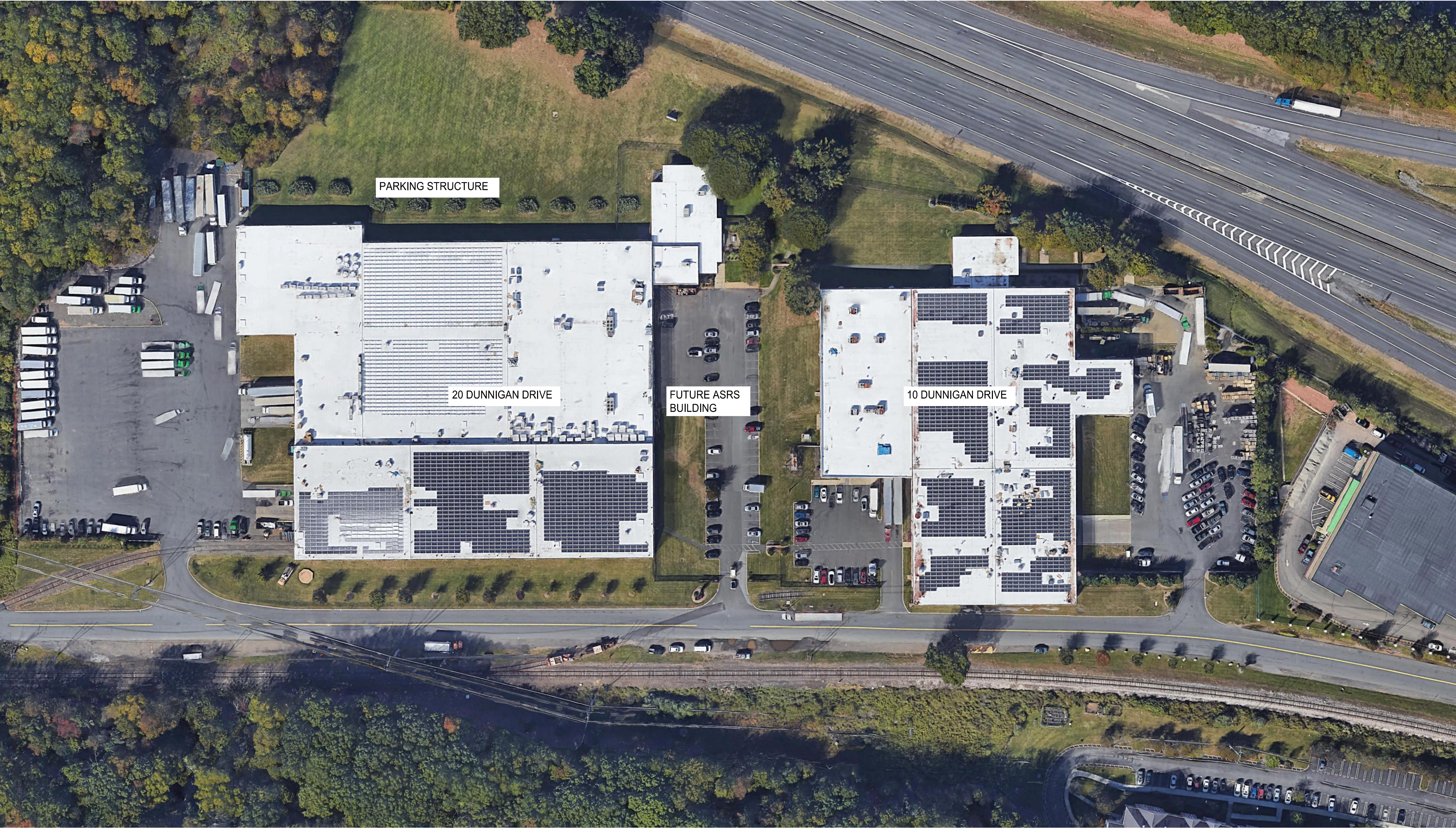


MANHATTAN BEER DISTRIBUTORS

20 DUNNIGAN DRIVE, SUFFERN, NY
STRUCTURAL REPAIRS



SITE LOCATION MAP - OVERALL LAYOUT
(NOT TO SCALE)



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GEI PROJECT No. 2101670

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0	ISSUED FOR DOB SUBMISSION	DATE
1	ISSUED FOR BID	10.15.21
2	CD SET	11.30.21
3	REVISED BID SET	01.20.22

DRAWN BY :	D. CHECHURIN
CHECKED BY :	P. BRUNO
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SCALE :	AS SHOWN

DRAWING TITLE :	
COVER SHEET	

DWG NUMBER :	S-000
1 OF 60	

GENERAL NOTES

1. DESIGN AND CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE FOLLOWING CODES:
- A. NEW YORK STATE BUILDING CODE.
 - B. AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318).
 - C. AMERICAN CONCRETE INSTITUTE SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI-301)
 - D. AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL AND CODE.
 - E. OCCUPATIONAL HEALTH CODE.
 - F. AMERICAN NATIONAL STANDARDS FOR CONSTRUCTION (ANSI).
 - G. USE LATEST EDITIONS INCLUDING ADDENDA, SUPPLEMENTS, AMENDMENTS, REVISIONS AND ERRATA U.N.O., EXCEPT AS MODIFIED BY THE CONTRACT DOCUMENTS.
 - H. THE AMERICAN WELDING SOCIETY CODE (A.W.S. D1.1).
 - I. STEEL DECK INSTITUTE (SDI)
 - J. STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS, LOAD AND WEIGHT TABLES (SJI)
 - K. AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
2. PLANS AND APPLICATION FORMS MUST BE FILED AND APPROVED BY THE LOCAL BUILDING DEPARTMENT BEFORE STARTING ANY WORK OR ORDERING ANY MATERIALS.
3. CONTRACTOR TO PROPERLY NOTIFY BUILDING DEPARTMENT PRIOR TO ANY WORK REQUIRING INSPECTION BY THE BUILDING DEPARTMENT AS CALLED FOR IN THE NEW YORK STATE BUILDING CODE.
4. A TESTING AND INSPECTION AGENCY ACCEPTABLE TO THE STRUCTURAL ENGINEER AND RETAINED BY THE OWNER SHALL PERFORM ALL REQUIRED TESTING AND INSPECTIONS IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE REGULATIONS (SEE SPECIAL INSPECTION NOTES).
5. THE CONTRACTOR IS RESPONSIBLE FOR ARRANGING WITH TESTING AGENCY WHEN INSPECTION SHALL BE PERFORMED. THE CONTRACTOR SHALL PROVIDE AMPLE TIME FOR THE TESTING AGENCY TO COMPLETE THEIR WORK.
6. TESTING AND INSPECTION OF SOILS, CONCRETE, REBARS, STEEL, WELDING AND H.S. BOLTING, ETC., SHALL BE IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE. (SEE SPECIAL INSPECTION NOTES)
7. CONTRACTOR SHALL PROVIDE, INSTALL, AND MAINTAIN ALL NECESSARY FALSEWORK, SHORING, BRACING AND ALL OTHER TEMPORARY CONSTRUCTION AND EQUIPMENT REQUIRED AND SHALL BE RESPONSIBLE FOR SITE SAFETY, STRUCTURAL STABILITY AND PROTECTION OF THE PUBLIC, STRUCTURES, AND PROPERTY INCLUDING PROTECTION FROM THE ELEMENTS DURING ANY PHASE OF THE WORK. SHORING DESIGN SHALL BE PERFORMED BY A NEW YORK STATE LICENSED P.E.
8. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELEVATOR DRAWINGS, AND SPECIFICATIONS. IN ADDITION, REFER TO THESE DRAWINGS FOR ADDITIONAL STRUCTURAL WORK NOT SHOWN ON STRUCTURAL DRAWINGS SUCH AS OPENINGS, INSERTS, SLEEVES, CURBS, ETC.
9. AS PART OF THE CONTRACTOR'S SCOPE OF WORK, THE CONTRACTOR SHALL REPLACE, REPAIR AND PATCH TO ITS ORIGINAL CONDITION ANY AREAS ALTERED OR DAMAGED DURING PROCESS OF WORK AT NO ADDITIONAL COST TO THE OWNER AND WITH ALL NECESSARY GUARANTEES AND WARRANTIES.
10. THE CONTRACTOR SHALL TAKE FIELD MEASUREMENTS, ELEVATIONS, OPENINGS AND VERIFY FIELD CONDITIONS AND SHALL CAREFULLY COMPARE SUCH FIELD MEASUREMENTS AND CONDITIONS AND OTHER INFORMATION KNOWN TO THE CONTRACTOR, WITH THE CONTRACT DOCUMENTS, BEFORE COMMENCING WORK AND PRIOR TO PREPARING SHOP DRAWINGS AND ORDERING MATERIALS AND DETAILS. ERRORS, DISCREPANCIES, INCONSISTENCIES, OR OMISSIONS DISCOVERED WHICH MAY INTERFERE WITH THE PROPER EXECUTION OF THE WORK, SHALL BE REPORTED TO THE ENGINEER IMMEDIATELY. DEVIATIONS FROM THESE PLANS MAY OCCUR DUE TO FIELD CONDITIONS.
11. THE CONTRACTOR SHALL FILE AND OBTAIN AND PAY FOR ALL REQUIRED PERMITS, FEES AND LICENSES BEFORE STARTING ANY WORK OR ORDERING ANY MATERIAL. DO NOT BEGIN CONSTRUCTION UNLESS AND UNTIL A WRITTEN PERMIT HAS BEEN ISSUED IN ACCORDANCE WITH THE REQUIREMENTS OF THE BUILDING CODE. SUBMIT ORIGINAL COPIES TO OWNER BEFORE PROCEEDING.
12. THESE DRAWINGS REPRESENT STRUCTURAL COMPONENTS IN THEIR FINAL AND FINISHED STATE. CONSTRUCTION PROCEDURE, METHODS, SAFETY PRECAUTIONS OR MECHANICAL REQUIREMENTS USED TO ERECT THEM ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR OR SUB-CONTRACTOR DOING THE WORK, AND SHOULD FOLLOW CHAPTER 33 OF THE NEW YORK STATE BUILDING CODE.
13. THE CONTRACTOR IS CAUTIONED TO MAKE CONTINUOUS OBSERVATIONS AND SAFEGUARDS OF THE EXISTING STRUCTURES DURING THE PERFORMANCE OF HIS WORK. SHOULD HE BECOME AWARE OF ANY EXISTING UNFORESEEN STRUCTURAL CONDITIONS OR SITUATIONS THAT REQUIRE FURTHER INVESTIGATION OR STUDY, (SUCH AS EXCESSIVE DETERIORATION, CRACKS IN MASONRY OR SLABS, MOVEMENT, ADDITIONAL DEFLECTION, ETC.), HE SHALL IMMEDIATELY NOTIFY THE ENGINEER/ARCHITECT OF RECORD.
14. SHOP DRAWINGS AND SAMPLE PRODUCTS ARE REQUIRED FOR ALL WORK AND SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY ARE STAMPED 'APPROVED' AND INITIALED BY THE CONTRACTOR ATTESTING THAT IT CONFORMS TO THE CONTRACT DOCUMENTS. SHOP DRAWINGS SHALL BE PREPARED, SIGNED AND SEALED BY A NEW YORK STATE LICENSED P.E. RETAINED BY THE CONTRACTOR.
15. THE CONTRACTOR SHALL RETAIN HIS OWN CONSULTANTS, INCLUDING STRUCTURAL CONSULTANTS, REGARDING CONTRACTORS DEMOLITION AND CONSTRUCTION PROCEDURES, SHORING AND BRACING PROTECTION, STRUCTURAL STABILITY REVIEWS DURING CONSTRUCTION AGAINST DAMAGE COLLAPSE, DISTORTION AND OFF-ALIGNMENT, ETC., AND ON SITE SAFETY RESPONSIBILITIES AND BUILDING DEPARTMENT REPORTING AND FILING REQUIREMENTS.
16. THE CONTRACTOR SHALL SAFEGUARD ALL ELECTRICAL AND MECHANICAL CONDUITS AND UTILITIES AND COORDINATE ALL EXISTING LINES, PIPES, DUCTS AND OTHER UTILITIES WHICH ARE TO BE RELOCATED AND REINSTALLED AS A RESULT OF THE NEW WORK.
19. EXISTING FOUNDATIONS, WALLS, COLUMNS, SLABS, BEAMS, FLOORS, (CONCRETE, STEEL, TIMBER, ETC.), ARE ASSUMED TO BE IN GOOD CONDITION. THIS MUST BE VERIFIED IN THE FIELD BY THE CONTRACTOR. CONTRACTOR SHALL NOTIFY ENGINEER-OF-RECORD OF ANY CONDITIONS REQUIRING REPAIRS PRIOR TO CONSTRUCTION.
20. THE ENGINEER OF RECORD SHALL NOT HAVE CONTROL OR CHARGE OF AND SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURE, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK FOR THE ACTS OR OMISSIONS OF THE CONTRACTORS, SUBCONTRACTOR, OR ANY OTHER PERSON PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

STRUCTURAL CONCRETE NOTES

1. MATERIALS SHALL CONFORM WITH THE FOLLOWING STANDARDS:
- A. PORTLAND CEMENT AS PER ASTM C-150, TYPE I / II.
 - B. CONCRETE AGGREGATES AS PER ASTM C-33 AND C-330.
 - C. WATER SHALL BE CLEAN AND FREE FROM INJURIOUS OILS, ACIDS, ALKALIS, SALT, ORGANIC MATERIALS AND DELETERIOUS MATERIALS.
 - D. REBARS SHALL BE DEFORMED BARS CONFORMING TO ASTM A-615, GRADE 60. TIES MAY BE A-615, GRADE 40. FOR BARS REQUIRING WELDING CONFORM TO ASTM A-706.
 - E. WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM 185 WITH ULTIMATE TENSILE STRENGTH OF 70 K.S.I. WIRE FABRIC SHALL CONFORM TO ASTM 475.
2. CONCRETE DESIGN STRENGTH: CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES FOR THE VARIOUS COMPONENTS, UNLESS NOTED OTHERWISE
- | | SPECIFIED MINIMUM
28-DAY COMPRESSIVE STRENGT
(f'c) (PSI) | ENTRAINED
AIR CONTENT | DENSITY
(PCF) | MAX.
W/C RATIO |
|---|--|--------------------------|------------------|-------------------|
| GRADE BEAMS,
FOUNDATION FOOTINGS & WALLS | 5000 | 6% | 145 | 0.45 |
- THE REQUIRED AVERAGE CONCRETE MIX DESIGN (F'CR) USED AS A BASIS OF SELECTION OF CONCRETE PROPORTIONS SHALL BE DETERMINED FROM A CURVE OF CONCRETE STRENGTH VERSUS WATER-CEMENT RATIO WHERE THE AVERAGE CONCRETE MIX DESIGN SHALL COMPLY WITH ACI 318.
3. CONCRETE SHALL BE PRODUCED FROM APPROVED BATCH PLANTS, BASED ON PRELIMINARY TEST DESIGN AND RESULTING INTO MIX STRENGTH SPECIFIED. PRODUCER SHALL CERTIFY CONFORMANCE OF QUALITY AND CONDITION OF MATERIALS TO ACI-318 AND THAT INGREDIENTS ARE THE SAME OR EQUAL TO THOSE USED FOR THE PRELIMINARY TESTS. ATTESTATION OF QUALITY INSPECTION AT THIS BATCH PLANT SHALL APPEAR ON THE TICKET ACCOMPANYING

EACH LOAD OF CONCRETE.

4. CONTRACTOR SHALL SUBMIT PRELIMINARY TESTS AND DESIGN MIX REPORTS IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE FOR REVIEW BY THE STRUCTURAL ENGINEER BEFORE COMMENCING WORK. NO CONCRETE SHALL BE PLACED UNTIL THE DESIGN MIXES WITH ACCOMPANYING COMPRESSIVE TEST RESULTS HAVE BEEN APPROVED BY THE ENGINEER OF RECORD.
5. COMPRESSION TEST SAMPLES SHALL BE TAKEN FROM THE MIXER IN ACCORDANCE WITH ASTM C172, CURED IN ACCORDANCE WITH ASTM C-31, AND TESTED AT 28-DAYS IN ACCORDANCE WITH ASTM C-39.
6. THE APPROVED INSPECTION AGENCY WILL CONDUCT ALL NECESSARY TESTS AT THE LABORATORY, AND WILL HAVE ONE QUALIFIED CONCRETE TECHNICIAN TO BE STATIONED AT THE MIXING PLANT AND A MINIMUM OF ONE QUALIFIED CONCRETE TECHNICIAN AT THE JOB SITE. THE TECHNICIAN AT THE JOB SITE WILL PERFORM THE TESTS AND PREPARE THE SPECIMENS AS REQUIRED UNDER THE CODE.
- THE TESTING AGENCY SHALL PROVIDE ACI CERTIFIED TECHNICIANS.
7. ALL CONCRETE USED IN THE STRUCTURE SHALL CONFORM IN ALL RESPECTS TO THE MATERIAL AND PROPORTIONS OF THESE MATERIALS IDENTIFIED IN THE APPROVED DESIGN MIX. THE USE OF ANY ADDITIVES NOT PRESENT IN THE APPROVED DESIGN MIX IS PROHIBITED.
8. WHEN CHANGES TO BRAND, TYPE, SIZE OR SOURCE OF CEMENTITIOUS MATERIALS, AGGREGATES, WATER, ICE OR ADMIXTURES ARE PROPOSED, SUBMIT NEW FIELD DATA, DATE FROM NEW TRIAL MIXTURES, OR OTHER EVIDENCE THAT THE CHANGE WILL NOT ADVERSELY AFFECT THE RELEVANT PROPERTIES OF THE CONCRETE. DATA SHALL BE SUBMITTED FOR ACCEPTANCE BEFORE CHANGES ARE MADE.
9. CONCRETE TEST CYLINDERS AND SLUMP. INSPECTION OF REINFORCING BARS AND PLACING OF CONCRETE TO BE TAKEN AND TESTED IN ACCORDANCE WITH ACI-318 AND AS REQUIRED BY THE NEW YORK STATE BUILDING CODE. (SEE SPECIAL INSPECTION NOTE)
10. CONTRACTOR TO PROVIDE A LOCATION WHICH IS LEVEL, SAFE AND FOR THE STORING OF CURING BOXES IN ORDER FOR THE TESTING LABORATORY TO PREPARE TEST CYLINDERS.
11. THE TEST CYLINDERS IMMEDIATELY AFTER MOLDING AND FINISHING, SHALL BE STORED IN CURING BOXES (SUPPLIED BY THE TESTING AGENCY) ON-SITE FROM 24-HOURS TO 48-HOURS IN A TEMPERATURE RANGE FROM 60F TO 80F IN AN ENVIRONMENT PREVENTING MOISTURE LOSS FROM THE SPECIMENS BEFORE BEING TRANSPORTED TO THE TESTING LABORATORY.
12. NO CHLORIDES SHALL BE INTRODUCED INTO THE CONCRETE OR GROUT, ETC., UNDER ANY CIRCUMSTANCES.
13. ALL CONTINUOUS REINFORCING BARS SHALL BE EMBEDDED AT ENDS OF MEMBERS AND LAPPED AT SPLICES AND CORNERS AS PER THE CRSI LATEST EDITION. SPLICES FOR BOTTOM STEEL TO BE LOCATED AT SUPPORTS; SPLICES FOR TOP STEEL TO BE LOCATED AT MID-SPAN. NO SPLICES ARE PERMITTED IN TENSION ZONE, CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT. ALL DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS SHALL COMPLY WITH THE REQUIREMENTS OF ACI 315 AND ACI 318-02, CHAPTERS 7 AND 12, AND THE NEW YORK STATE BUILDING CODE.
- A. ALL SPLICES FOR SLABS AND BEAMS SHALL BE TENSION SPLICES, U.O.N.
 - B. COMPRESSION SPLICES SHALL BE USED IN PIERS AND COLUMNS, U.O.N.
 - C. EMBEDMENT FOR DOWELS, ETC. SHALL BE TENSION DEVELOPED, U.O.N.
14. MECHANICAL SPLICES MAY BE USED. WHERE THESE SPLICES ARE USED, SPLICES SHOULD BE STAGGERED AND SHALL DEVELOP 133 PERCENT OF THE SPECIFIED YIELD STRENGTH IN TENSION OR COMPRESSION.
15. MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE
- | | |
|--|-----------|
| UNFORMED SURFACE IN CONTACT WITH THE GROUND | 3 IN. |
| FORMED SURFACES EXPOSED TO EARTH | |
| #6 BARS AND LARGER | 2 IN. |
| #5 BARS AND SMALLER | 1-1/2 IN. |
| FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER | |
| BEAMS, GIRDERS, AND COLUMNS | 1-1/2 IN. |
| SLABS, WALLS, AND JOISTS: | |
| #11 BARS AND SMALLER | 3/4 IN. |
| #14 BARS #18 BARS | 1-1/2 IN. |
| SURVEY REBAR ELEVATIONS BEFORE PLACING CONCRETE. | |
16. ALL BAR SIZES DESIGNATED ARE BASED ON NUMBER OF EIGHTHS OF AN INCH NOMINAL DIAMETER, NOT METRIC UNITS.
17. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL THE NECESSARY CHAIRS, REBARS, TIES, SPACERS, ETC., TO SECURE AND SUPPORT THE REINFORCING. PROPERLY INSTALLED SPACERS SHALL BE UTILIZED TO PROPERLY MAINTAIN APPROPRIATE CONCRETE COVER WHILE PLACING CONCRETE. LIFTING OF BARS OR MESH DURING PLACEMENT OF CONCRETE IS NOT PERMITTED.
- NOTE: PROVIDE PLASTIC TIPPED BOLSTERS AND CHAIRS AT THOSE LOCATIONS WHERE THE CONCRETE SURFACE IN CONTACT WITH THE BOLSTERS OR CHAIRS ARE EXPOSED.
18. THE CONTRACTOR SHALL SUBMIT CHECKED SHOP DRAWINGS CONSISTING OF COMPLETE PLANS AND DETAILS OF REINFORCEMENT, LOCATIONS OF POUR LINES, CONSTRUCTION JOINTS, ETC., FOR APPROVAL BEFORE PROCEEDING WITH THE WORK. SUCH DRAWINGS SHALL BE OF SUFFICIENT DETAIL TO PERMIT PLACING OF REINFORCEMENT WITHOUT REFERENCE TO THE STRUCTURAL DRAWINGS.
19. NO COLD WEATHER CONSTRUCTION OR HOT WEATHER CONSTRUCTION IS PERMITTED WITHOUT WRITTEN APPROVAL OF THE ARCHITECT/ENGINEER. SUBMIT PROCEDURES FOR REVIEW.
20. LOCATION OF ALL CONSTRUCTION AND CONTRACTION JOINTS, NOT SHOWN IN DRAWINGS, SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL PRIOR TO DETAILING OF REINFORCEMENT. ALL CONSTRUCTION JOINTS TO BE CLEARLY INDICATED ON REBAR DETAIL DRAWING. MAXIMUM CONTRACTION JOINT SPACING = 40"o" O.C.
21. ALL STRUCTURAL MEMBERS SHALL BE POURED FOR THEIR FULL DEPTHS IN ONE OPERATION. CONSTRUCTION JOINTS SUCH AS DAYS END POUR SHALL BE LOCATED IN THE MIDDLE THIRD OF THE SPAN. MAIN REINFORCING TO RUN THROUGH THE JOINT. KEY AND ROUGHEN JOINTS TO EXPOSE AGGREGATE FOR CHEMICAL BOND. NO HORIZONTAL CONSTRUCTION JOINTS PERMITTED IN BEAMS OR WALLS WITHOUT THE EXPRESS CONSENT OF ENGINEER-OF-RECORD. NO CONSTRUCTION JOINTS ARE PERMITTED IN BEAMS CARRYING COLUMN LOADS OR OTHER CONCENTRATED LOADS. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS. ALL REINFORCING IS TO BE CONTINUOUS THROUGH JOINT.
- ROUGHENED SURFACE AT INTERFACE OF SEPARATE CONCRETE POURS (JOINTS) SHALL BE PREPARED AS FOLLOWS:
- A)ROUGHEN SURFACE TO A FULL AMPLITUDE OF APPROXIMATELY 1/4" WITH STIFF BROOM AFTER INITIAL SET.
B)BEFORE PLACING FRESH CONCRETE, CLEAN SURFACE AND REMOVE LAITANCE WITH WIRE BRUSH.
C)IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, WET SURFACE AND REMOVE STANDING WATER.
22. ALL OPENINGS IN WALLS, UNLESS OTHERWISE NOTED, SHALL HAVE TWO #6 BARS ON SIDES AND SHALL EXTEND 2'-0" BEYOND EDGES OF OPENINGS, U.O.N.
23. ANCHOR BOLTS TO BE ASTM F1554 (WELDABLE) FY=55KSI UNLESS OTHERWISE NOTED.
24. NO PIPES OR CONDUITS EXCEEDING 1/3 SLAB THICKNESS IN OUTSIDE DIAMETER SHALL BE EMBEDDED IN THE STRUCTURAL CONCRETE FLOOR OR ROOF SLAB. WHERE CONDUITS OR PIPES ARE PERMITTED, THEY SHALL BE PLACED NOT CLOSER THAN 3 DIAMETER O.C. AND SHALL BE LOCATED SO AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE. ALUMINUM CONDUITS ARE NOT PERMITTED.
25. PROVIDE 4-#6 DOWELS X 3'-0" LONG BETWEEN ALL PIERS, FOOTINGS, GRADE BEAMS, ETC., U.N.
26. CONCRETE DIMENSIONAL TOLERANCE SHALL CONFORM TO ACI 117.
27. ALL SLAB-ON-GROUND SHALL HAVE THICKENING, DEPRESSIONS, OPENINGS, ETC., AS REQUIRED OR AS SHOWN HEREIN, OR ON ARCHITECTURAL, ELECTRICAL OR MECHANICAL DRAWINGS.
28. THE METHOD FOR CONVEYING CONCRETE TO THE PLACE OF DEPOSIT SHALL COMPLY WITH THE NEW YORK STATE BUILDING CODE. CONCRETE MAY BE CONVEYED BY PUMPING. PUMPING METHODS SHALL COMPLY WITH DEPARTMENT OF BUILDING REGULATIONS. ALUMINUM OR ALUMINUM ALLOY PIPE SHALL NOT BE USED.
29. THE MEANS AND METHODS OF CONCRETE PLACEMENT, INCLUDING POUR HEIGHTS, TEMPERATURE CONTROL, LATERAL PRESSURE AND UPLIFT PRESSURE CONTROL, FORMWORK DESIGN, AND OTHER REQUIRED CONSTRUCTION METHODS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR DOING THE WORK. THE CONTRACTOR SHALL EMPLOY ALL NECESSARY PRECAUTIONS IN ORDER TO AVOID ANY MOVEMENT OR

DISLOCATION OF ALL M/E SYSTEMS, PIPING PENETRATIONS, ETC., INCLUDING THE CONCRETE PLACEMENT. ALL M/E EQUIPMENT SHALL BE INSTALLED AS INDICATED ON THE M/E DRAWINGS.

