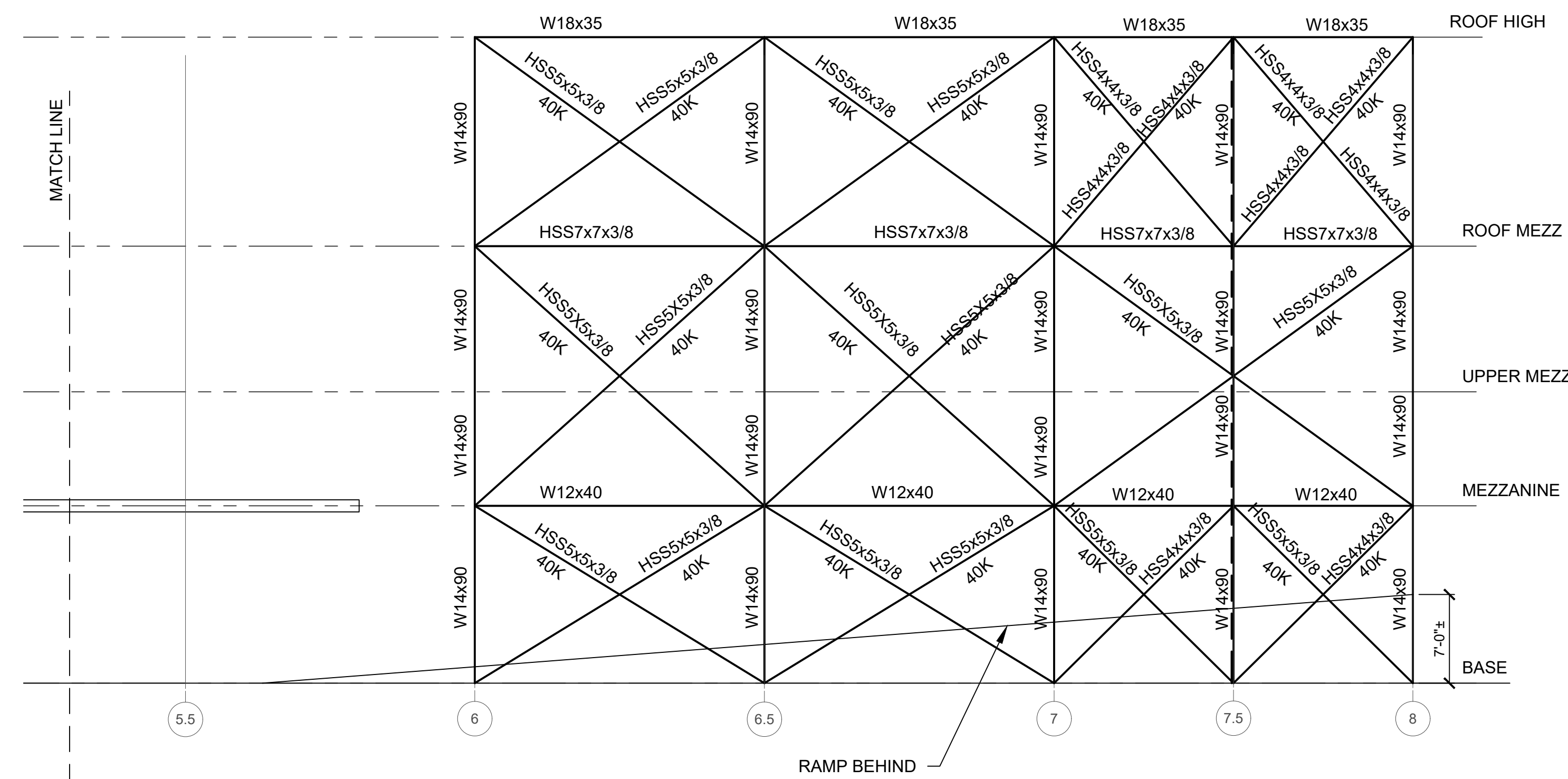
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KEY PLAN
 SCALE: N.T.S.



ASRS BUILDING SOUTH ELEVATION
GRIDLINE I FRAME
 SCALE: 1/8" = 1'-0"



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LATERAL FRAMING
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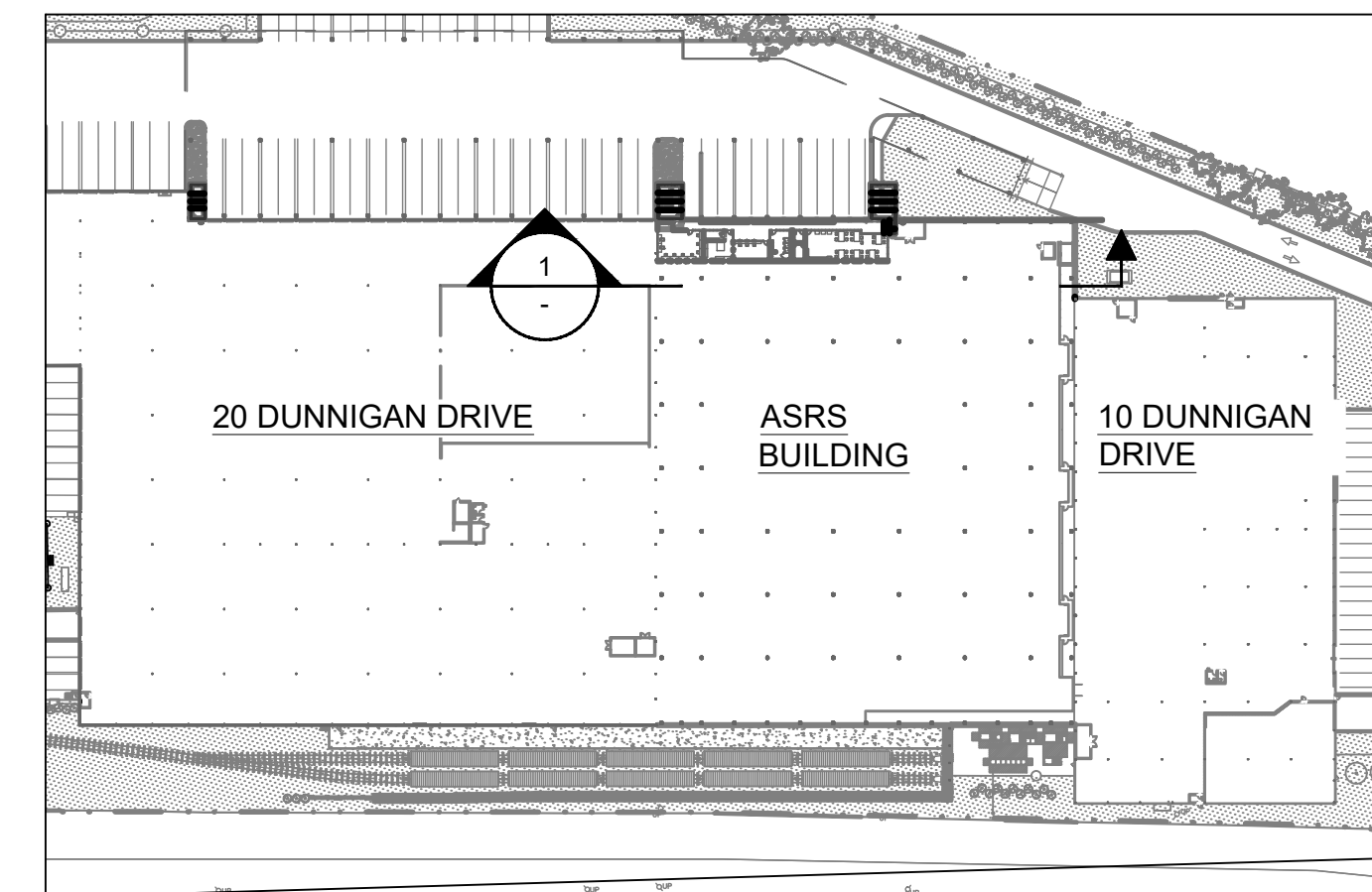
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S-301
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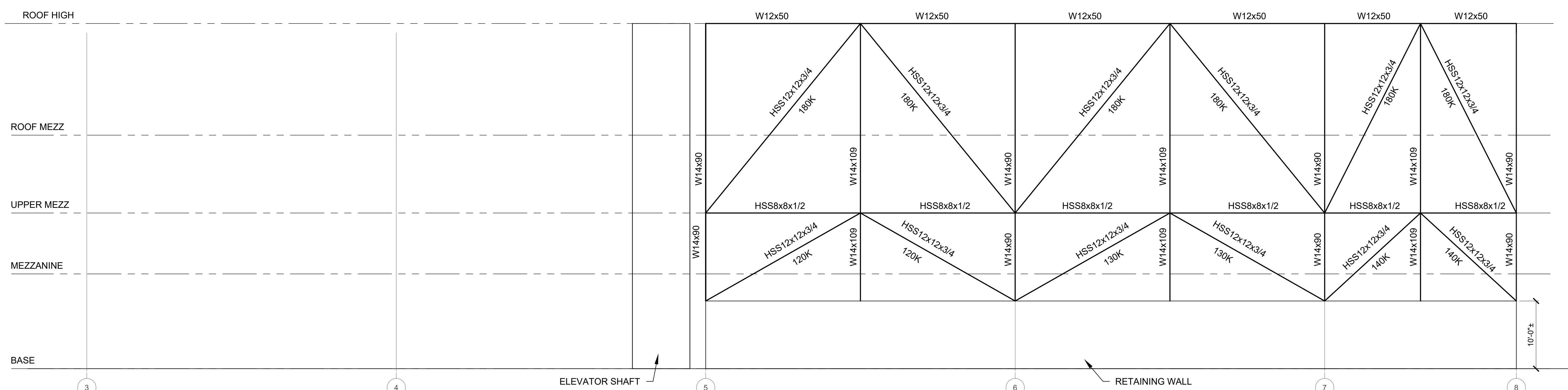
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KEY PLAN
 SCALE: N.T.S.



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**ASRS BUILDING NORTH ELEVATION
 GRIDLINE A FRAME**
 SCALE: 1/8" = 1'-0"

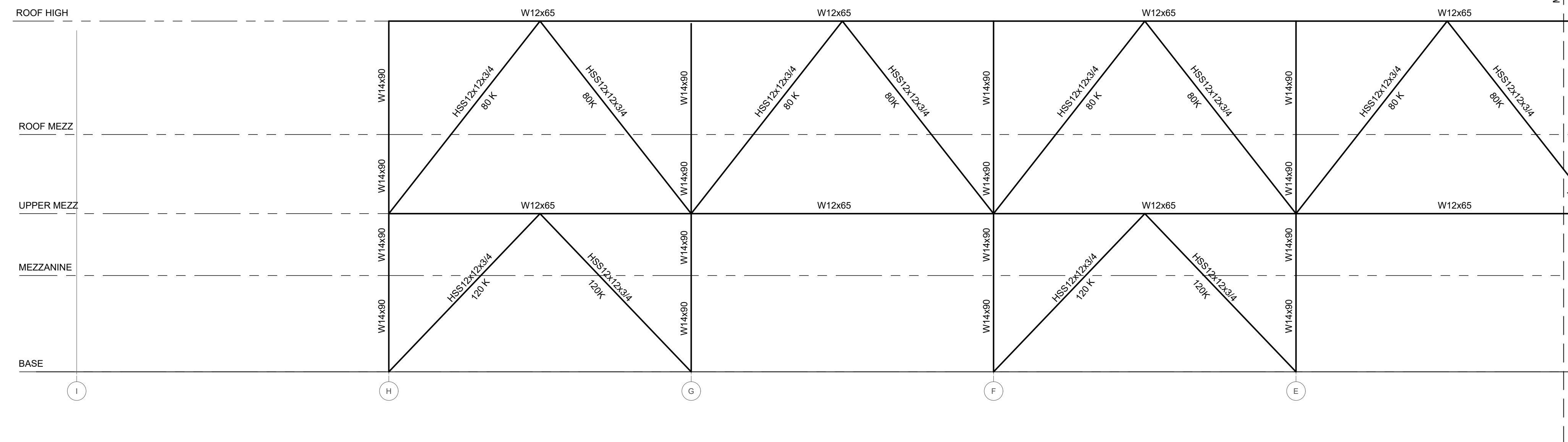
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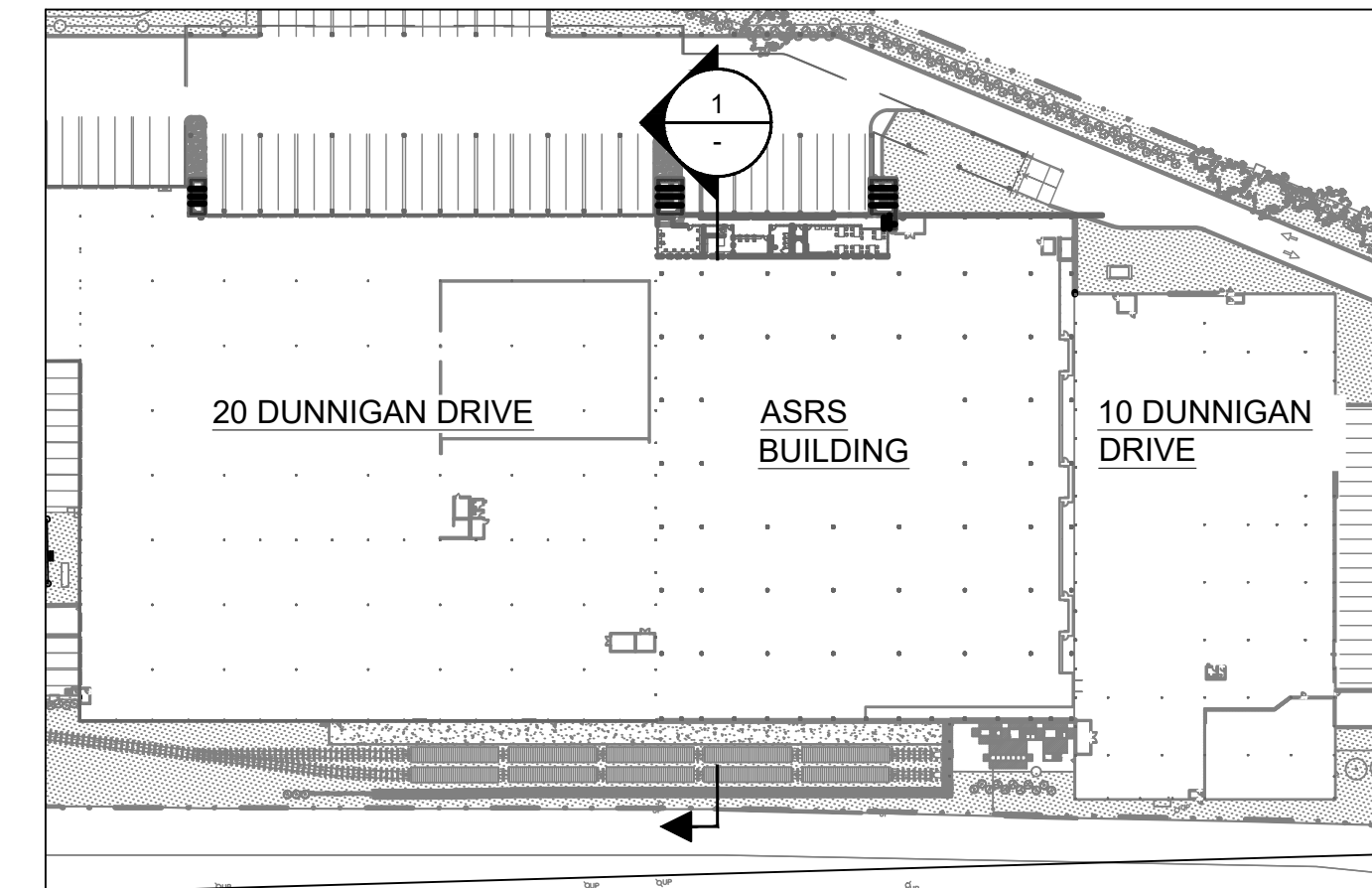
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**LATERAL FRAMING
 ELEVATIONS**

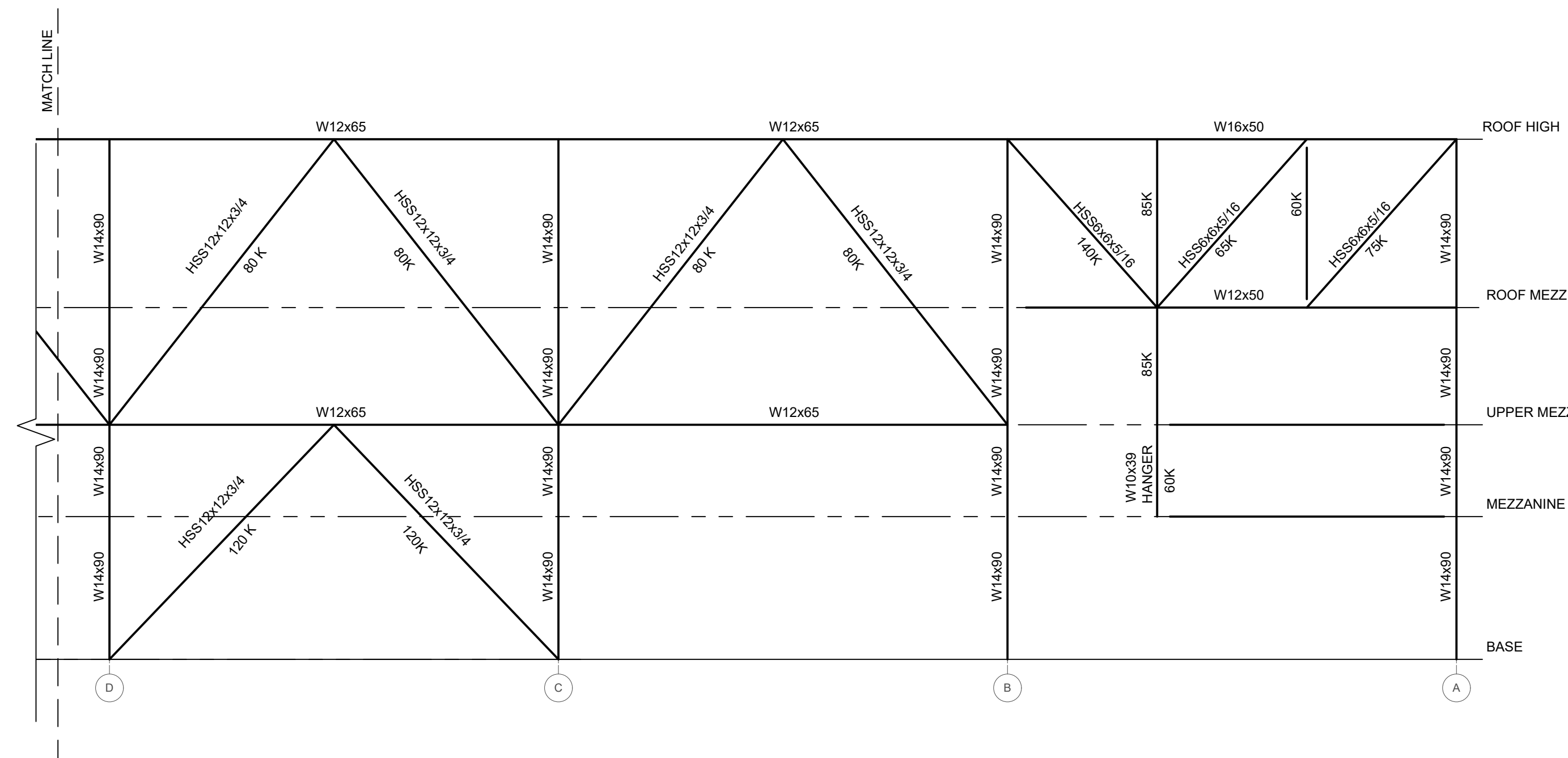
DWG NUMBER :
S-302
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ASRS BUILDING WEST ELEVATION
GRIDLINE 1 FRAME
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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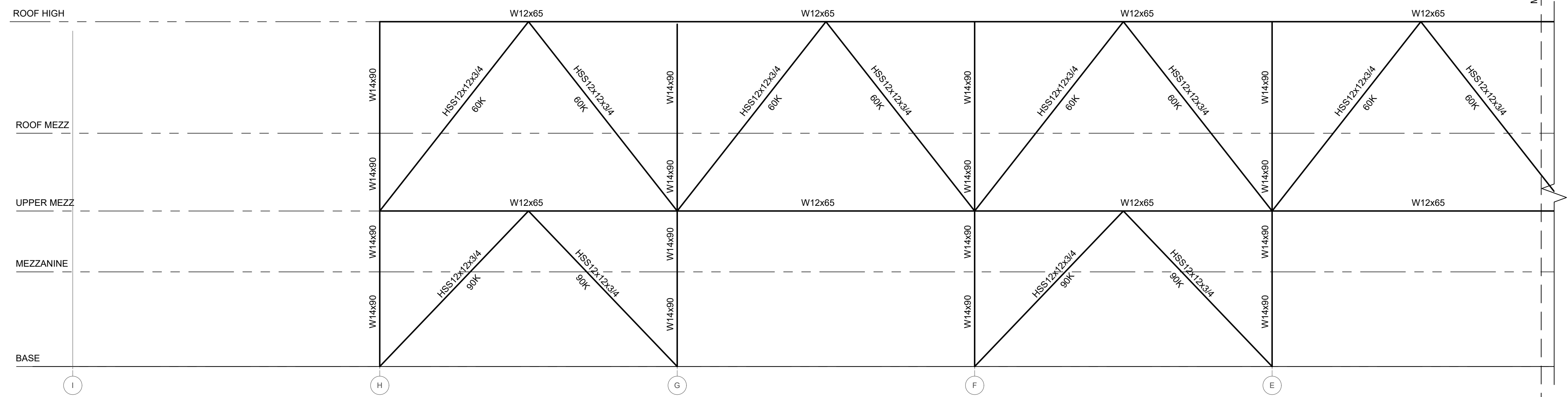
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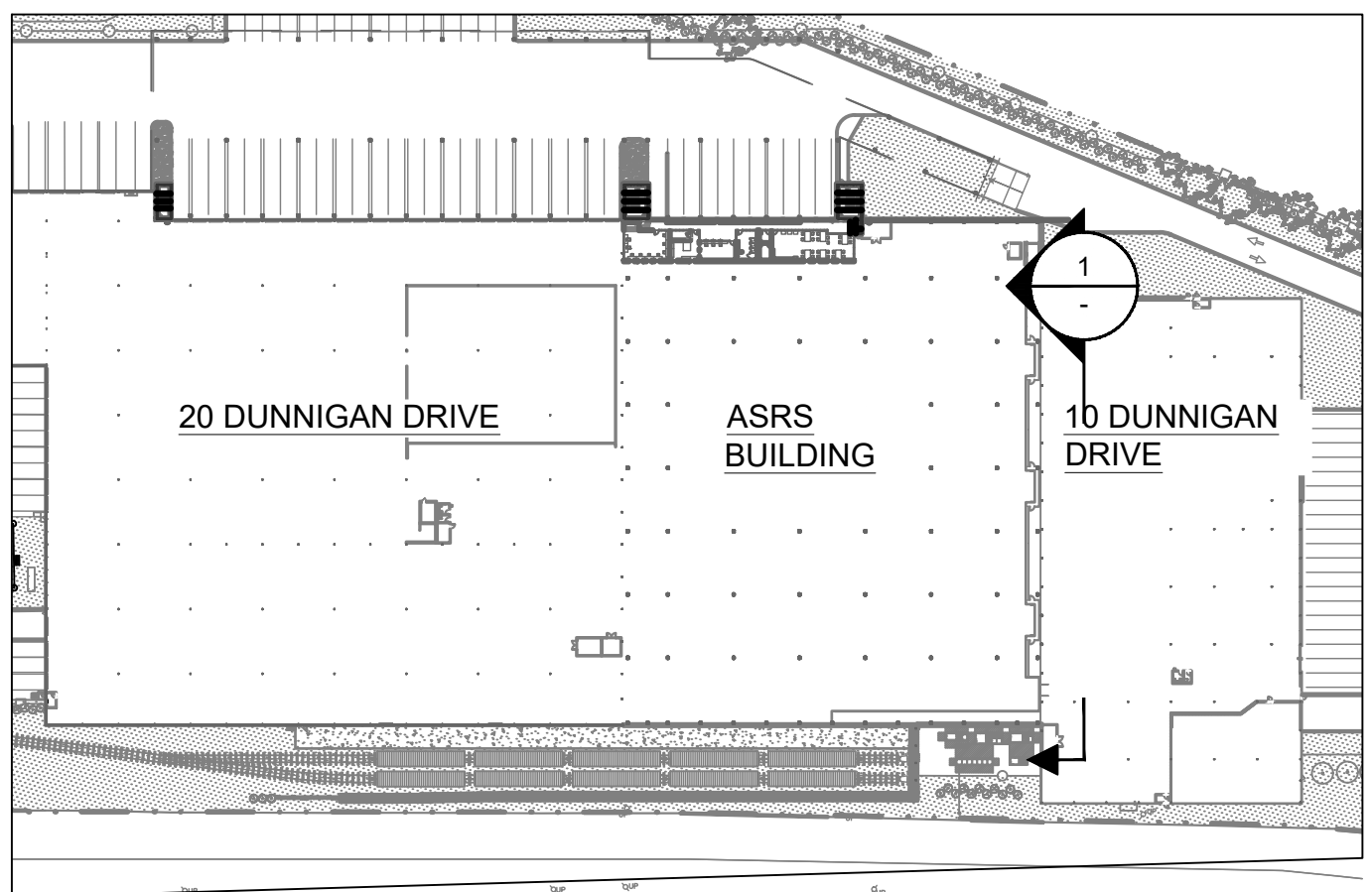
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LATERAL FRAMING
ELEVATIONS