30. SEE ARCHITECTURAL DRAWINGS FOR FINISH OF ALL EXPOSED CONCRETE.

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0	ISSUED FOR BID	10.15.21
1	CD SET	11.30.21
2	REVISED BID SET	01.20.22
3		

DRAWN BY : D.CHECHURIN

CHECKED BY : P. BRUNO

APPROVED BY : P. BRUNO

DATE : 9/10/2021

SCALE : AS SHOWN

DRAWING TITLE :

NOTES

DWG NUMBER :

S-001

2 OF 60

LOADING SCHEDULE

SUPERIMPOSED DEAD LOADING: SELF WEIGHT OF CONCRETE
SLAB AND METAL DECK IS NOT INCLUDED IN THE FOLLOWING VALUES.

- SNOW LOADING:
- GROUND SNOW LOAD PG= 30 PSF
 - SNOW EXPOSURE FACTOR CE= .9
 - IMPORTANCE FACTOR IS= 1
 - THERMAL FACTOR CT= 1
 - FLAT ROOF SNOW LOAD PF= 20 PSF
 - SLOPE ROOF FACTOR CS= 1.0
 - SLOPE ROOF SNOW LOAD PS= 20 PSF
 - DRIFT
 - SNOW DENSITY = 17.9 PCF
 - DRIFT HEIGHT HD = 3.52 FT
 - MAXIMUM DRIFT LOAD = 83 PSF
 - DRIFT LOAD AT END = 20 PSF
 - DRIFT WIDTH W= 14 FT
 - FROST DEPTH = 4'-0" BELOW GRADE

- WIND LOADING:
- BASIC WIND VELOCITY V= 120 MPH
 - EXPOSURE FACTOR B
 - IMPORTANCE FACTOR IW= 1
 - ASCE 7-10 SIMPLIFIED PROCEDURE WIND LOADS
 - BUILDING OCCUPANCY II
 - a = 4.0
 - TERRAIN FACTOR KZT= 1.0
 - ROOF MEAN HEIGHT HR = 50 FT

- WIND FORCE MWFRS HORIZONTAL PRESSURE.
 - ZONE = 28 PSF

- WIND FORCE MWFRS VERTICAL PRESSURE.
 - ZONE = -18 PSF

- TOTAL BASE SHEAR PARKING BUILDING.
 - N-S = 207 KIPS
 - E-W = 65 KIPS

- TOTAL BASE SHEAR ASRS BUILDING.
 - N-S = 215 KIPS
 - E-W = 260 KIPS

- COMPONENTS AND CLADDING
- DISTANCE a= 21.0 FT
 - EFFECTIVE WIND AREA OF C&C = 50 SF.
 - WIND FORCE C&C HORIZONTAL PRESSURE.
 - WALL INTERIOR=31 PSF
 - WALL CORNERS=37 PSF

- WIND FORCE C&C VERTICAL PRESSURE.
 - ROOF EDGES = -43 PSF
 - ROOF CORNERS = -56 PSF

- SEISMIC LOADING PARKING BUILDING:
- SEISMIC LOADING CALCULATED AS PER ASCE 7-10.
 - SEISMIC OCCUPANCY II
 - SS= 0.26g
 - S1= 0.07g
 - SPECTRAL RESPONSE COEFFICIENT SDS= 0.27 g
 - SPECTRAL RESPONSE COEFFICIENT SD1= 0.114 g
 - SITE CLASS D
 - SEISMIC DESIGN CATEGORY B
 - BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL ORDINARY REINFORCED CONCRETE SHEAR WALLS
 - RESPONSE MODIFICATION COEFFICIENT 'R' = 5
 - SYSTEM OVER STRENGTH FACTOR 'Wo' = 2.50
 - DEFLECTION AMPLIFICATION FACTOR 'Cd' = 4.5
 - CS= 0.055%g
 - TOTAL BASE SHEAR = 629 KIPS

- SEISMIC LOADING ASRS BUILDING:
- BASIC SEISMIC RESISTING FROM SYSTEM: ORDINARY CONCENTRIC BRACED FRAMES
 - R = 3.25
 - = 2.0
 - Cc = 3.25
 - CS = 0.084g
 - TOTAL BASE SHEAR = 284 KIPS

NEW YORK STATE SPECIAL INSPECTION NOTES

THE DESIGNATED INSPECTING AGENCY FOR SPECIAL INSPECTION APPROVED BY THE CODE ENFORCEMENT OFFICIAL AND ACCEPTABLE TO THE STRUCTURAL ENGINEER RETAINED BY THE OWNER/CONTRACTOR SHALL PERFORM ON SITE INSPECTION AND TESTING IN ACCORDANCE WITH NEW YORK STATE BUILDING CODE REGULATIONS UNDER THE SUPERVISION OF A NEW YORK STATE LICENSED PROFESSIONAL. SPECIAL INSPECTION ITEMS INCLUDE THE FOLLOWING LISTED BELOW.

THE SPECIAL INSPECTION ENGINEERS ARE RESPONSIBLE FOR FILING AND OBTAINING APPROVAL OF ALL STATEMENTS, TEST AND INSPECTION REPORTS, INCLUDING STEEL, CONCRETE, MASONRY PRODUCER'S CERTIFICATES. COPIES TO BE SENT TO ENGINEER OF RECORD.

ALL TESTING AGENCY REPORTS SHALL BE SIGNED AND SEALED BY A NEW YORK STATE LICENSED PROFESSIONAL ENGINEER. TECHNICAL REPORTS SHALL BE FILED WITH THE BUILDING DEPARTMENT FOR APPROVAL OF SAID REPORTS.

CONTRACTOR TO NOTIFY THE SPECIAL INSPECTION ENGINEERS AT LEAST 72 HOURS BEFORE THE SPECIFIC WORK COMMENCES. THE CONTRACTOR SHALL PROVIDE AMPLE TIME FOR THE TESTING AGENCY TO COMPLETE THEIR WORK.

CONTRACTOR SHALL NOT CONCEAL ANY WORK REQUIRING SPECIAL INSPECTION UNTIL SUCH WORK IS INSPECTED OR TESTED AND DEEMED TO MEET PROJECT CRITERIA AND CODE REQUIREMENTS.

SPECIAL INSPECTION	CODE SECTION	EMPLOYED BY
CONCRETE - CAST IN PLACE	TABLE 1704.4	OWNER
CONCRETE DESIGN MIX	1904 & 1905	CONTRACTOR
CONCRETE TEST CYLINDERS *	1905.6	OWNER
STRUCTURAL STEEL - ERECTION & BOLTING	1704.3.3 & TABLE 1704.3	OWNER
STRUCTURAL STEEL - WELDING	1704.3.1	OWNER
STRUCTURAL SAFETY - STRUCTURAL STABILITY	1704.19	CONTRACTOR
ADHESIVE ANCHOR AS REQ'D BY MANUFACTURER	N/A	OWNER

SEISMIC SPECIAL INSPECTION

INSPECTION AND TESTING	REFERENCE STANDARD	BCNYS STANDARD
A. SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE		
STRUCTURAL STEEL	AISC 341	1707.2
MECHANICAL AND ELECTRICAL COMPONENTS		1707.8
B. STRUCTURAL TESTING FOR SEISMIC RESISTANCE		
TESTING FOR SEISMIC RESISTANCE		1708.2
STRUCTURAL STEEL	AISC 341, AWS D1.1	1708.4
SEISMIC QUALIFICATION OF MECHANICAL AND ELECTRICAL EQUIPMENT		1708.5

LOADING SCHEDULE									
SUPERIMPOSED DEAD LOADING (PSF)	PARKING AREA	ADMIN BUILDING OFFICE	ADMIN BUILDING CORRIDOR UPPER FLOORS	ADMIN BUILDING TERRACE	ASRS ROOF	OP'S BUILDING OFFICE	OP'S BUILDING CORRIDOR UPPER FLOORS	OP'S BUILDING FIRST FLOOR AND LOBBIES	OP'S BUILDING ROOF
CEILING	1	2	2	2		2	2	2	2
FIREPROOFING	1	2	2	2		2	2	2	2
MEP	1	2	2	2	2	2	2	2	2
WATERPROOFING	5			5	6				
INSULATION	6			3	6				
DRAINAGE MAT	4			4					
SLAB TOPPING 4"	50								
FLOORING		3	9	25		3	3	3	3
	68	9	15	43	14	9	9	9	9
LIVE LOADING (PSF)									
LIVE LOADING	100	50	80	100	20	50	80	100	50
PARTITION		15				15			15

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0	ISSUED FOR DOB SUBMISSION	DATE
1	ISSUED FOR BID	10.15.21
2	CD SET	11.30.21
3	REVISED BID SET	01.20.22

DRAWN BY : D.CHECHURIN

CHECKED BY : P. BRUNO

APPROVED BY : P. BRUNO

DATE : 9/10/2021

SCALE : AS SHOWN

DRAWING TITLE :

NOTES

DWG NUMBER :

S-002

STRUCTURAL STEEL NOTES

- DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE LATEST AISC SPECIFICATION PERMITTED BY THE NEW YORK STATE BUILDING CODE. ALL STEEL SHALL CONFORM TO:

 - STRUCTURAL STEEL SHAPES: ASTM A 992 STEEL (GR50) FY = 50 KSI
 - STEEL PIPE COLUMNS: ASTM A501 FY = 36 KSI
 - STRUCTURAL TUBES: ASTM A500 GRADE B FY = 46 KS
(SEE NOTE 24 FOR GALVANIZING)
 - STEEL ANGLES, PLATES & CHANNELS: ASTM A36 FY = 36 KSI
 - THREADED RODS: STAINLESS STEEL AISI TYPE 316. FY = 64 KSI

A.I.S.C. SPECIFICATIONS FOR DESIGN FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS SHALL APPLY, OR ANY MORE RESTRICTIVE REQUIREMENTS OF THE NEW YORK STATE BUILDING CODE.

 - CONTRACTOR SHALL PROVIDE CERTIFIED MILL REPORTS AND FABRICATOR SHALL FILE AN AFFIDAVIT OF THE PRODUCER OF THE STEEL CERTIFYING THAT THE PROVISIONS OF THE BUILDING CODE ARE MET. SUBMIT ABOVE FOR REVIEW TO STRUCTURAL ENGINEER PRIOR TO FABRICATION AND ORDERING OF MATERIALS.
 - ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE, EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS. (LATEST EDITION).
 - THE CONTRACTOR SHALL COORDINATE ALL DIMENSIONS SHOWN ON THE PLANS WITH THE VARIOUS TRADES BEFORE DETAILING AND FABRICATING STEEL. REFER TO MECHANICAL AND ELECTRICAL DRAWINGS FOR ADDITIONAL STRUCTURAL WORK NOT SHOWN ON THE STRUCTURAL DRAWINGS.
 - ALL DETAILING FABRICATION AND ERECTION SHALL CONFORM TO AISC "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATIONS FOR STRUCTURAL STEEL BUILDING" OR "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN," AND AISC "CODE OF STANDARD PRACTICE," LATEST EDITION.
 - PRIOR TO SUBMITTING ANY SHOP DRAWINGS, THE CONTRACTOR SHALL SUBMIT A SHOP DRAWING SCHEDULE TO THE ENGINEER-OF-RECORD FOR APPROVAL. THE SCHEDULE SHOULD INCLUDE THE DATES, WHEN THE DRAWINGS ARE TO BE SUBMITTED, THE NUMBER OF DRAWINGS AND TYPE OF DETAILS (ERECTION PLANS, SCHEDULES, BEAMS, COLUMNS, ETC.).
 - THE FABRICATOR SHALL SUBMIT TO THE ENGINEER, FOR REVIEW AND APPROVAL, ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS. ERECTION PLANS, JOB STANDARDS, SCHEDULES, ETC. PRIOR TO THE START OF PIECE DETAILING.
 - THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS. CONNECTIONS SHOWN ON THE STRUCTURAL DRAWING ARE SCHEMATIC AND ARE ONLY INTENDED TO SHOW THE RELATIONSHIP OF MEMBERS CONNECTED. CONNECTION DETAILS INDICATED ON THE DRAWINGS SHALL BE INCORPORATED INTO FABRICATOR'S CONNECTION DESIGN. SEE SPECIFICATIONS. CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE STRUCTURE IS LOCATED. ENGINEER'S SEAL MAY BE QUALIFIED "FOR DESIGN OF CONNECTIONS ONLY."

ENGINEER-OF-RECORD'S REVIEW OF THE SHOP DRAWINGS IS ONLY FOR THE LIMITED PURPOSE OF CHECKING FOR CONFORMANCE WITH THE DESIGN INTENT AND INFORMATION EXPRESSED IN THE CONTRACT DRAWINGS.

 - ALL COMPOSITE BEAM CONNECTIONS TO DEVELOP MINIMUM ONE-HALF OF THE TOTAL LOAD CAPACITY TABULATED IN THE AISC STEEL HANDBOOK (LATEST EDITION) TIMES 1.5 FOR THE GIVEN SHAPE AND SPAN OF THE BEAM OR THE REACTION SHOWN ON THE DRAWINGS, OR THE SCHEDULE SHOWN HEREIN, WHICHEVER IS GREATER.

ALL BEAM CONNECTIONS TO DEVELOP MINIMUM ONE-HALF OF THE TOTAL LOAD CAPACITY TABULATED IN THE AISC STEEL HANDBOOK (LATEST EDITION) FOR THE GIVEN SHAPE AND SPAN OF THE BEAM, OR THE REACTION SHOWN ON THE DRAWING, OR THE SCHEDULE SHOWN BELOW, WHICHEVER IS GREATER.

 - FOR FRAMED BEAM CONNECTIONS, INCLUDING SINGLE PLATE CONNECTIONS, PROVIDE THE MINIMUM NUMBER OF HORIZONTAL BOLT ROWS AS SHOWN BELOW, BASED ON 3" C/C UNLESS NOTED OTHERWISE.

SHAPE	REACTION (K)	MINIMUM NUMBER OF FASTENERS
W8, C8	20	2
W10, C10	28	2
W12, C12	35	3
W14	35	3
W16	50	4
W18	85	4
W21	85	5
W24	100	6
W27	100	6
W30	120	7
W33	160	8
W36	220	9
W40	240	11
W44	260	11

PROVIDE REINFORCING AT CONNECTIONS WHERE CUTS OR COPES REDUCE THE SHEAR OR MOMENT CAPACITY BELOW THAT REQUIRED TO SUSTAIN THE REACTION. REFER TO PLAN FOR REACTIONS LARGER THAN SHOWN HERE.

 - DETAILS OF CONNECTIONS SHALL CONFORM TO AISC STANDARDS AND THE MANUAL OF STEEL CONSTRUCTION (LATEST EDITION), AND NEW YORK STATE BUILDING CODE..
 - TYPICALLY, ALL BEAM TO COLUMN CONNECTIONS SHALL BE FRAMED BEAM CONNECTIONS.
 - TYPICALLY, BEAM TO BEAM CONNECTIONS SHALL BE FRAMED BEAM CONNECTIONS.
 - IF A ONE-SIDED CONNECTION IS USED FOR BEAMS 18" DEEP AND UNDER, THE CONNECTION SHALL BE DESIGNED IN ACCORDANCE WITH THE AISC MANUAL (LATEST EDITION), AND SHALL HAVE A MINIMUM OF TWO LINES OF BOLTS IN EACH LEG OF THE ONE-SIDED CONNECTION ANGLE.
 - WHEN THE TABULATED AISC FRAMED CONNECTION TABLES CANNOT BE USED OR WHERE THEY ARE UNABLE TO CARRY THE REQUIRED REACTIONS, SPECIAL CONNECTIONS OF ADEQUATE STRENGTH SHALL BE PROVIDED.
 - SHOP CONNECTIONS SHALL BE HIGH-STRENGTH BOLTED, OR WELDED.
 - FIELD CONNECTIONS SHALL BE HIGH-STRENGTH BOLTED, EXCEPT WHERE WELDED CONNECTIONS ARE SHOWN ON DRAWINGS.
 - ALL BEAMS CARRYING OR OVER COLUMNS, AND MOMENT SPLICES SHALL BE HIGH STRENGTH BOLTED.
 - BOLTED CONNECTIONS**
 - ALL BOLTS SHALL BE HIGH-STRENGTH CONFORMING TO ASTM A325 N. (A325 SLIP CRITICAL, CLASS A, FULLY TENSIONED TO COMPLY WITH AISC TABLE J 3.7).
 - ALL BOLTS SHALL BE 3/4" MINIMUM DIAMETER WITH HARDENED WASHERS UNDER THE TURNING ELEMENT (INCLUDING TURN OF THE NUT METHOD), UNLESS OTHERWISE NOTED.
 - HIGH-STRENGTH BOLTED CONNECTIONS TYPE N SHALL BE INSTALLED AND CONFORM TO AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS," APPROVED NOV. 13, 1985 AND ADOPTED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS OF THE ENGINEERING FOUNDATION.
 - ALL CONTACT SURFACES, INCLUDING SURFACES ADJACENT TO THE BOLT HEAD AND NUT, SHALL BE FREE OF SCALE, OIL, PAINT, LACQUER, AND OTHER FOREIGN MATERIAL. BURRS THAT WOULD PREVENT SOLID SEATING OF THE CONNECTED PARTS SHALL BE REMOVED. CONTACT SURFACES THAT ARE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 AND ROUGHENED BY MEANS OF HAND WIRE BRUSHING (POWER BRUSHING IS PROHIBITED) WILL BE PERMITTED.
 - ALL BOLTED CONNECTIONS SHALL MEET STRUCTURAL INTEGRITY REQUIREMENTS.
 - PROVIDE TWO BOLTS PER CONNECTION. MINIMUM CONNECTION STRENGTH SHALL BE 6 KIPS.
 - ALL HOLES SHALL BE DRILLED OR PUNCHED. NO BURNED IN HOLES SHALL BE ACCEPTABLE.

13. WELDED CONNECTIONS

- A. WELDING OPERATIONS AND DESIGN SHALL BE IN ACCORDANCE WITH THE NEW YORK STATE BUILDING CODE AND AISC SPECIFICATIONS, INCLUDING:
 1. AISC SPECIFICATION, J2.
 2. STRUCTURAL WELDING CODE, AWS D1.1, AS MODIFIED BY AISC SPECIFICATION SECTION J2.
 - B. WELDING SHALL BE PERFORMED BY LICENSED WELDERS AND MEET THE REQUIREMENTS OF THE NEW YORK STATE BUILDING CODE AND AWS.
 - C. AWS CLASS E70XX SERIES LOW HYDROGEN ELECTRODES SHALL BE USED FOR WELDING NEW STRUCTURAL STEEL.
 - D. ALL CONTACT SURFACES AND AREAS WITHIN TWO INCHES OF WELD DURING WELDING SHALL BE FREE OF SCALE, OIL, PAINT LACQUER, GALVANIZING, AND ANY OTHER FOREIGN MATERIAL.
 - E. BUTT WELDS SHALL BE 100% PENETRATION WELDS AND FILLET WELDS A MINIMUM OF 1/4" UNLESS OTHERWISE NOTED.
 - F. ALL WELDED CONNECTIONS SHALL MEET STRUCTURAL INTEGRITY REQUIREMENTS OF THE NEW YORK STATE BUILDING CODE.
 - G. WELDING SHALL NOT BE PERMITTED WHEN: THE AMBIENT TEMPERATURE IS BELOW 15F; SURFACES ARE WET, EXPOSED TO RAIN, SNOW OR HIGH WIND, WELDERS ARE EXPOSED TO INCLEMENT CONDITIONS. CONTRACTOR SHALL COORDINATE WITH OWNER TO PROVIDE FOR THE SHUT-OFF OF FIRE ALARMS, FIREWATCH, AND NOTIFICATION TO THE FIRE DEPARTMENT DURING WELDING OPERATIONS.
 - H. ALL FILLER METAL SHALL HAVE A MINIMUM CVN (CHARPY V-NOTCH) TOUGHNESS OF 20 FT.-LBS. AT MINUS 20F.
14. CONTRACTOR SHALL SUBMIT WRITTEN WELDING PROCEDURE SPECIFICATIONS (WPS) FOR ALL WELDING ON THE JOB, INCLUDING PREQUALIFIED PROCEDURES.
15. THE CONTRACTOR MUST HAVE WPS THAT COVER EVERY WELD TO BE MADE ON THE JOB INCLUDING THOSE OF THE ERECTOR. WPS MUST BE AVAILABLE FOR TESTING LABORATORY INSPECTOR OR ENGINEER.
16. WPS MUST INCLUDE: WELDING PROCESS, BASE METAL THICKNESS AND STEEL GRADE OF BASE METAL AND BACKUP MATERIAL, JOINT TYPE, WELD TYPE, JOINT DESIGN DETAIL, BACKLOGGING, ELECTRODE DIAMETER, NUMBER AND POSITION OF WELD PASSES, VOLTAGE, POLARITY, PREHEAT, AND INSPECTION REQUIREMENTS, APPROVED WRITTEN WELDING PROCEDURES IN CLOSE PROXIMITY TO AN USED BY THE WELDERS. THE WPS MUST BE PROPERLY FOLLOWED, AND THE WELDER MUST USE THE RIGHT TOOLS (THE CORRECT TEMPERATURE AND INDICATING CRAYON). MAINTAIN THE MINIMUM SPECIFIED PREHEAT TEMPERATURE, WELD SIZE, MAXIMUM INTERPASS TEMPERATURE EXCEEDED, ELECTRODE, TRAVEL SPEED.
17. CONTRACTOR SHALL TAKE SPECIAL PRECAUTIONS DURING STEEL ERECTION TO INSURE STABILITY UNTIL STRUCTURE IS COMPLETE. ALL STRUCTURAL STEEL SHALL BE ADEQUATELY BRACED ALONG ITS MAJOR AND MINOR AXIS DURING ERECTION.
18. SHORING OF FLOOR MEMBERS TO CONTROL SLAB THICKNESS, FLOOR LEVEL TOLERANCES, AND CONCRETE PONDING IS THE CONTRACTOR'S OPTION. FLOORS TO BE POURED SO AS TO MAINTAIN UNIFORM SLAB THICKNESS ACROSS TOP OF STEEL MEMBERS.
19. STRUCTURAL STEEL CONTRACTOR TO PROVIDE DECK SUPPORT ANGLES AS REQUIRED.
20. BOLTS, NUTS AND WASHERS FOR STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE GALVANIZED.
21. STEEL, NUTS, BOLTS AND WASHERS TO BE PROVIDED BY A SINGLE DOMESTIC SOURCE.
22. ALL CUTS, HOLES, COPES REQUIRED FOR CONSTRUCTION BY OTHER TRADES, SHALL BE SHOWN ON APPROVED SHOP DRAWINGS AND MADE IN THE SHOP. FIELD BURNINGS OF CUTS OR HOLES IN THE STEEL MEMBERS WILL NOT BE PERMITTED.
23. ALL STEEL EXPOSED TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED. LINTEL ANGLES, BRACING MEMBERS OR STEEL EMBEDDED IN MASONRY SHALL RECEIVE TWO SHOP COATS OF PAINT. ALL OTHER STEEL SHALL BE LEFT UNPAINTED. NOTE: OMIT PAINT AND GALVANIZATION FROM STEEL AREAS WHICH ARE TO BE WELDED. TOUCH UP ALL AREAS WHERE THE GALVANIZING HAS BEEN REMOVED OR DAMAGED.
- NOTE: STRUCTURAL STEEL TUBING WHICH IS EXPOSED TO THE WEATHER SHOULD BE GALVANIZED AND HAVE A RADIUS EQUAL TO THREE (3) TIMES ITS THICKNESS AS PER ASTM A143/A143M.
24. FITTED STIFFENERS SHALL BE PROVIDED IN BEAMS OVER AND UNDER ALL COLUMNS EQUAL TO COLUMNS FLANGE THICKNESS.
25. ANCHOR BOLTS TO BE ASTM F1554 (WELDABLE) FY = 55 KSI.
26. ALL GROUT UNDER BILLET'S SHALL BE NON-SHRINK, PRE-MIX TYPE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5,000 P.S.I.
27. PROVIDE ADDITIONAL WIRE-MESH REINFORCEMENT, 4 X 4 -W 4 X 4 WVF X 5'-0" LONG IN TOP OF SLAB CONTINUOUSLY OVER ALL BEAMS WHICH RUN PARALLEL TO THE METAL DECK DIRECTION.
28. ALL STAIR STRUCTURES AND MISCELLANEOUS STEEL SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR. DRAWINGS AND CALCULATIONS SHALL BEAR THE SEAL AND SIGNATURE OF A NEW YORK STATE REGISTERED PROFESSIONAL ENGINEER HIRED BY THE CONTRACTOR.
29. IF FIELD BEAM SPLICE IS REQUIRED, IT SHALL DEVELOP THE FULL PLASTIC MOMENT CAPACITY OF THE MEMBER. COMPLETE DETAILS SHALL BE SUBMITTED ON THE SHOP DRAWINGS
30. SHOP AND FIELD TESTING OF WELDS AND BOLTS SHALL BE AS FOLLOWS:

- A. ALL WELDS SHALL BE VISUALLY INSPECTED AND SHALL BE ACCEPTABLE IF THE CRITERIA OR AWS D1.1 TABLE E.6.1 ARE SATISFIED.
 - B. FILLET WELDS FOR BEAM AND GIRDER SHEAR CONNECTION PLATES (15 PERCENT AT RANDOM) SHALL BE CHECKED BY MAGNETIC PARTICLE FOR FINAL PASS ONLY.
 - C. ULTRASONICALLY TEST 100 PERCENT OF ALL FULL PENETRATION WELDS.
 - D. CHECK BY CALIBRATED TORQUE WRENCH 25 PERCENT OF BOLTS IN EACH SHEAR CONNECTION, BUT NOT LESS THAN TWO (2) BOLTS PER CONNECTION.
 - E. THE TESTING AGENCY SHALL PERFORM ALL SHOP AND FIELD INSPECTION AND TESTING AS OUTLINED ABOVE.
 - F. THE STRUCTURAL STEEL FABRICATOR AND ERECTOR SHALL SCHEDULE ALL WORK TO ALLOW THE ABOVE TESTING REQUIREMENTS TO BE COMPLETED.
31. AFTER FABRICATION AND IN THE FIELD PRIOR TO THE APPLICATION OF SPRAY-ON OR TROWELED ON FIREPROOFING, ALL STEEL SHALL BE CLEANED OF ALL RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS. SEE ARCHITECTURAL DRAWING FOR FIREPROOFING REQUIREMENTS.
 32. NO FIELD CUTTING OF STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT APPROVAL OF ENGINEER-OF-RECORD.
 33. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY BRACING, GUYING, ETC. REQUIRED TO ERECT AND HOLD THE FRAME IN ORDER TO MAINTAIN CORRECT ALIGNMENT AND RESIST POSSIBLE COMBINATIONS OF DEAD, CONSTRUCTION, ERECTION, WIND, SEISMIC AND OTHER LATERAL LOAD COMBINATIONS.
 34. PROVIDE EXISTING CONNECTION REINFORCING AT LOCATIONS SHOWN ON PLAN S101. CONTRACTOR SHALL HIRE A NYSE TO DESIGN CONNECTION REINFORCING, CONTRACTOR SHALL SUBMIT THESE CALCULATIONS FOR REVIEW TO EOR.