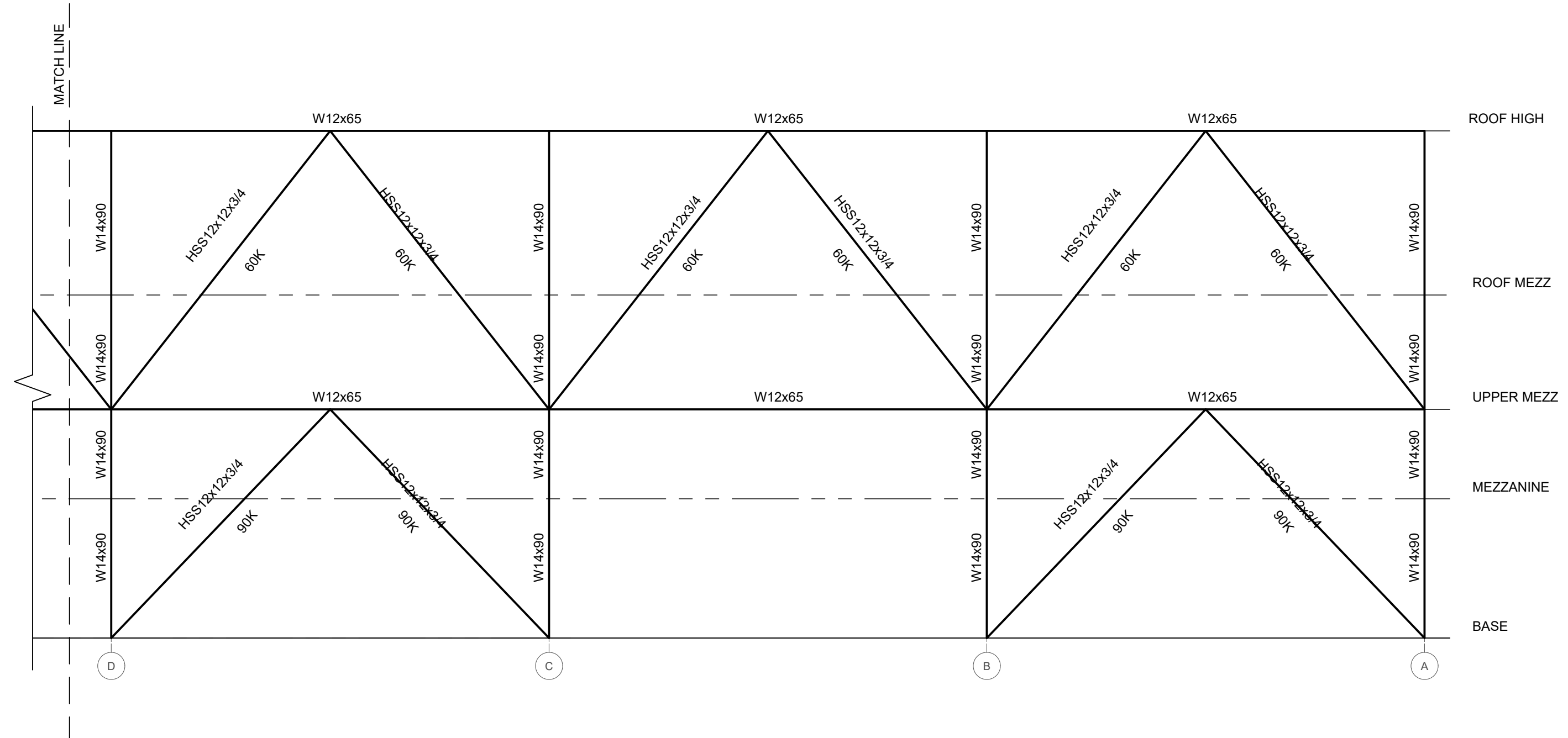
DWG NUMBER :
S-303
38 OF 60



ASRS BUILDING AT GRIDLINE 8
EAST ELEVATION
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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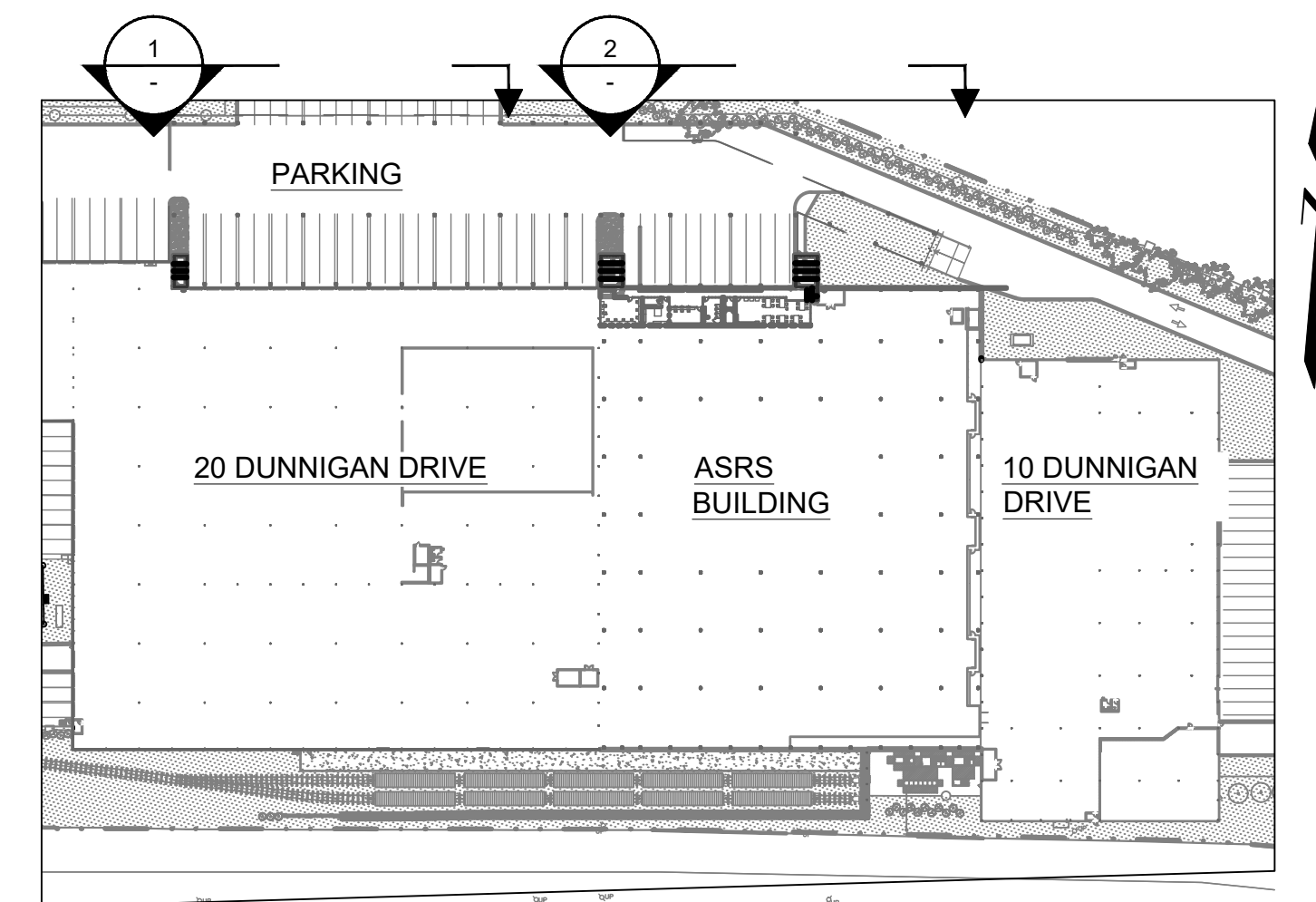
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DRAWING TITLE :

LATERAL FRAMING
ELEVATIONS

DWG NUMBER :
S-304
39 OF 60



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SCALE: N.T.S.

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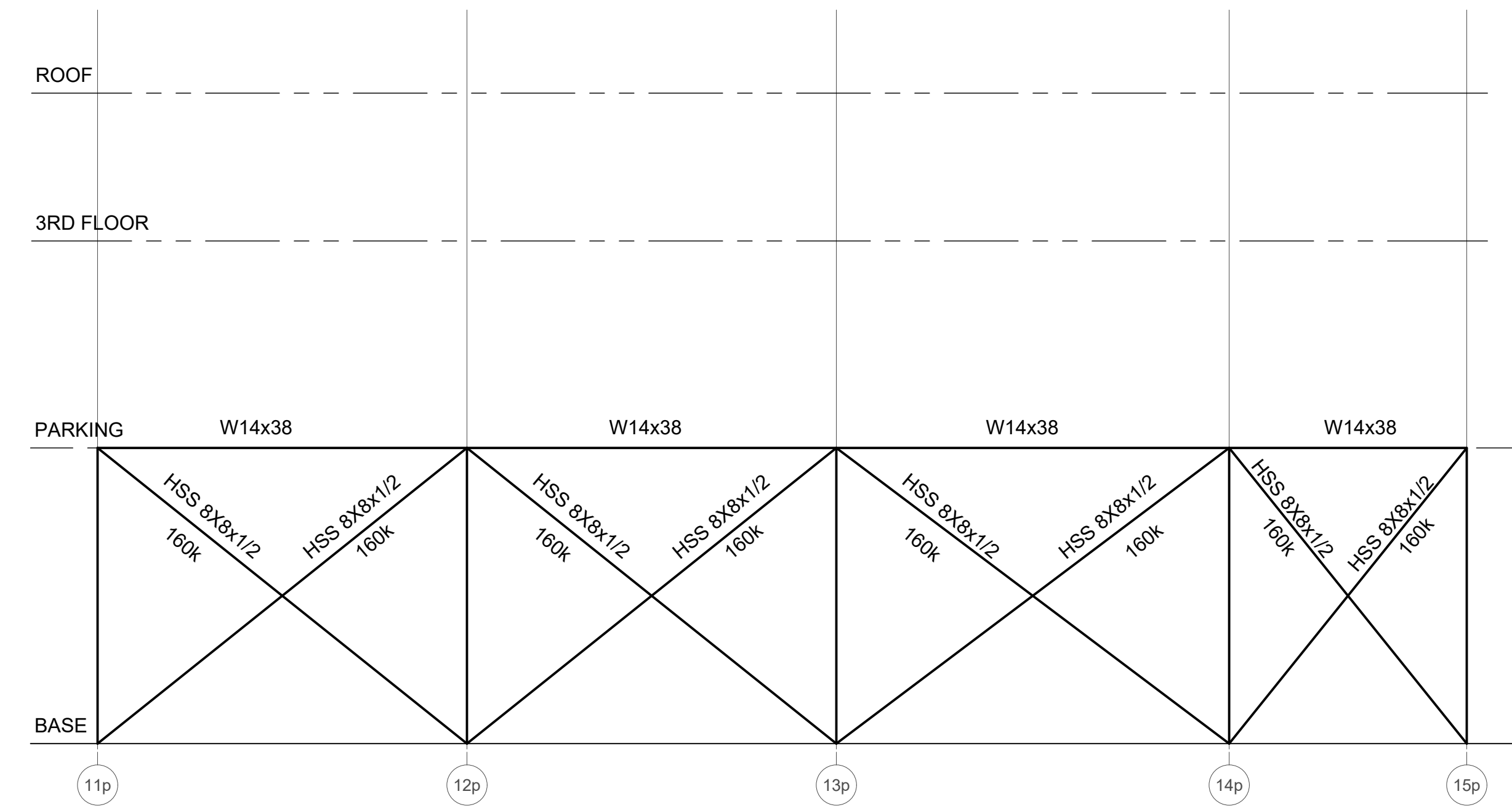
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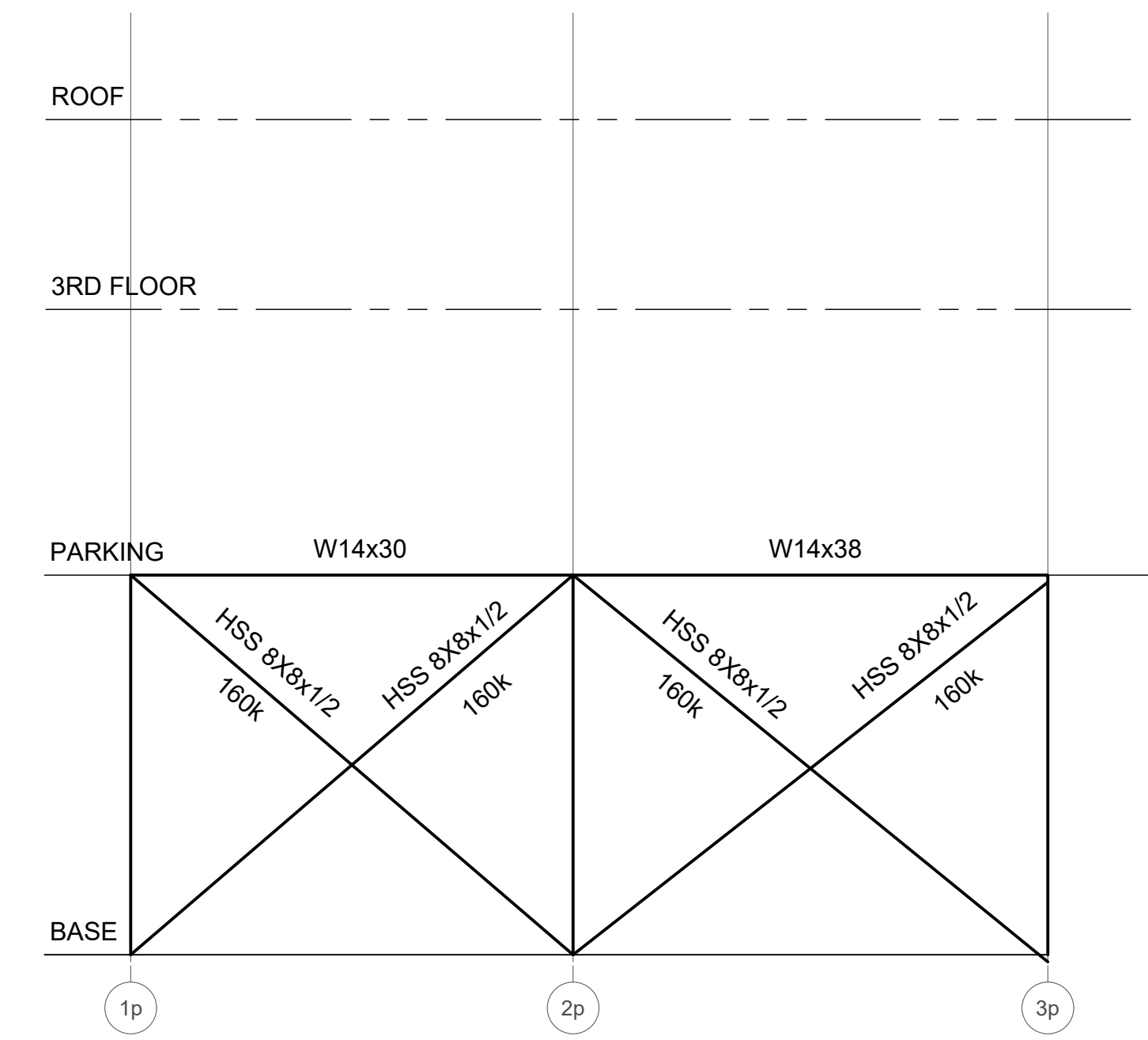
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**LATERAL FRAMING
 ELEVATIONS**

DWG NUMBER :
S-305
 40 OF 60



1
**PARKING AREA NORTH ELEVATION
 GRIDLINE Ap. EAST SIDE**
 SCALE: 1/8" = 1'-0"



2
**PARKING AREA NORTH ELEVATION
 GRIDLINE Ap - WEST SIDE**
 SCALE: 1/8" = 1'-0"

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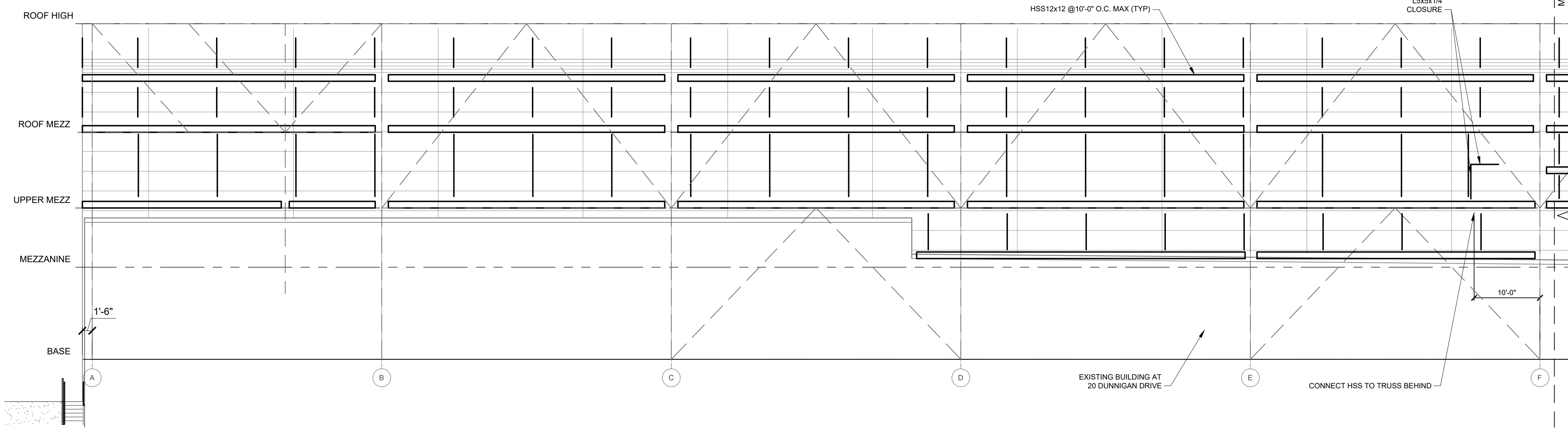
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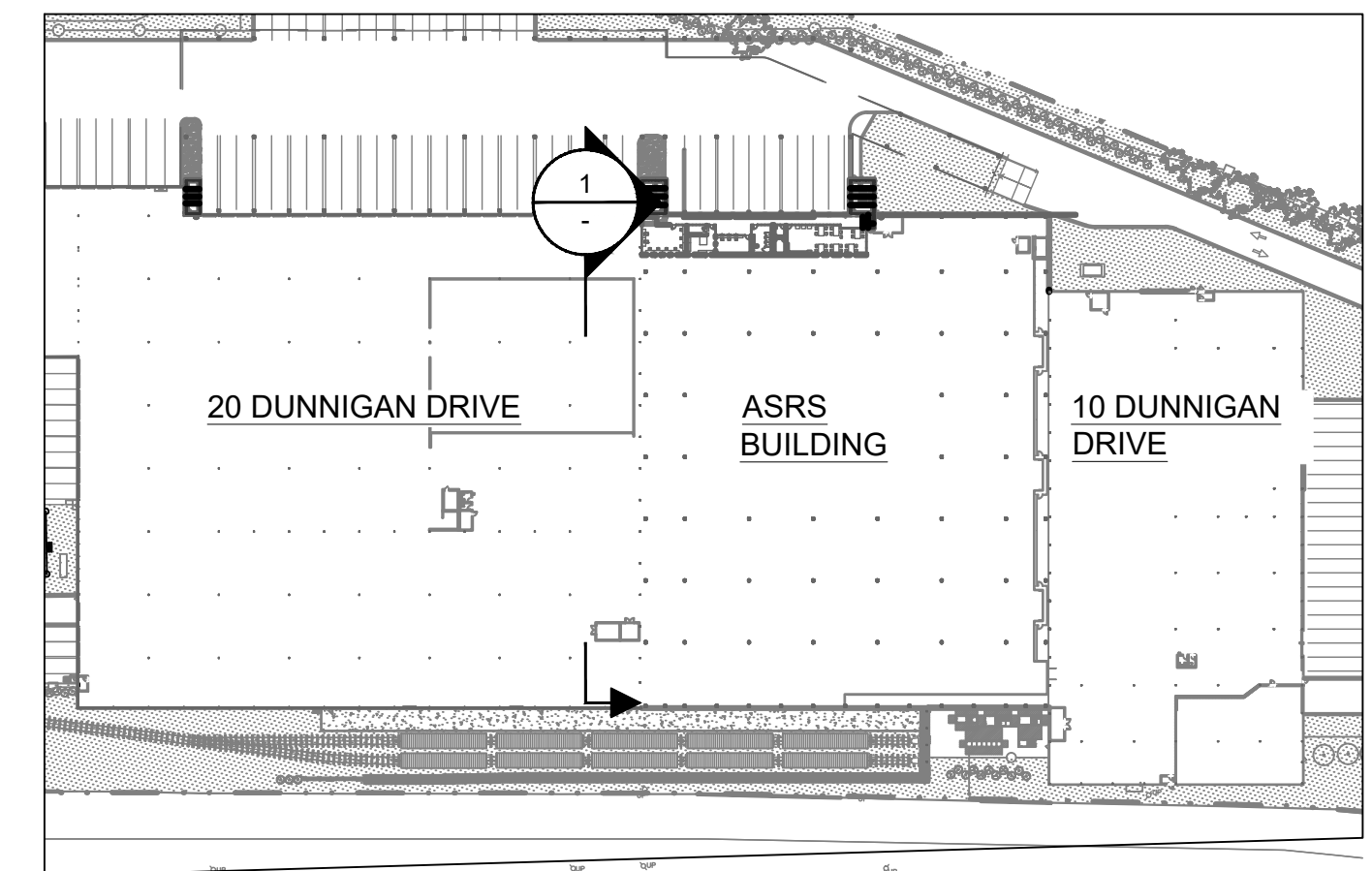
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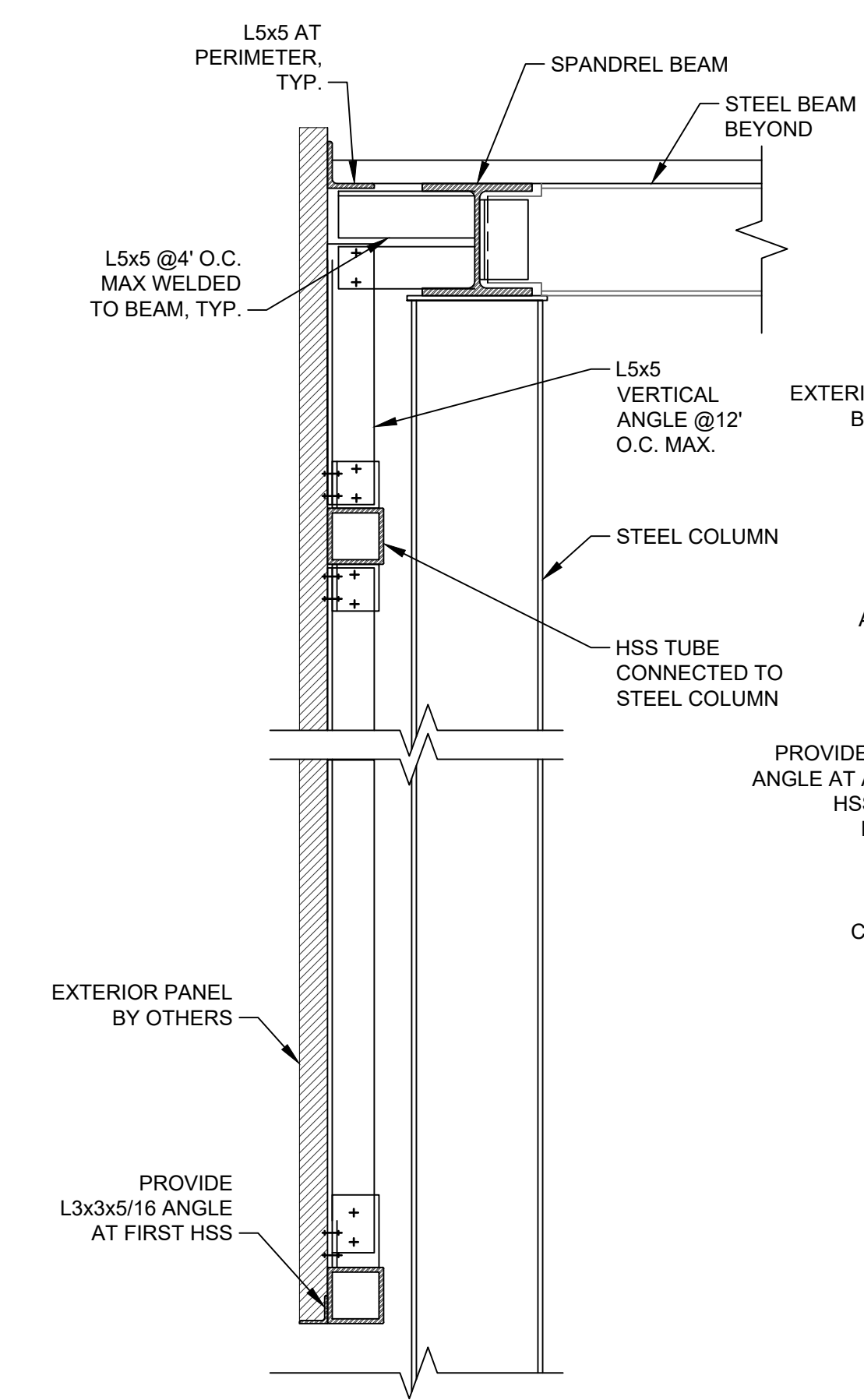
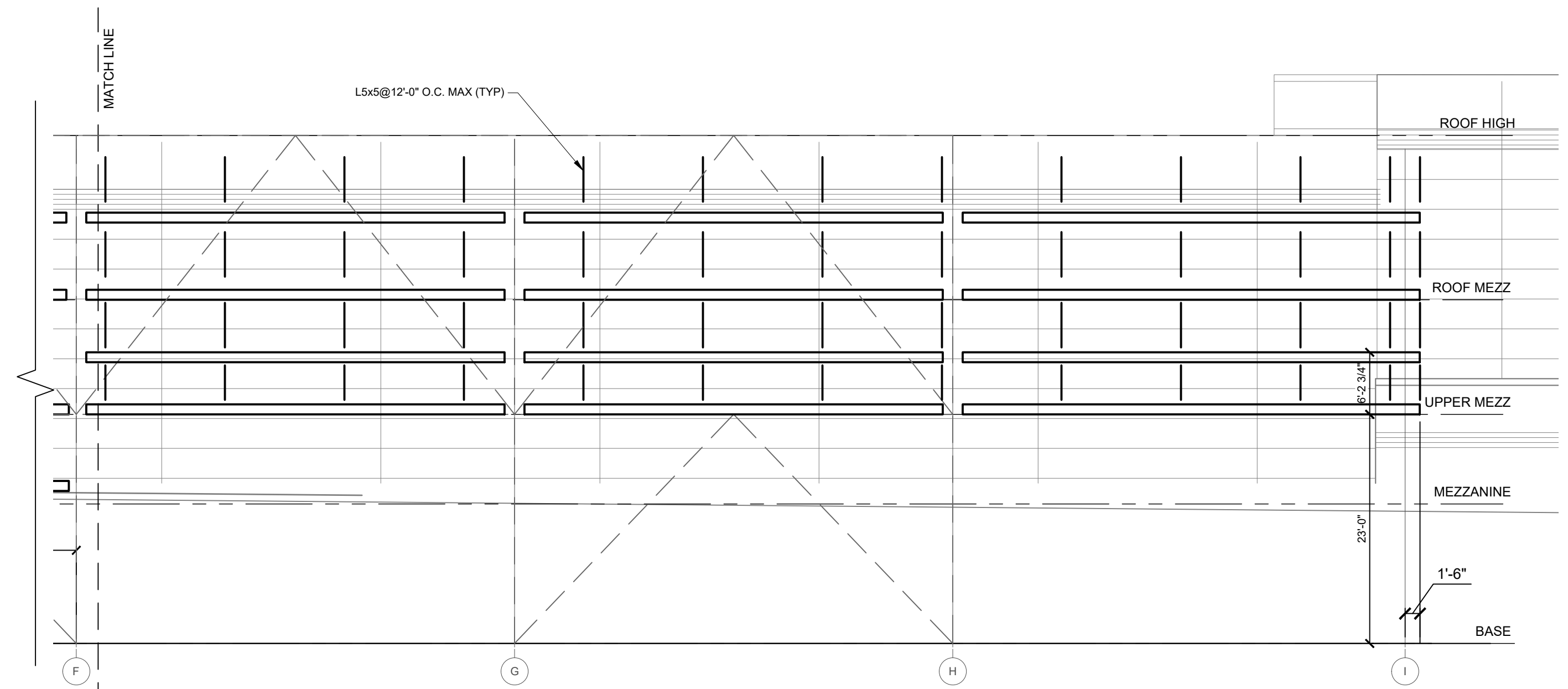
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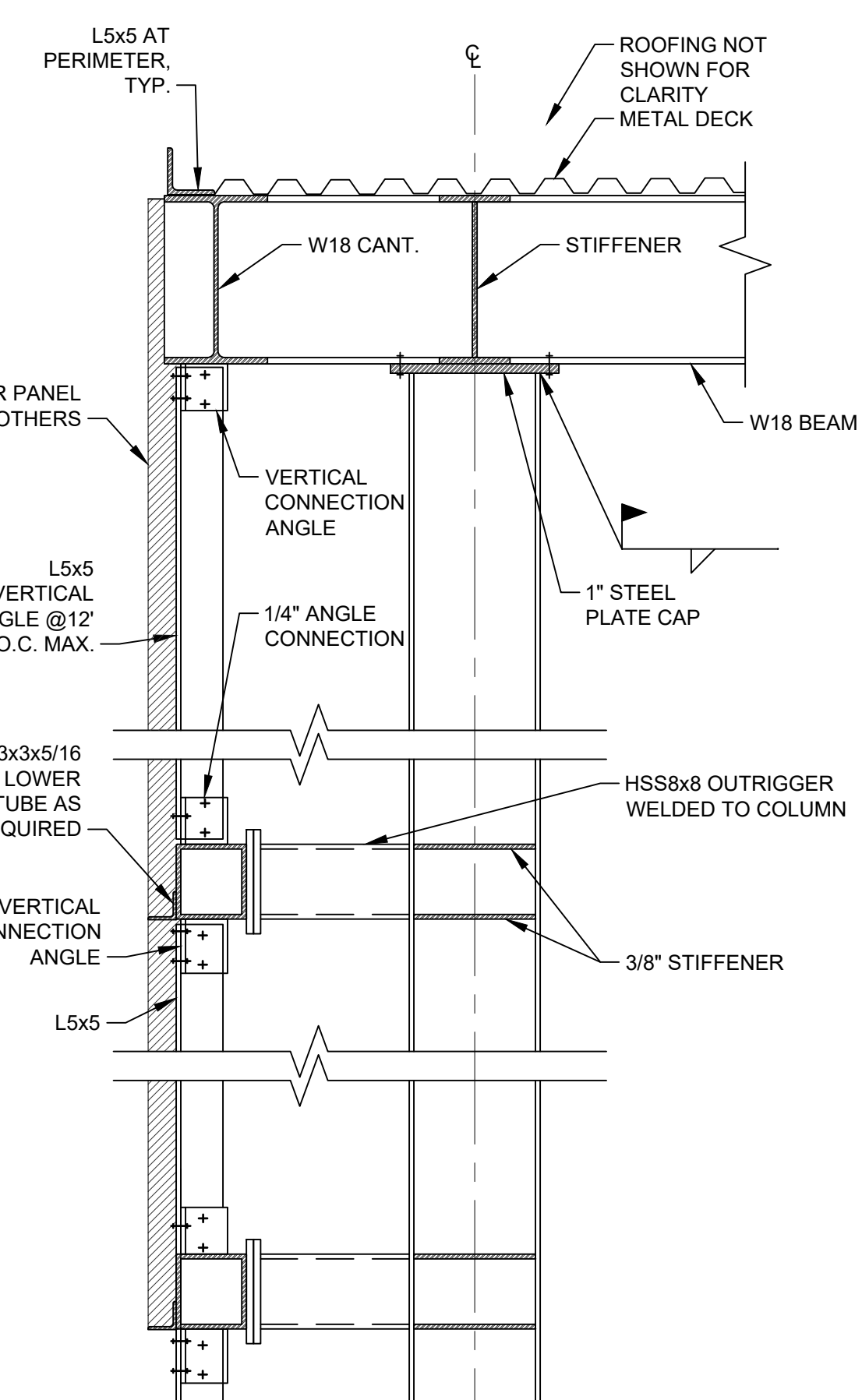
ASRS BUILDING WEST ELEVATION
 EXTERIOR PANEL SUPPORTS
 SCALE: 3/32" = 1'-0"



KEY PLAN
 SCALE: N.T.S.