STEEL JOISTS NOTES

1. ALL STEEL TRUSS GIRDERS/JOISTS, CONSTRUCTION, BRIDGING, ANCHORAGE, ETC., SHALL BE DESIGNED, DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS.
2. STEEL JOISTS SHALL BE STRUCTURAL STEEL HAVING A MINIMUM YIELD POINT OF $F_y=50,000$ PSI FOR CHORD SECTIONS AND A MINIMUM YIELD POINT OF $F_y=36,000$ PSI FOR WEB MEMBERS.
3. THE STEEL JOIST FABRICATOR SHALL SUBMIT AN AFFIDAVIT STATING COMPLIANCE WITH AISC, SJI AND AISI REQUIREMENTS. THE STEEL JOIST SUPPLIER SHALL BE A MEMBER OF SJI APPROVED FOR THE SPECIFIC JOIST

SERIES. STEEL JOIST FABRICATOR SHALL PROVIDE AN AFFIDAVIT BY A LICENSED PROFESSIONAL ENGINEER THAT ALL WELDS HAVE BEEN MADE TO SUSTAIN AND TRANSFER THE DESIGN STRESSES TO THE JOIST MEMBERS.

4. STEEL JOIST MANUFACTURER SHALL DESIGN AND FURNISH JOISTS AND BOTTOM CHORD BRACES AND BRIDGING FOR A NET UPLIFT OF 17 PSF MINIMUM. PROVIDE POSITIVE ANCHORAGE AT END OF THE BRIDGING LINE OR TO STRUCTURAL STEEL BEAMS. CONSTRUCTION LOADS MUST NOT BE APPLIED TO JOISTS UNTIL ALL BRIDGING IS ATTACHED TO THE JOISTS AND ANCHORED AT ITS ENDS. CROSS BRIDGING REQUIRED AT END BAYS OF JOISTS.
5. TOP AND BOTTOM CHORDS SHALL BE STRUCTURAL ANGLES.
6. JOIST SHALL BE BRIDGED WITH CONTINUOUS HORIZONTAL ANGLE BRIDGING TOP AND BOTTOM. PROVIDE X-BRIDGING IN LAST PANEL, OR WHERE CONTINUOUS BRIDGING STOPS. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON JOISTS
7. INDIVIDUAL JOIST MEMBERS SHALL BE CONNECTED DIRECTLY TO ONE ANOTHER BY WELDING IN ACCORDANCE WITH THE LATEST AWS CODE. ALL WELDERS SHALL BE CERTIFIED WELDERS AND LICENSED.
8. FOR ALL JOISTS FORTY(40) FEET AND LONGER STRICTLY FOLLOW MANUFACTURER'S INSTRUCTIONS REGARDING ERECTION.
9. JOISTS SHALL HAVE MINIMUM 4" BEARING ON MASONRY OR CONCRETE, AND 2-1/2" ON STEEL.
10. ALL STEEL JOISTS SHALL BE THOROUGHLY INSPECTED BEFORE SHIPMENT TO INSURE COMPLIANCE IN MATERIALS AND WORKMANSHIP.
11. ALL JOISTS TO RECEIVE ONE SHOP COAT OF APPROVED PAINT.
12. DESIGN OF JOISTS SHALL BE BY MANUFACTURER'S ENGINEER REGISTERED IN THE STATE OF NEW YORK FOR ALL LOADING CONDITIONS. SHOP DRAWINGS SHALL BEAR THE ENGINEERS SEAL AND SIGNATURE. SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION.
13. JOIST CONTRACTOR SHALL COORDINATE WITH ALL TRADES IN ORDER TO PROVIDE ALL ROOF OPENINGS WHERE REQUIRED.
14. WHEN HORIZONTAL BRIDGING INTERFERES WITH MECHANICAL PENETRATIONS THROUGH THE JOIST SYSTEM, THEN INSTALL TWO (2) ADJACENT BAYS OF BOLTED OR WELDED X-BRIDGING EACH SIDE OF THE PENETRATION. METAL DECKING SHALL BE SECURED IN PLACE AND BRIDGING INSTALLED PRIOR TO CUTTING AND REMOVING HORIZONTAL BRIDGING.
15. ALL CONCENTRATED LOADS SUCH AS FROM ROOFTOP UNITS, PIPING, SUSPENDED EQUIPMENT, ETC. SHALL BE SUPPORTED AT PANEL POINTS OF THE JOIST. IF LOADS NEED TO BE SUPPORTED BETWEEN PANEL POINTS PROVIDE ADDITIONAL REINFORCEMENT (SEE TYPICAL DETAILS).
16. PROVIDE CEILING EXTENSIONS WHERE REQUIRED BY ARCHITECTURAL DRAWINGS OR SPECIFICATIONS.
17. BRIDGING SHALL BE CONNECTED TO THE JOISTS WITH 1/8-INCH FILLET WELD 1-INCH LONG (MINIMUM) OR EQUIVALENT. HORIZONTAL BRIDGING SHALL HAVE A STANDARD 3-INCH LAP AND BE WELDED WITH 2-1/8 INCH FILLET WELDS 1-INCH LONG OR EQUIVALENT.
18. DEFLECTION DUE TO DESIGN LIVE LOAD SHALL NOT EXCEED 1/360 OF SPAN.
19. PROVIDE UPLIFT BRIDGING AT JOIST ENDS AT ALL ROOF JOISTS.

METAL DECKING

- STEEL DECKING SHALL BE CORRUGATED STEEL SHEETS, AND SHALL CONFORM TO THE STEEL DECKING INSTITUTE SPECIFICATIONS, AND MEET THE REQUIREMENTS SPECIFIED HEREIN. SEE DRAWINGS FOR SIZE, GAGE AND TYPE. STEEL SHALL MEET ASTM A-653, GRADE 33, MINIMUM YIELD OF 33 KSI. GALVANIZING SHALL CONFORM TO REQUIREMENTS OF ASTM A-653 COATING CLASS G90. STEEL FOR PAINTED METAL DECK SHALL CONFORM TO ASTM A1008 GRADE.
2. INSTALLATION:
- A. GENERAL: DECKING SHALL BE COMPLETE, INCLUDING ALL SHAPING, CUTTING, FITTING, DRILLING, WELDING, FASTENINGS, COLUMN CLOSURES, RIDGE AND VALLEY PLATES, SUMP AND CANT STRIPS AND ALL OTHER ACCESSORIES AND MISCELLANEOUS PIECES NECESSARY FOR PROPER EDGING INSTALLATION. CLOSURE ELEMENTS FOR CONTAINMENT OF CONCRETE DURING POUR DECKING SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. END LAPS SHALL BE TWO INCHES MINIMUM, SHALL OCCUR OVER SUPPORTS, AND SHALL BE CENTERED THEREON. ADJACENT ROWS SHALL BE SIDE LAP ONE CORRUGATION WITH PREVIOUSLY PLACED ROW.
- B. WELDING: SHALL CONFORM TO APPLICABLE REQUIREMENTS OF AMERICAN IRON AND STEEL INSTITUTE'S "SPECIFICATION FOR THE DESIGN OF LIGHT GAGE COLD-FORMED STEEL STRUCTURAL MEMBERS." WORK SHALL BE PERFORMED BY QUALIFIED WELDERS. SHEETS SHALL BE WELDED TO SUPPORTS IMMEDIATELY AFTER ALIGNMENT, AND ALL SHEETS SHALL BE WELDED BEFORE END OF WORKING DAY. WELDING SHALL MEET FOLLOWING MINIMUM REQUIREMENTS. USE ELECTRODE AWS D1.3 AND WELD TO ALL SUPPORTING MEMBERS IN ACCORDANCE WITH AWS REQUIREMENTS.
- 1) A MINIMUM VISIBLE 3/4" DIAMETER ARC PUDDLE WELD SHALL BE USED. WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL, AND SHALL HAVE GOOD FUSION TO SUPPORTING MEMBERS.
- 2) EXCEPT AS NOTED, DECK SHALL BE FASTENED BY 3/4" DIAMETER FUSION WELDS AT 12" O.C. AT ENDS AND INTERIOR SUPPORTS PERPENDICULAR TO DECK SPAN AND AT EDGE AND INTERIOR SUPPORTS PARALLEL TO THE DECK SPAN.
- 3) SIDE LAPS TO BE WELDED OR SCREWED WITH #12 SELF DRILLING SCREWS WITH MAXIMUM SPACING AT 3'-0" O.C.
3. METAL DECK TO BE INSTALLED CONTINUOUSLY OVER SUPPORTING MEMBERS WITH A MINIMUM OF THREE CONTINUOUS SPANS AND SHALL BEAR AT LEAST TWO INCHES MINIMUM BEARING AT FLOOR DECK SUPPORTS AND THREE INCH MINIMUM BEARING AT ROOF DECK SUPPORTS.
4. SHEAR STUDS SHALL BE CONSIDERED TO REPLACE WELDS.
5. FOR ONE OR TWO SPAN CONDITIONS, THE CONTRACTOR SHALL PROVIDE SHORING AS REQUIRED OR FURNISH HIGHER GAGE AS REQUIRED TO SUPPORT ALL APPLICABLE LOADS.
6. THE CONTRACTOR SHALL DEPOSIT ALL CONCRETE DURING PLACING IN SUCH A MANNER AS NOT TO OVERLOAD THE METAL DECK.
7. THE CONTRACTOR SHALL CALCULATE AND INCLUDE ALL ADDITIONAL CONCRETE THAT MAY BE REQUIRED DURING PLACING DUE TO DEFLECTION OF STRUCTURE.
8. USE ELECTRODE AWS D1.3 AND WELD TO ALL SUPPORTING MEMBERS IN ACCORDANCE WITH AWS REQUIREMENTS. ALL WELDING SHALL BE PERFORMED BY LICENSED WELDERS AND MEET THE REQUIREMENTS OF THE AWS AND THE LOCAL BUILDING CODE. WELDING SHALL NOT BE PERFORMED AT TEMPERATURES BELOW 32 DEGREES F.
9. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS WHICH INDICATE ALL INFORMATION NECESSARY FOR A COMPLETE FABRICATION AND ERECTION OF THE METAL WORK. DRAWINGS SHALL INCLUDE ALL SHAPING, CUTTING, FITTING, DRILLING, WELDING, CLOSURE STRIPS, CLOSURE PLATES, SUMP PANS, FILLERS, FASTENINGS, METAL LATH COLUMN CLOSURES AND ALL OTHER ACCESSORIES AND MISCELLANEOUS PIECES NECESSARY FOR PROPER INSTALLATION. TYPE, NUMBER OF WELDS, LOCATION AND METHOD OF WELDING SHALL BE INDICATED ON SHOP DRAWINGS.
10. STANDARD CLOSURES AND EDGE ANGLE POUR STOPS (SCREED ANGLES) SHALL BE GALVANIZED AND CONFORM TO ASTM A653, CLASS G60, AND PROVIDED AS REQUIRED.
11. BOTTOM OF METAL DECK MUST BE FREE OF DUST, LOOSE SCALE AND OIL PRIOR TO APPLYING ANY CEMENTITIOUS MIXTURE.
12. FLOOR DECK SHALL BE COMPOSITE TYPE AND MANUFACTURED WITH INTEGRAL HANGER TABS FOR VENTING PURPOSES ONLY.
13. PROVIDE ADDITIONAL ANGLE SUPPORTS UNDER DECK AT ALL COLUMNS.
14. NO LOADS SUCH AS MECHANICAL EQUIPMENT, PIPING, DUCTS, LIGHTING, ETC., SHALL BE HUNG DIRECTLY FROM METAL DECK. HANGERS FOR DUCTWORK, PIPING, ETC. SHALL BE HUNG DIRECTLY FROM STRUCTURAL STEEL MEMBER OR SUPPLEMENTARY MEMBERS OR ANCHORS EMBEDDED IN CONCRETE. SUBMIT HANGER LOAD DETAILS FOR REVIEW.
15. DECK CONTRACTOR SHALL COORDINATE OPENING SIZES AND LOCATIONS IN FLOORS AND ROOF FROM ARCHITECTURAL AND MECHANICAL DRAWINGS. HE SHALL PROVIDE ANY HEADER MEMBERS OR REINFORCEMENT AS REQUIRED EVEN IF NOT SHOWN ON PLANS.
16. CONSTRUCTION JOINTS FOR SLABS ON METAL DECK SHALL BE LOCATED MIDWAY BETWEEN BEAMS WHERE THE JOINT IS PARALLEL TO THE BEAM SPAN. WHERE THE JOINT IS PERPENDICULAR TO THE BEAM SPAN THE JOINTS SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF SPAN.

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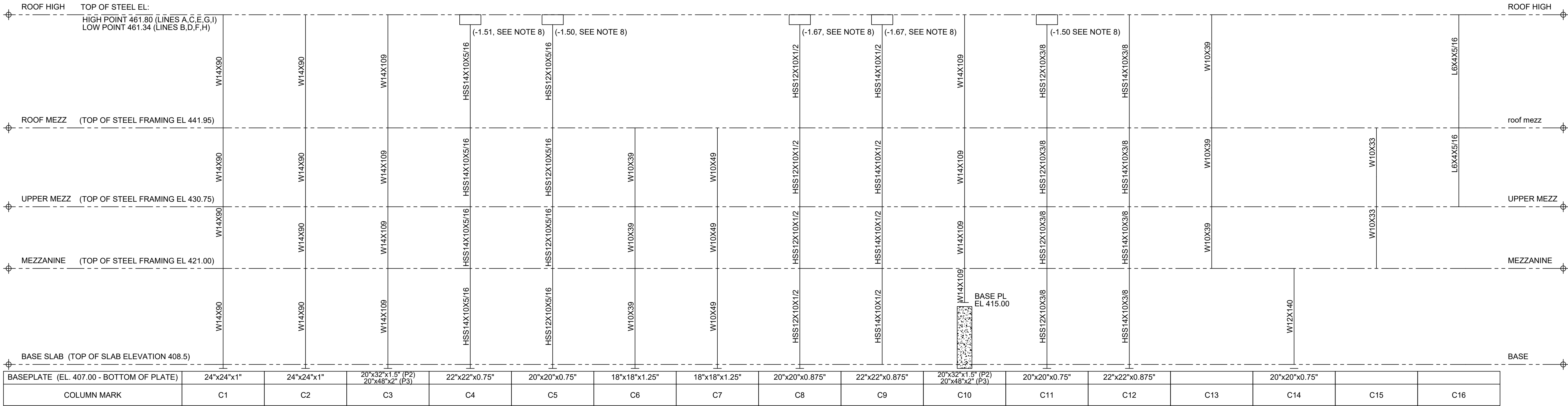
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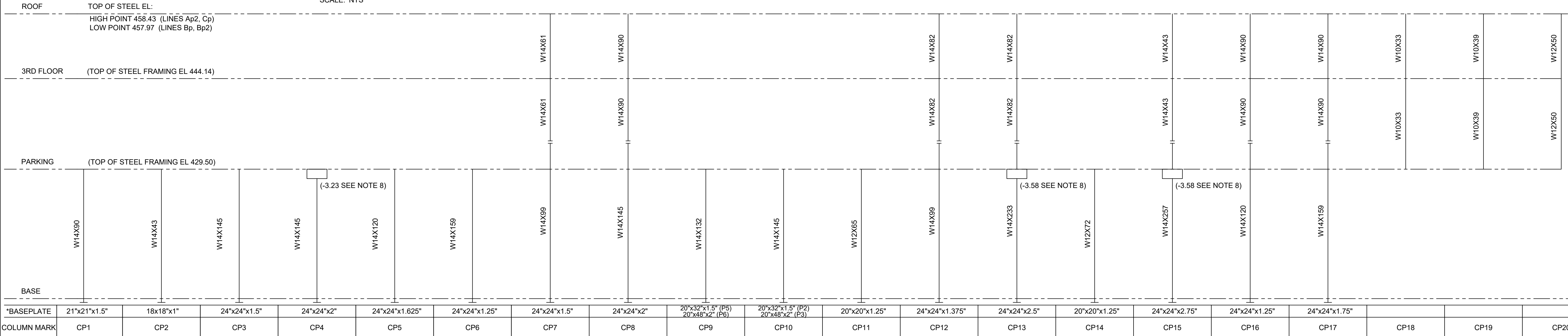
NOTES

DWG NUMBER :
S-003
4 OF 60



ASRS BUILDING COLUMN SCHEDULE

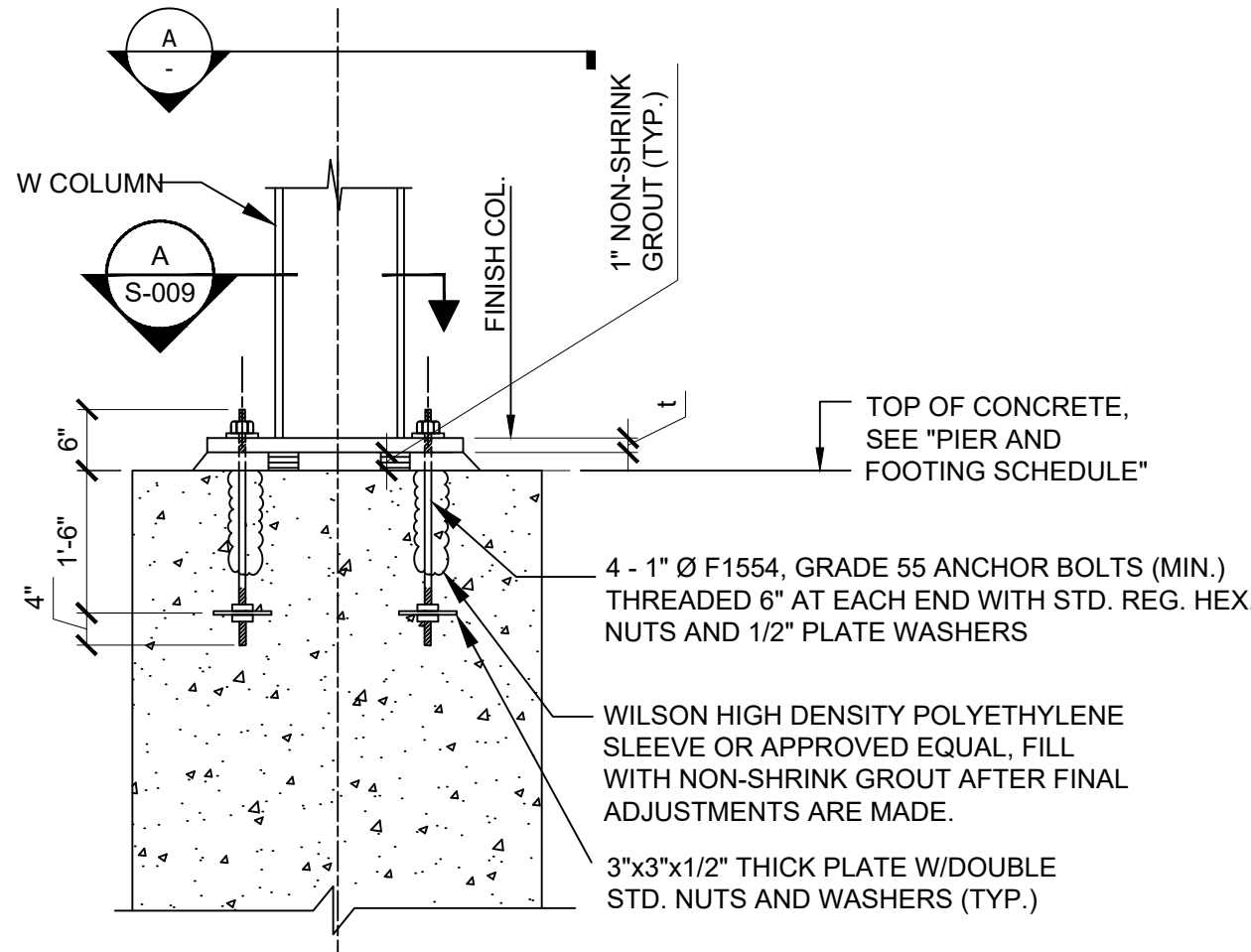
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*SEE FOUNDATION PLANS FOR TOP OF FOOTING ELEVATIONS AT THE PARKING STRUCTURE. BOTTOM OF BASE PLATE ELEVATIONS ARE 1" ABOVE THE TOP OF FOOTING ELEVATIONS.

PARKING AREA COLUMN SCHEDULE

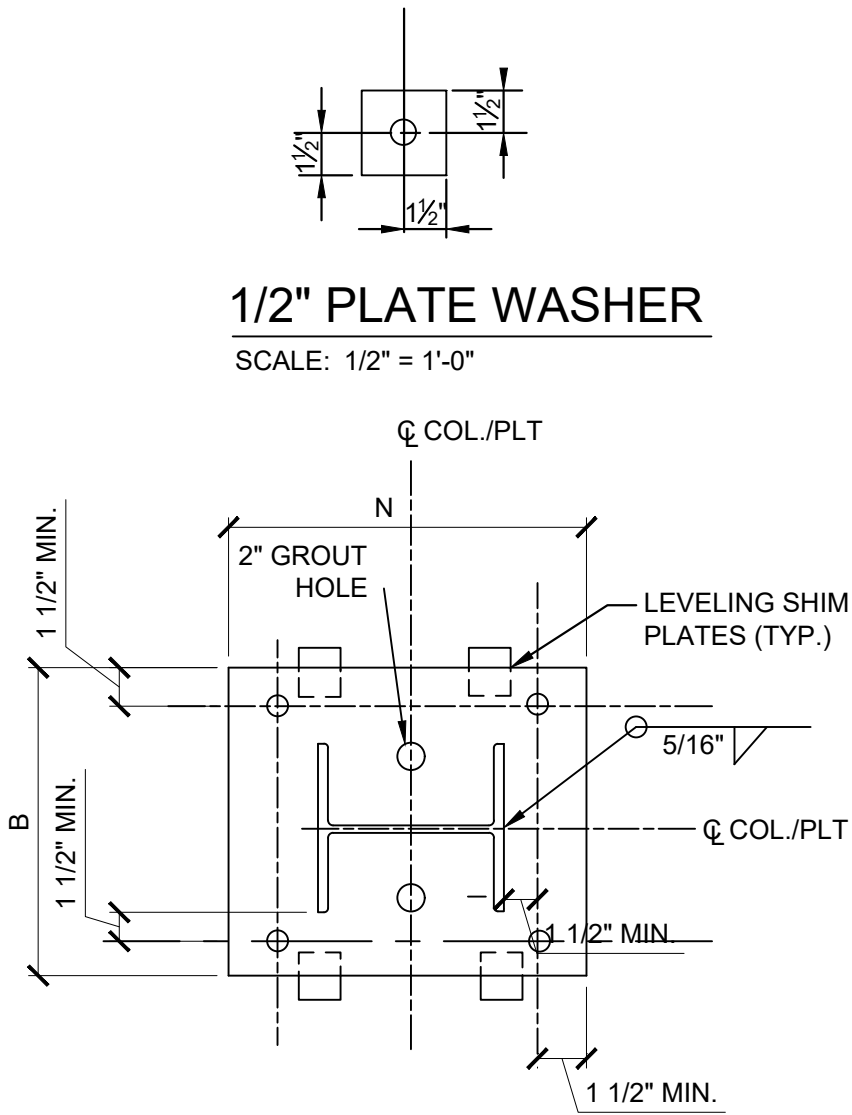
SCALE: NTS



GRAVITY COLUMN BASEPLATE

SCALE: 1/2" = 1'-0"

- NOTES:
1. FOR BASE PLATE DIMENSIONS B, N AND L, SEE COLUMN SCHEDULE.
 2. FOR SIZE OF COLUMN, SEE COLUMN SCHEDULE; FOR ORIENTATION, SEE FRAMING PLANS.
 3. WASHER PLATES SHALL HAVE HOLES WHICH ARE 1/16" LARGER THAN THE BOLT DIAMETER.
 4. HOLE DIAMETER SHALL BE EQUAL TO 'BOLT DIAMETER + 5/16"
 5. GROUT SHALL COMPLETELY FILL THE SPACE UNDERNEATH THE PLATE.
 6. WASHER MATERIAL TO MEET ASTM A36.
 7. FOR CLARITY, FIREPROOFING IS NOT SHOWN.
 8. TOP OF COLUMNS NOTED ARE LOWER THAN THE TYPICAL TOP OF STEEL BY THE DIMENSION SHOWN (-XX.X FEET)



PLAN VIEW

SCALE: 1/2" = 1'-0"

- NOTES:
- HSS POST BASEPLATE DETAIL SIMILAR.

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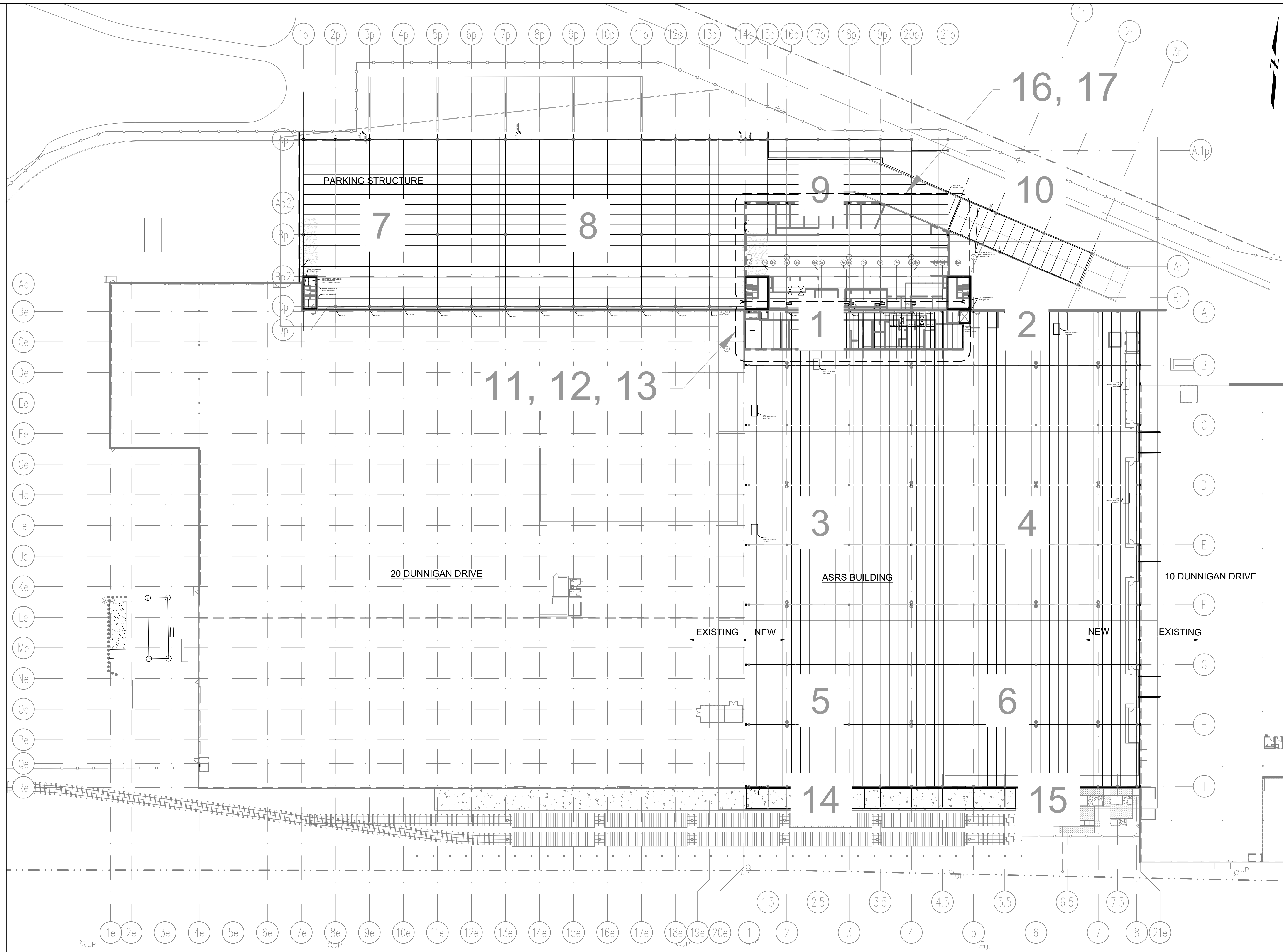
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COLUMN SCHEDULE

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FRAMING KEY PLAN
SCALE: 1/32" = 1'-0"

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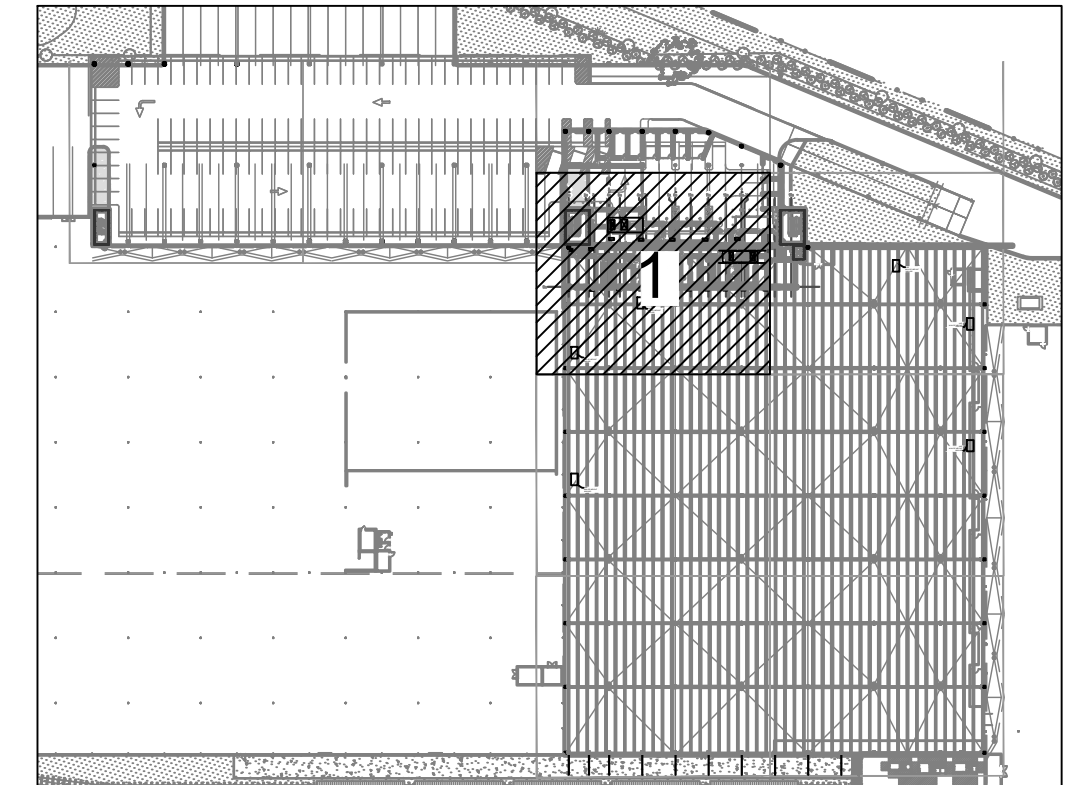
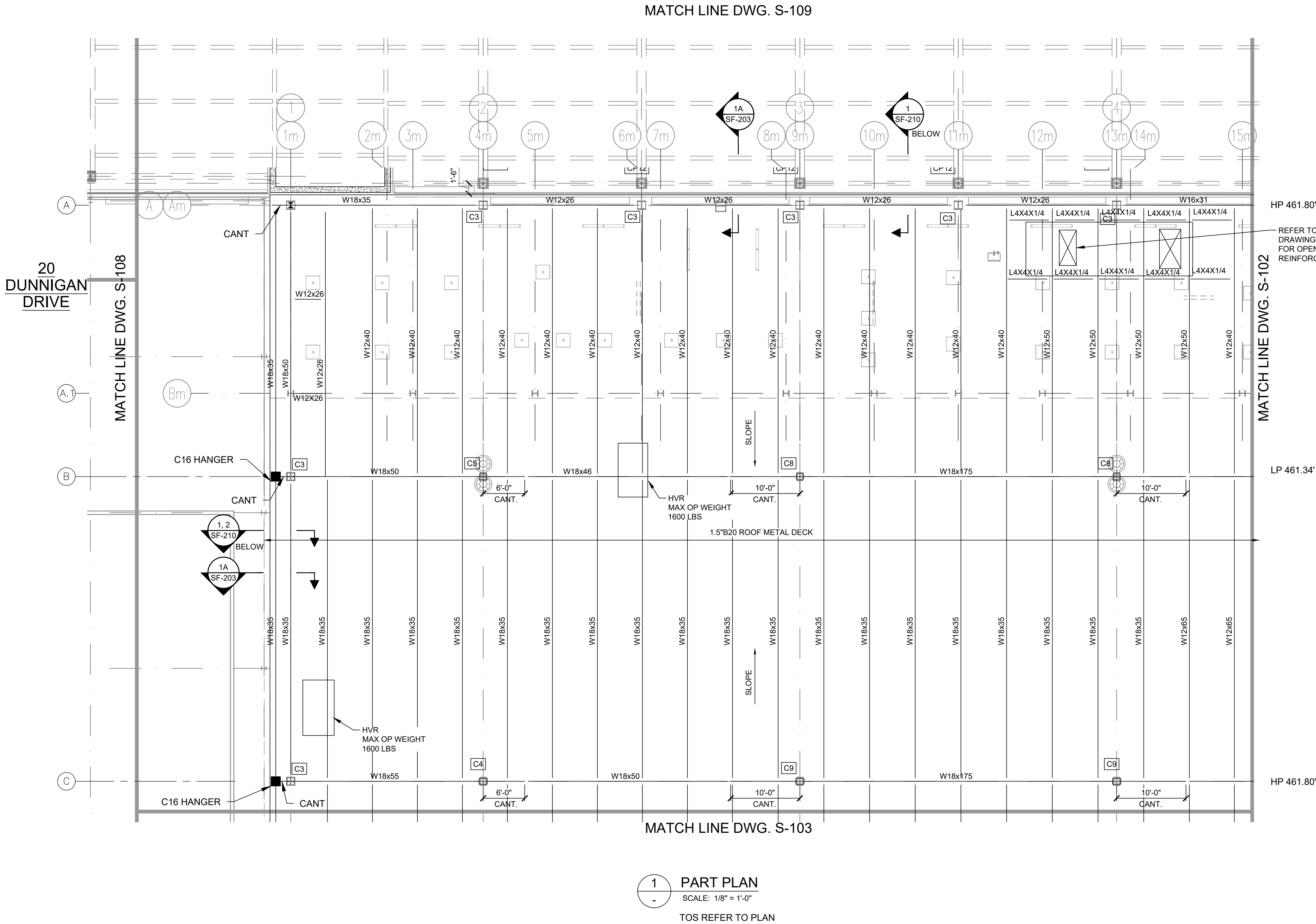
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FRAMING - KEY PLAN

DWG NUMBER :
S-100

5 OF 60



LEGEND

TOS - DENOTES TOP OF STEEL ELEVATION

HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION

LP - DENOTES TOP OF STEEL LOW POINT ELEVATION

C1 - COLUMN #

NOTE:

ASRS ROOF DECK WELDING
SHALL BE 3/4\" PUDDLE WELD
W/36/7 WELD PATTERN
W10-#12 SIDELAP FASTENERS

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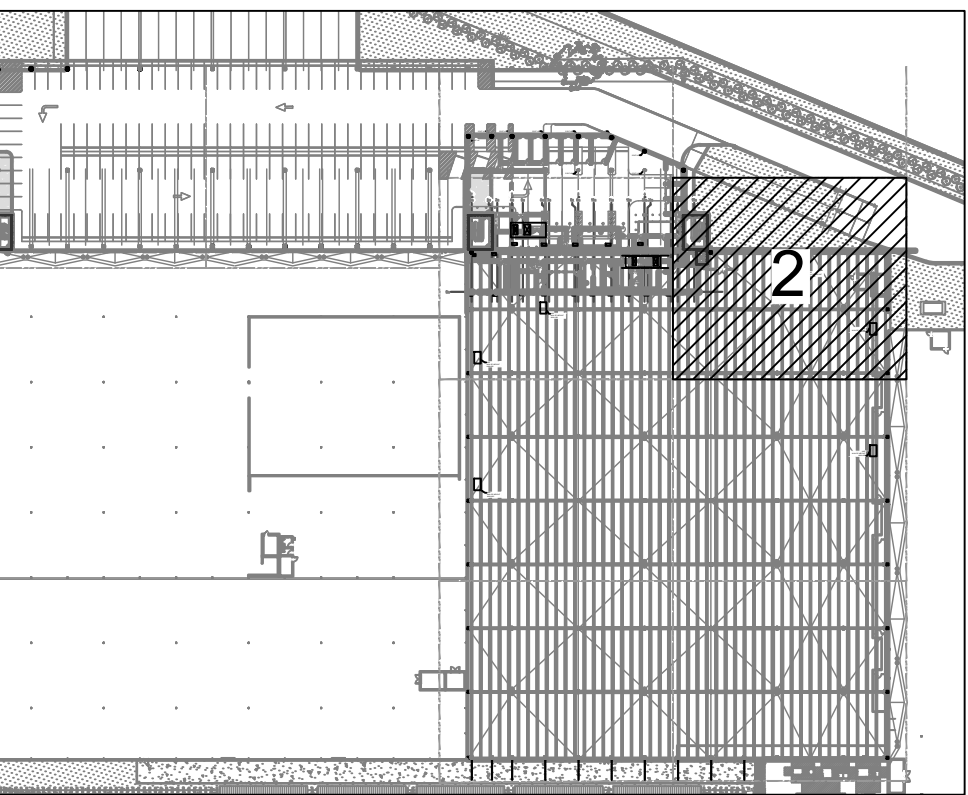
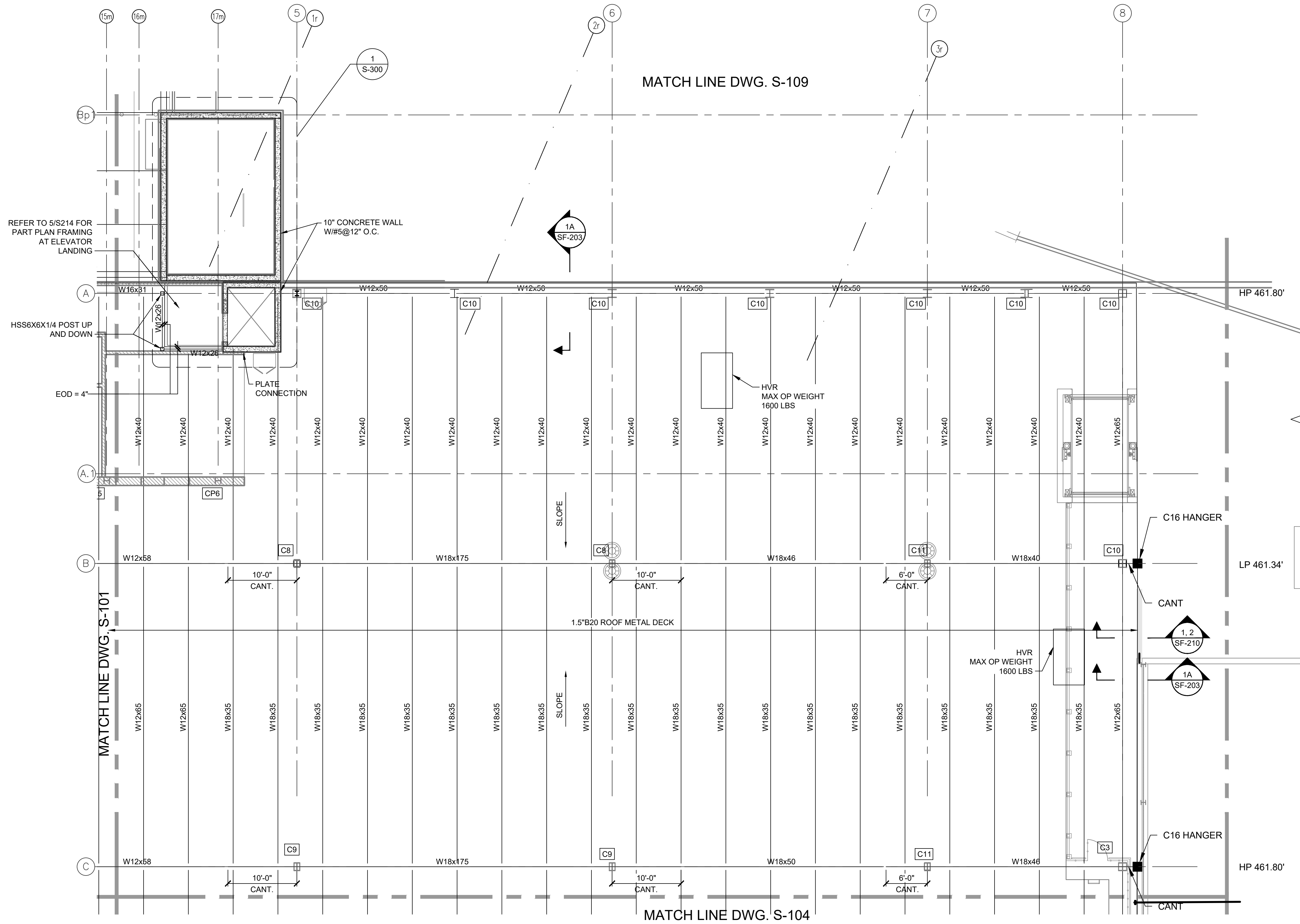
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AS/RS FRAMING PART PLAN

DWG NUMBER :

S-101

6 OF 60



KEY PLAN
SCALE: N.T.S.

- LEGEND
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 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #

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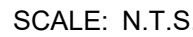
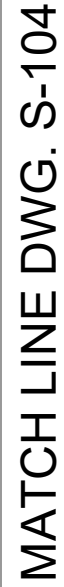
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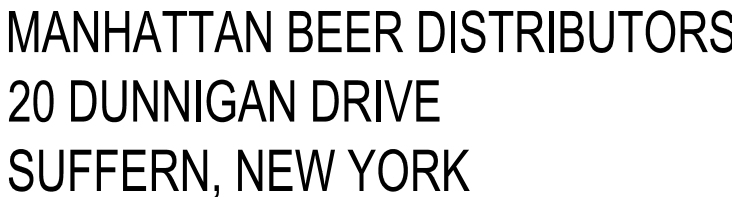
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C1 - COLUMN

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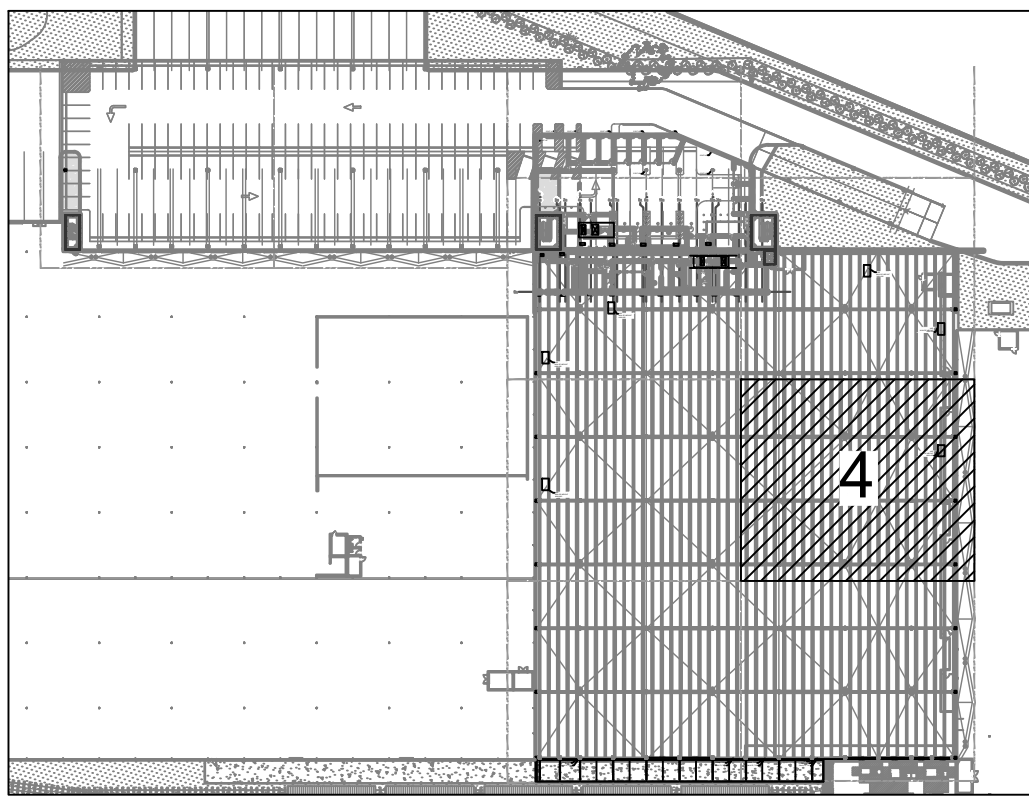
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AS/RS FRAMING PART PLAN

S-103

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KEY PLAN
SCALE: N.T.S.