1 TYP. EXTERIOR SECTION
 SCALE: 3/4" = 1'-0"



2 TYP. EXTERIOR SECTION AT EAST AND WEST WALL
 SCALE: 3/4" = 1'-0"

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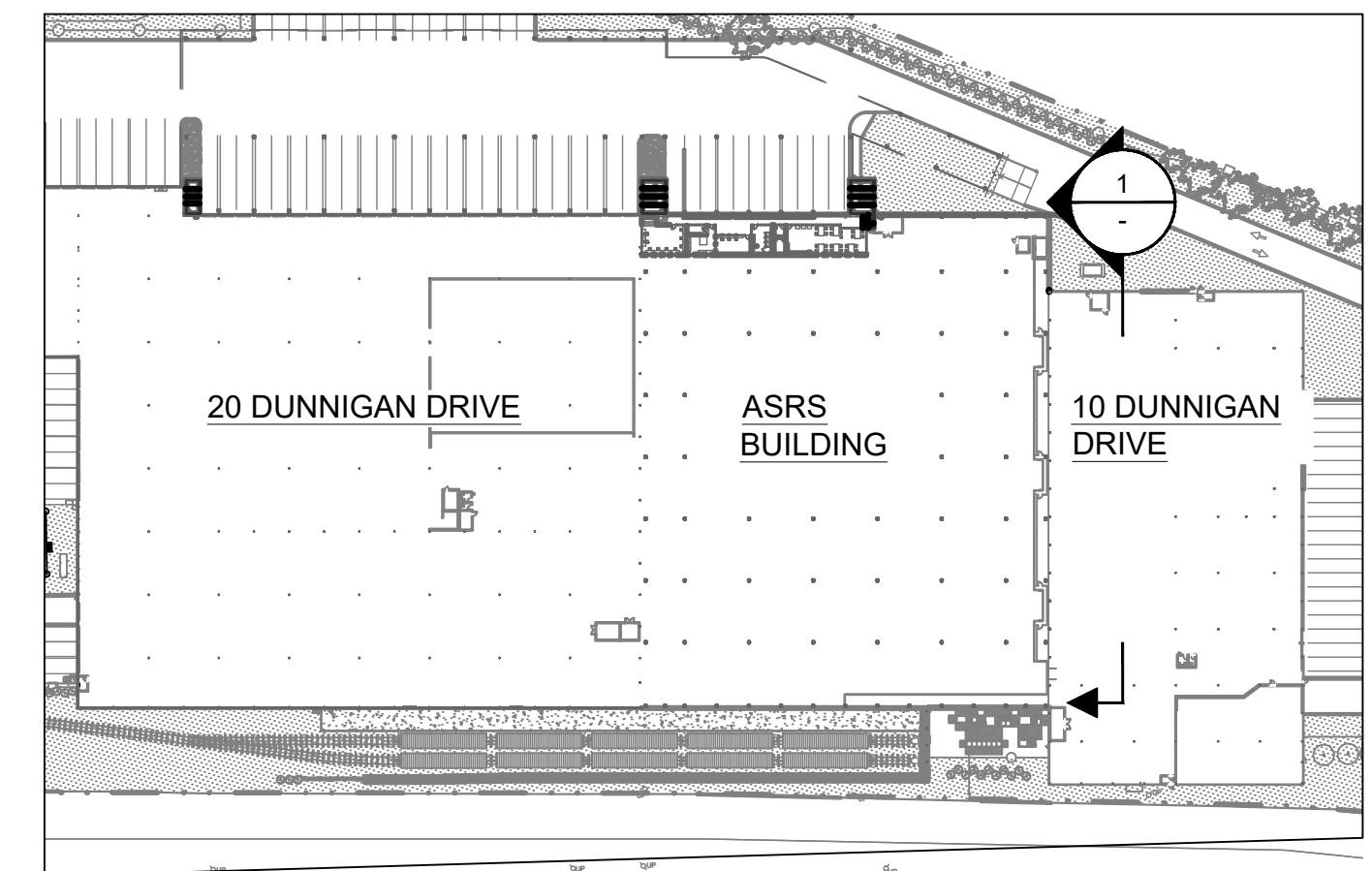
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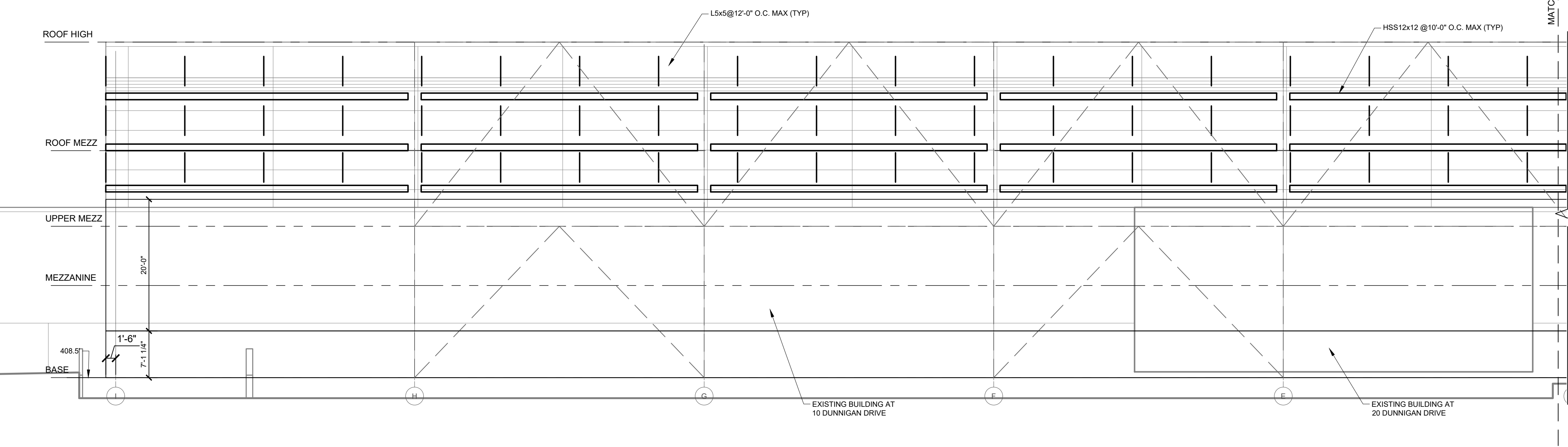
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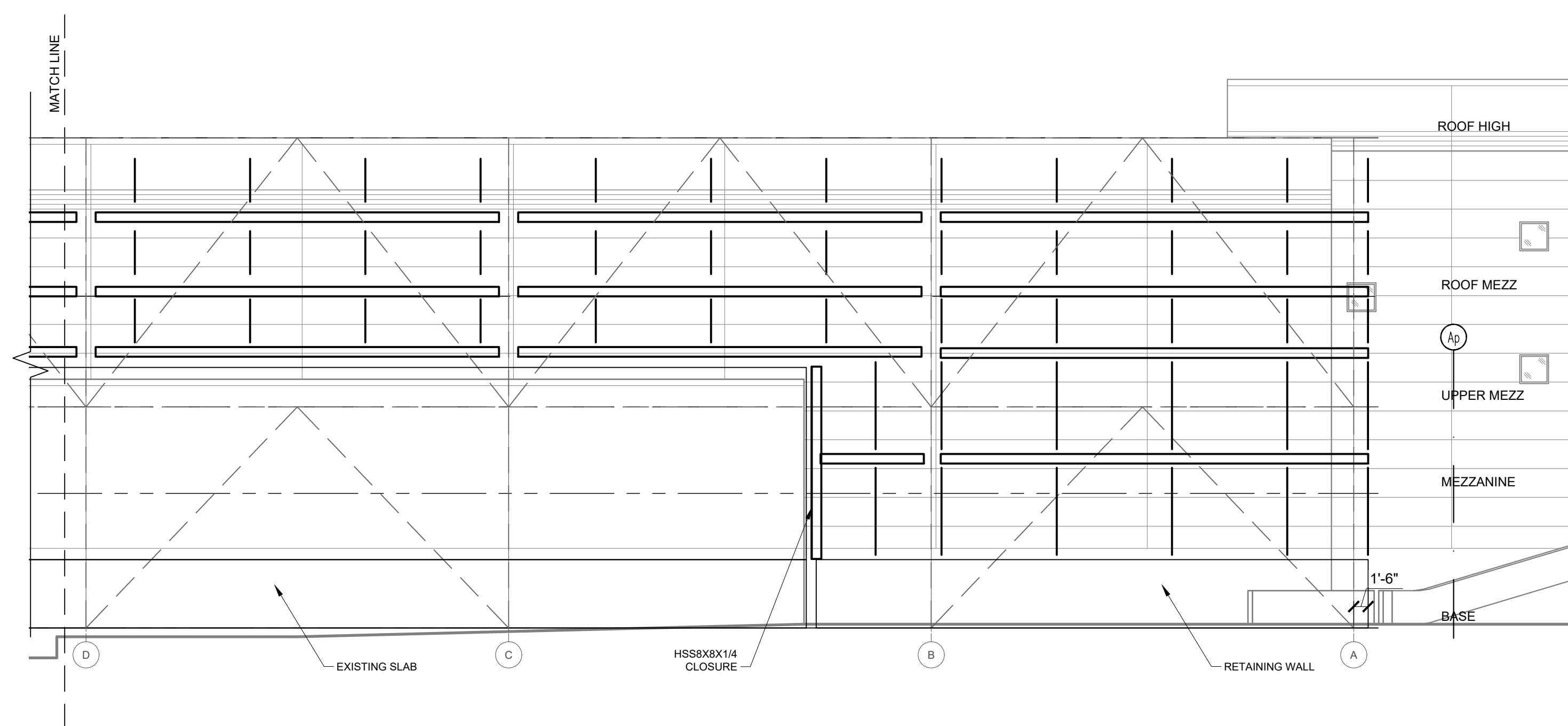
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KEY PLAN
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**ASRS BUILDING EAST ELEVATION
 EXTERIOR PANEL SUPPORTS**
 SCALE: 3/32" = 1'-0"



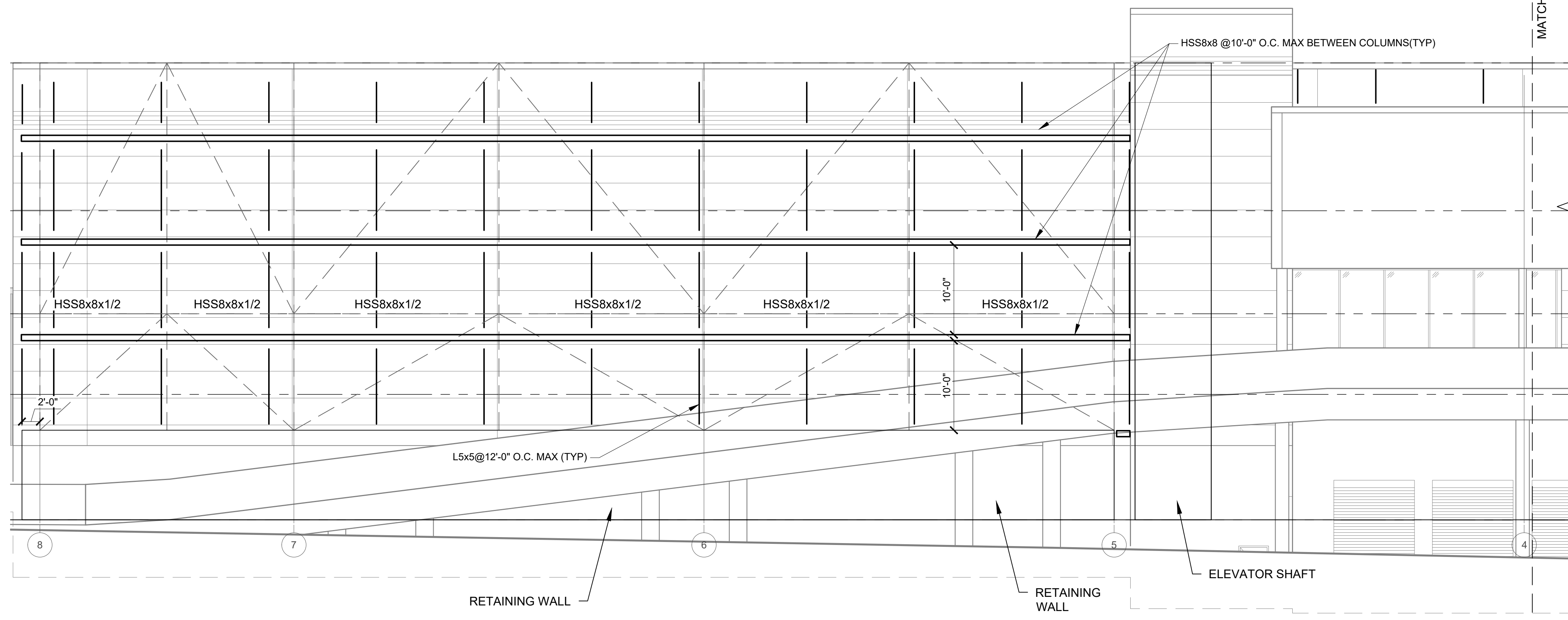
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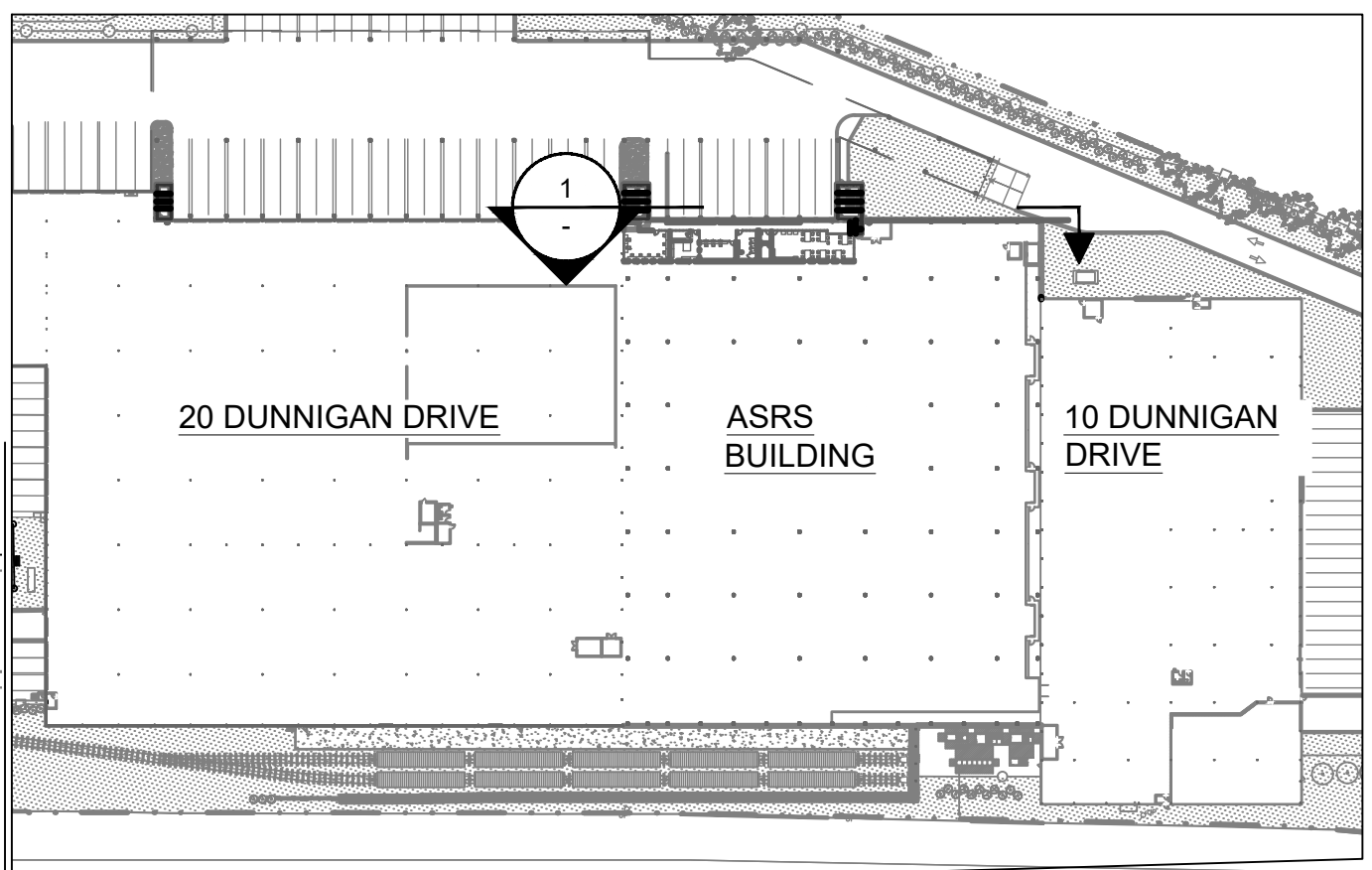
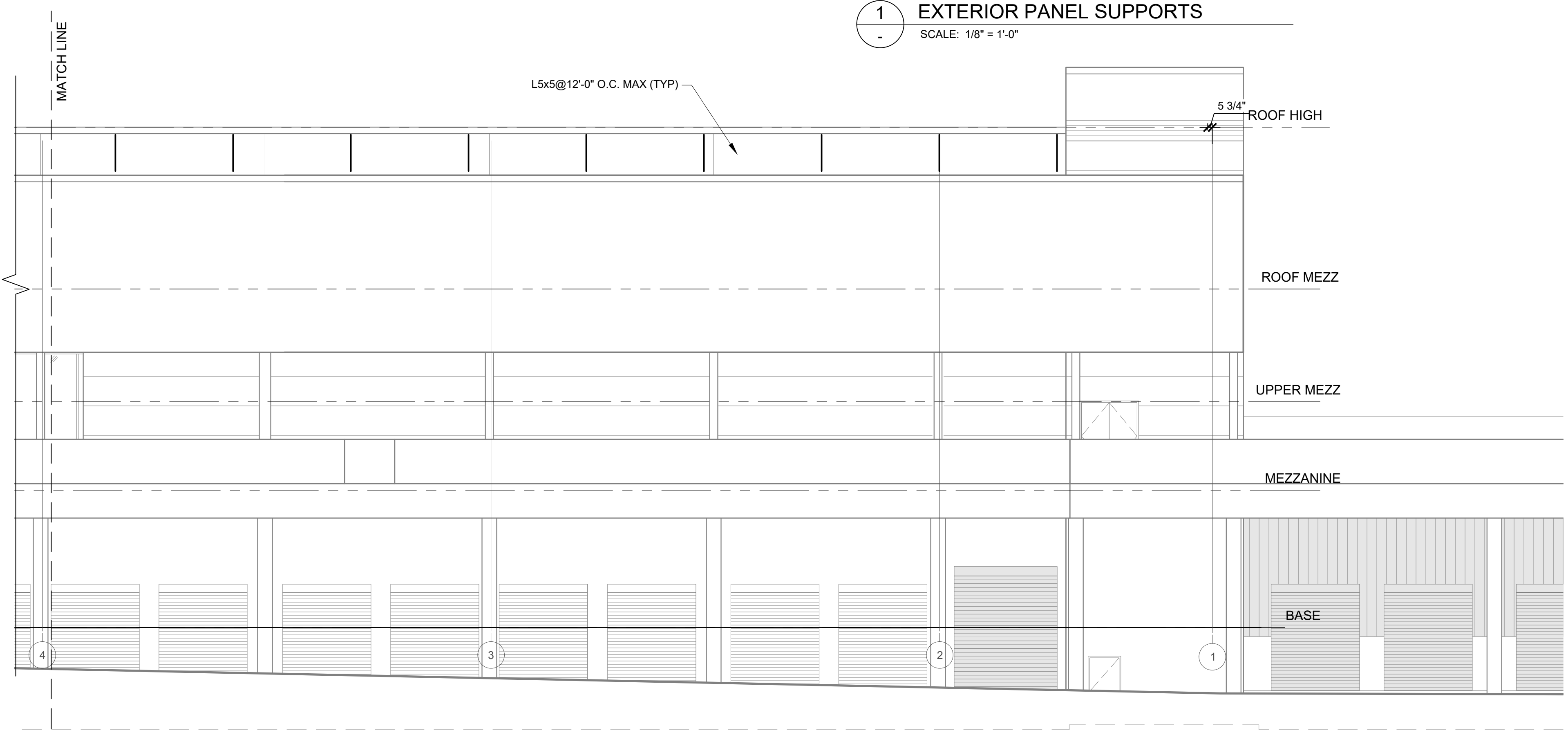
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**EXTERIOR PANEL
 SUPPORTS**

DWG NUMBER :
S-311
 42 OF 60



ASRS BUILDING NORTH ELEVATION
EXTERIOR PANEL SUPPORTS
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.

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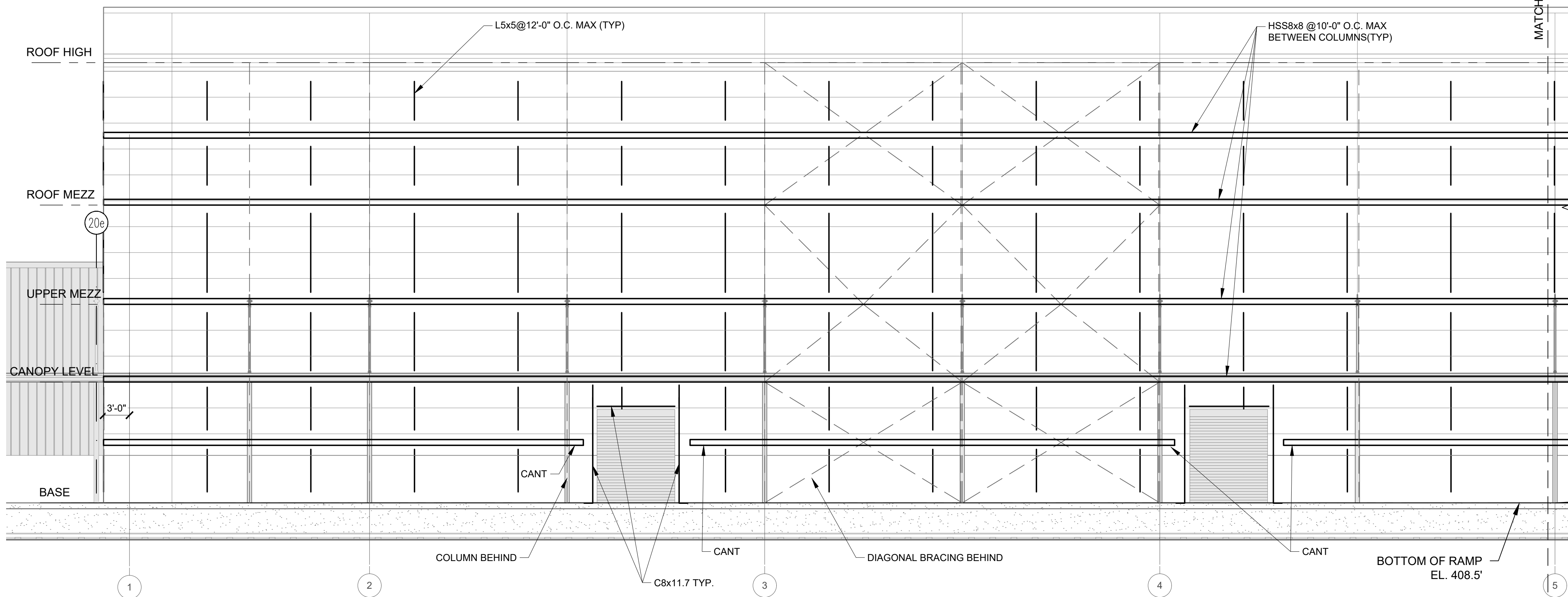
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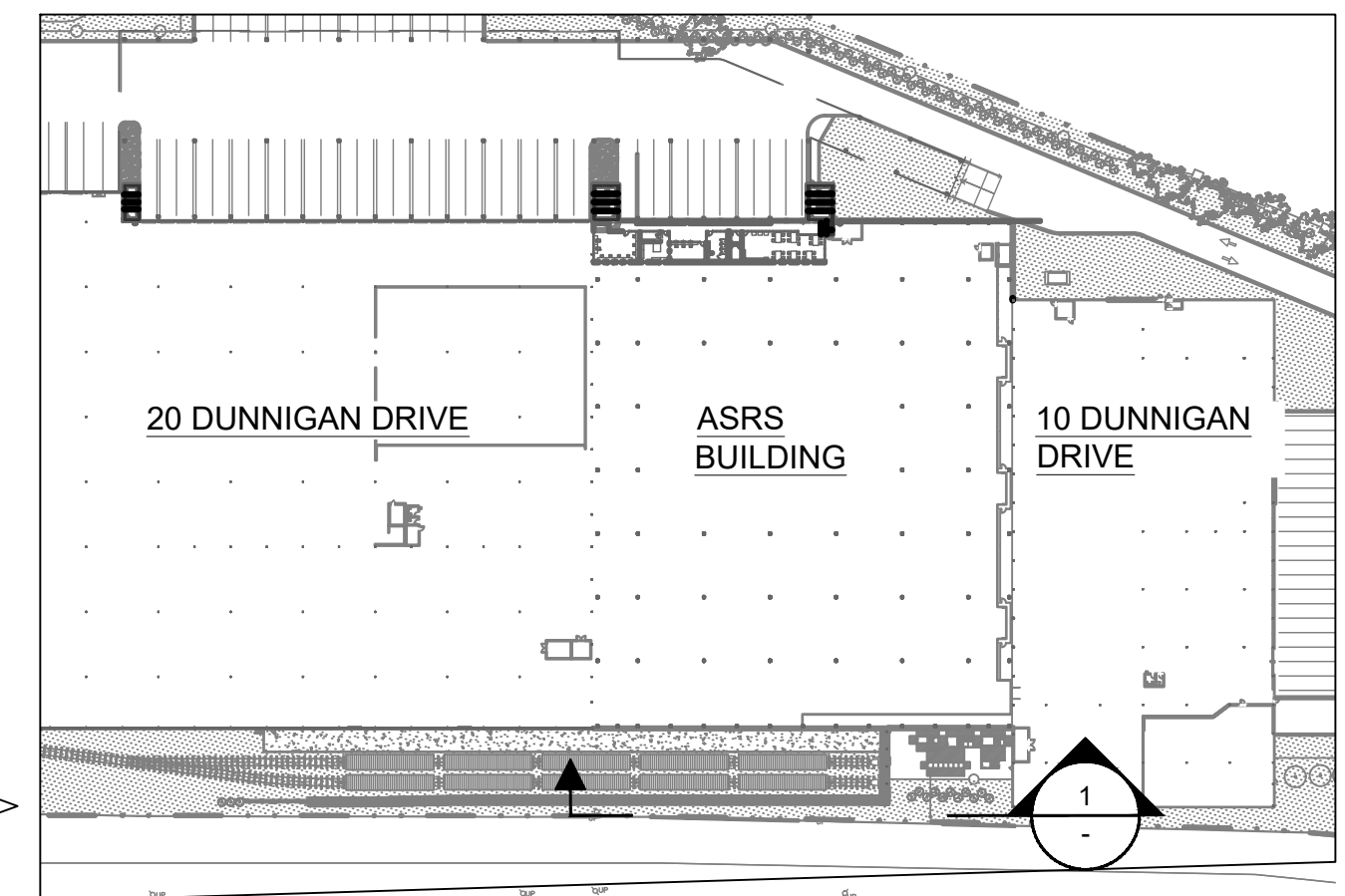
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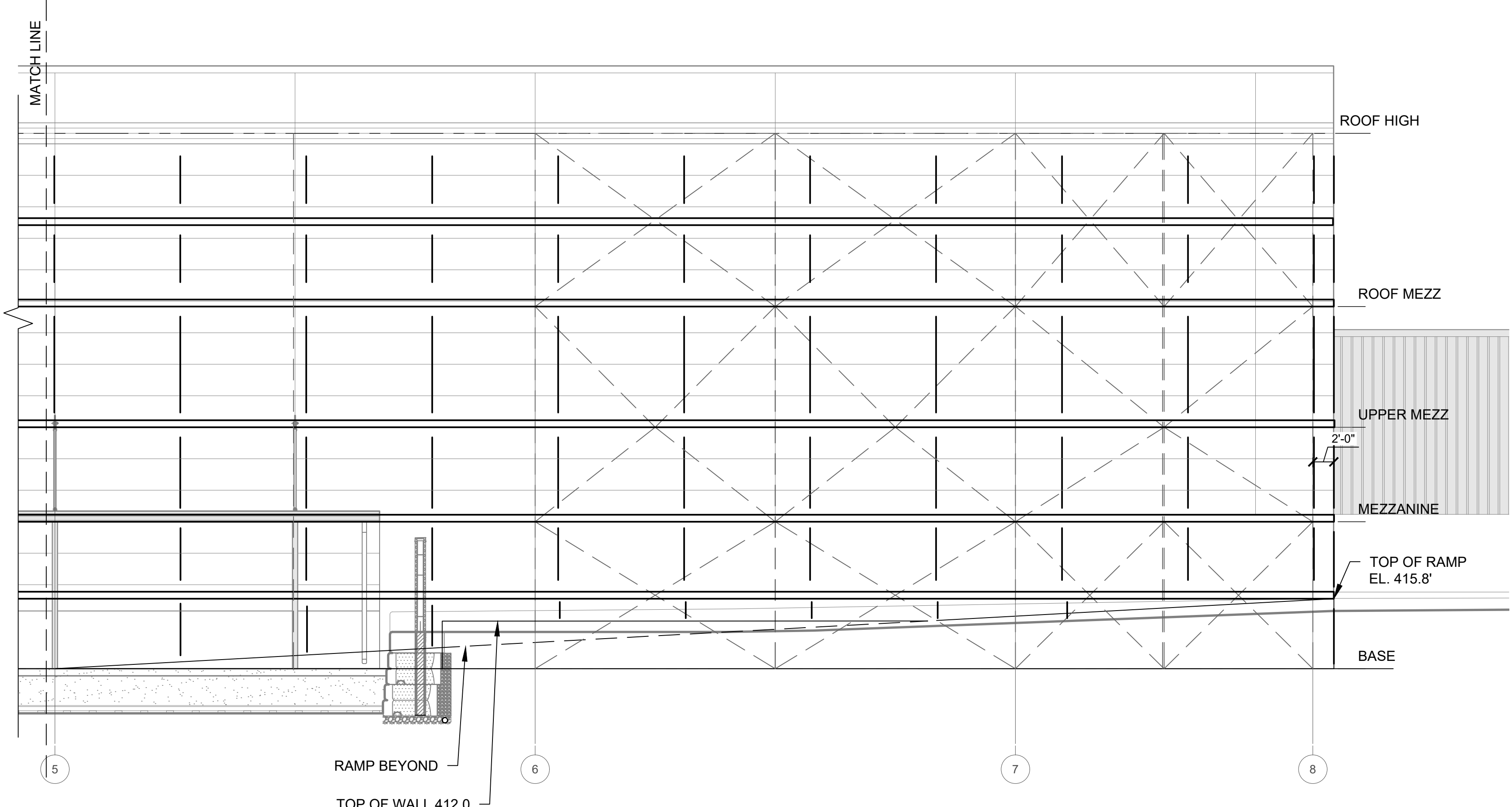
DWG NUMBER :
S-312
43 OF 60