- LEGEND
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #

XX.X

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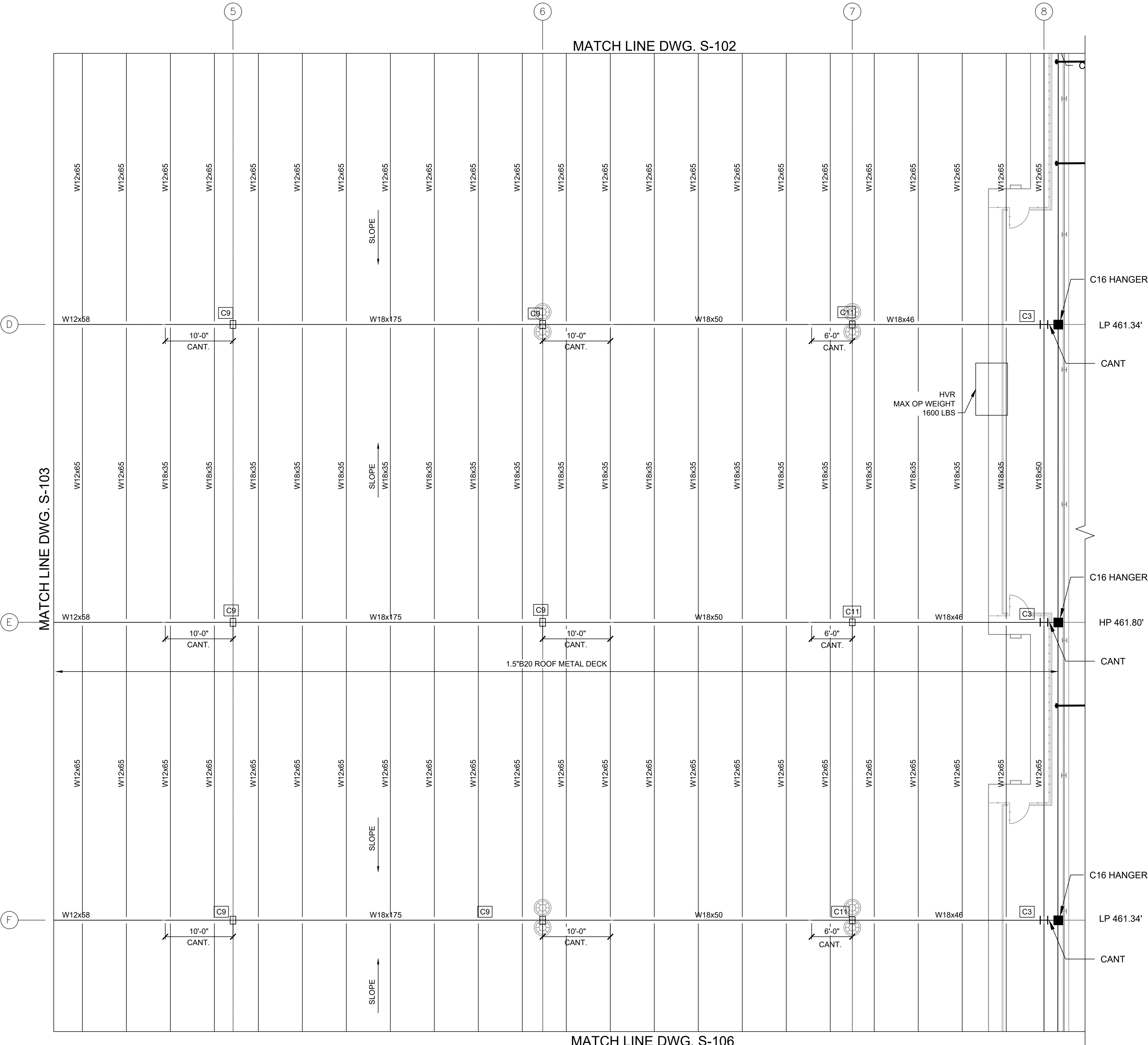
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MATCH LINE DWG. S-106

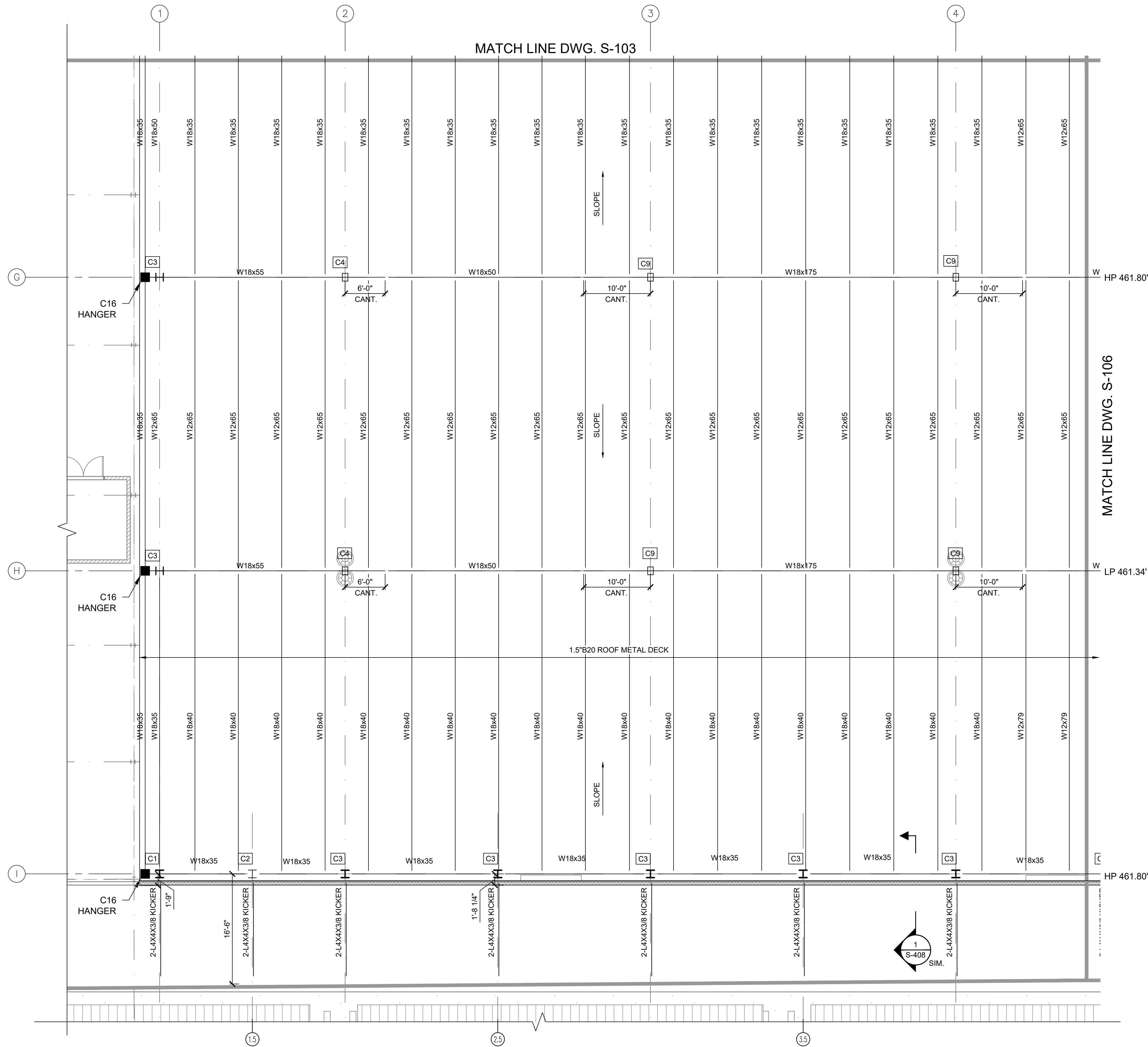
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PART PLAN

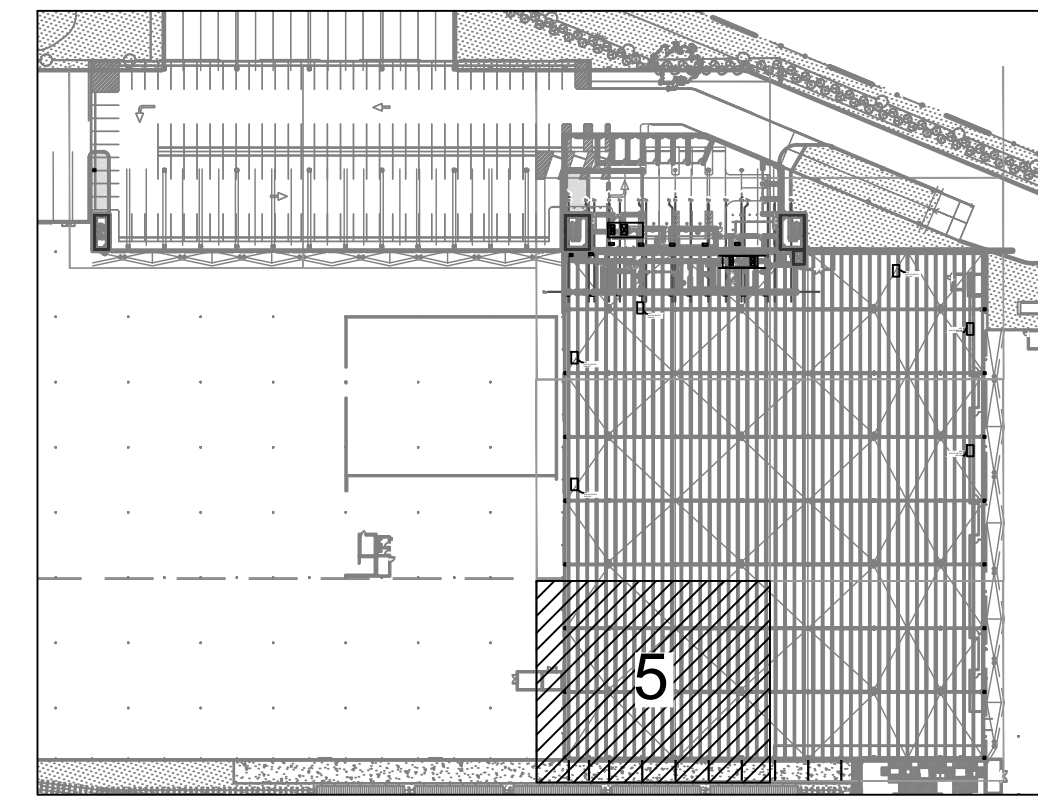
SCALE: 1/8" = 1'-0"

TOS REFER TO PLAN

20 DUNNIGAN
DRIVE



5 PART PLAN
SCALE: 1/8" = 1'-0"
TOS REFER TO PLAN



KEY PLAN
SCALE: N.T.S.

LEGEND

- TOS - DENOTES TOP OF STEEL ELEVATION
HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
C1 - COLUMN #

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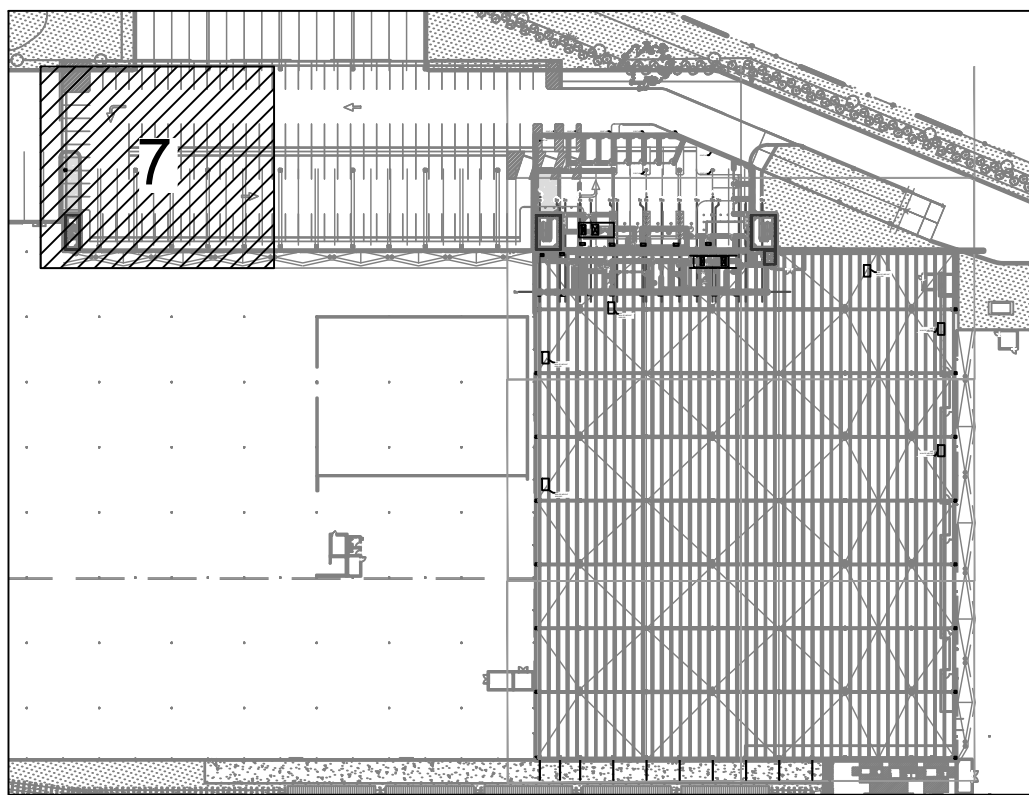
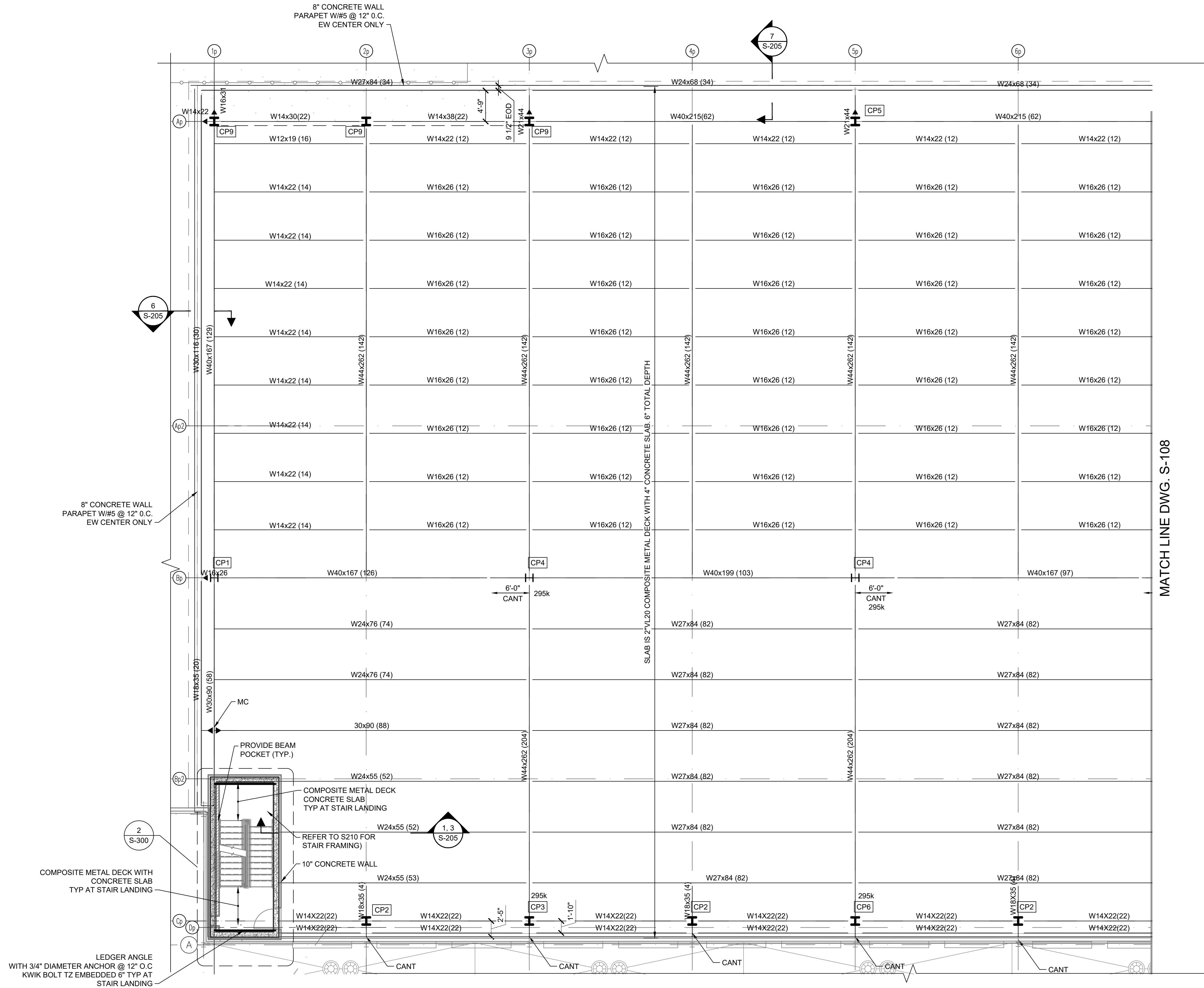
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- HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
- LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
- C1 - COLUMN #
- ◄ - DENOTES MOMENT CONNECTION TO DEVELOP 80% MP

NOTE
SHEAR STUDS SHALL BE 3/4"-5" LONG (TYP) FOR 6" SLAB.
INSTALL SHEAR STUD AT 1' O.C. MAXIMUM AT EVERY
BEAM MINIMUM (UNO)

PARKING SLAB SHALL BE REINFORCED WITH
6X6-W2.9XW2.9 WWF

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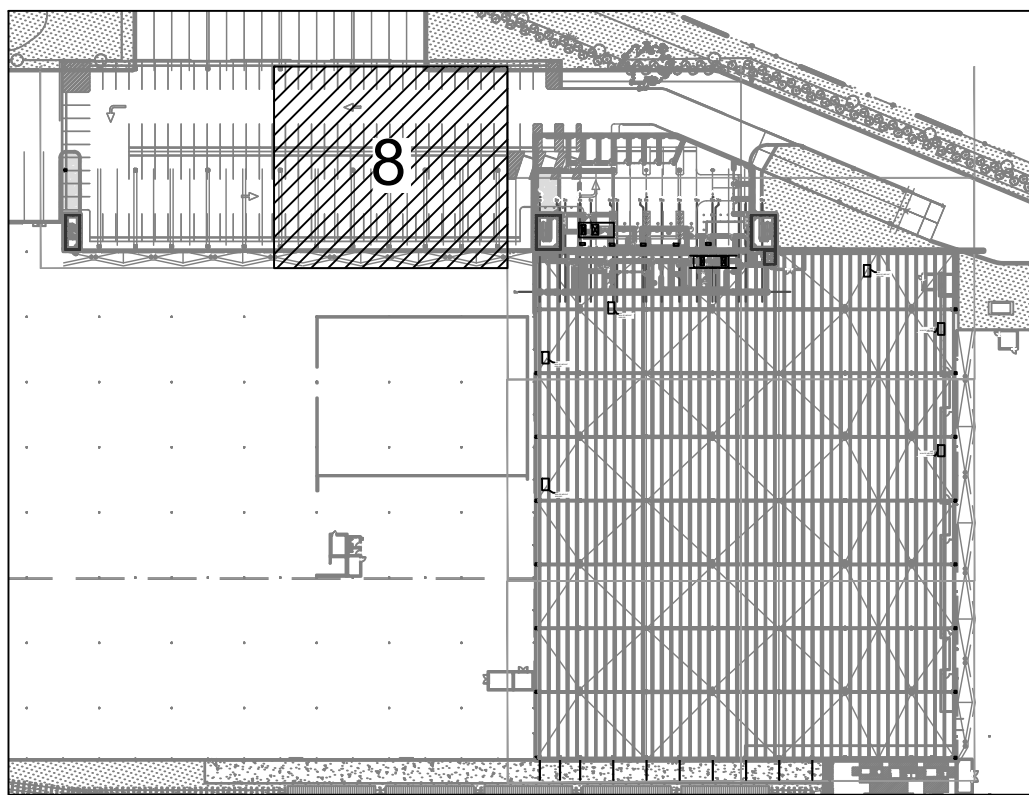
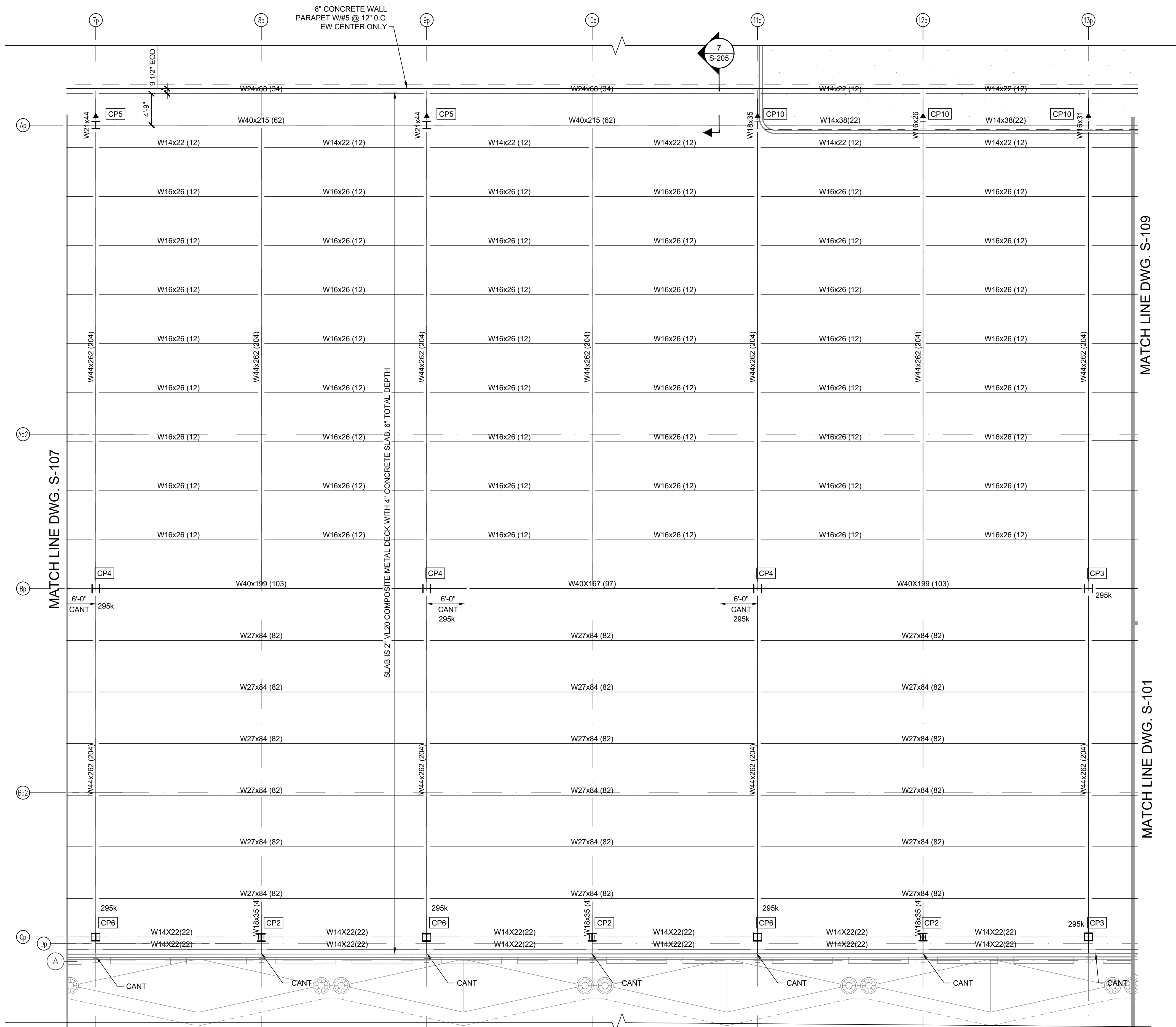
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PARKING FRAMING
PART PLAN

DWG NUMBER :
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 - C1 - COLUMN #

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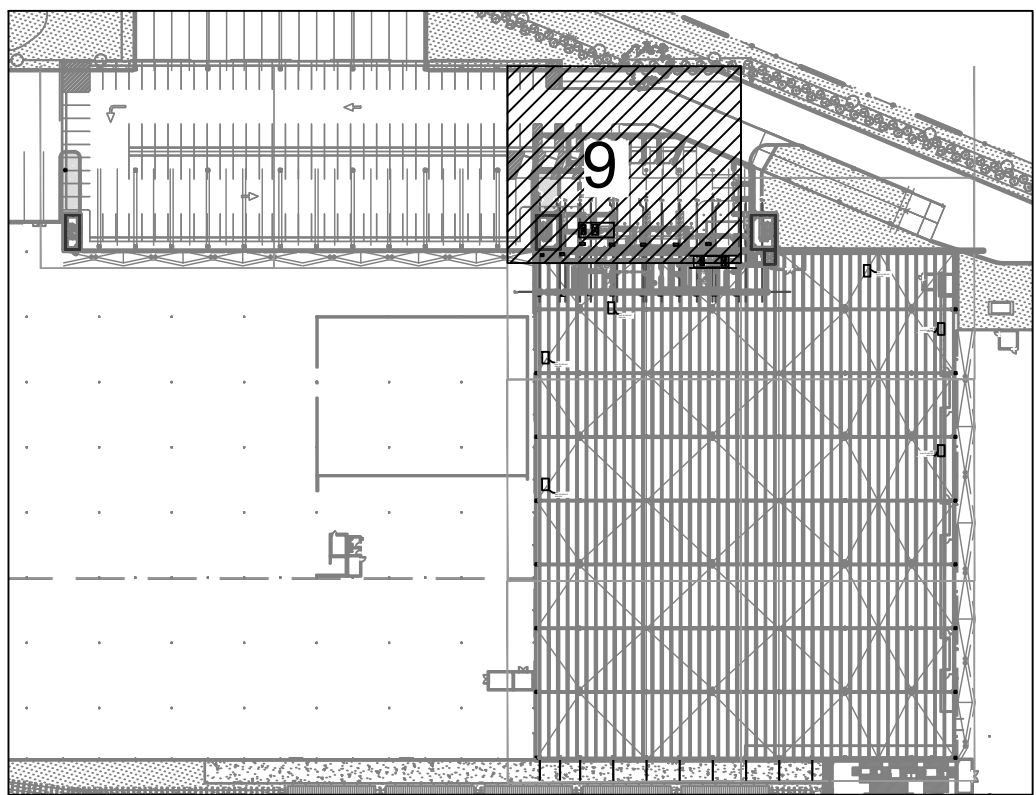
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DATE : 9/10/2021
SCALE : AS SHOWN

DRAWING TITLE :

PARKING FRAMING
PART PLAN

DWG NUMBER :
S-108
13 OF 60



KEY PLAN
SCALE: N.T.S.