**ASRS BUILDING SOUTH ELEVATION
EXTERIOR PANEL SUPPORTS**
SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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DRAWING TITLE :

**EXTERIOR PANEL
SUPPORTS**

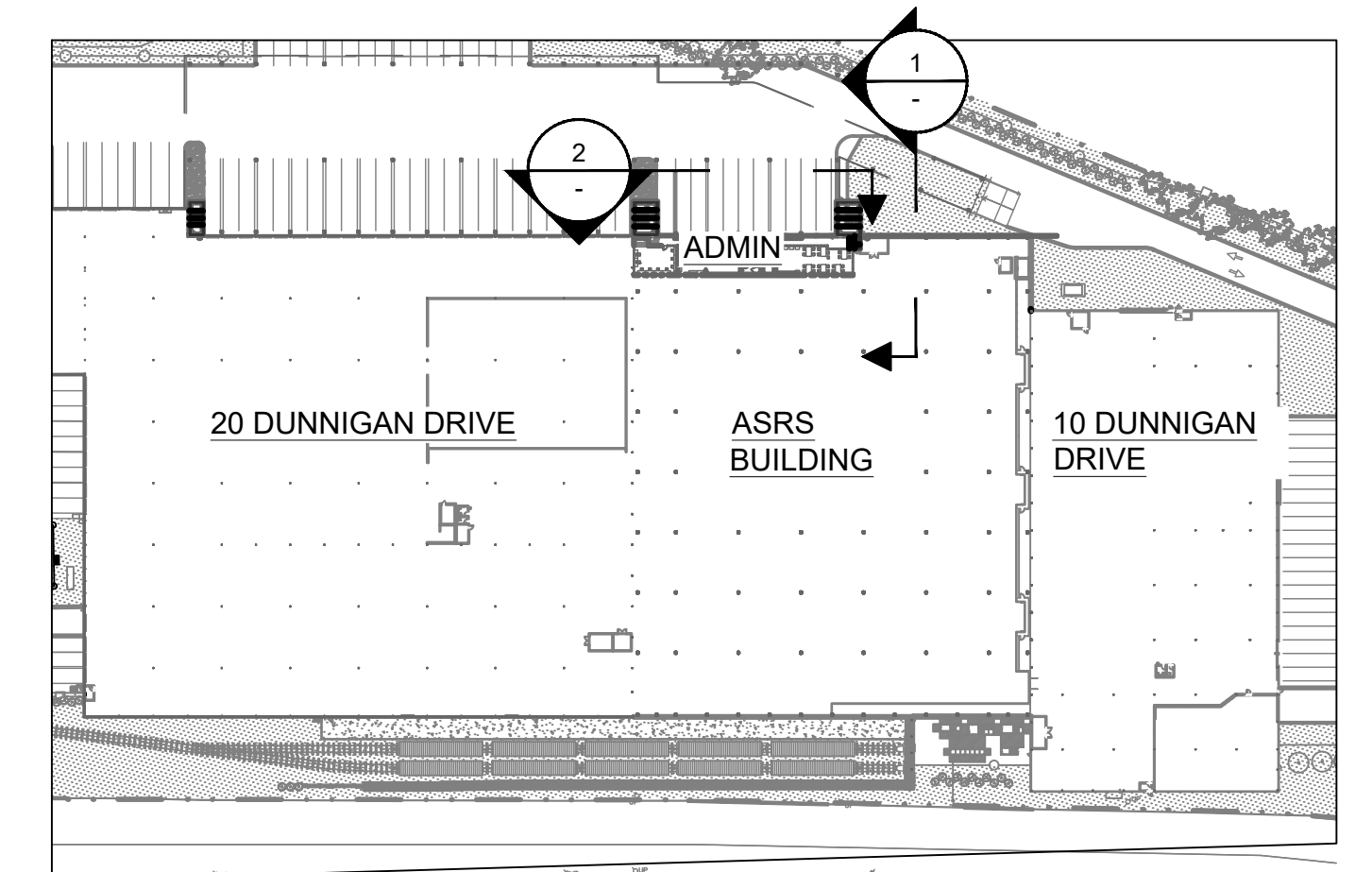
DWG NUMBER :
S-313
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KEY PLAN
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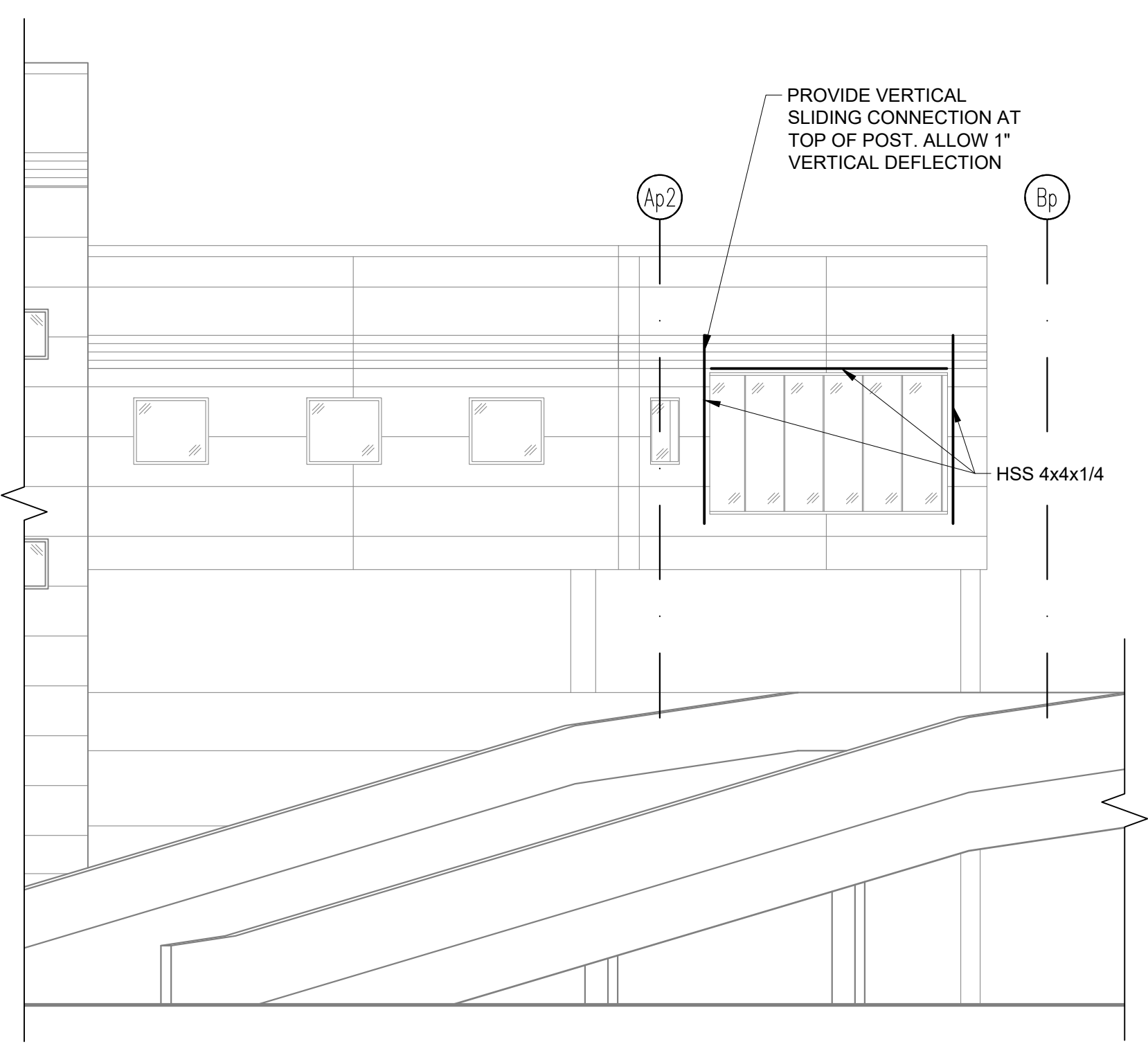
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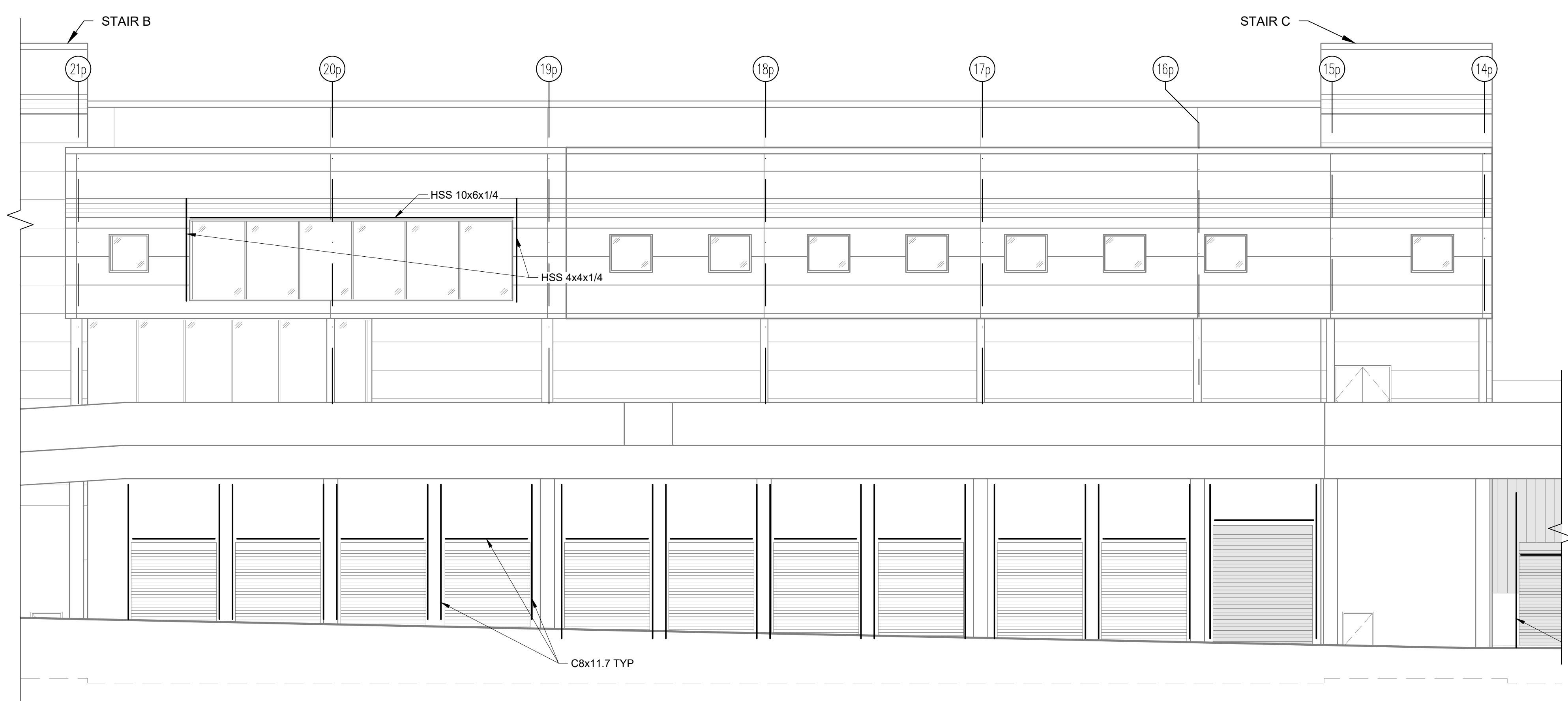
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EXTERIOR PANEL SUPPORTS

DWG NUMBER :
S-314
 45 OF 60



ADMIN BUILDING EAST ELEVATION EXTERIOR PANEL SUPPORTS
 SCALE: 1/8" = 1'-0"



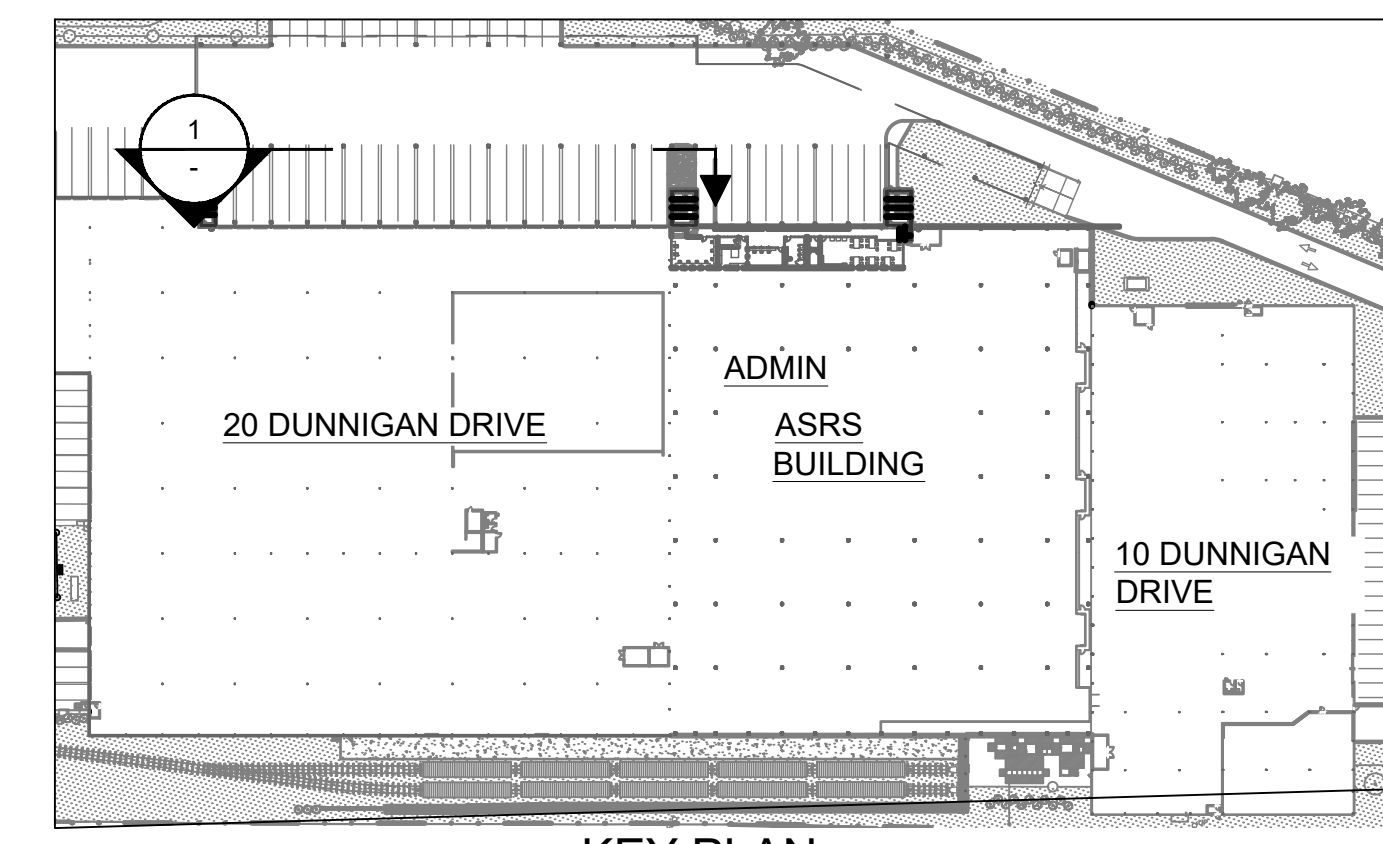
ADMIN BUILDING NORTH ELEVATION EXTERIOR PANEL SUPPORTS
 SCALE: 1/8" = 1'-0"

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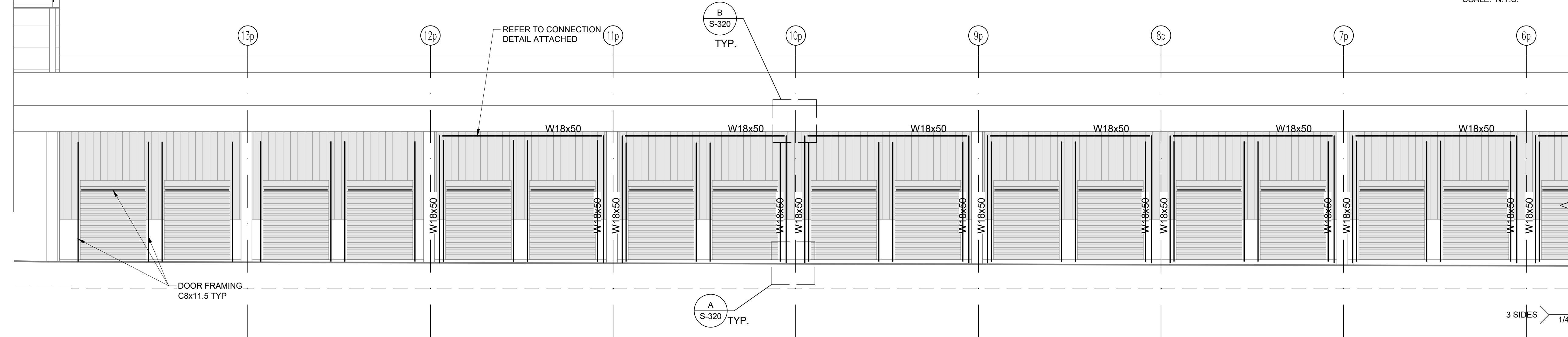
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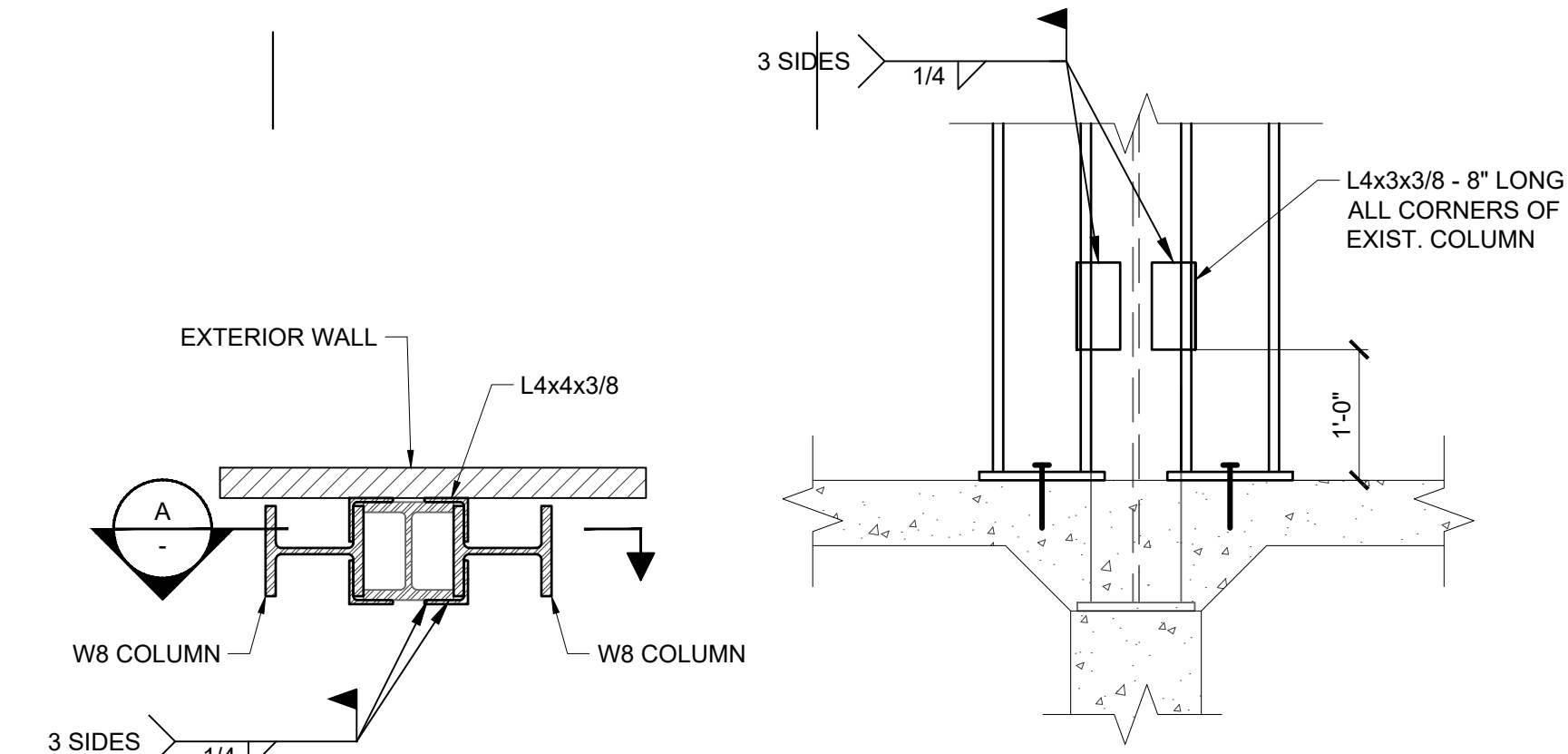
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KEY PLAN
 SCALE: N.T.S.

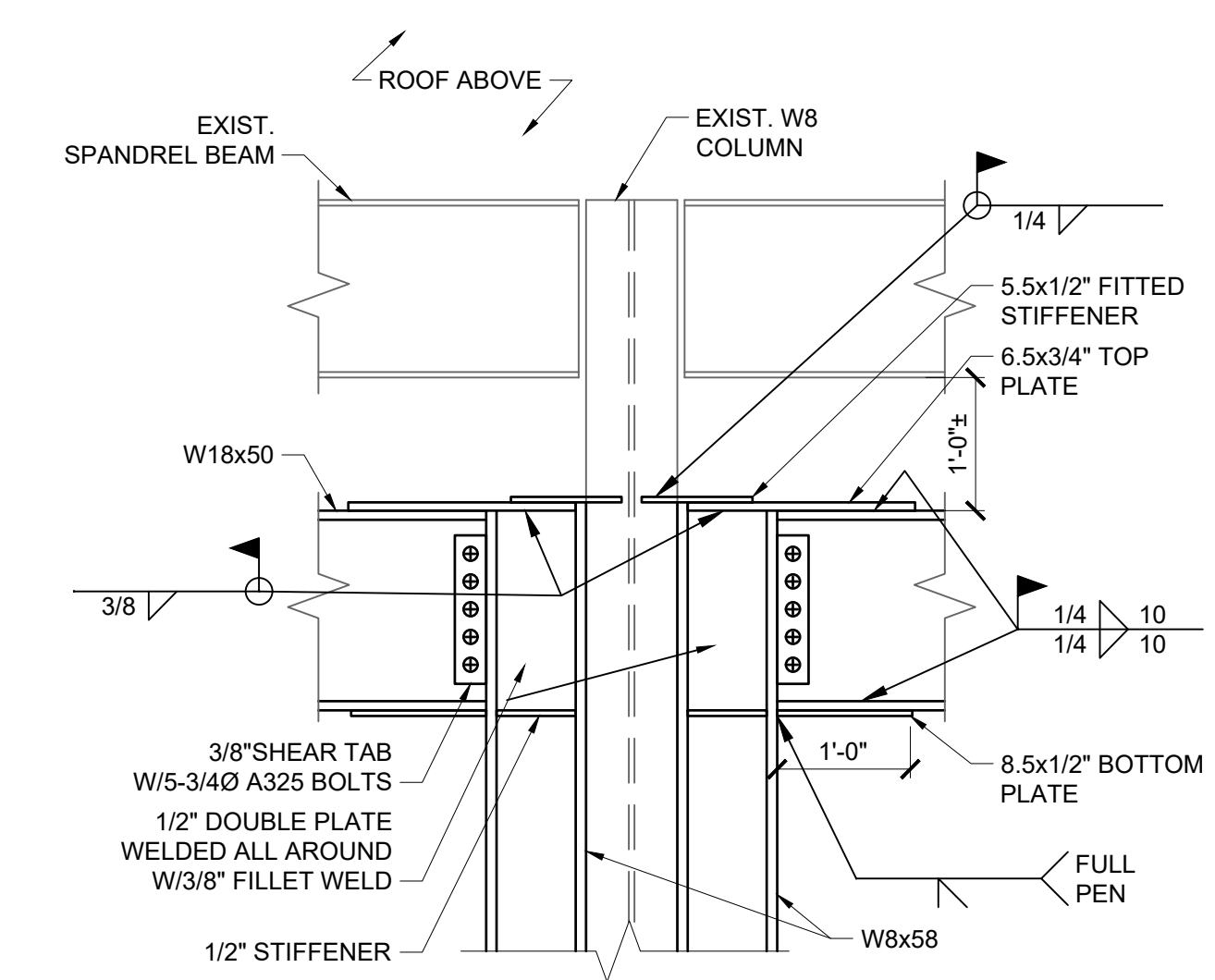


1 NORTH ELEVATION 20 DUNNIGAN DRIVE
 SCALE: 1/8" = 1'-0"



1 DETAIL
 SCALE: 3/4" = 1'-0"

A SECTION
 SCALE: 3/4" = 1'-0"



B SECTION
 SCALE: 3/4" = 1'-0"

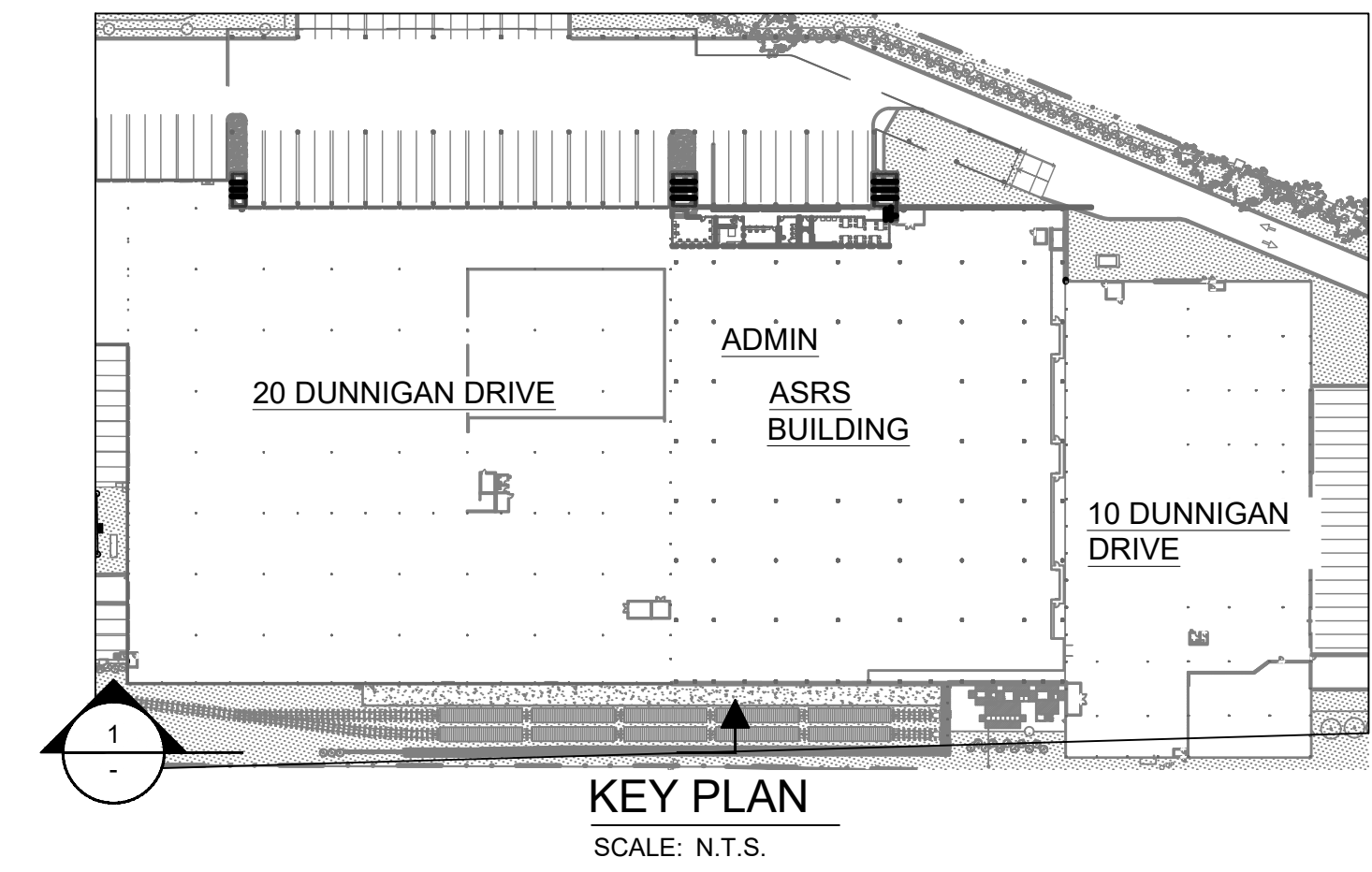
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NORTH ELEVATION 20 DUNNIGAN DRIVE

DWG NUMBER:
S-320
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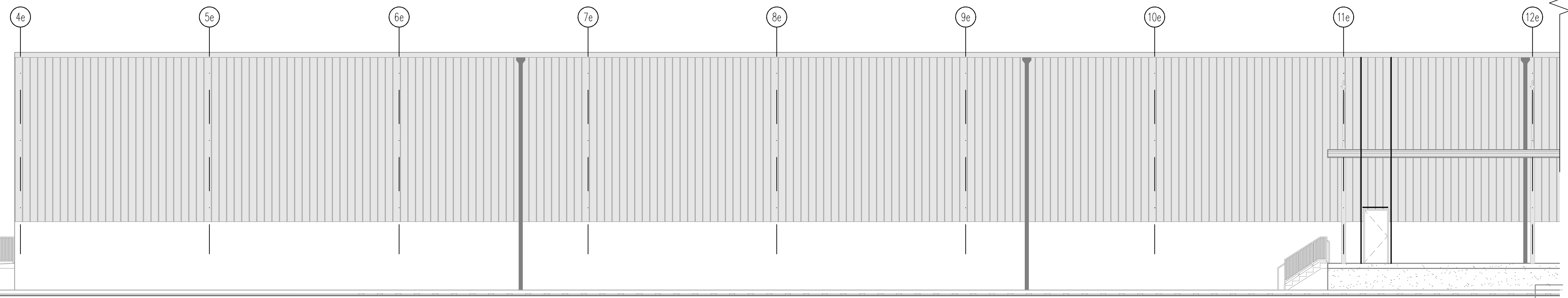
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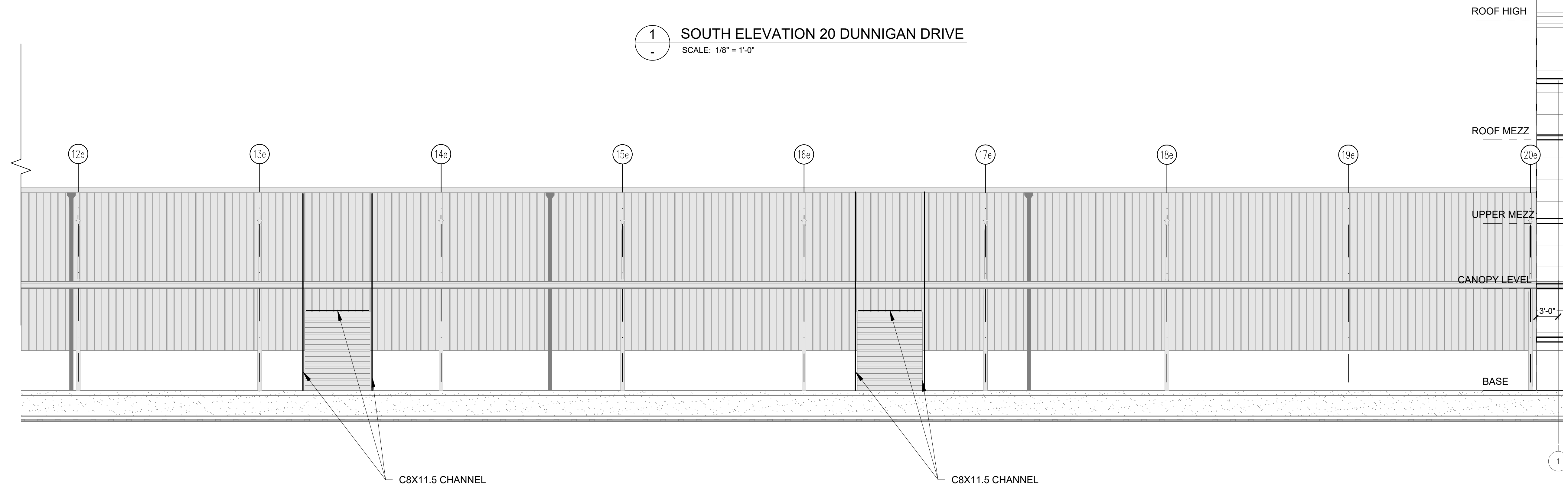
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1 SOUTH ELEVATION 20 DUNNIGAN DRIVE
 SCALE: 1/8" = 1'-0"



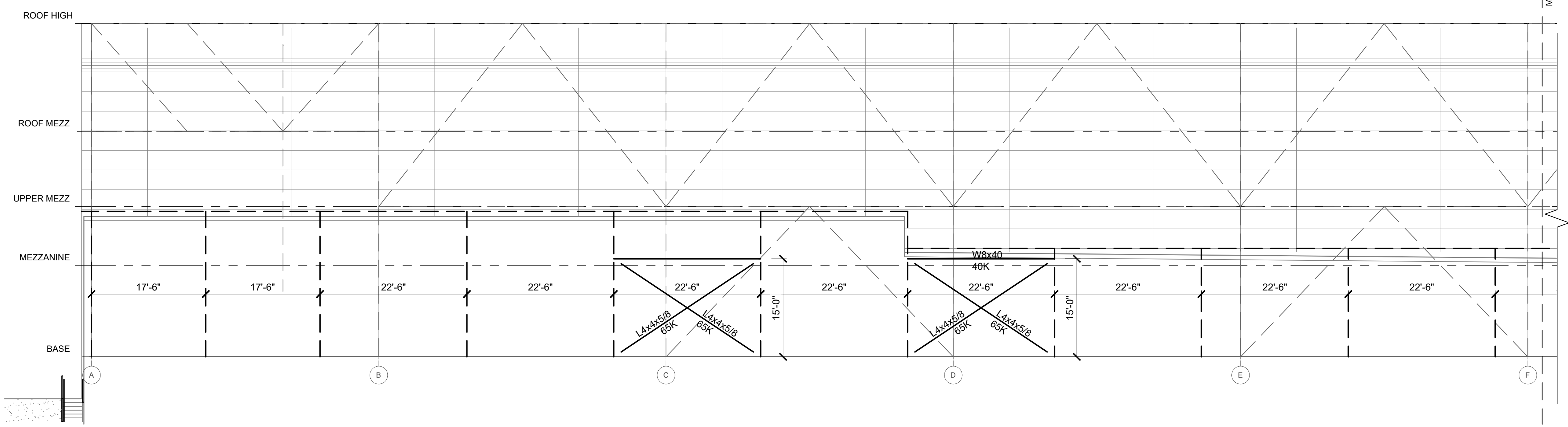
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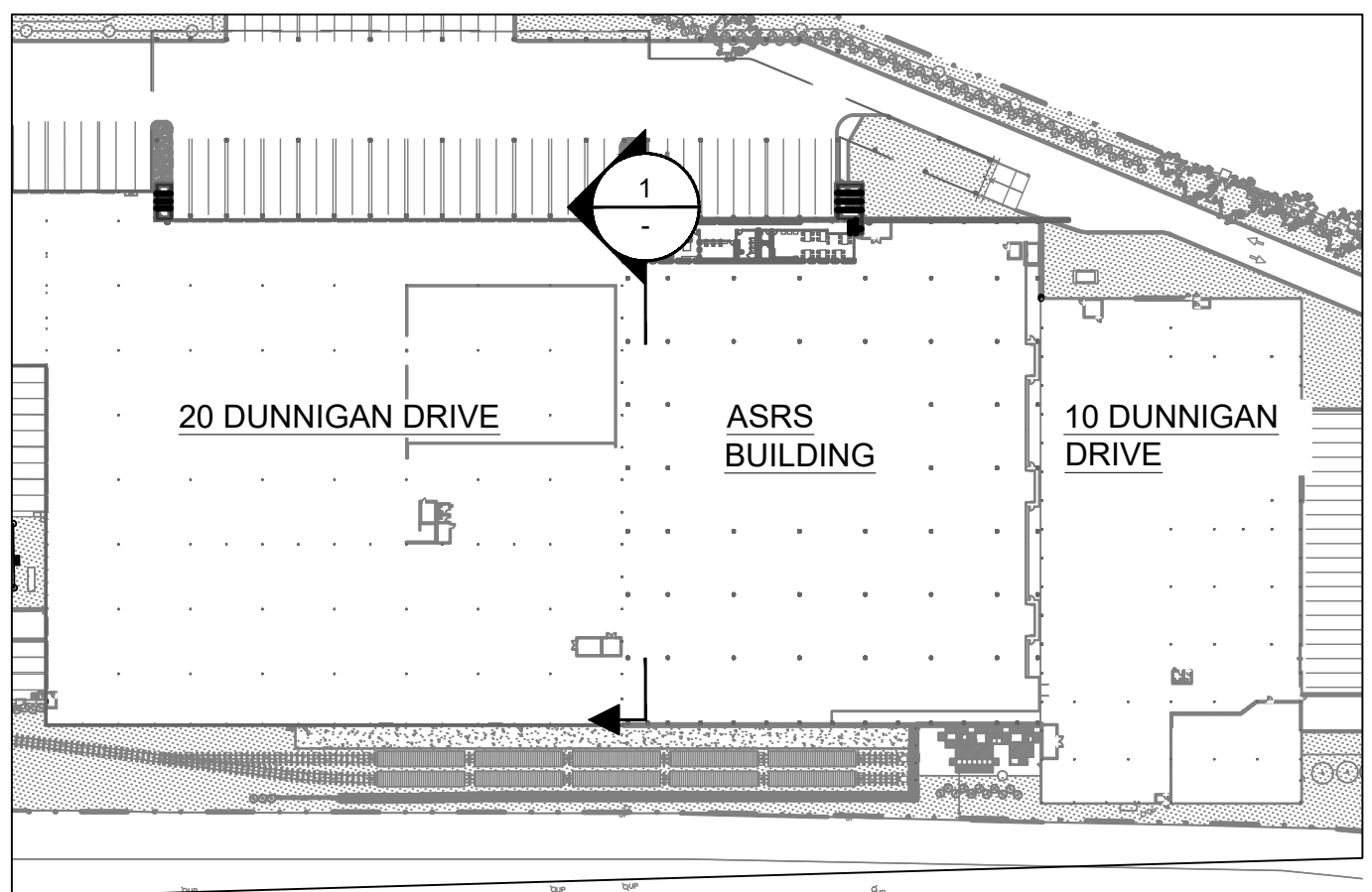
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SOUTH ELEVATION 20 DUNNIGAN

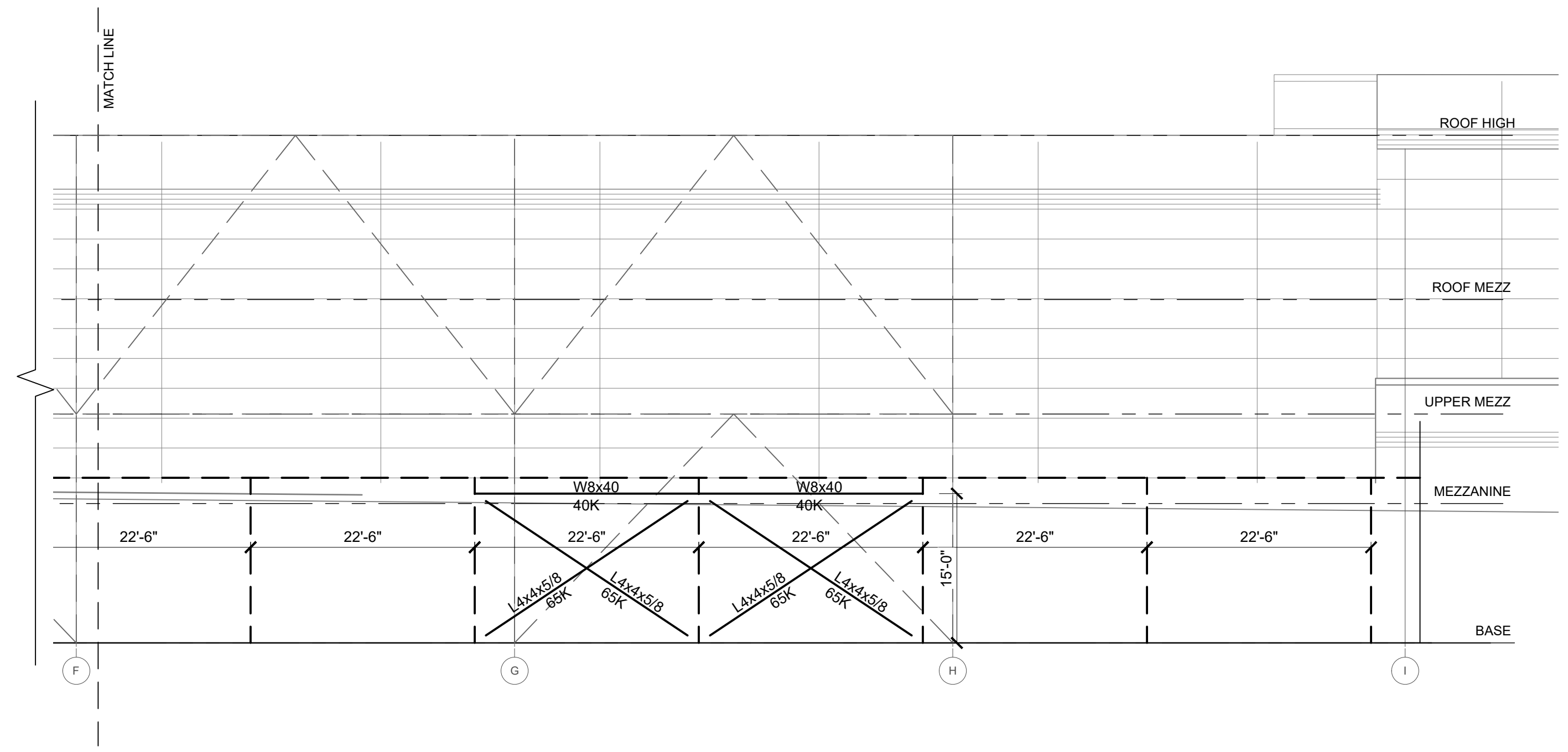
DWG NUMBER :
S-321
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ASRS BUILDING WEST ELEVATION
EXTERIOR PANEL SUPPORTS
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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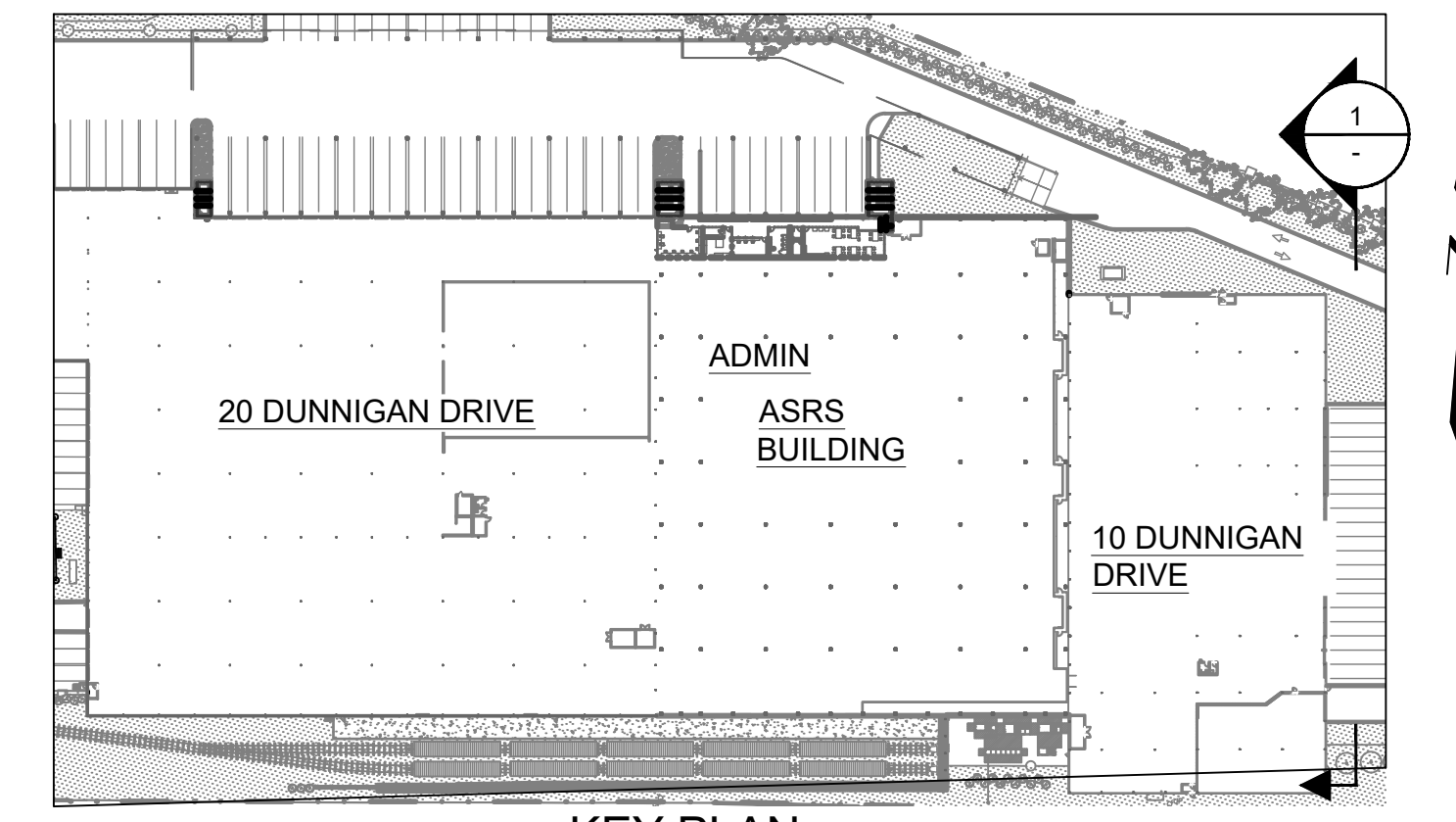
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DRAWING TITLE :

20 DUNNIGAN DRIVE BRACING
FRAME ELEVATION

DWG NUMBER :
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KEY PLAN
SCALE: N.T.S.

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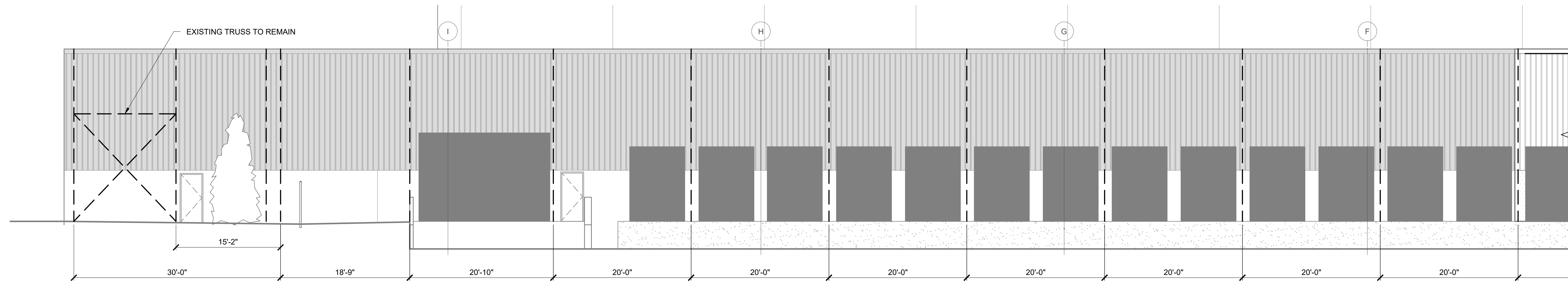
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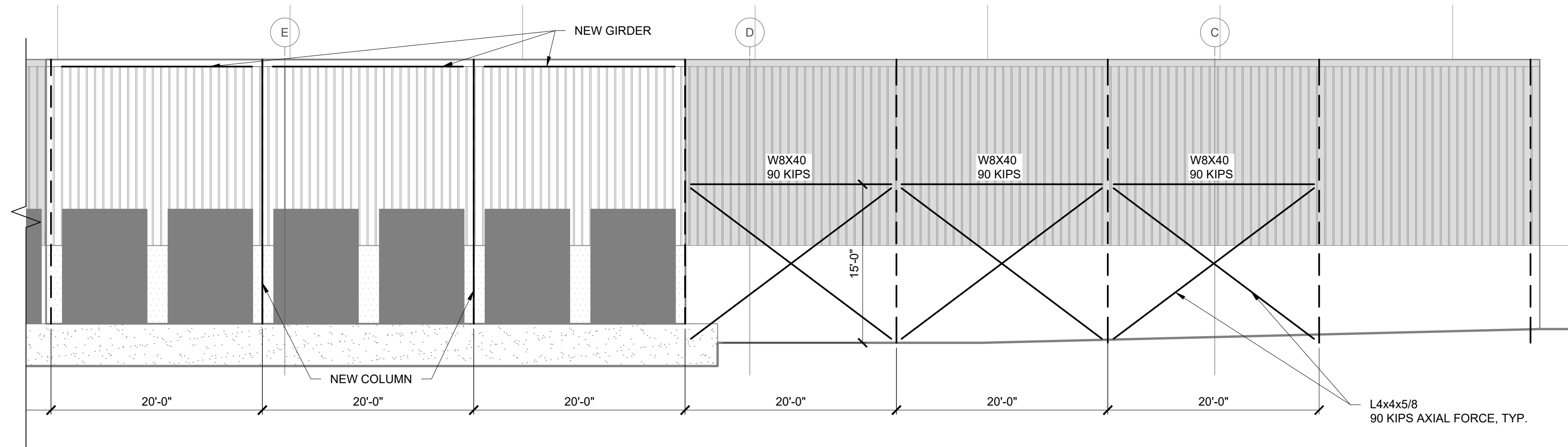
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 20 DUNNIGAN DRIVE
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1 EAST ELEVATION 10 DUNNIGAN DRIVE
 SCALE: 1/8" = 1'-0"



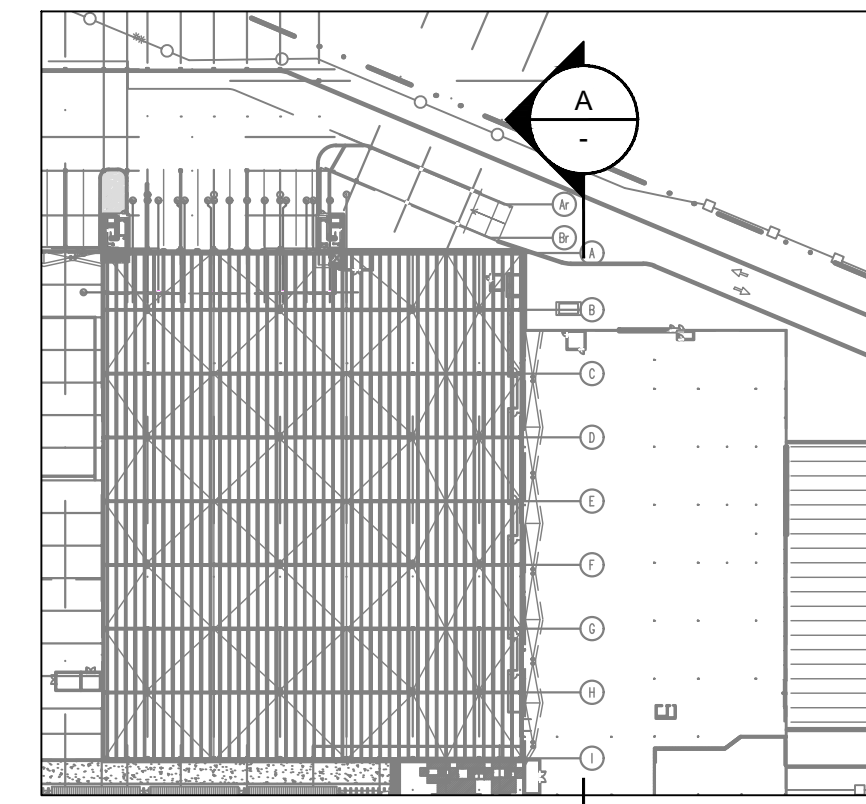
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1	ISSUED FOR BID	10.15.21
2	CD SET	11.30.21

DRAWN BY : D. CHECHURIN
 CHECKED BY : P. BRUNO
 APPROVED BY : P. BRUNO
 DATE : 9/10/2021
 SCALE : AS SHOWN

DRAWING TITLE :

**EAST ELEVATION 10
 DUNNIGAN**

DWG NUMBER :
S-323
 49 OF 60



KEY PLAN
SCALE: N.T.S.