LEGEND

- TOS - DENOTES TOP OF STEEL ELEVATION
HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
C1 - COLUMN #



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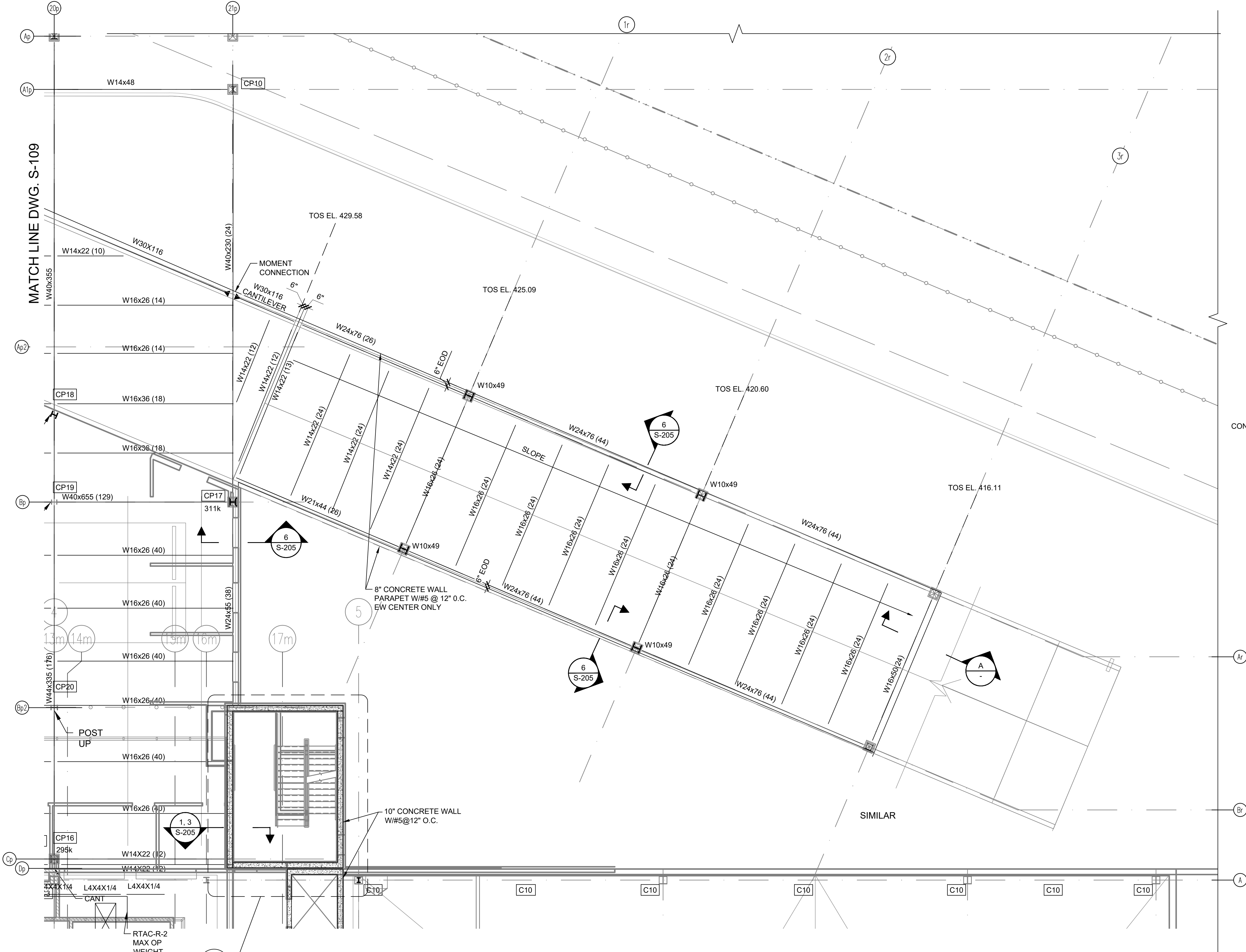
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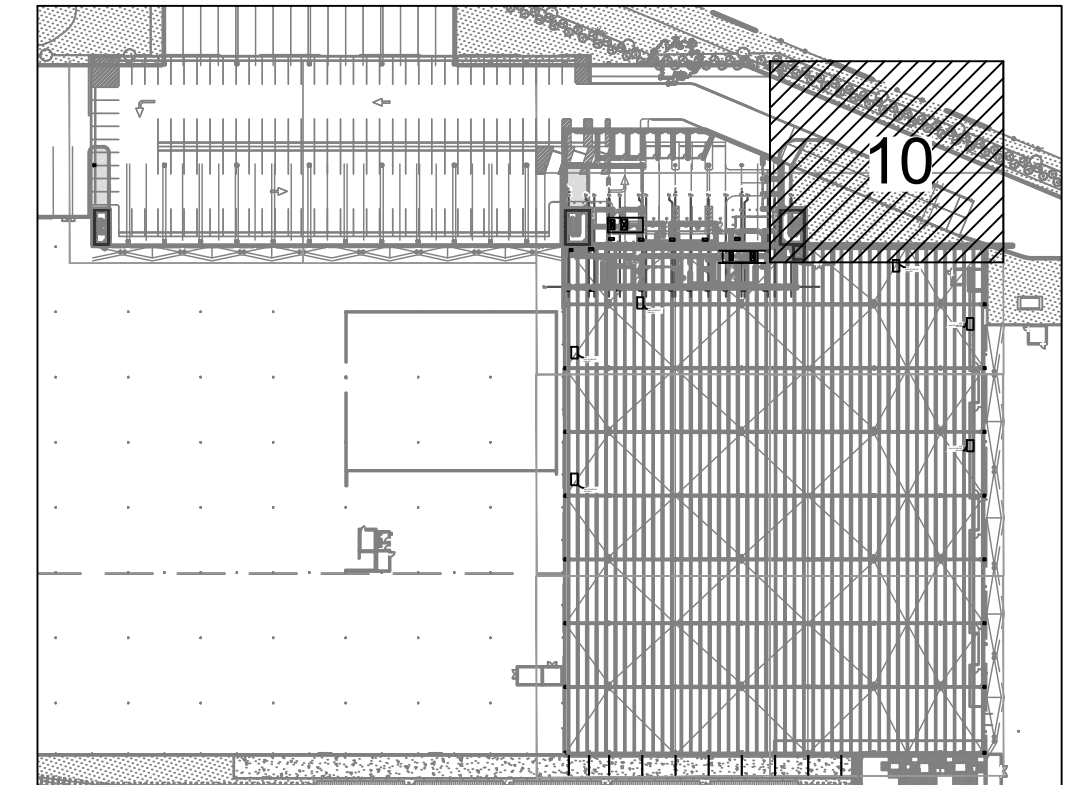
PARKING FRAMING
PART PLAN

DWG NUMBER :
S-109
14 OF 60

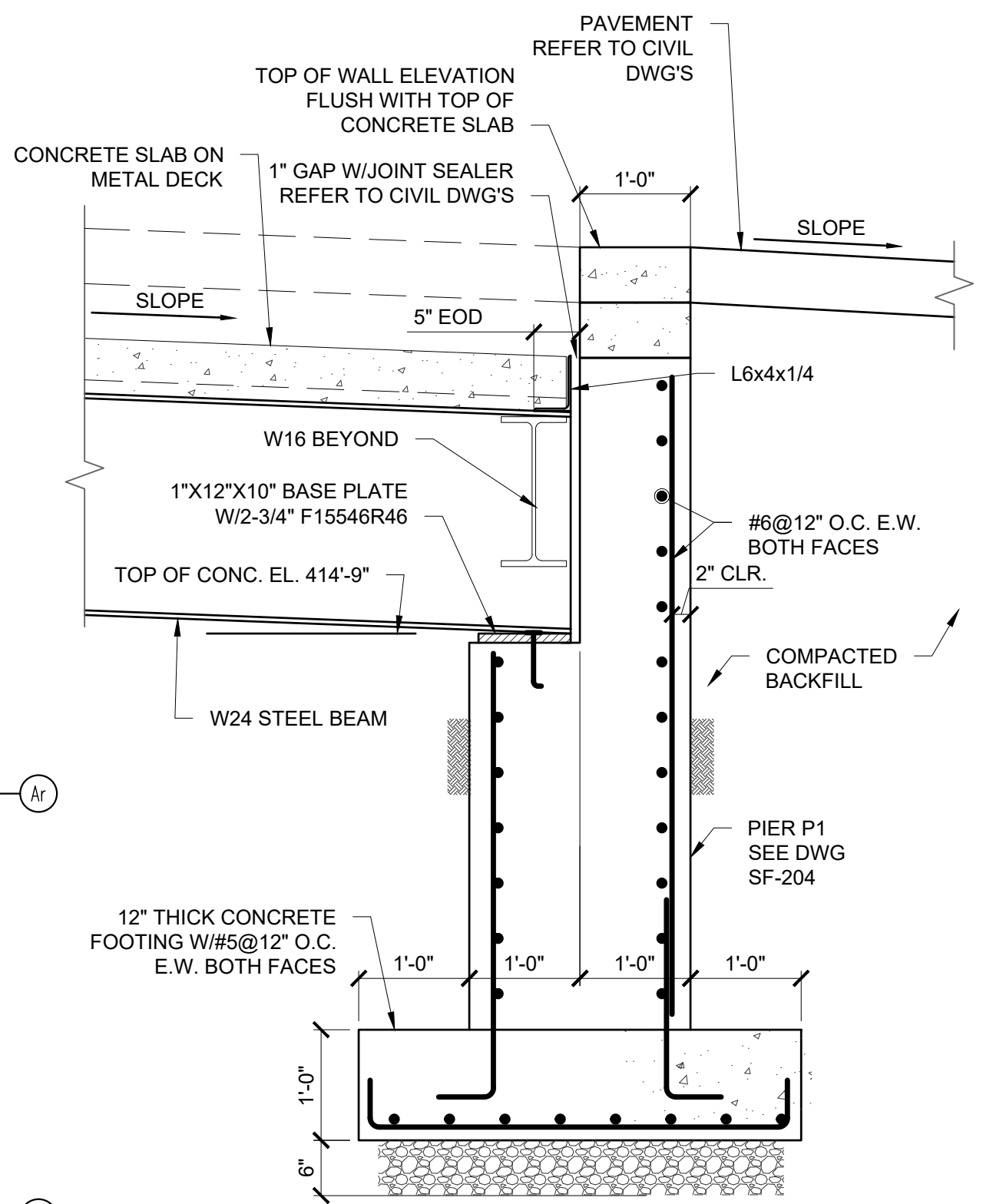


MATCH LINE DWG. S-102
10 PART PLAN
SCALE: 1/8" = 1'-0"

TOS 429.58 UNO.
REFER TO PLAN FOR
RAMP ELEVATIONS



KEY PLAN
SCALE: N.T.S.



A BUTTRESS SECTION
SCALE: 3/4" = 1'-0"

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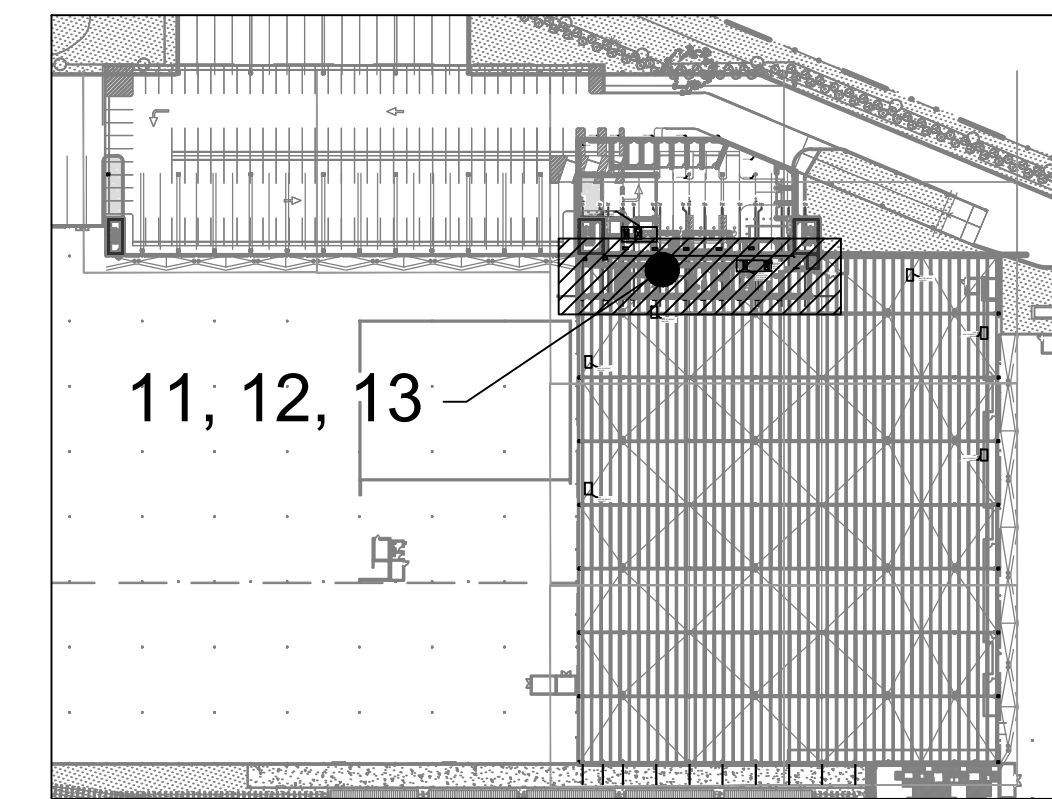
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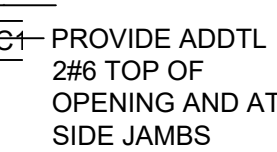
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PARKING FRAMING
PART PLAN

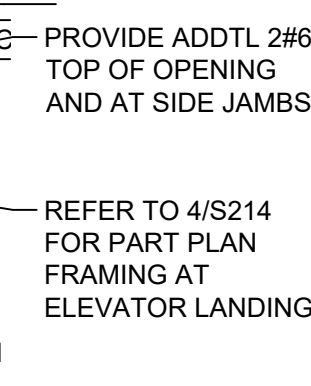
DWG NUMBER:
S-110
15 OF 60



11 MEZZANINE FRAMING PLAN
SCALE: 1/8" = 1'-0"
TOS 421.00'



12 UPPER MEZZANINE FRAMING PLAN
SCALE: 1/8" = 1'-0"
TOS 430.75'



13 OP BUILDING ROOF PLAN
SCALE: 1/8" = 1'-0"
TOS 441.95'

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DATE :	9/10/2021
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SCALE : AS SHOWN

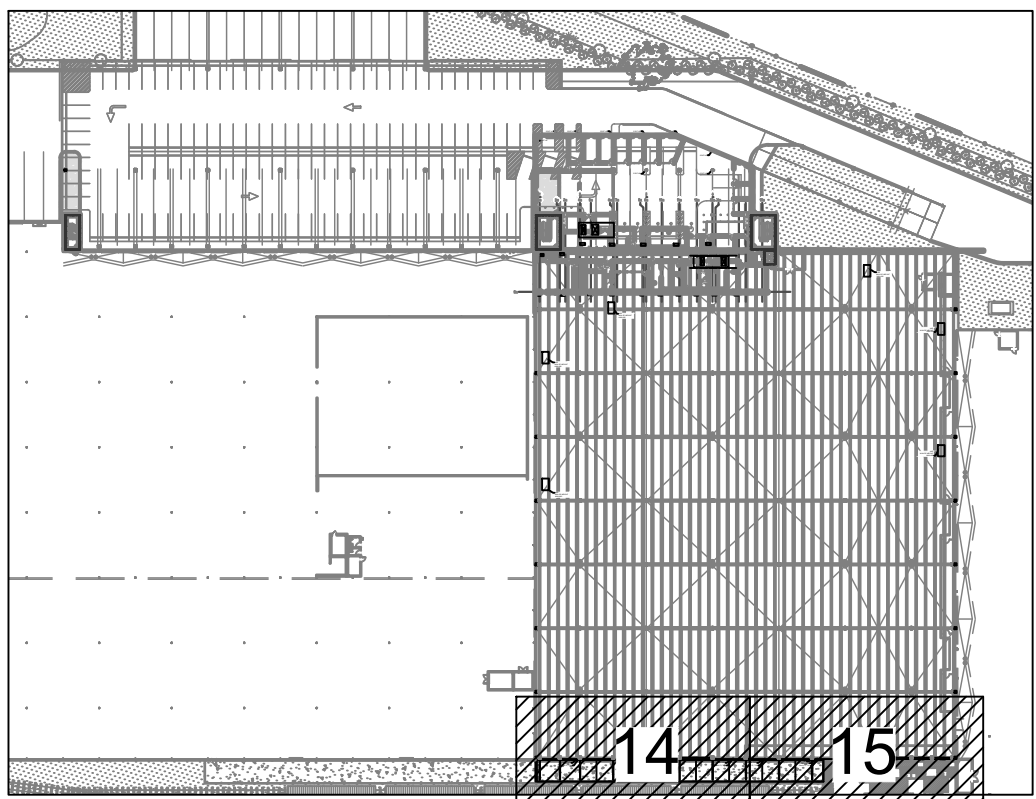
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MEZZANINE
FRAMING PLAN

DWG NUMBER :

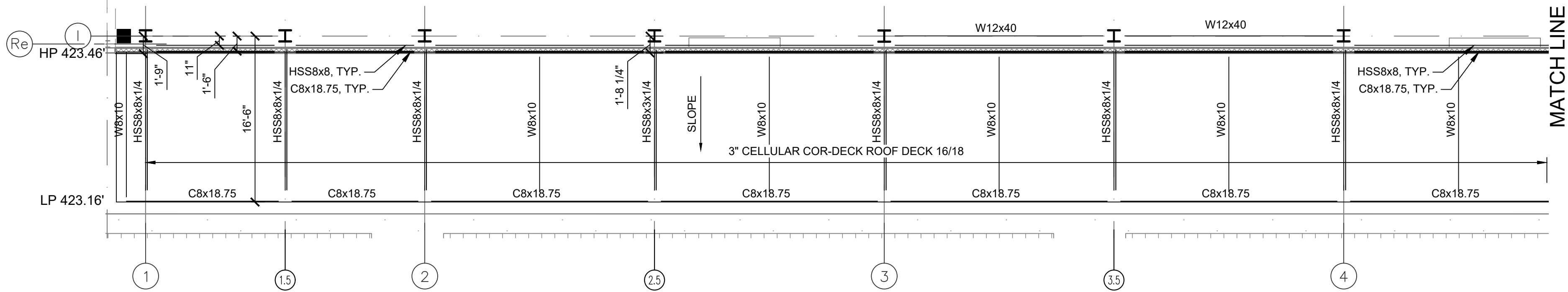
S-111

16 OF 60

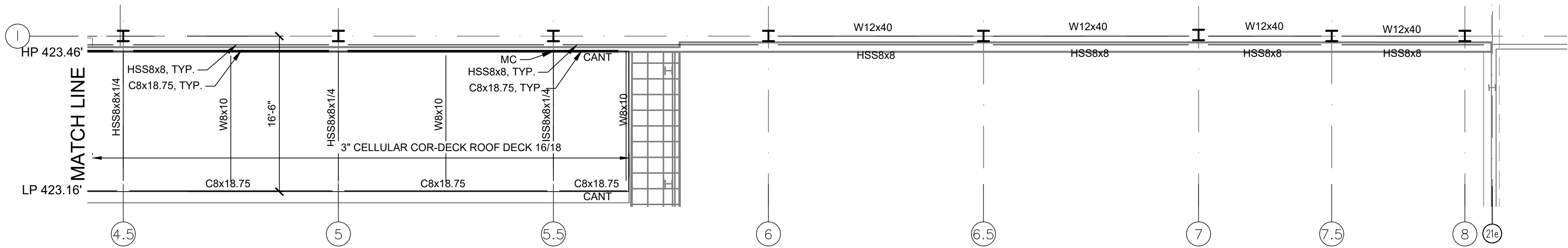


KEY PLAN
SCALE: N.T.S.

- LEGEND
- TOS - DENOTES TOP OF STEEL ELEVATION
 - HP - DENOTES TOP OF STEEL HIGH POINT ELEVATION
 - LP - DENOTES TOP OF STEEL LOW POINT ELEVATION
 - C1 - COLUMN #



14 CANOPY FRAMING
SCALE: 1/8" = 1'-0"



15 CANOPY FRAMING
SCALE: 1/8" = 1'-0"

TOS REFER TO PLAN

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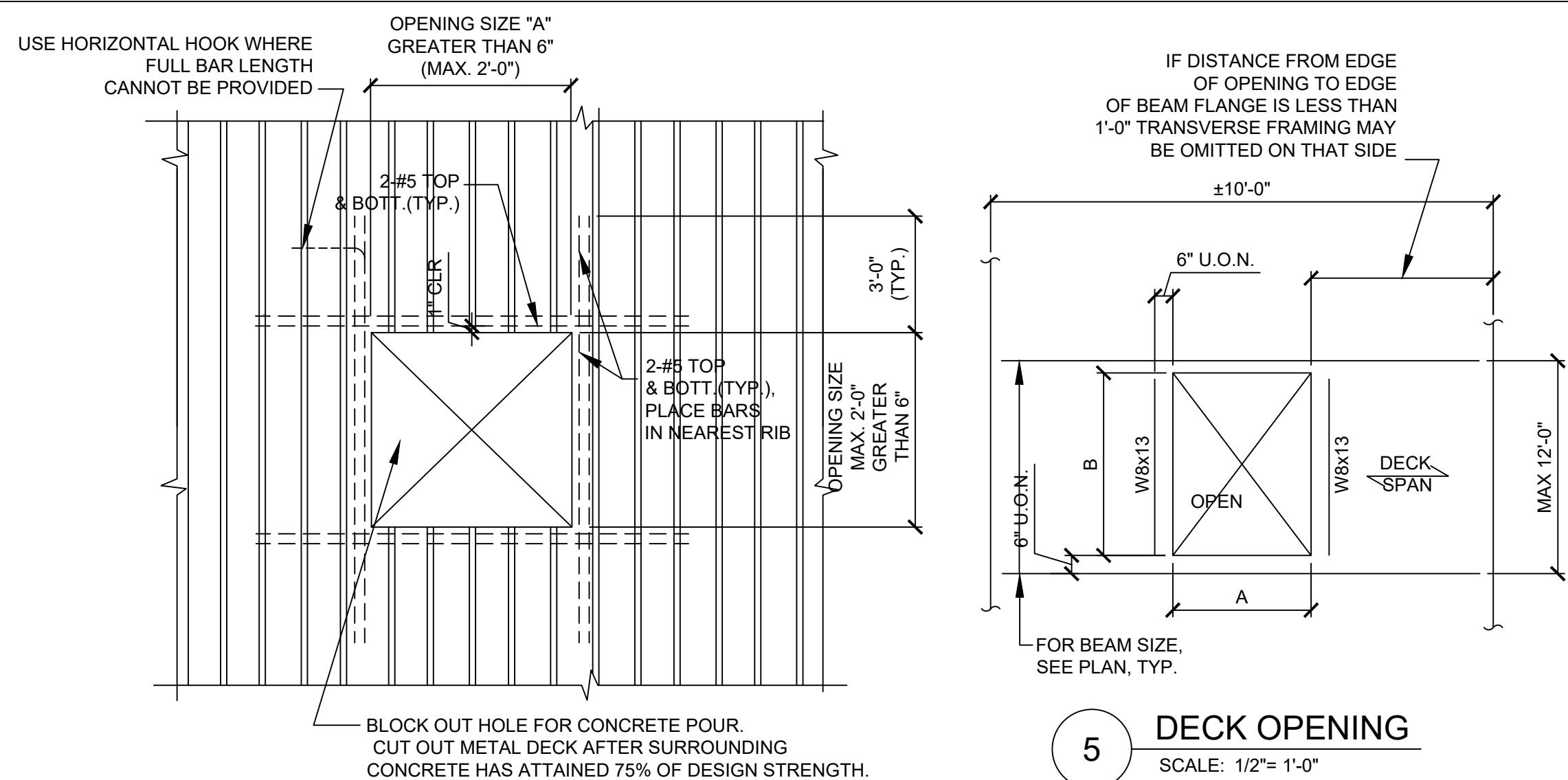
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DRAWING TITLE :

CANOPY FRAMING

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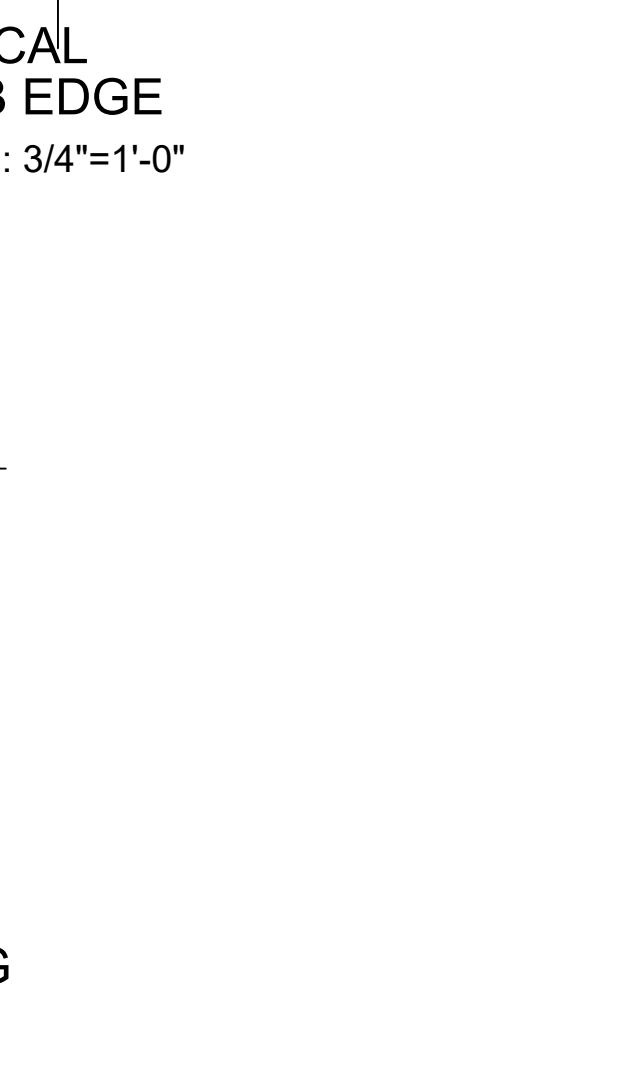
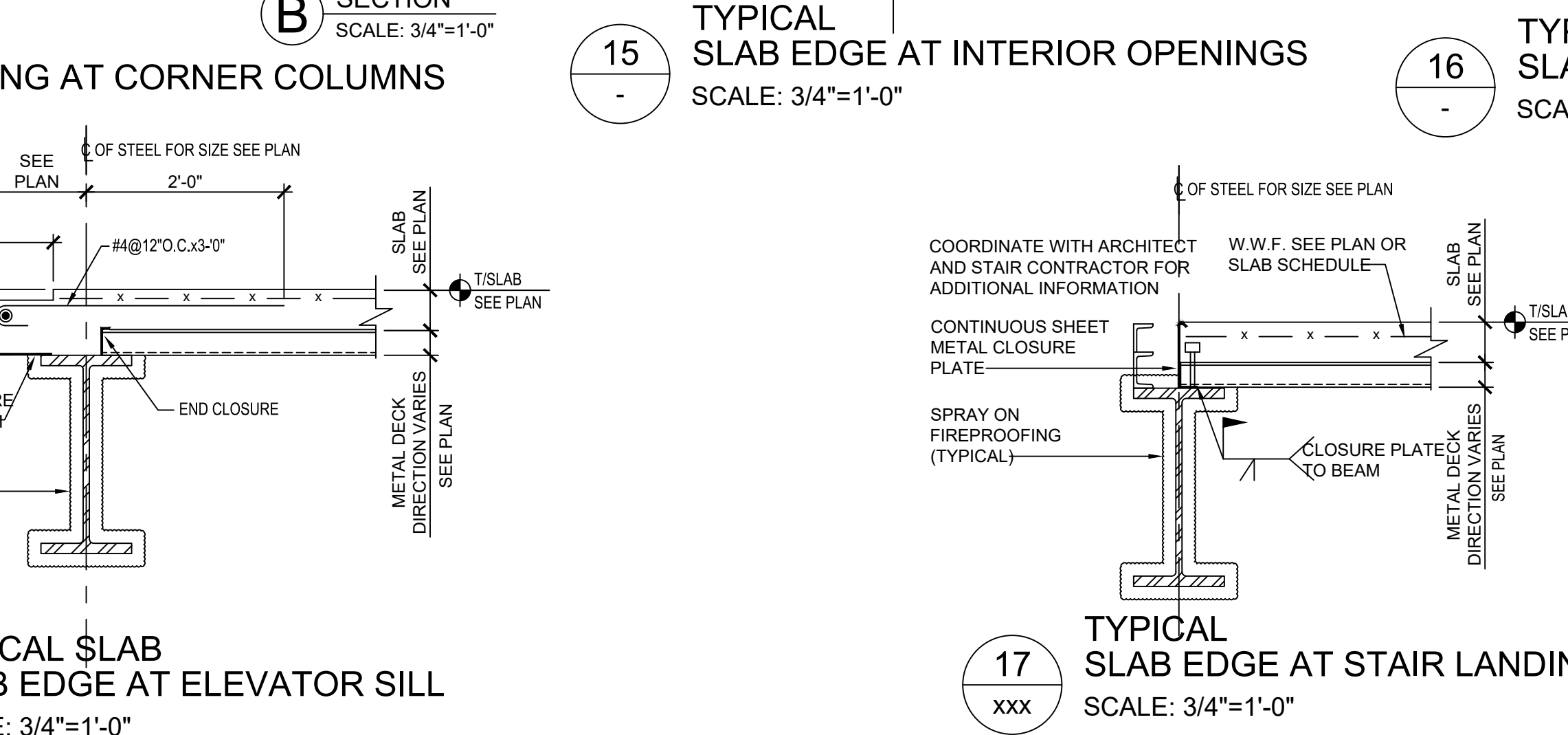
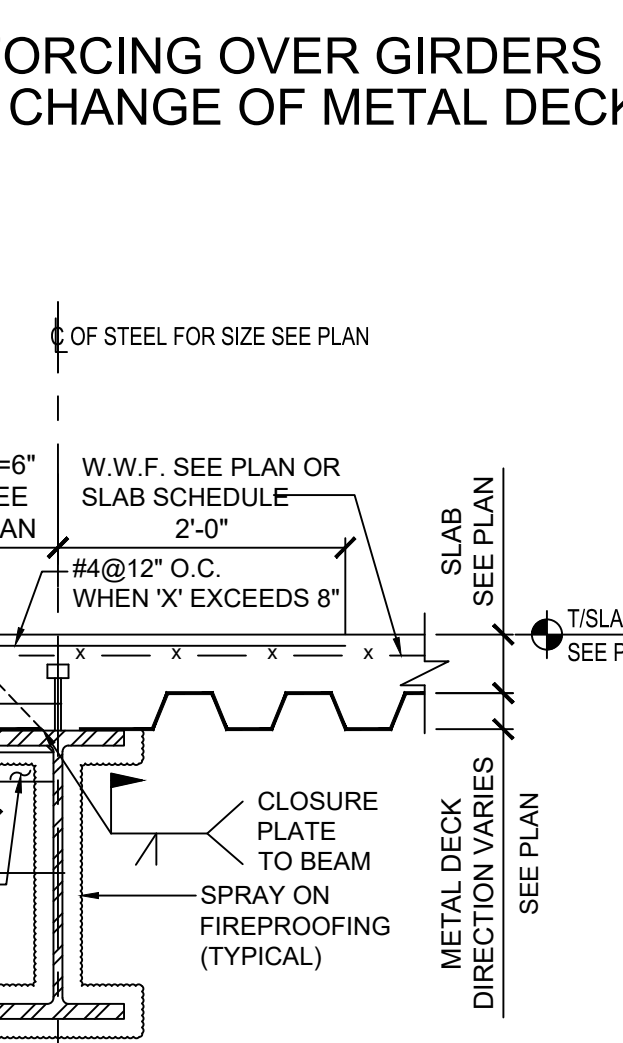
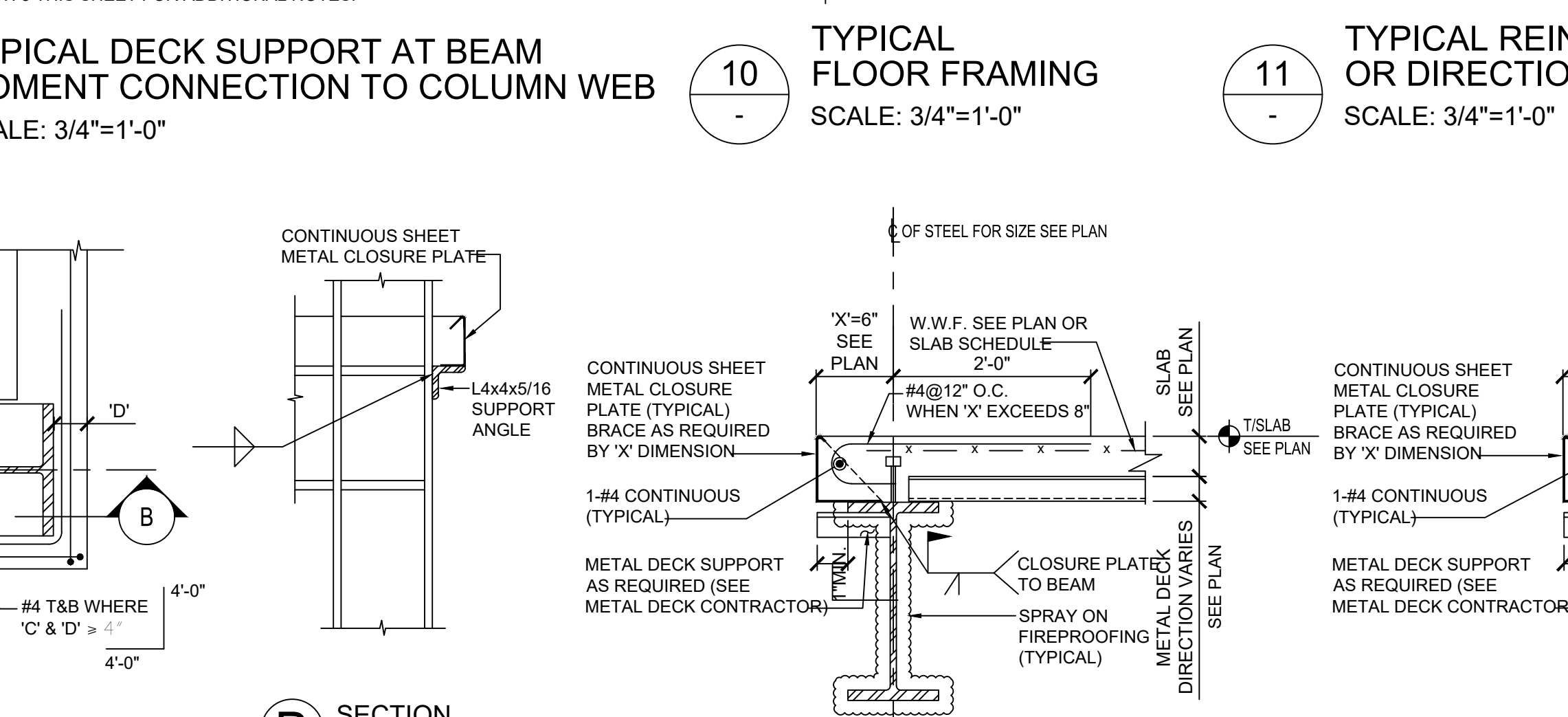
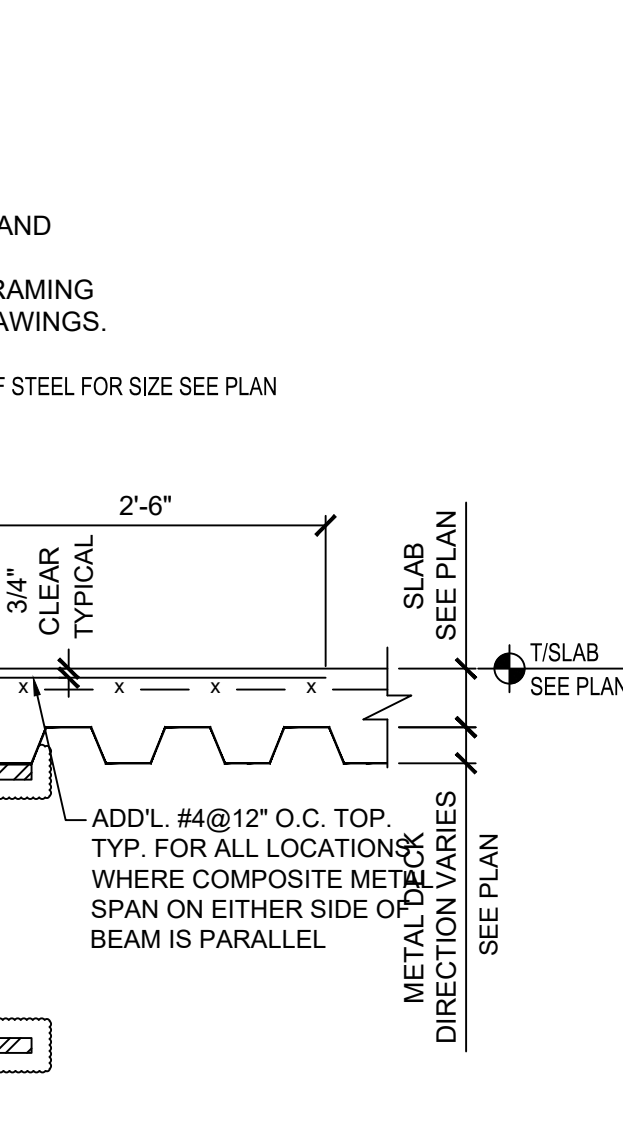
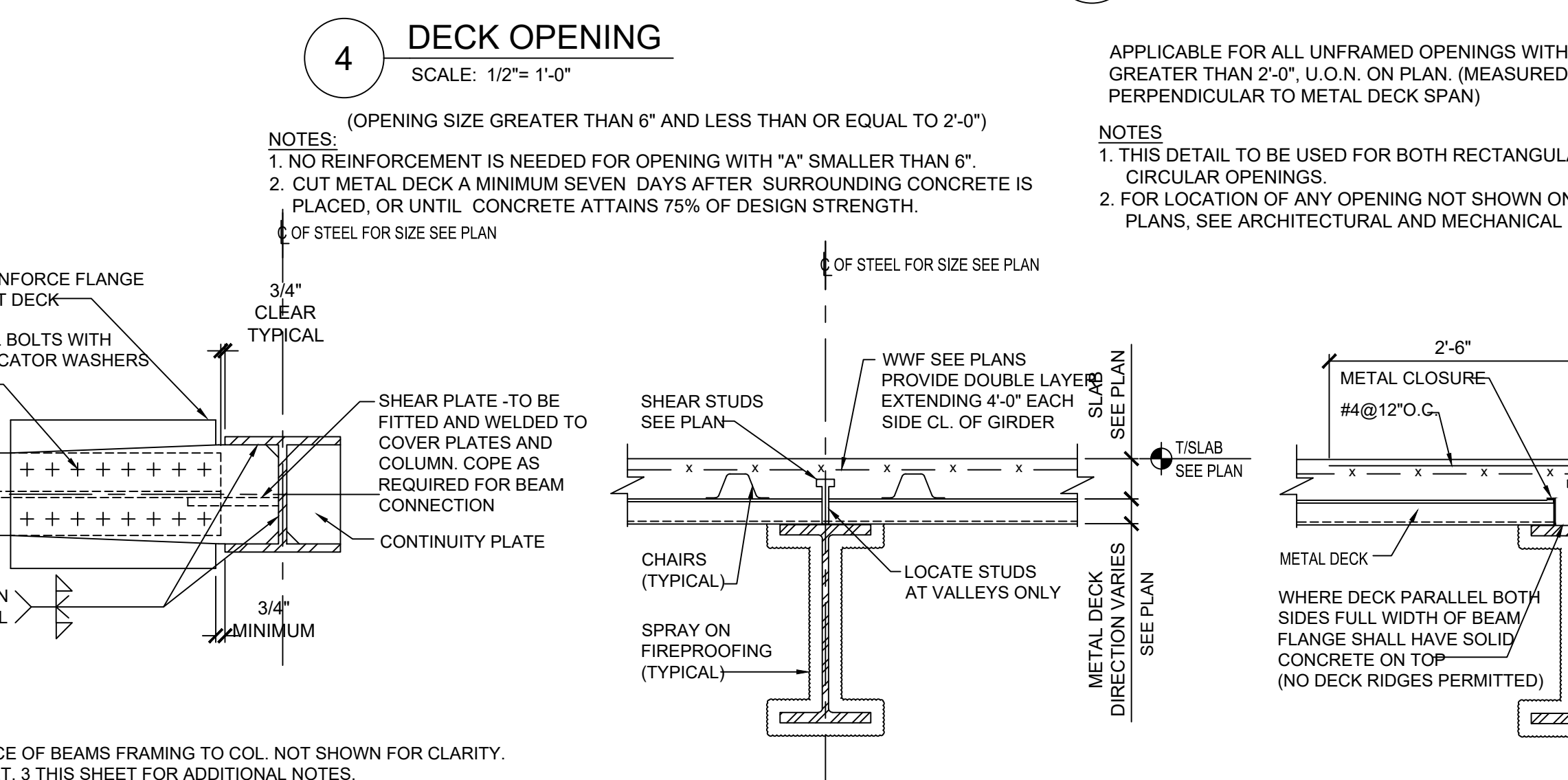
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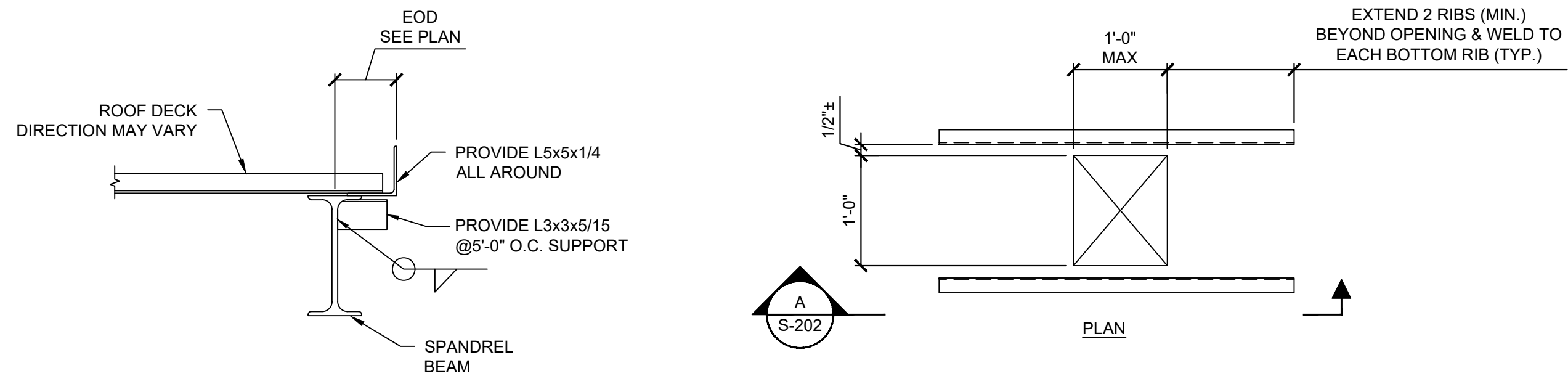
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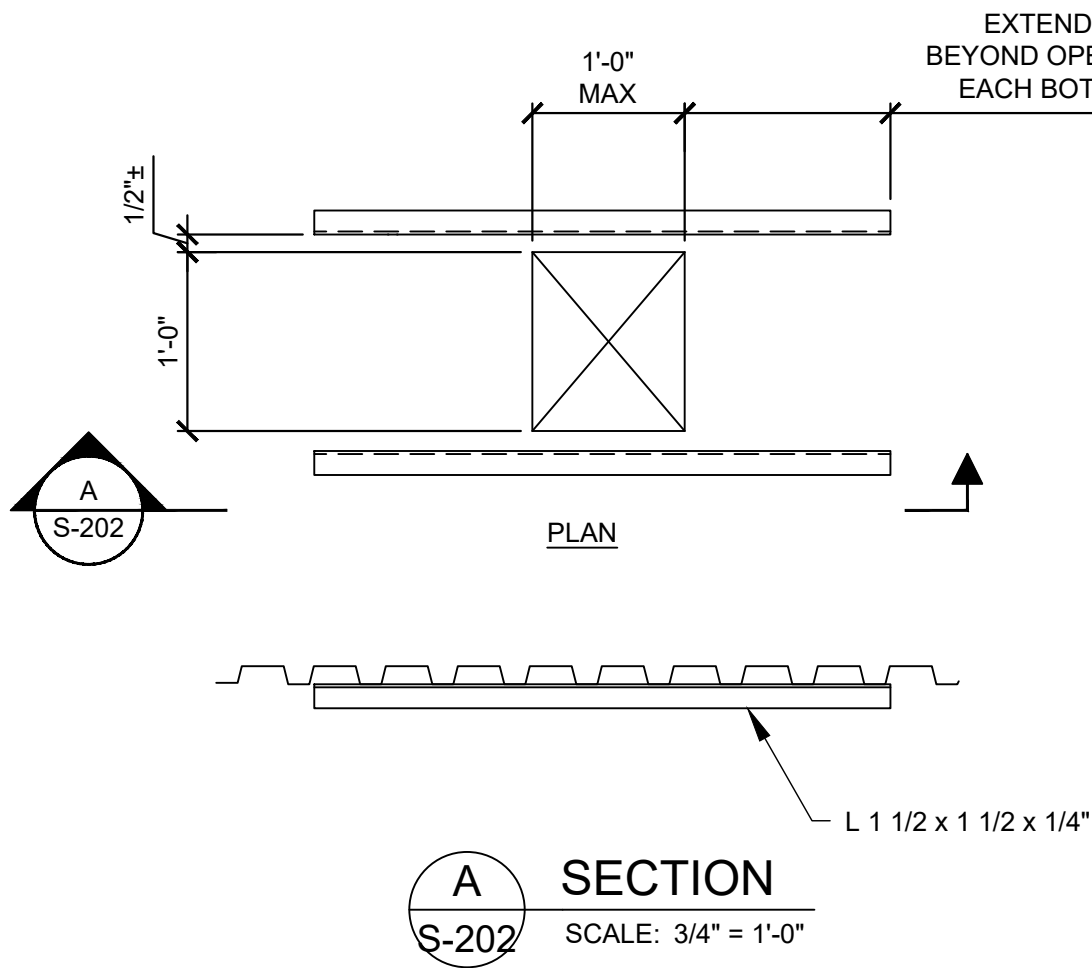
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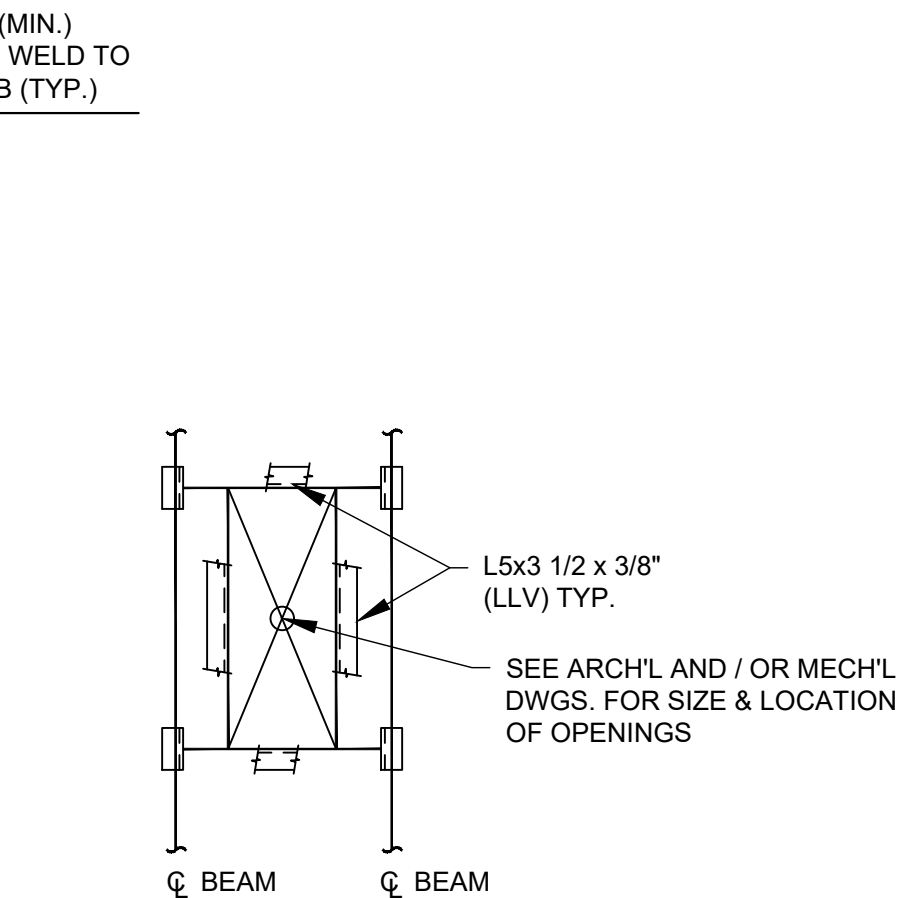
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DATE : 9/10/2021		
SCALE : AS SHOWN		
DRAWING TITLE :		
METAL DECK DETAILS		
DWG NUMBER :	S-202	
		21 OF 60



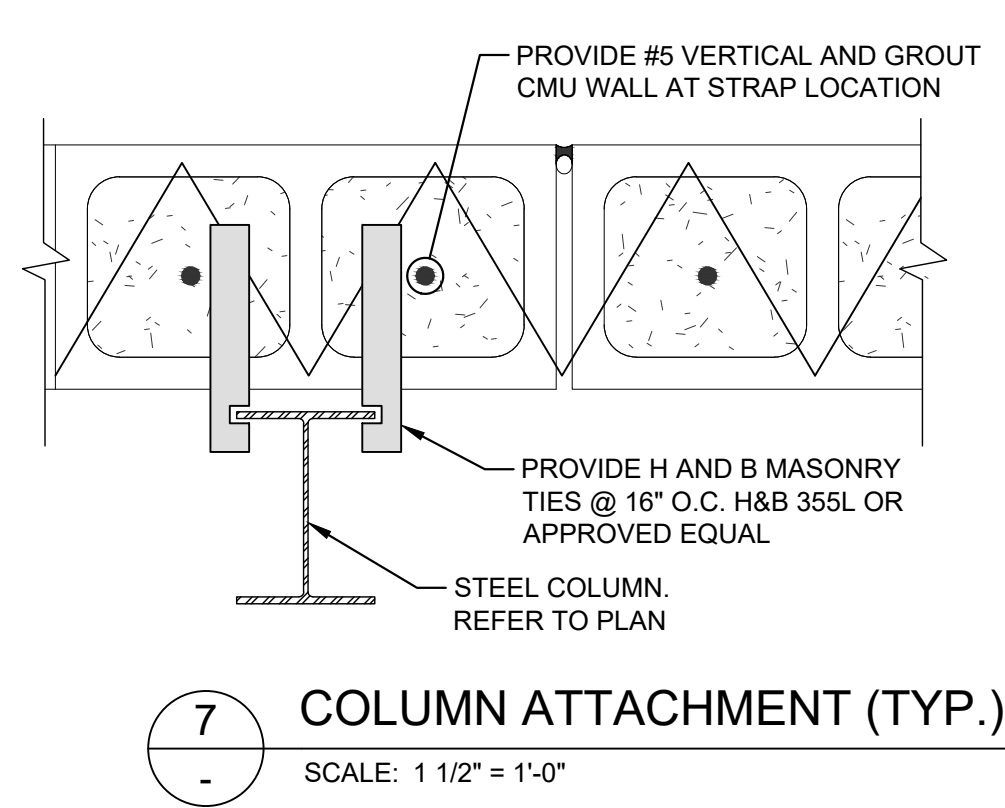
1A TYP. PERIMETER BEAM DETAIL @ ROOF
SCALE: 1" = 1'-0"