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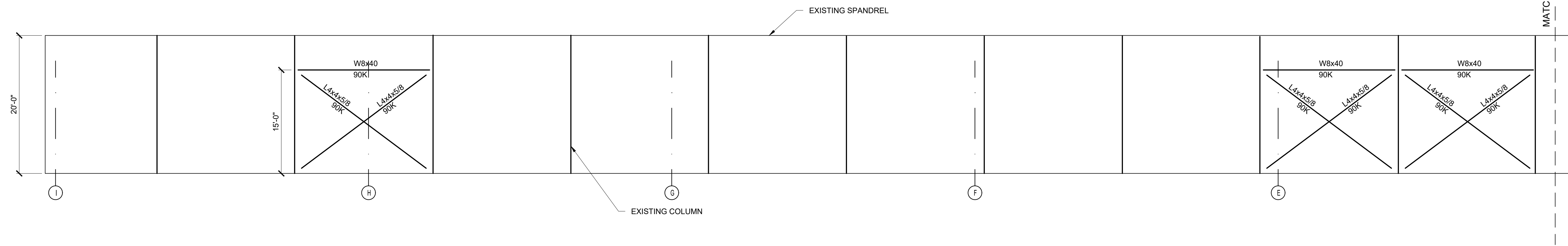
CIVIL PLANNING ENGINEER
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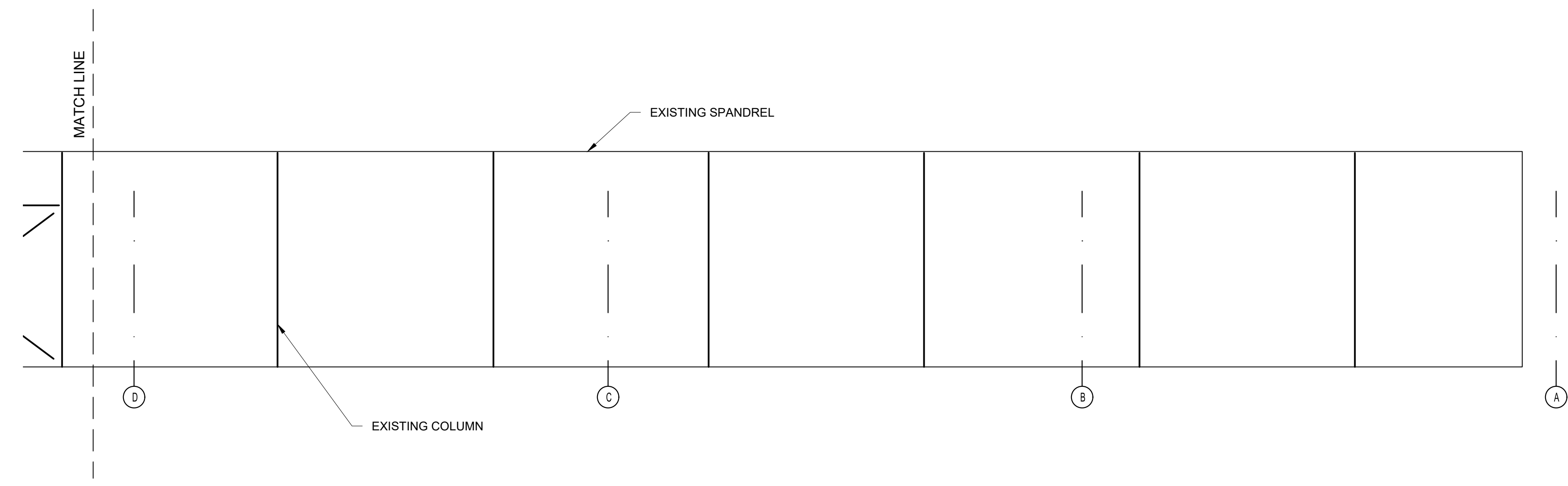
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BRACING ELEVATION AT 10
DUNNIGAN WEST WAL
SCALE: 1/8" = 1'-0"

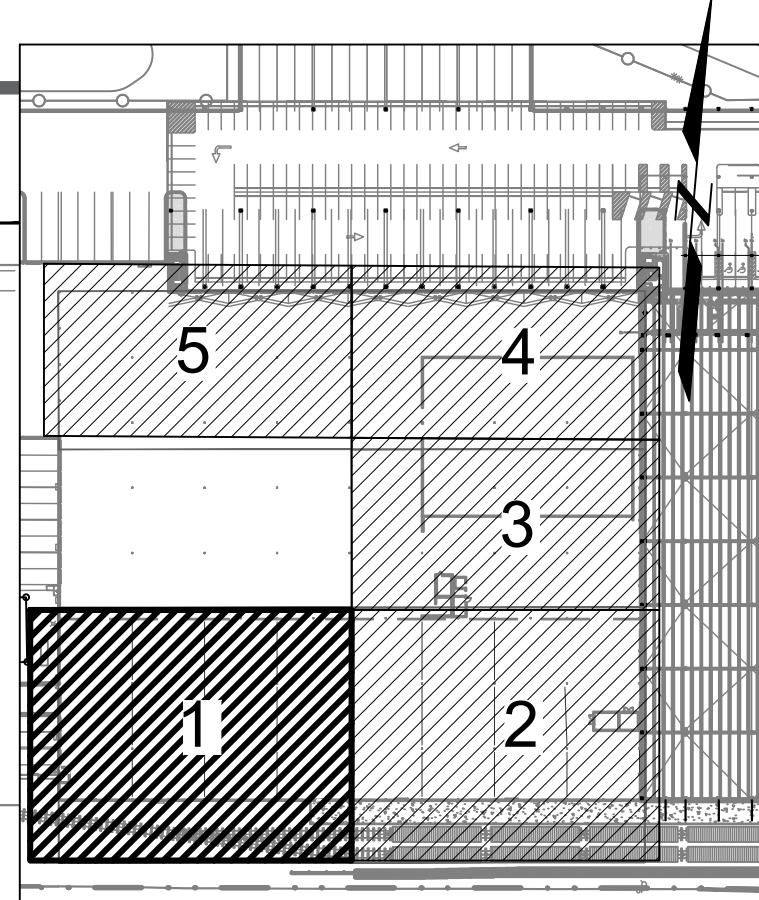
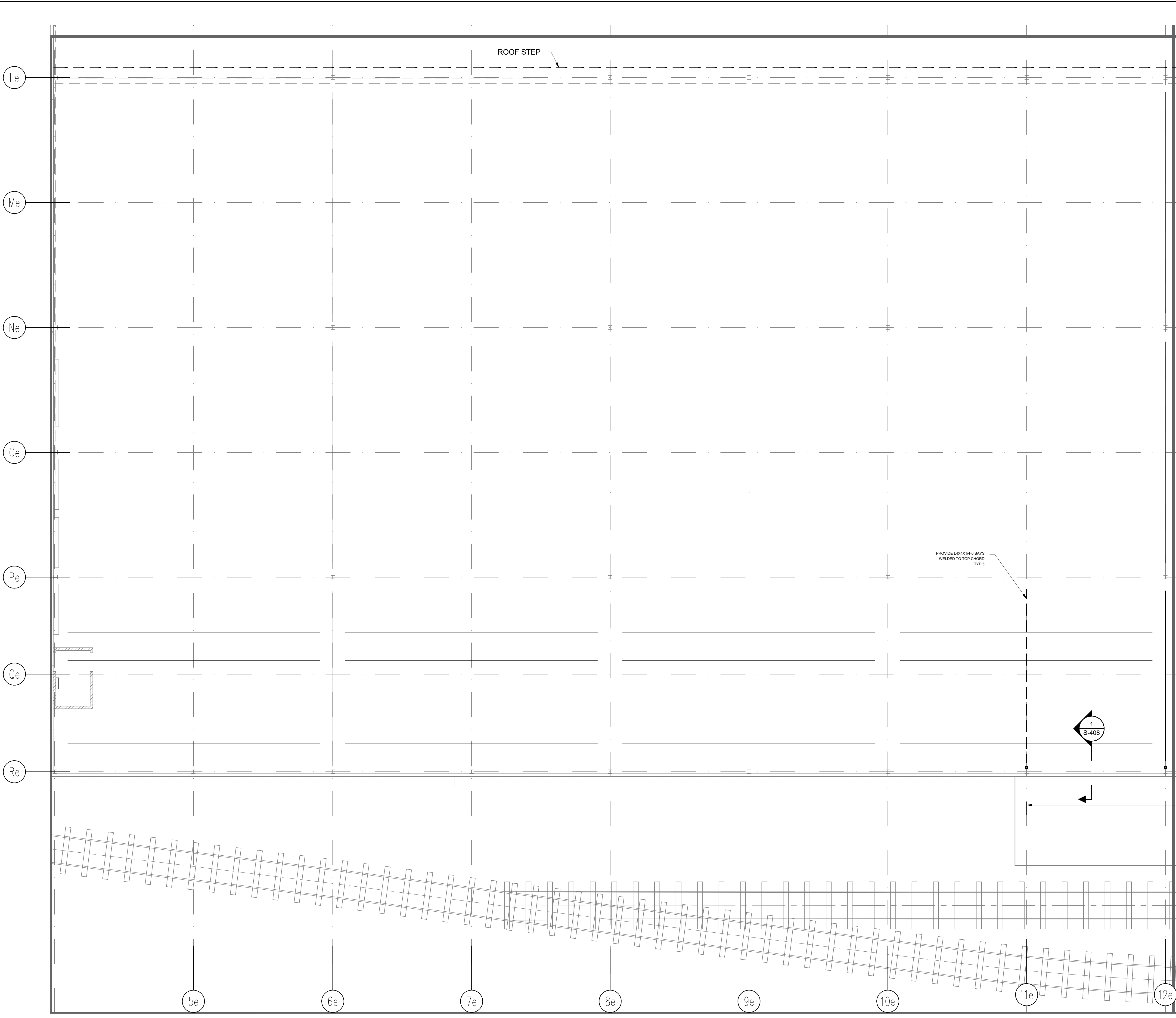


0	ISSUED FOR DOB SUBMISSION	DATE
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DATE : 9/10/2021
SCALE : AS SHOWN

DRAWING TITLE :
**BRACING ELEVATION AT
10 DUNNIGAN WEST WAL**

DWG NUMBER :
S-324
50 OF 60



KEY PLAN
SCALE: N.T.S.

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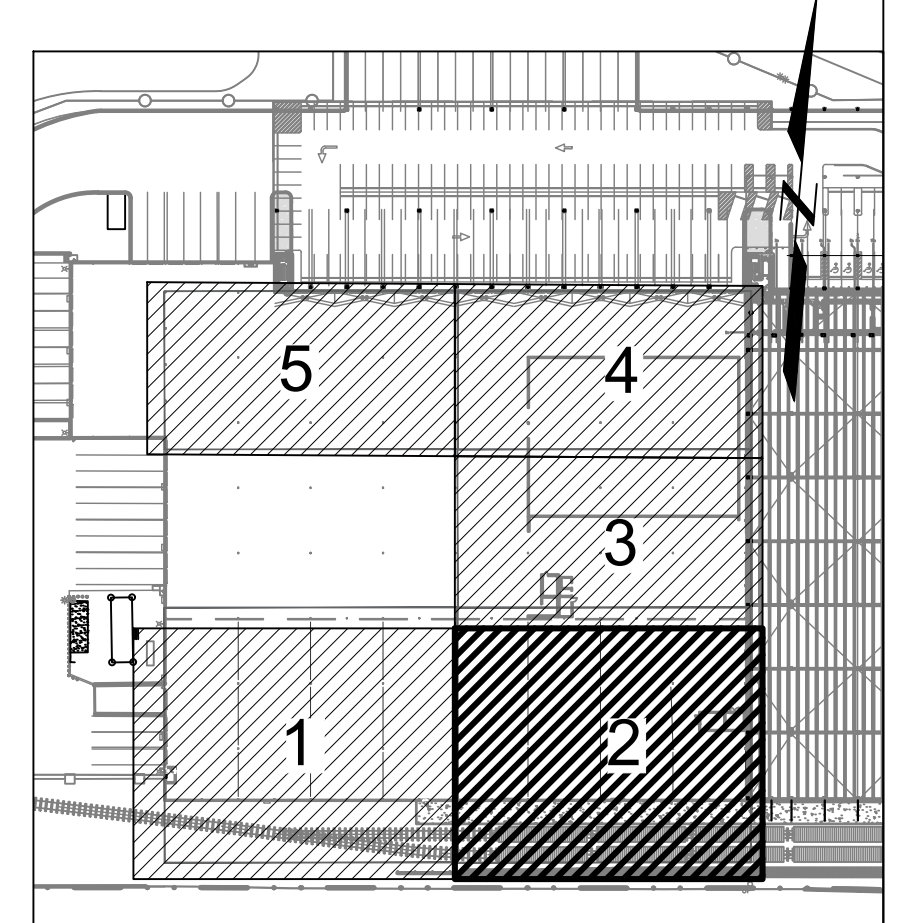
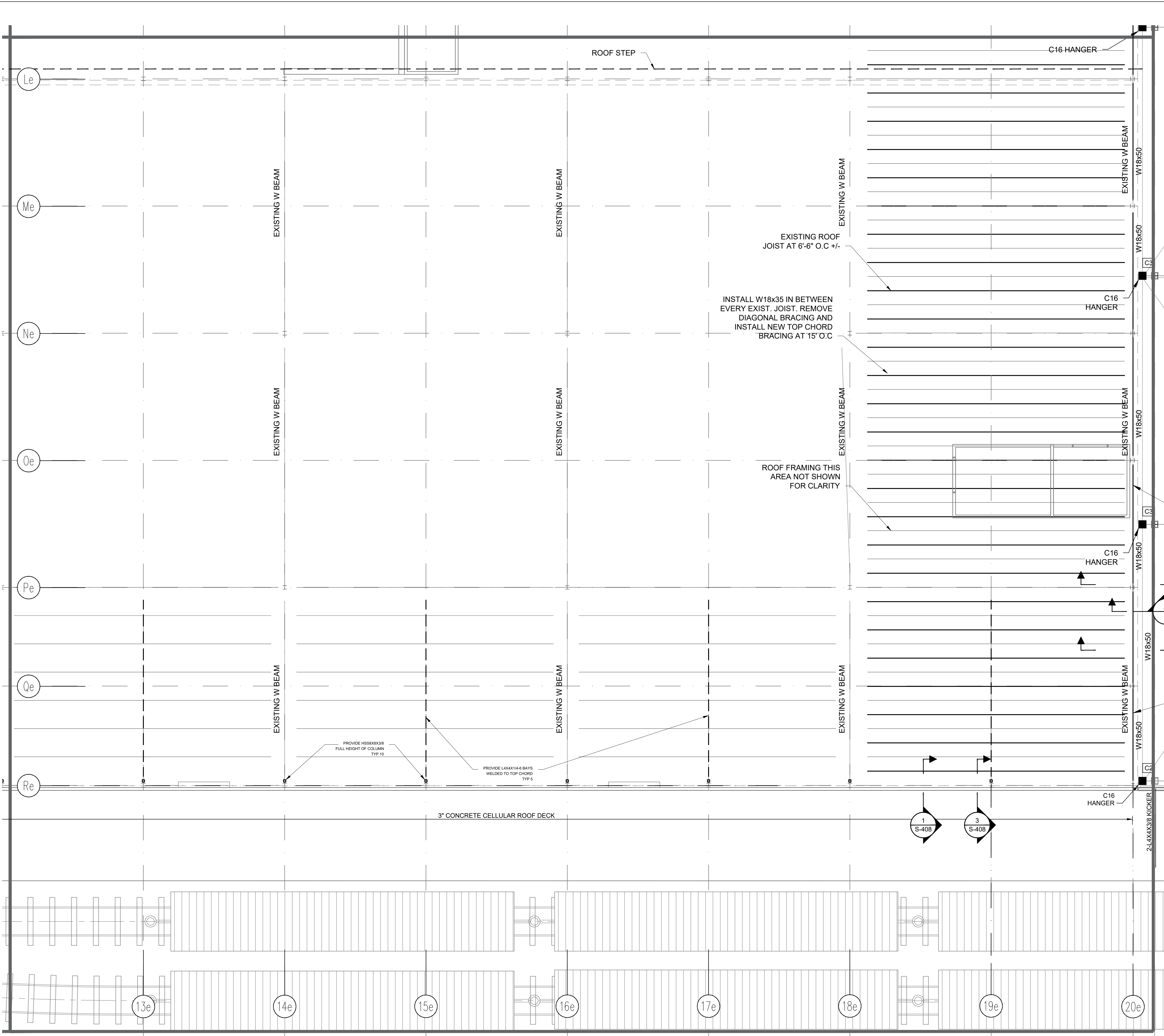
0	ISSUED FOR DOB SUBMISSION	DATE
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 SCALE : AS SHOWN

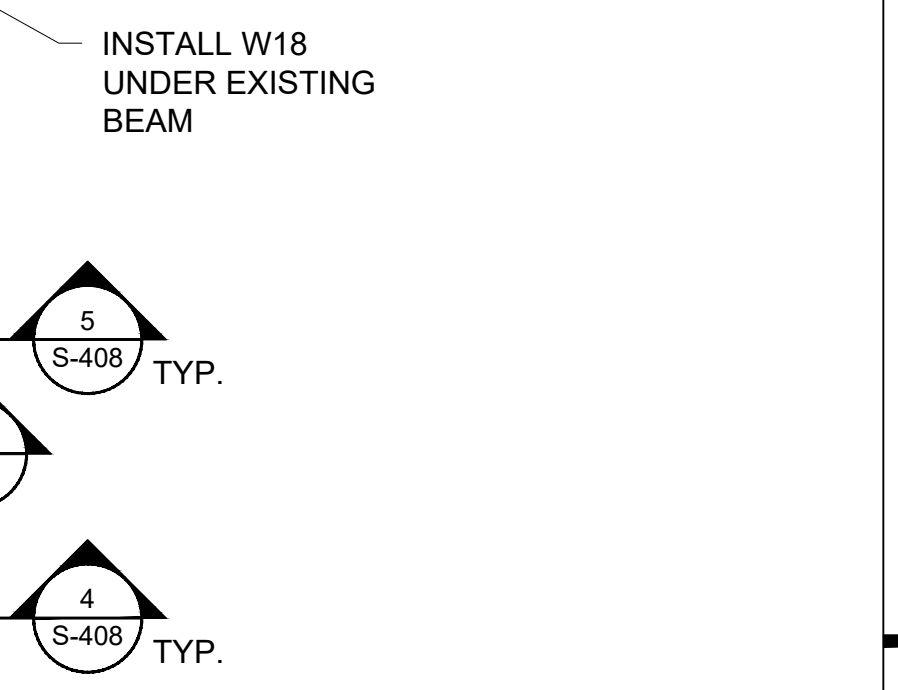
DRAWING TITLE :
**20 DUNNIGAN DRIVE ROOF
 REINFORCING FRAMING PLAN**

DWG NUMBER :
S-400
 51 OF 60

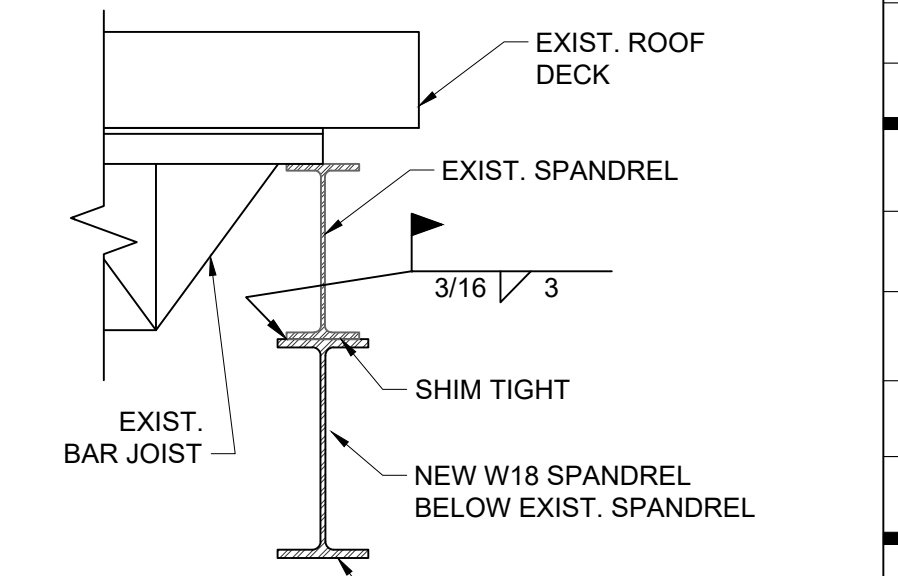
1 PART PLAN
 SCALE: 1/8" = 1'-0"



KEY PLAN
SCALE: N.T.S.



SECTION SPANDREL BEAM REINFORCEMENT, TYP.
SCALE: 1/8" = 1'-0"



PART PLAN
SCALE: 1/8" = 1'-0"

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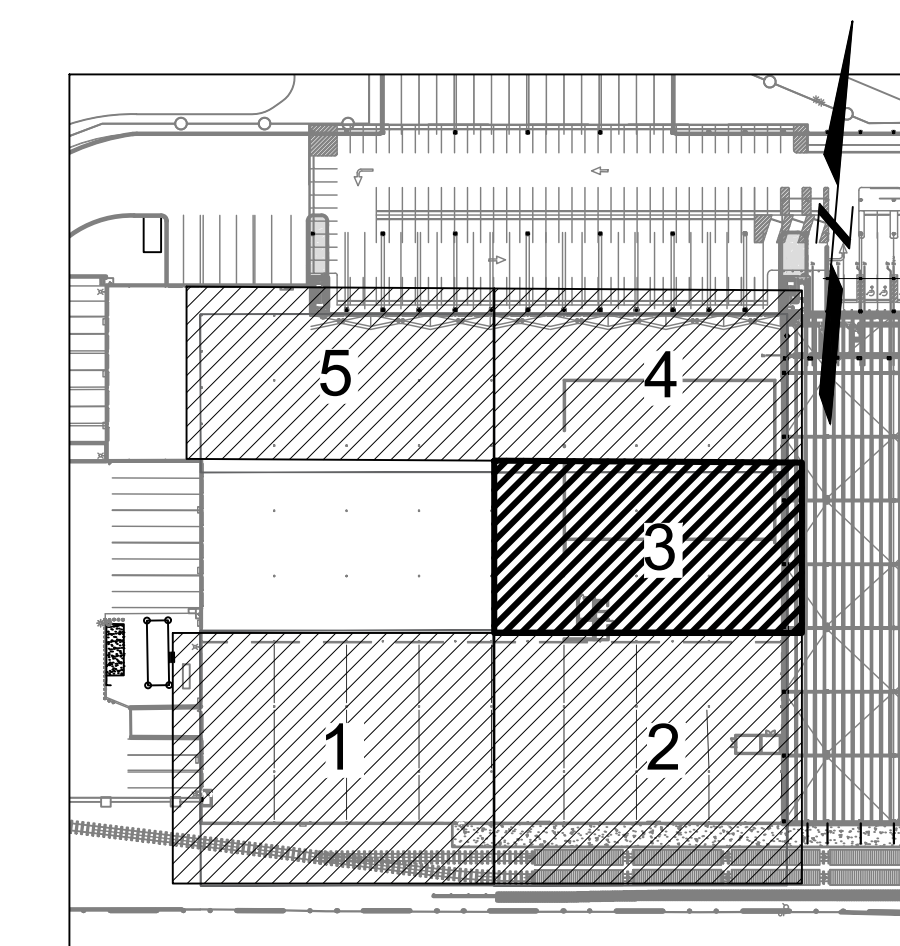
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20 DUNNIGAN DRIVE ROOF REINFORCING FRAMING PLAN

DWG NUMBER :
S-401
52 OF 60



KEY PLAN
SCALE: N.T.S.

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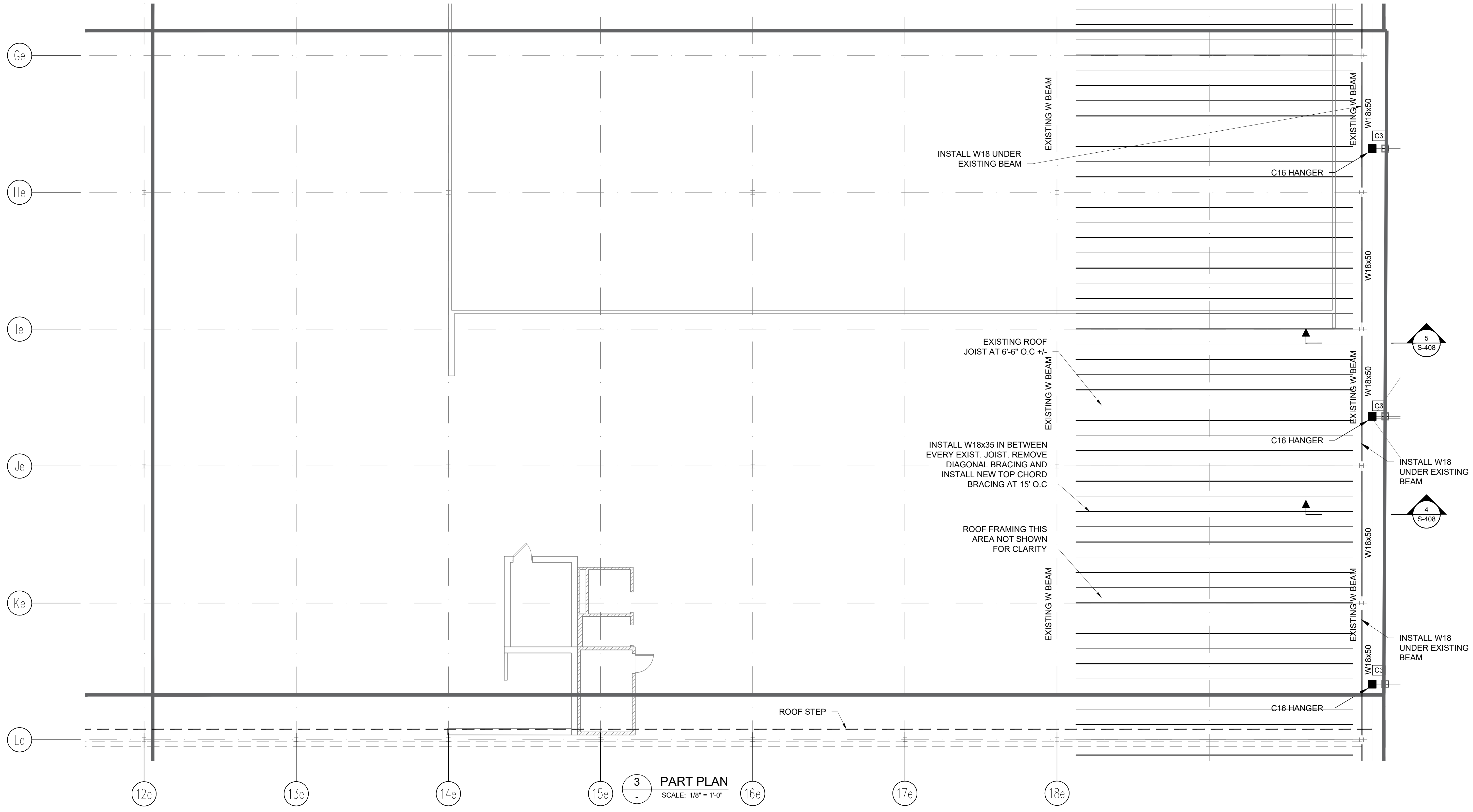
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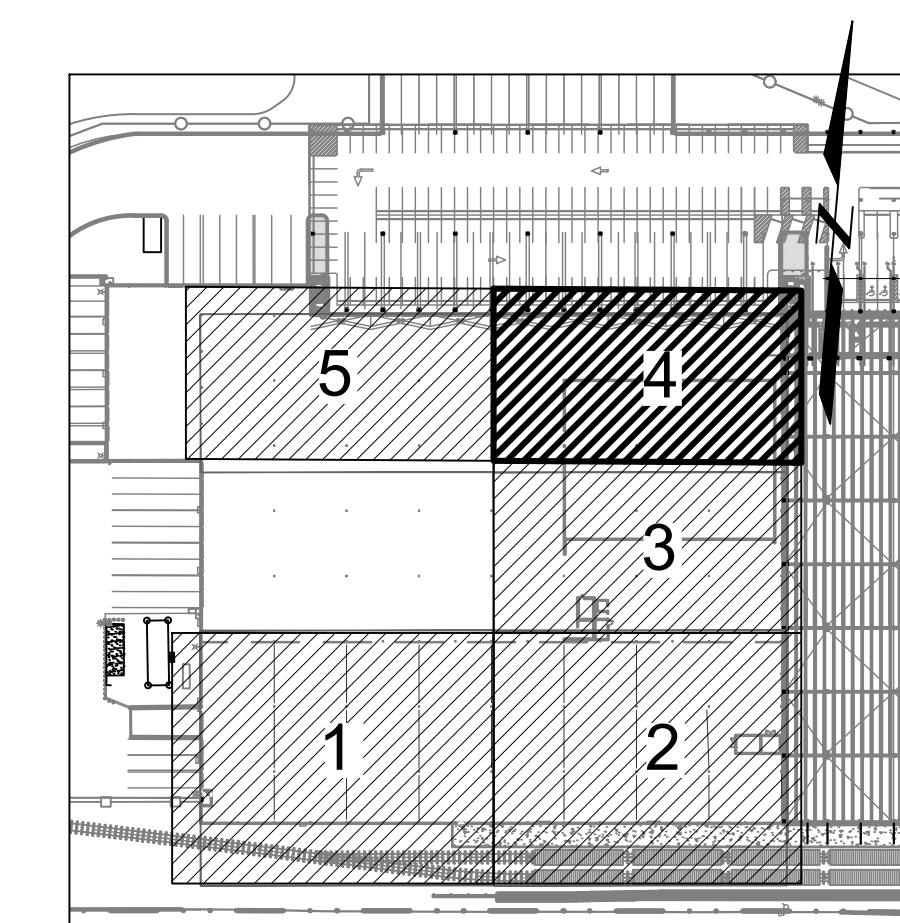
3 PART PLAN
SCALE: 1/8" = 1'-0"

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SCALE :	AS SHOWN
DRAWING TITLE :	

20 DUNNIGAN DRIVE ROOF REINFORCING FRAMING PLAN

DWG NUMBER :
S-402
53 OF 60



KEY PLAN
SCALE: N.T.S.

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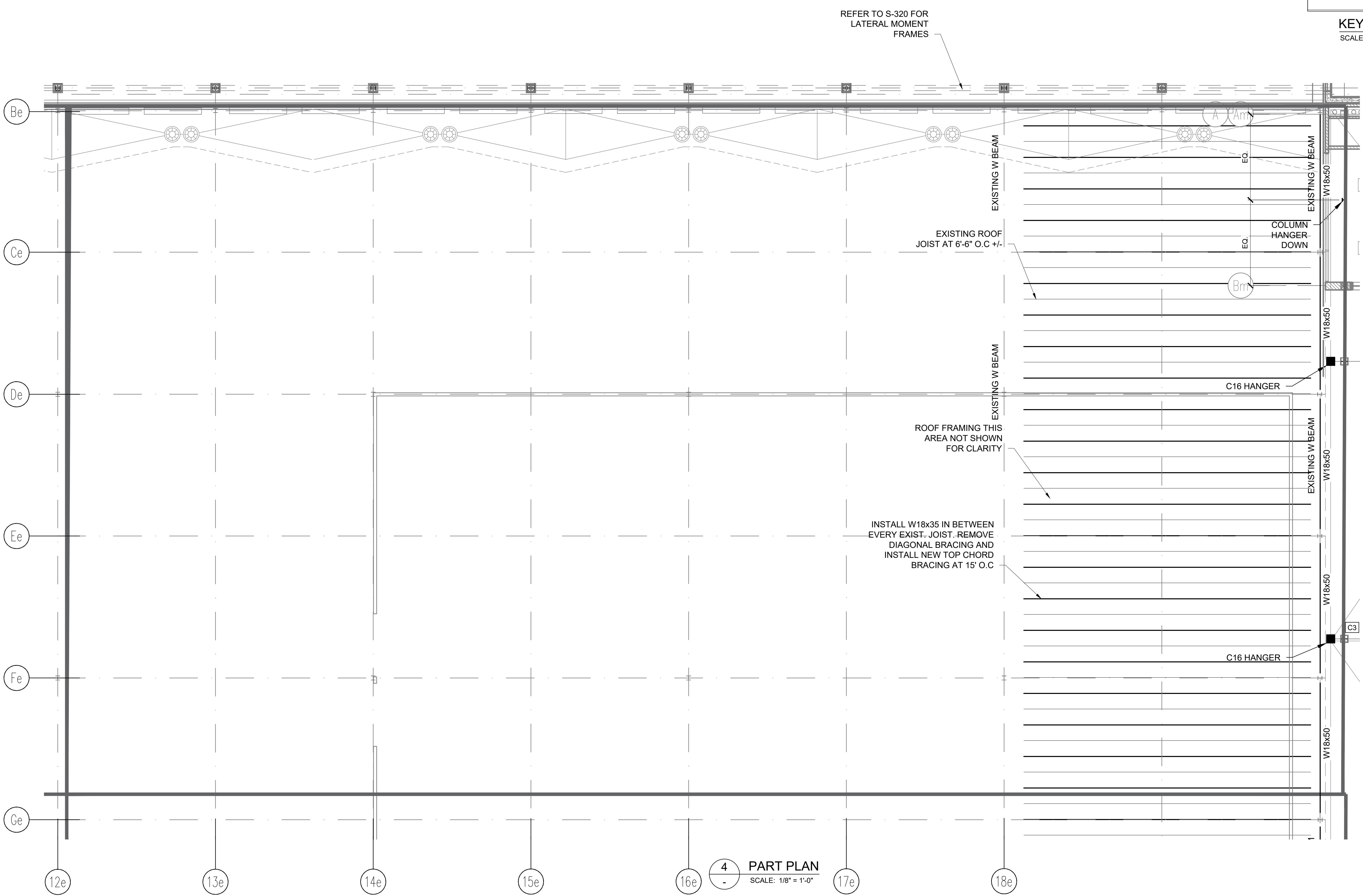
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APPROVED BY : P. BRUNO
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SCALE : AS SHOWN

DRAWING TITLE :

**20 DUNNIGAN DRIVE ROOF
REINFORCING FRAMING PLAN**

DWG NUMBER :
S-403
54 OF 60



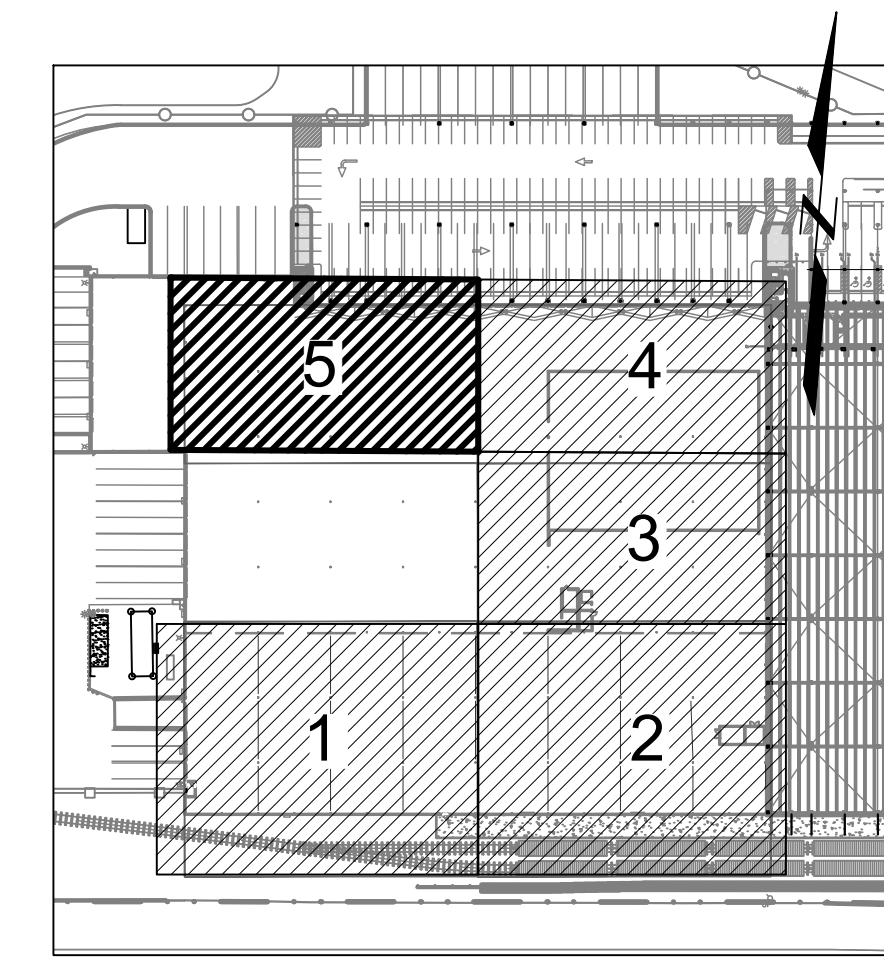
PART PLAN
SCALE: 1/8" = 1'-0"

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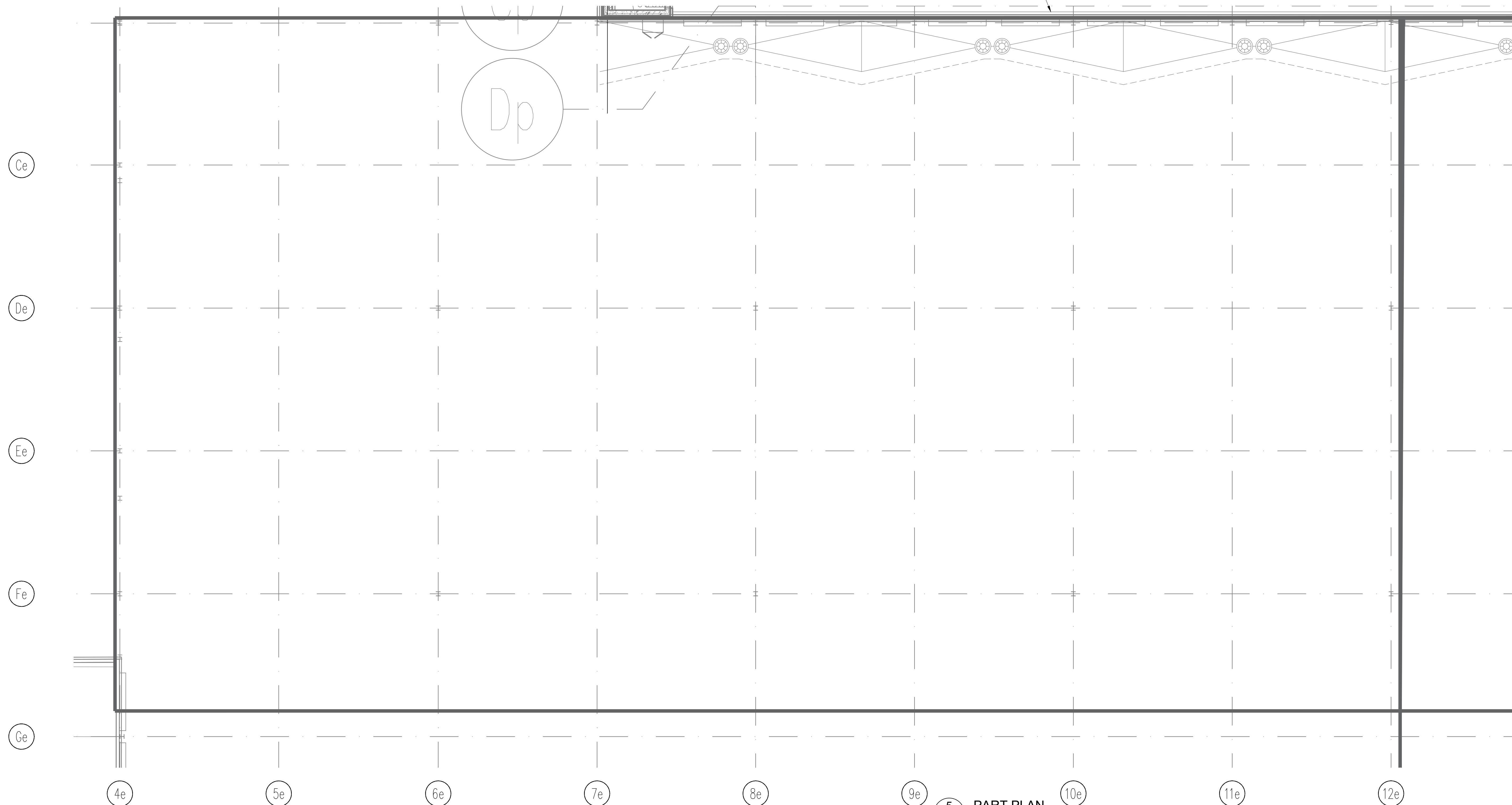
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KEY PLAN
 SCALE: N.T.

REFER TO S-320 FOR
 LATERAL MOMENT
 FRAMES



5 PART PLAN
 SCALE: 1/8" = 1'-0"

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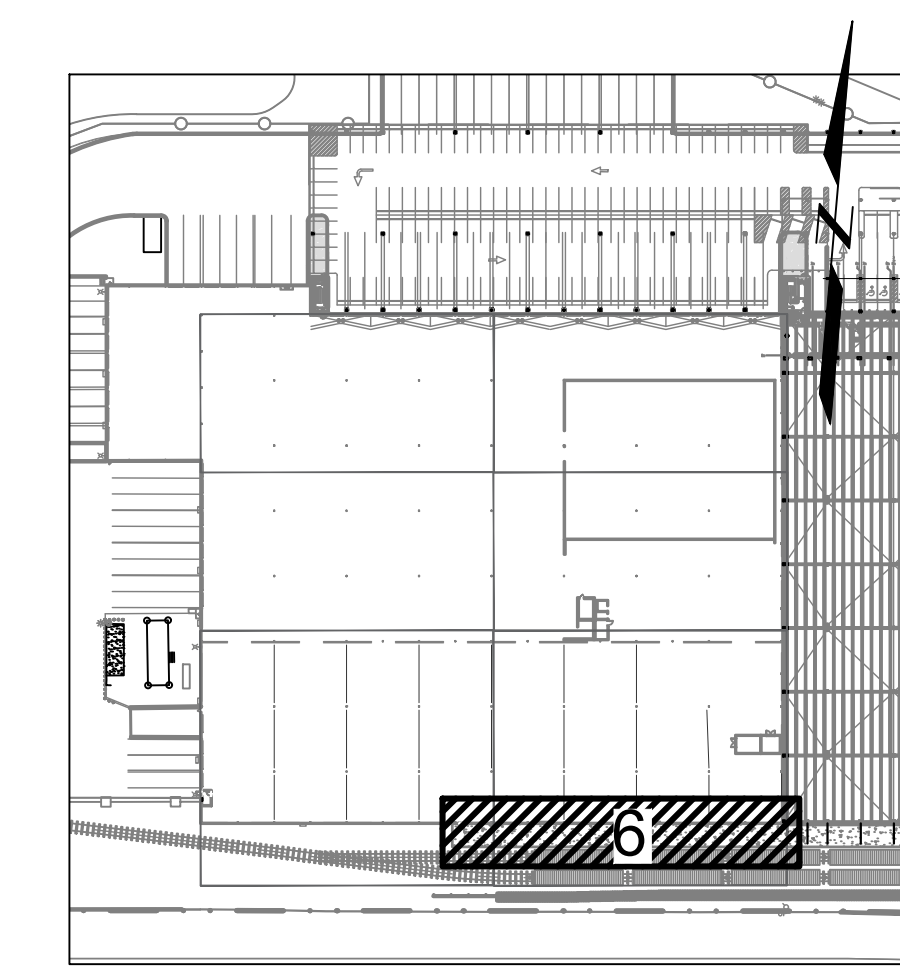
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 SCALE : AS SHOWN

DRAWING TITLE :
 20 DUNNIGAN DRIVE ROOF
 REINFORCING FRAMING PLAN

DWG NUMBER :
S-404
 55 OF 60



KEY PLAN
SCALE: N.T.S.

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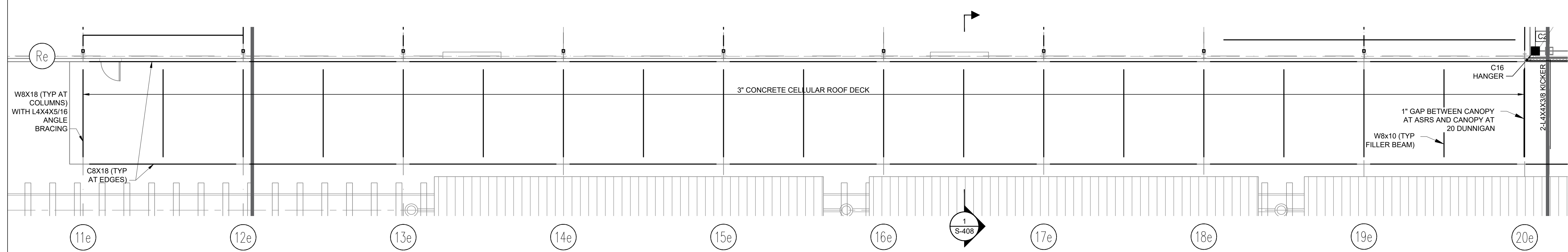
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6 PART PLAN
SCALE: 1/8" = 1'-0"

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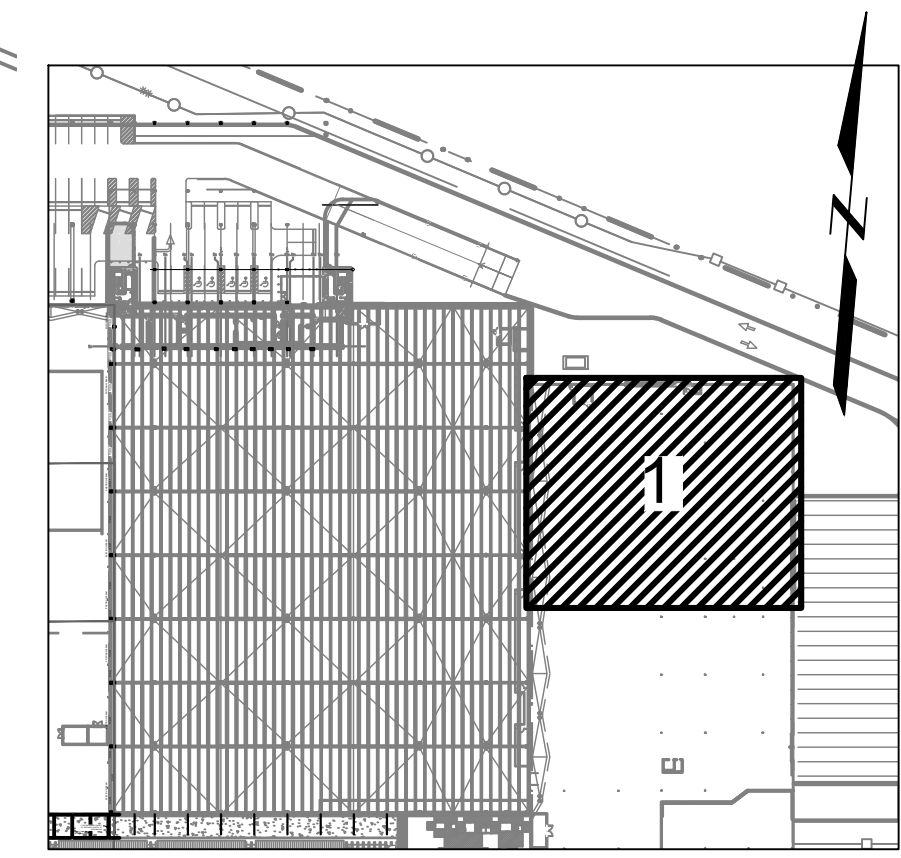
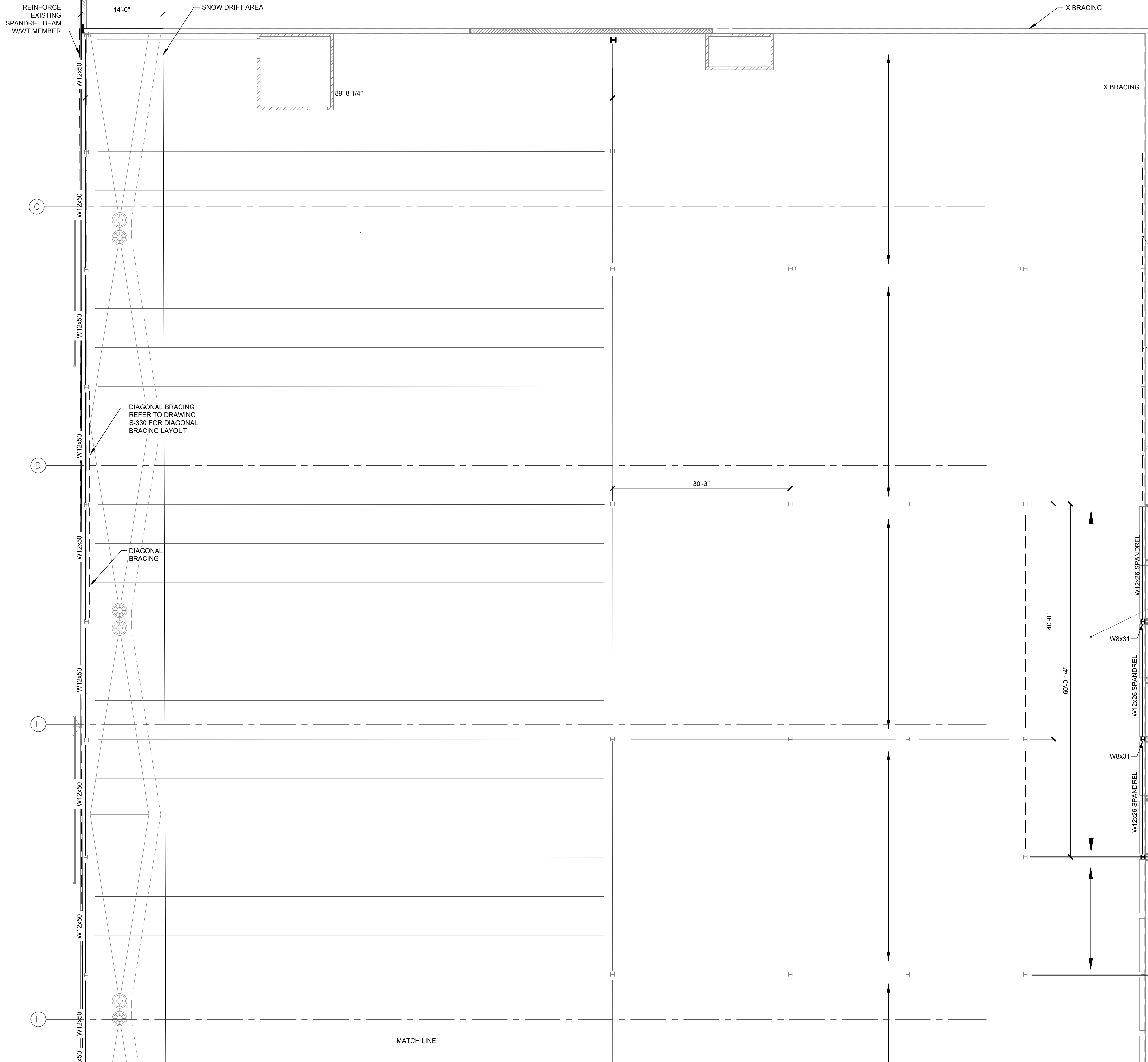
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DATE :	9/10/2021
SCALE :	AS SHOWN

DRAWING TITLE :

**PROPOSED CANOPY FRAMING
PLAN 20 DUNNIGAN DRIVE**

DWG NUMBER :
S-405

56 OF 60



KEY PLAN
SCALE: N.T.S.

NEW DIAGONAL BRACING
REFER TO BUILDING
ELEVATION

EXISTING ROOF JOISTS
TO REMAIN

1 PART PLAN
SCALE: 1/8" = 1'-0"

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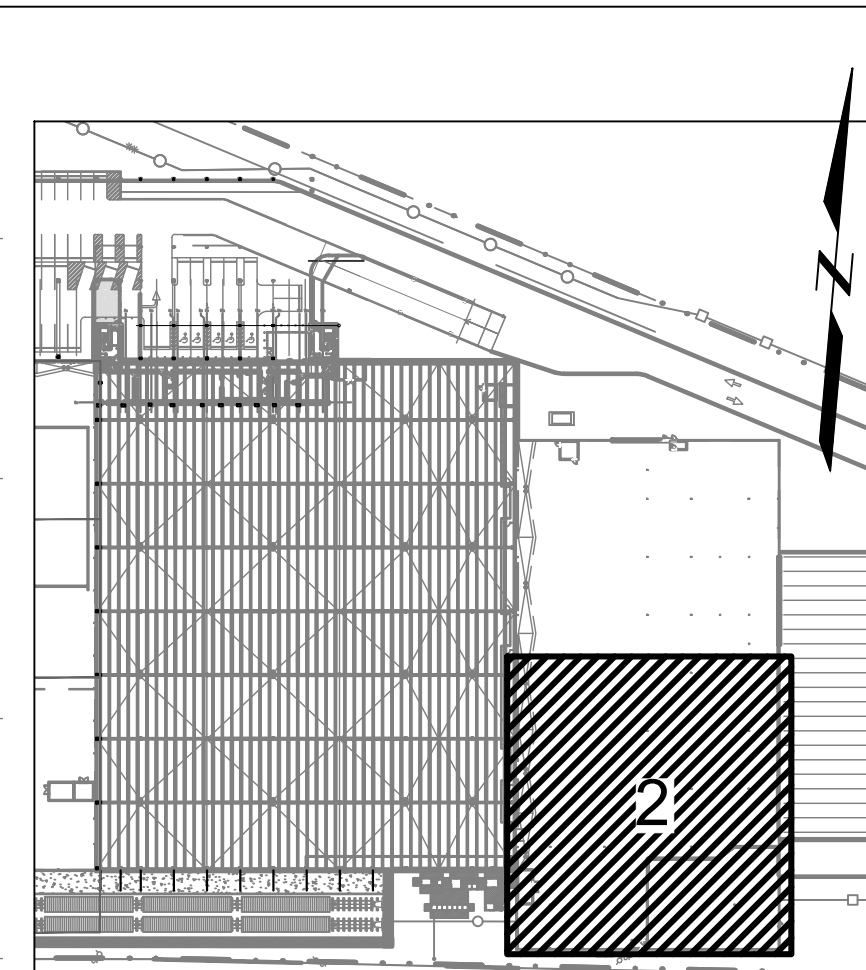
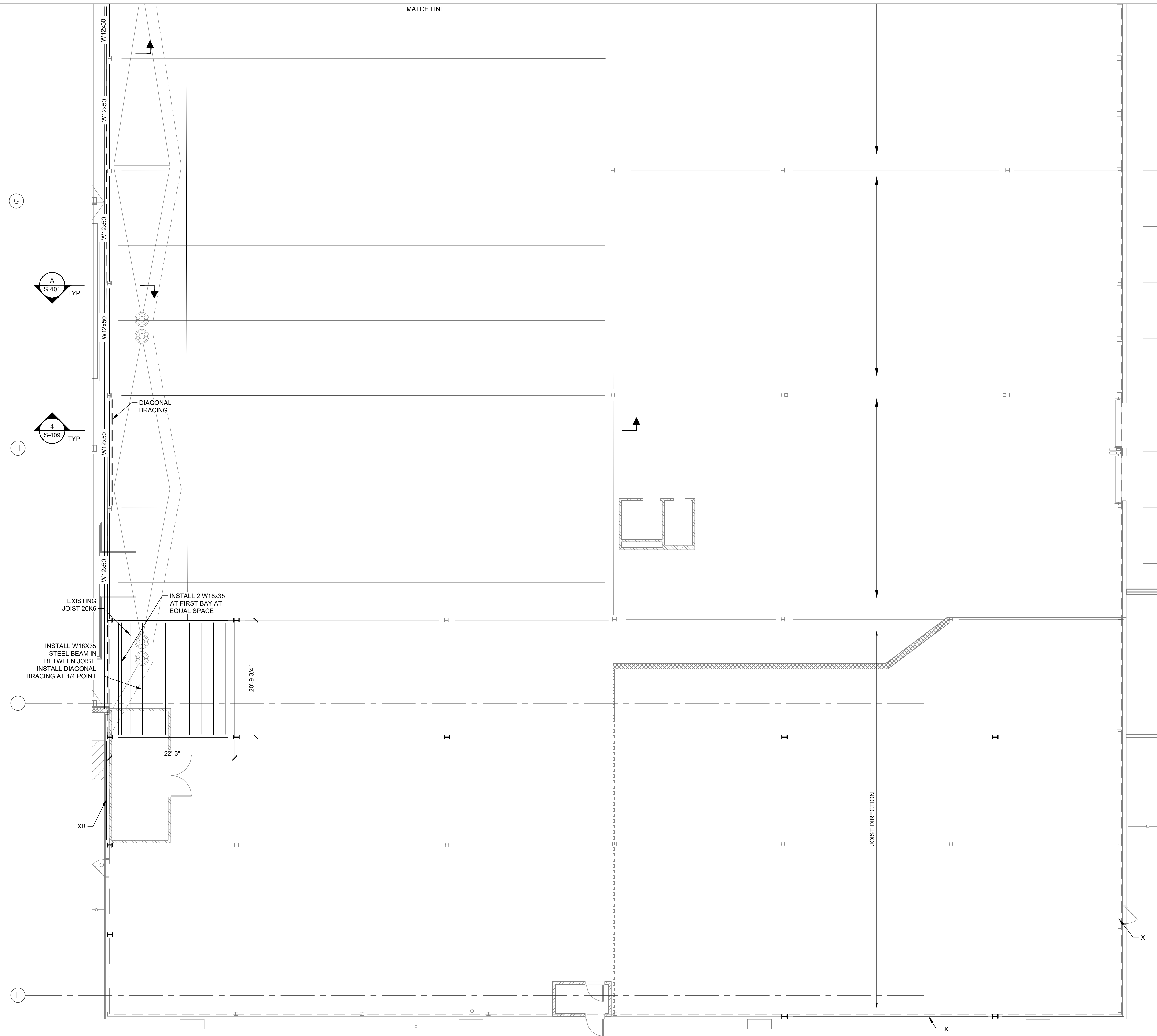
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APPROVED BY : P. BRUNO
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SCALE : AS SHOWN

DRAWING TITLE :
**10 DUNNIGAN DRIVE
ROOF FRAMING PLAN**

DWG NUMBER :
S-406
57 OF 60



KEY PLAN
SCALE: N.T.S.

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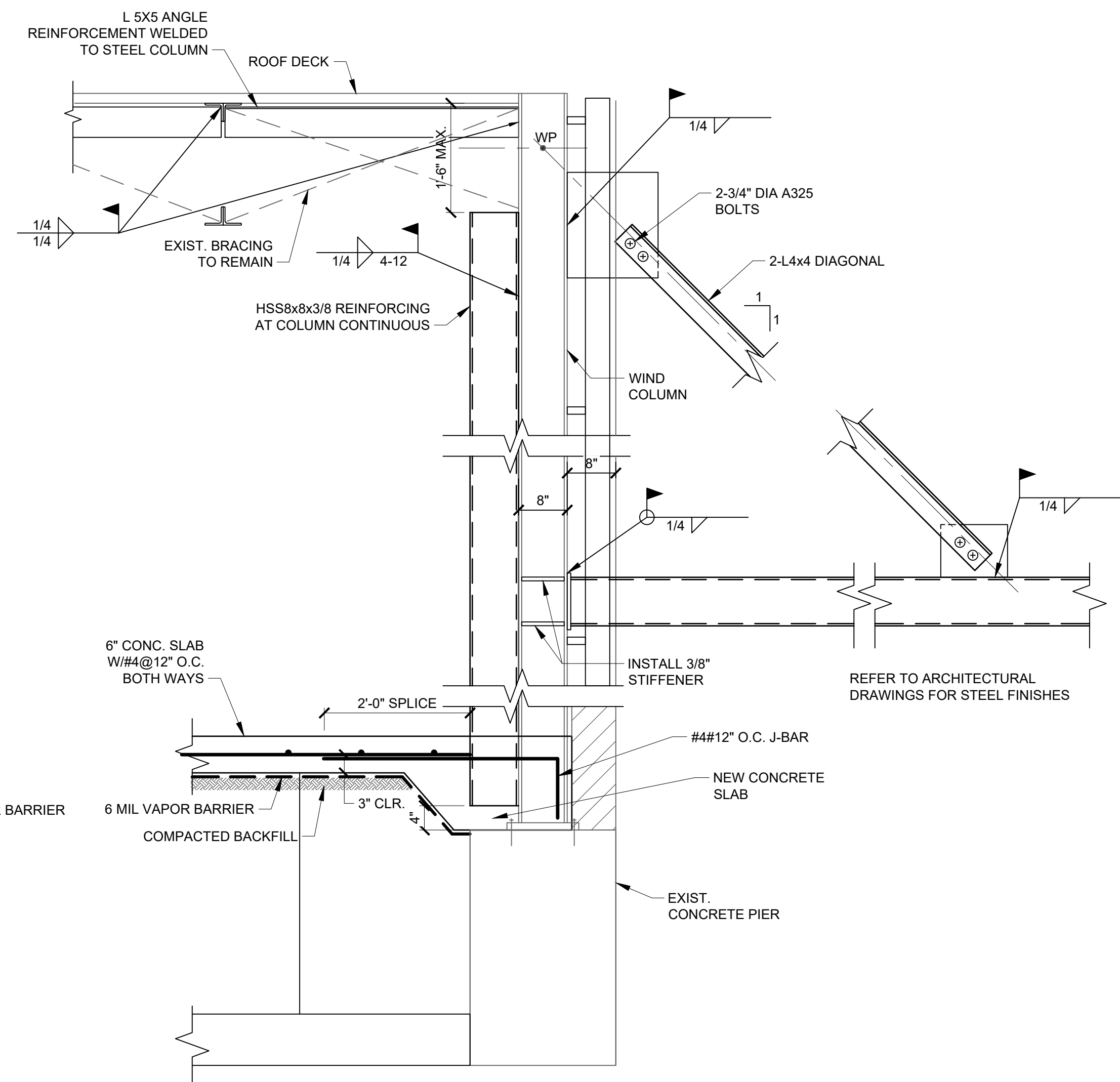
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CHECKED BY : P. BRUNO
APPROVED BY : P. BRUNO
DATE : 9/10/2021
SCALE : AS SHOWN

DRAWING TITLE :

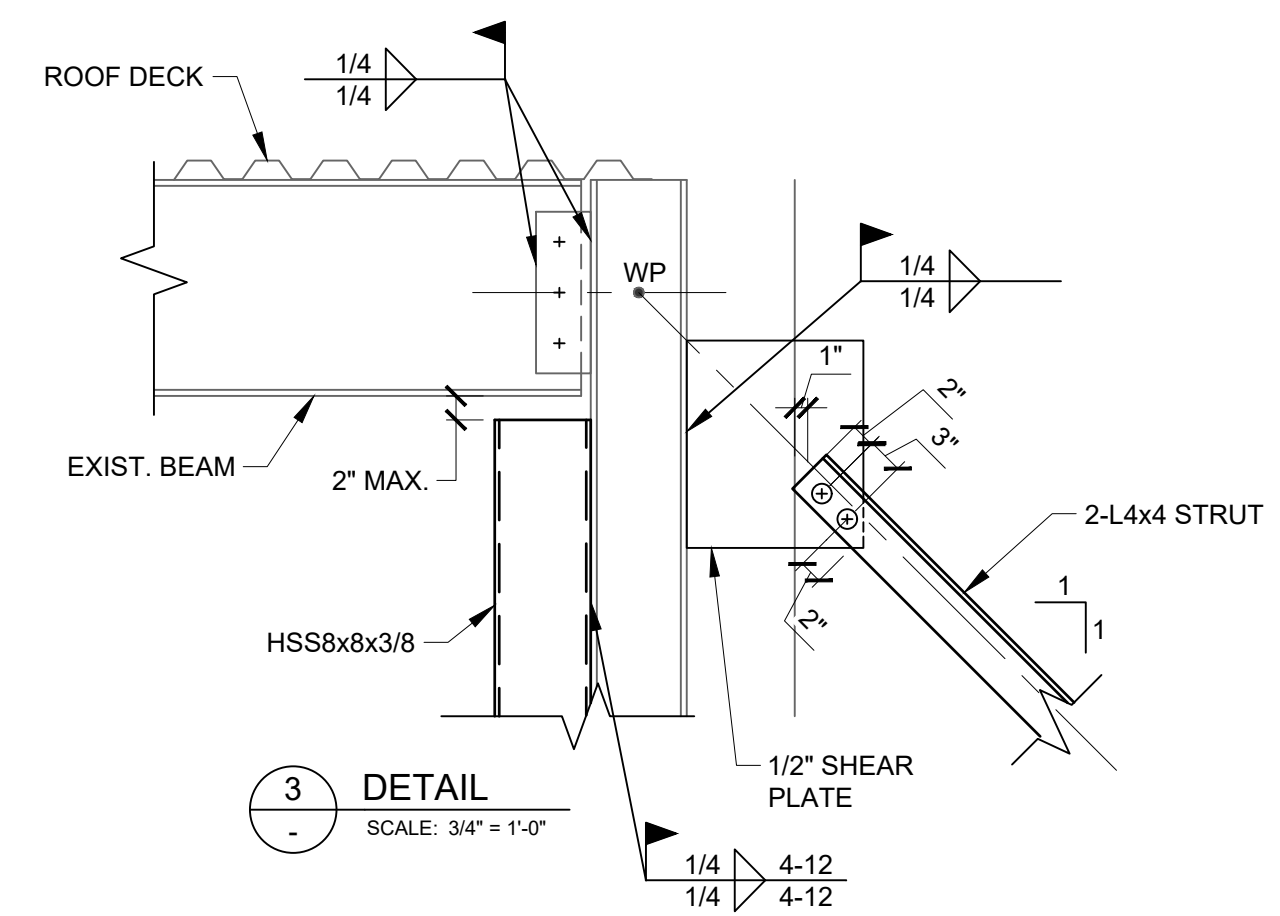
**10 DUNNIGAN DRIVE
ROOF FRAMING PLAN**

2 PART PLAN
SCALE: 1/8" = 1'-0"

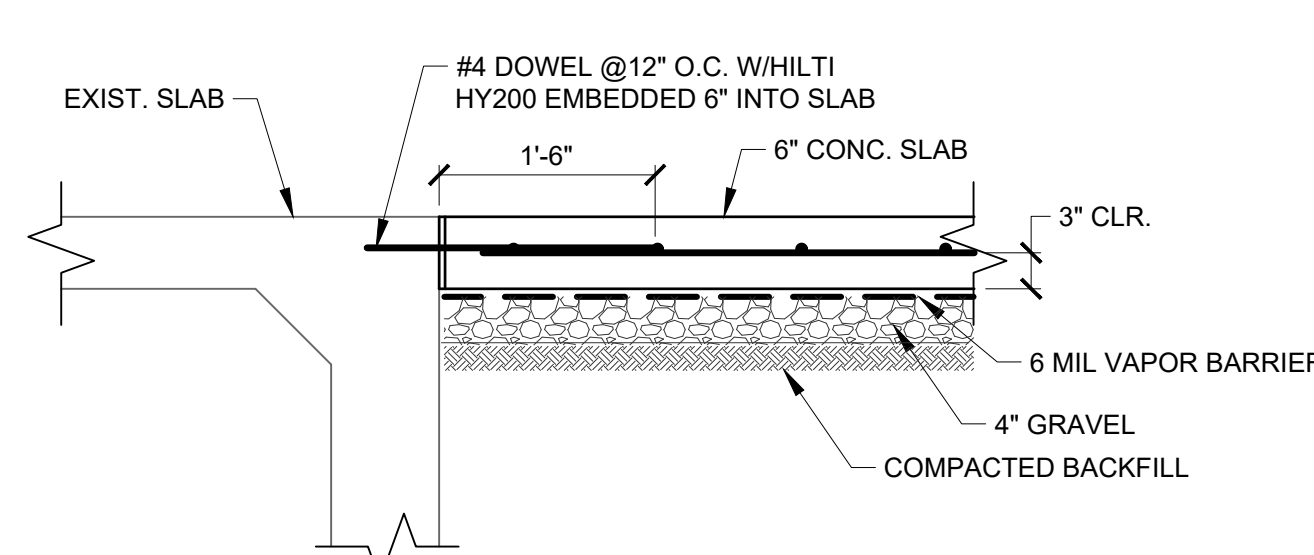
DWG NUMBER :
S-407
58 OF 60



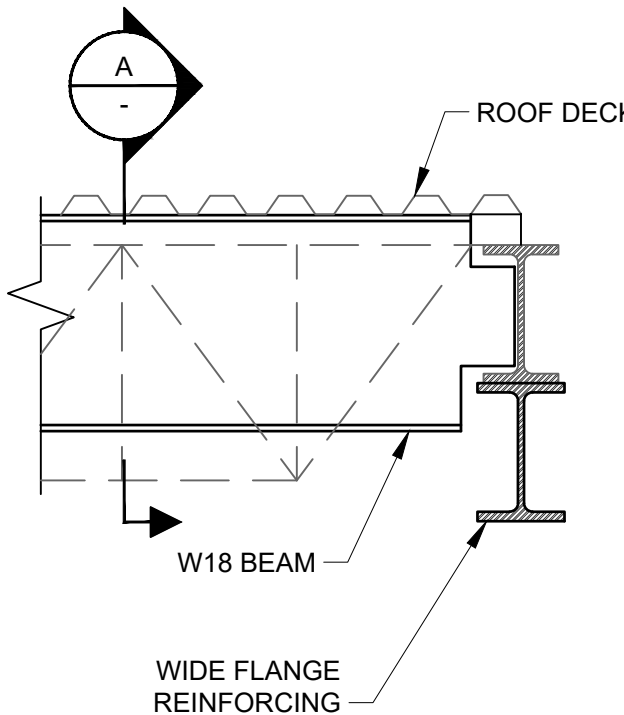
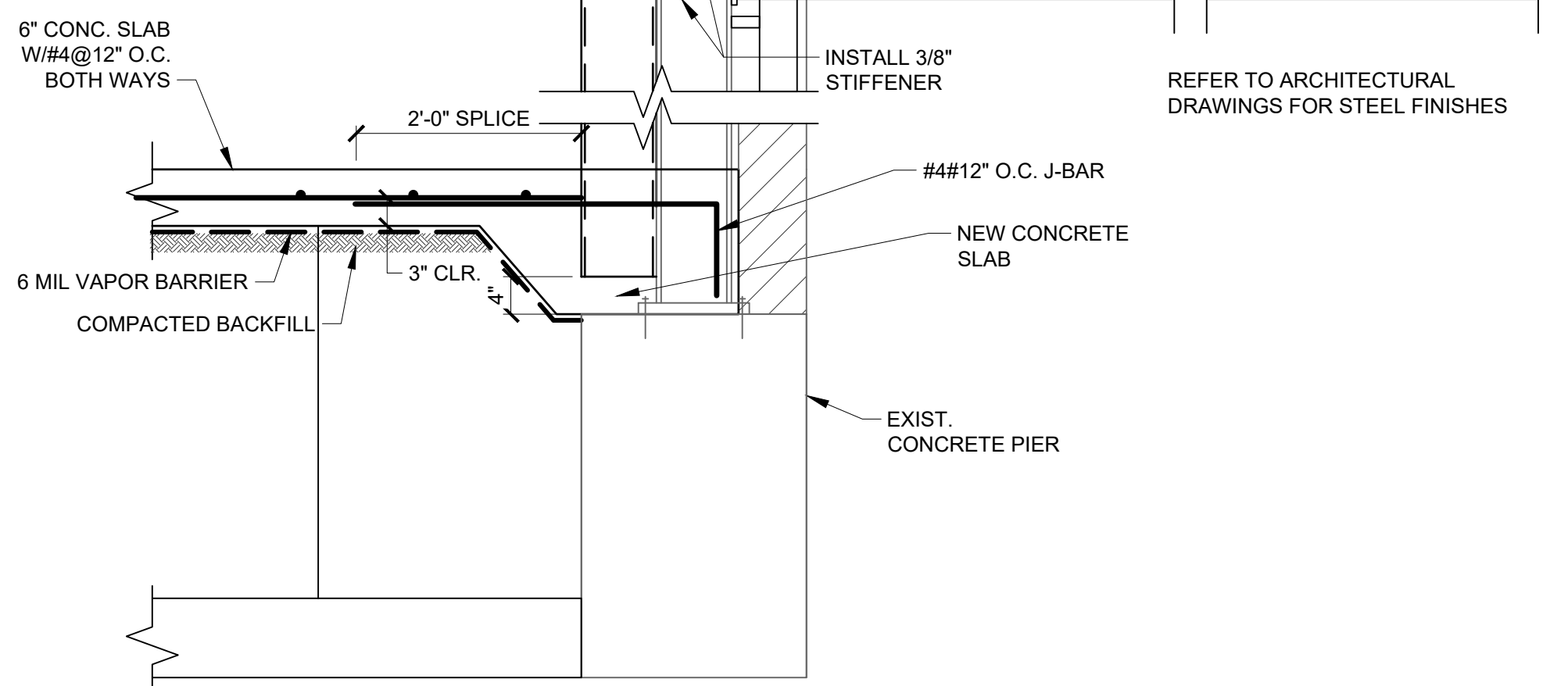
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-
DETAIL
SCALE: 3/4" = 1'-0"



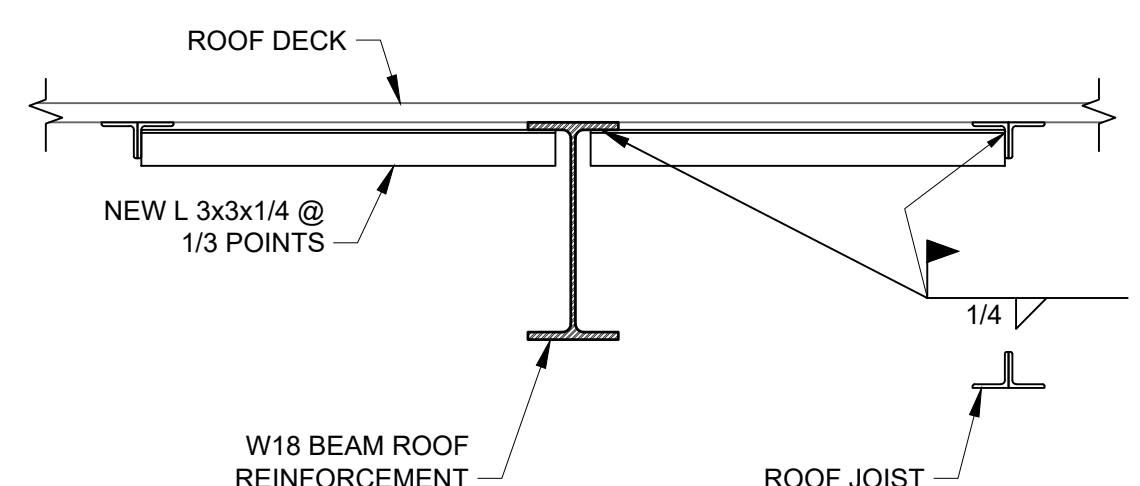
3
-
DETAIL
SCALE: 3/4" = 1'-0"



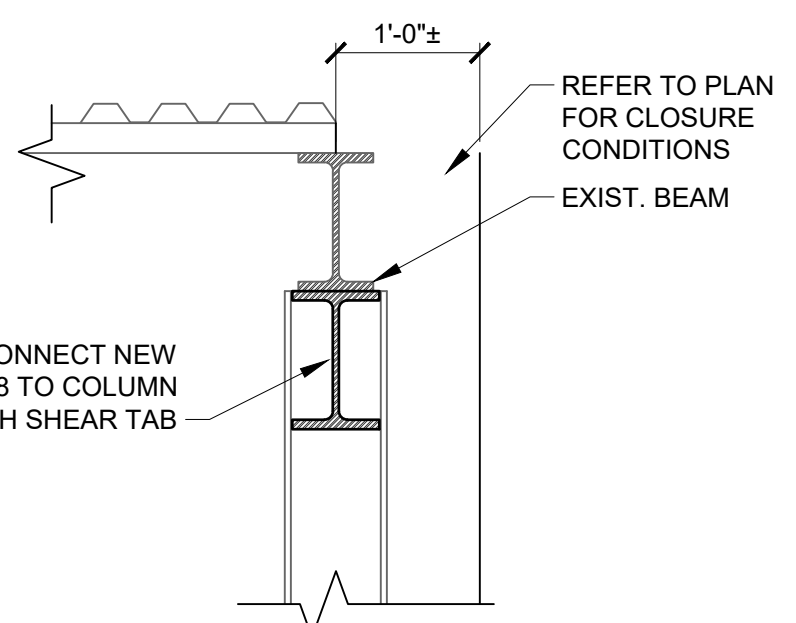
2
-
DETAIL
SCALE: 3/4" = 1'-0"



4
-
DETAIL
SCALE: 3/4" = 1'-0"



A
-
SECTION
SCALE: 3/4" = 1'-0"



5
-
DETAIL
SCALE: 3/4" = 1'-0"



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DRAWING TITLE :

20 DUNNIGAN DRIVE
DETAILS

DWG NUMBER :
S-408
59 OF 60



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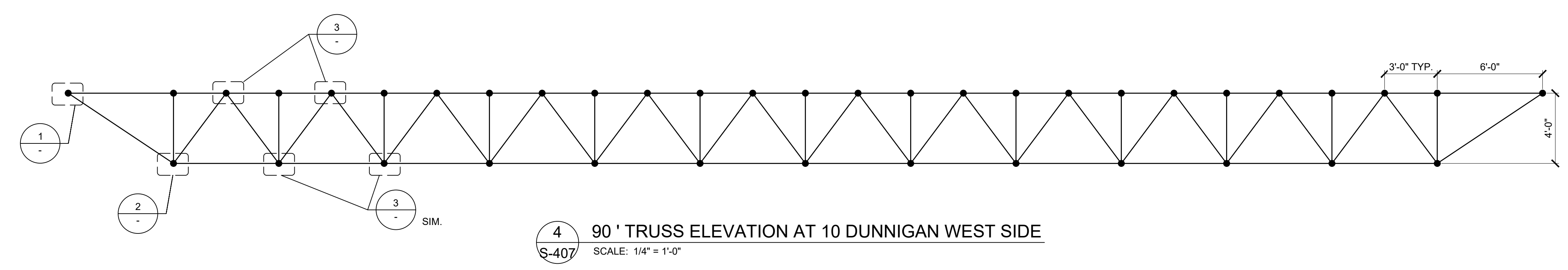
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DRAWN BY : D. CHECHURIN
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 SCALE : AS SHOWN

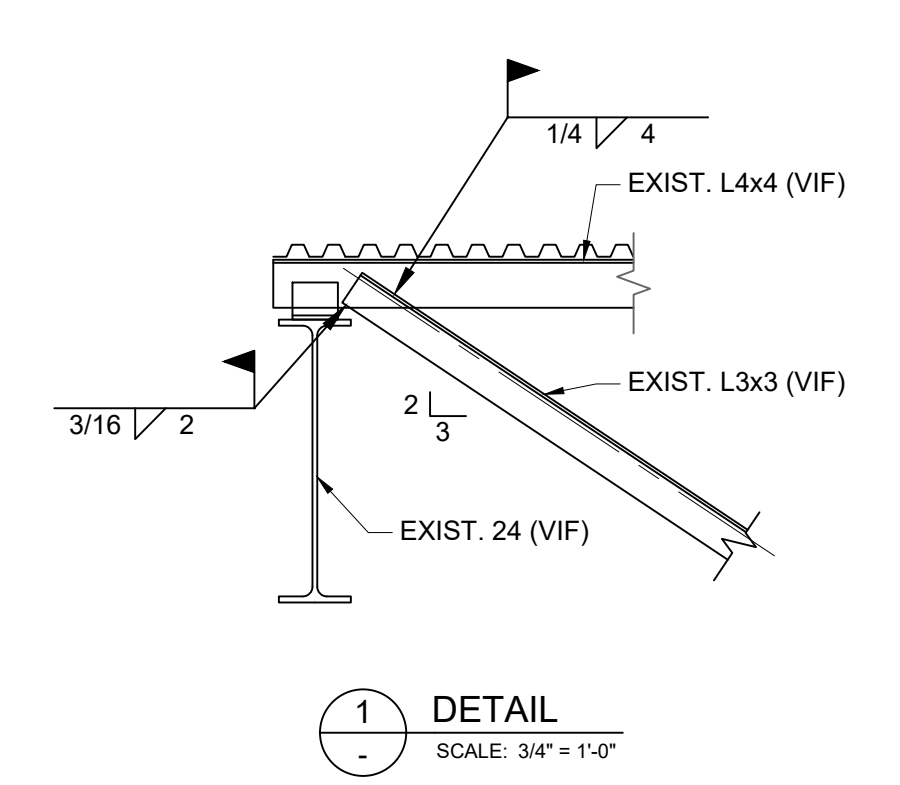
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20 DUNNIGAN DRIVE
 DETAILS

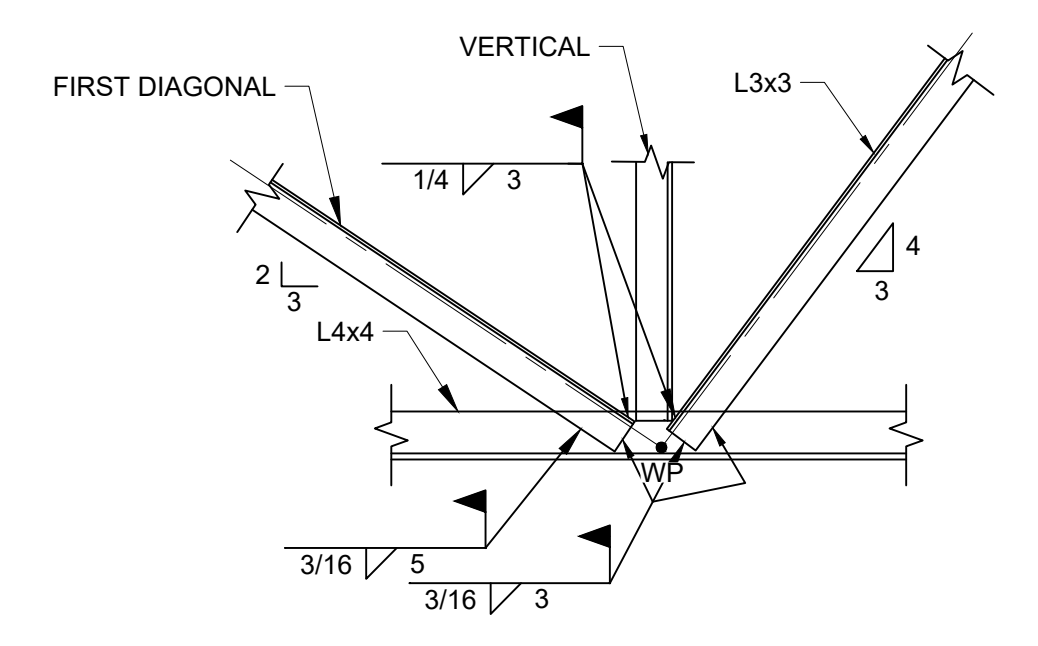
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S-409
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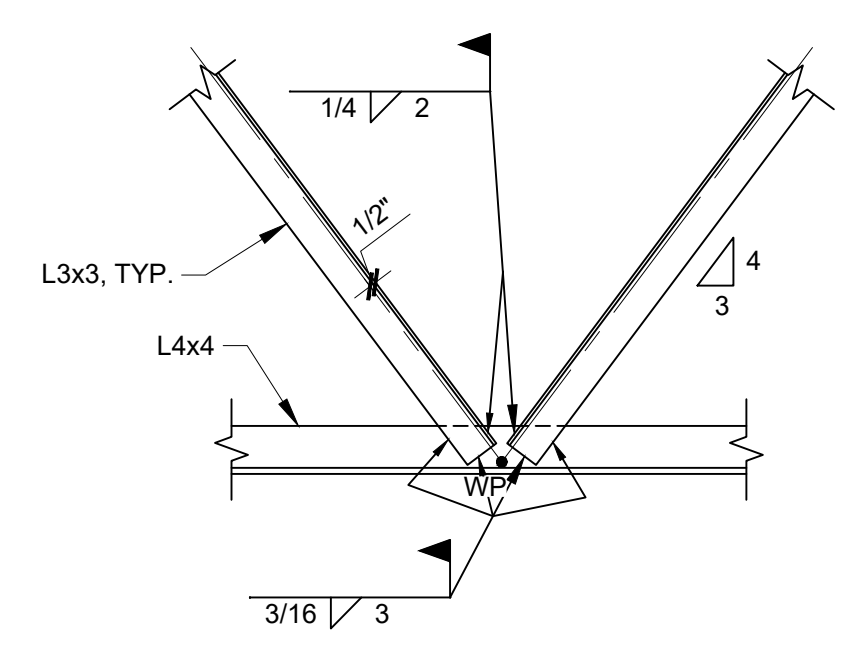
4 90' TRUSS ELEVATION AT 10 DUNNIGAN WEST SIDE
 S-407 SCALE: 1/4" = 1'-0"



1 DETAIL
 SCALE: 3/4" = 1'-0"



2 DETAIL
 SCALE: 3/4" = 1'-0"



3 TYP. CONNECTION AT INTERIOR PANEL POINT WHERE SHOWN ON ELEVATION
 SCALE: 3/4" = 1'-0"