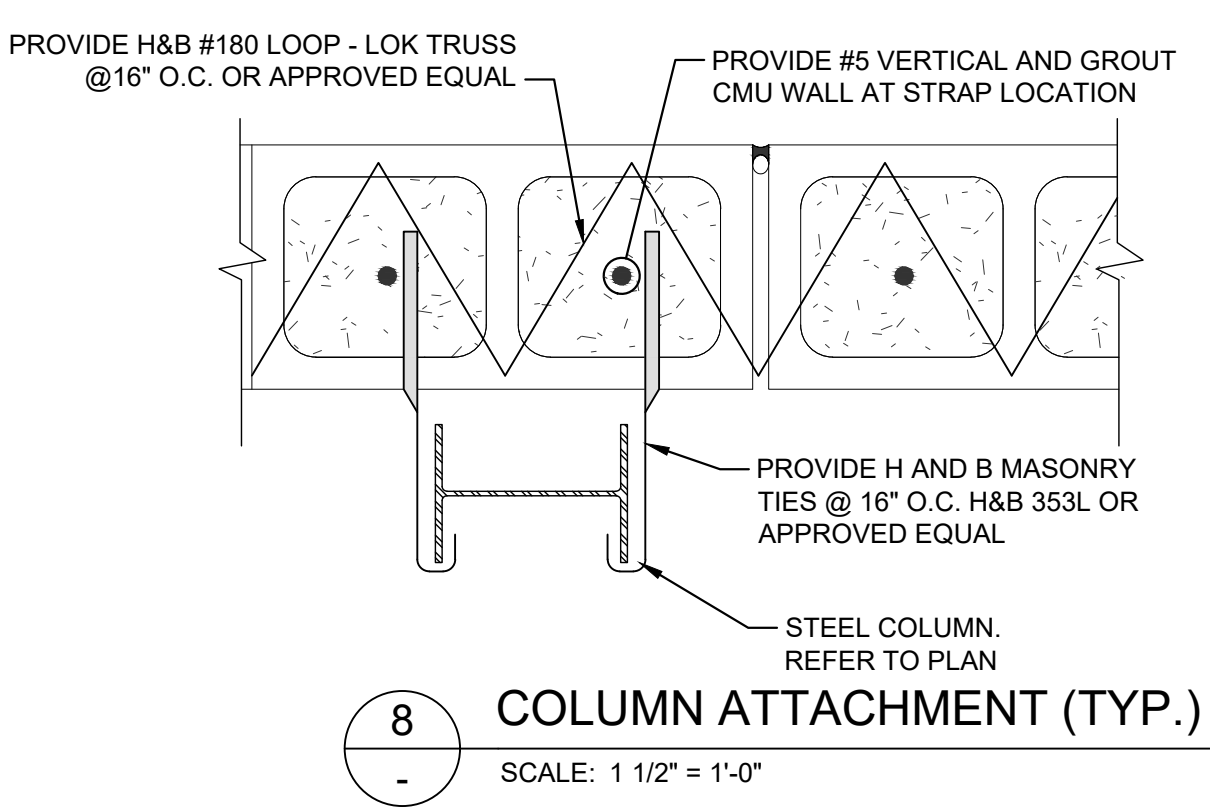
ROOF DECK PENETRATION AT SMALL OPENINGS
SCALE: 3/4" = 1'-0"



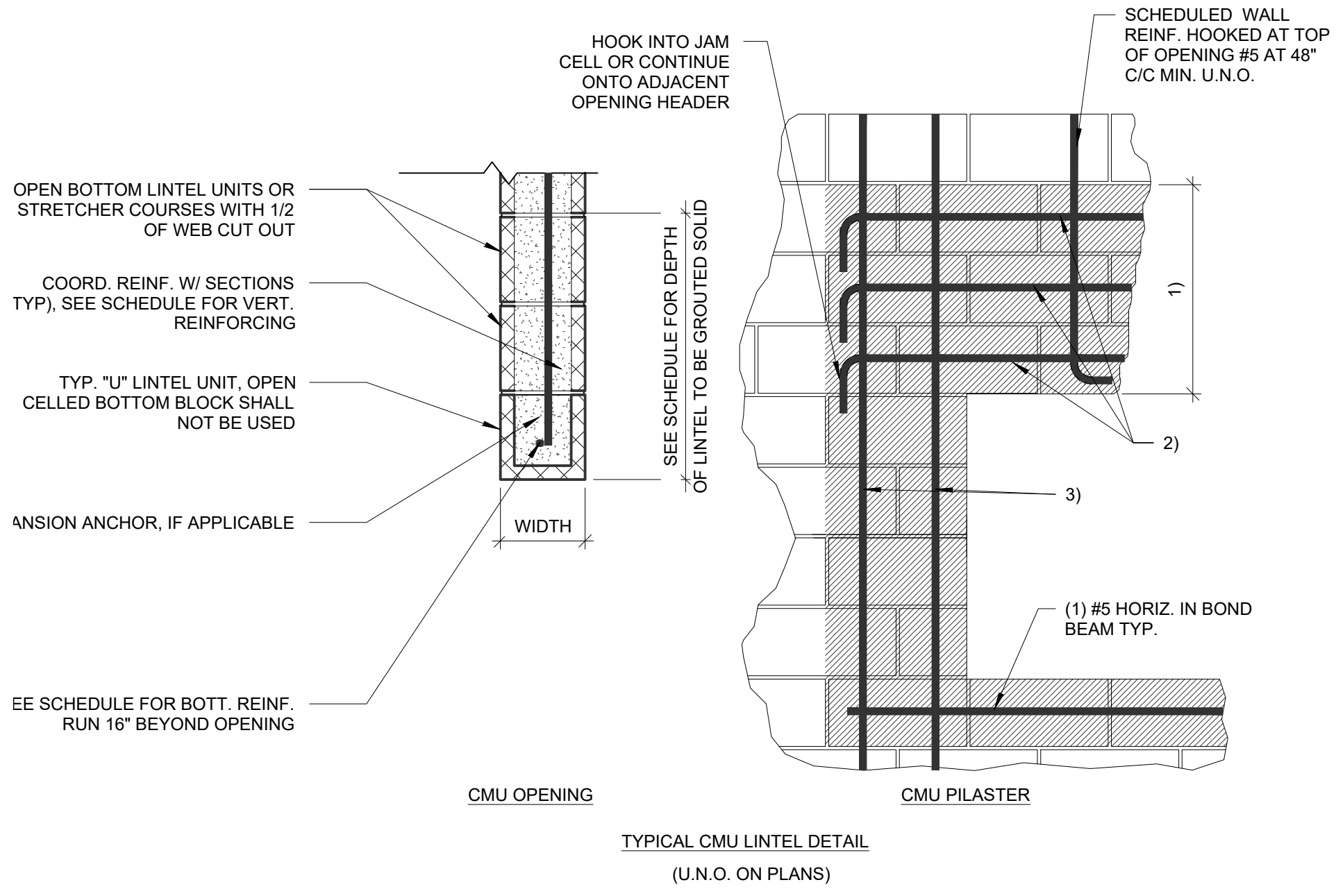
TYP. FRAMING AT ROOF OPENINGS
SCALE: 3/4" = 1'-0"



7 COLUMN ATTACHMENT (TYP.)
SCALE: 1 1/2" = 1'-0"



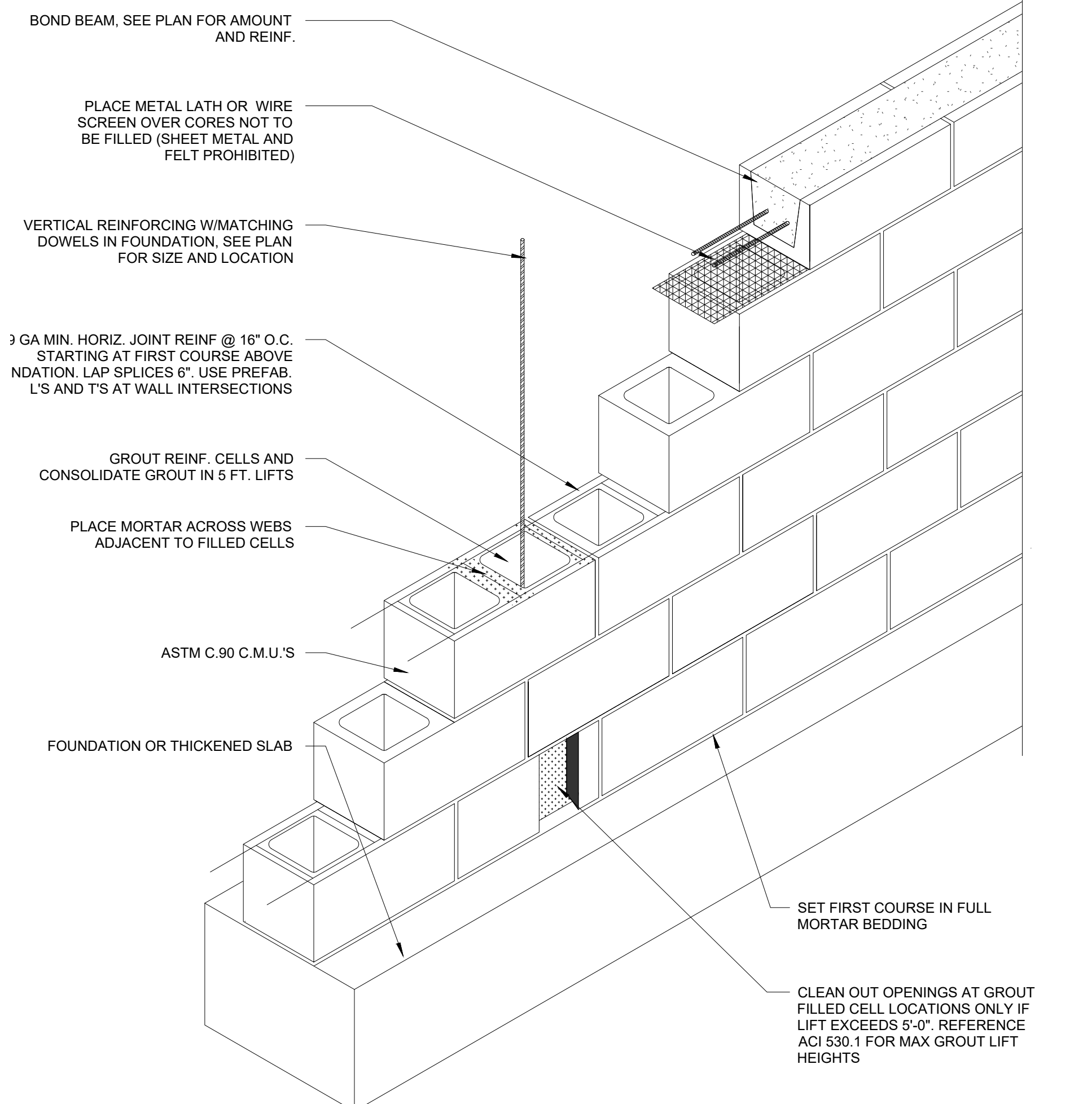
8 COLUMN ATTACHMENT (TYP.)
SCALE: 1 1/2" = 1'-0"



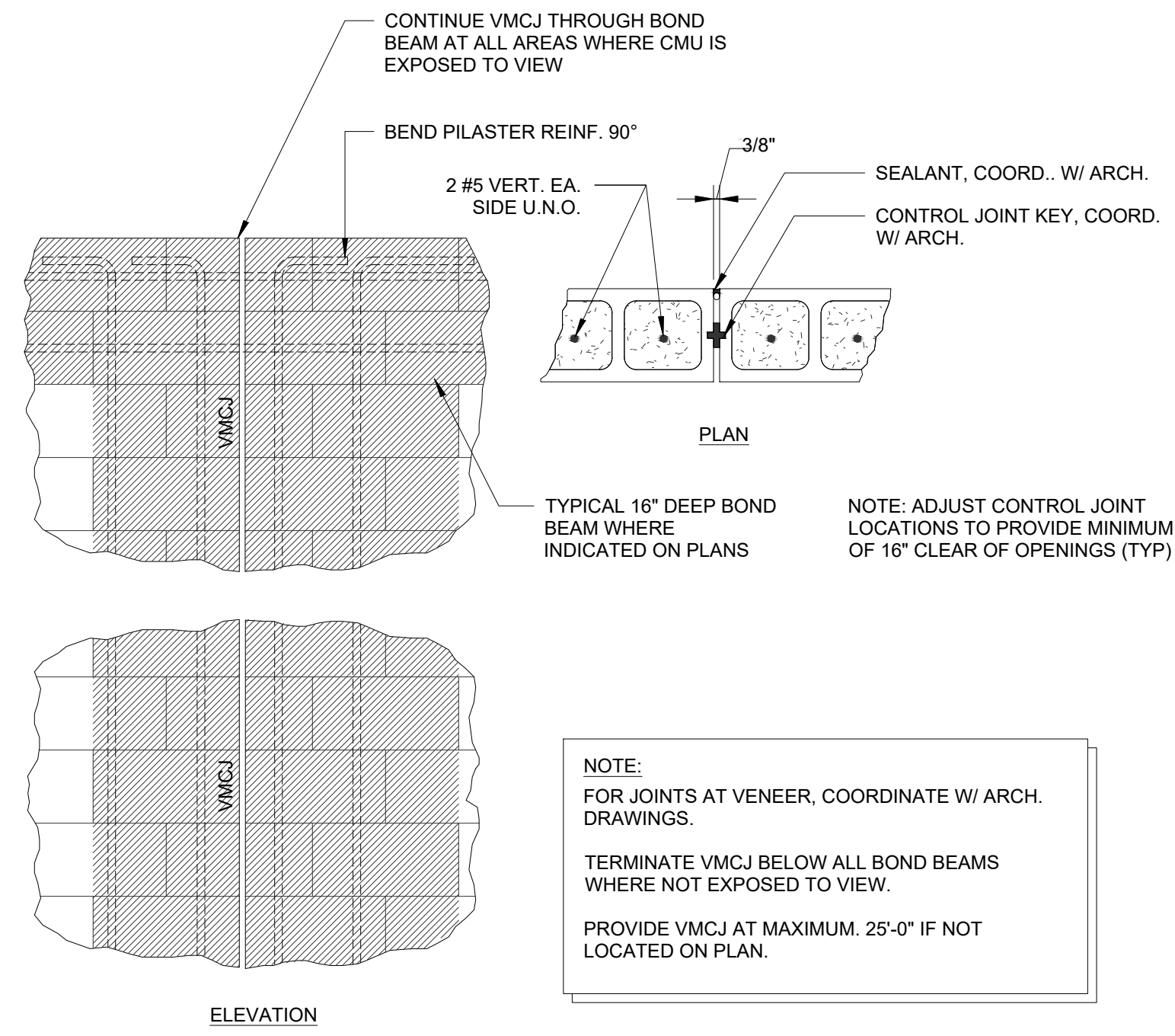
3 CMU LINTEL SCHEDULE
1" = 1'-0"

CMU LINTEL/JAMB SCHEDULE					
MAX OPENING SPAN	LOCATION	WIDTH	1) MIN. DEPTH OF FULL GROUTED CELLS	2) HORIZONTAL BOTT. REINF. U.N.O. (1 HORIZ.) PER COURSE	3) MIN. ADJACENT FULLY GROUTED CELLS AND REINF. (1 VERT.) PER CELL U.N.O.
≤ 4'-4"	TYP.	8"	8"	(1) #5 BOTT.	8" #5
4'-4" < OPNG. ≤ 6'-4"	TYP.	8"	16"	(1) #5 BOTT.	16" (2) #5
6'-4" < OPNG. ≤ 8'-4"	TYP.	8"	24"	(1) #5 BOTT.	16" (2) #6
8'-4" < OPNG. ≤ 10'-8"	TYP.	8"	32"	(2) #5 BOTT.	16" (2) #6

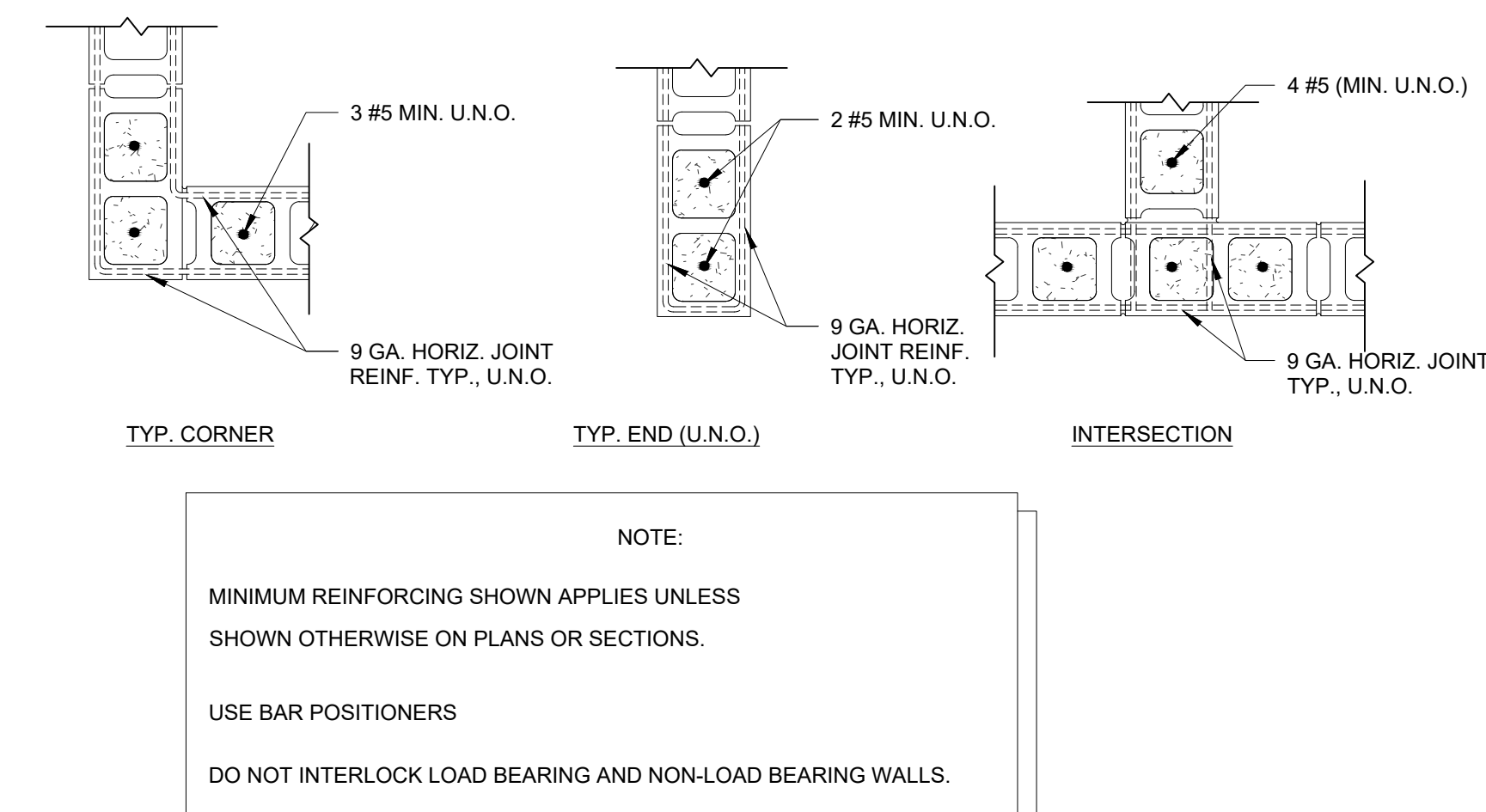
- NOTES:
- #6 VERT. SHALL START ABOVE FOOTING WITH A MINIMUM LAP LENGTH OF 30".
 - VERIFY ALL CMU OPENINGS W/ ARCHITECT DWG'S.
 - ALL CMU LINTEL BEAMS ABOVE BOTTOM COURSE SHALL BE CONSTRUCTED OF OPEN BOTTOM LINTEL UNITS, OR STRETCHER COURSES WITH 1/2 WEB CUT OUT.
 - COORDINATE W/ SECTIONS, DETAILS, & GENERAL NOTES.
 - PRE-CAST CONCRETE LINTEL MAY BE USED IN LIEU OF LINTEL BLOCK.
 - REFER TO PLANS FOR FULLY GROUTED AREAS.



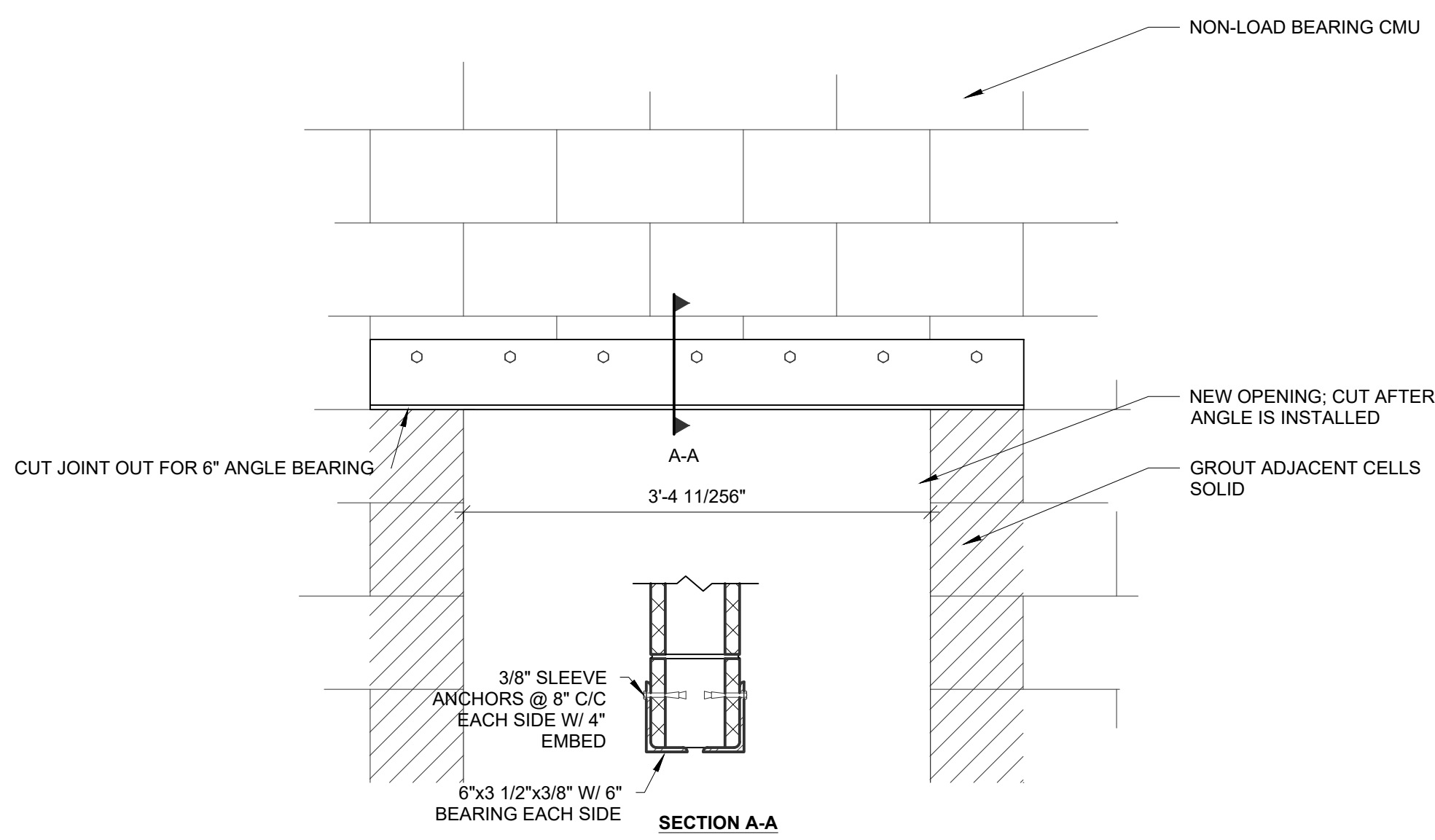
2 TYPICAL MASONRY WALL CONSTRUCTION
3/16" = 1'-0"



4 TYPICAL VMCJ DETAILS



5 TYPICAL CMU DETAILS



6 CMU OPENING

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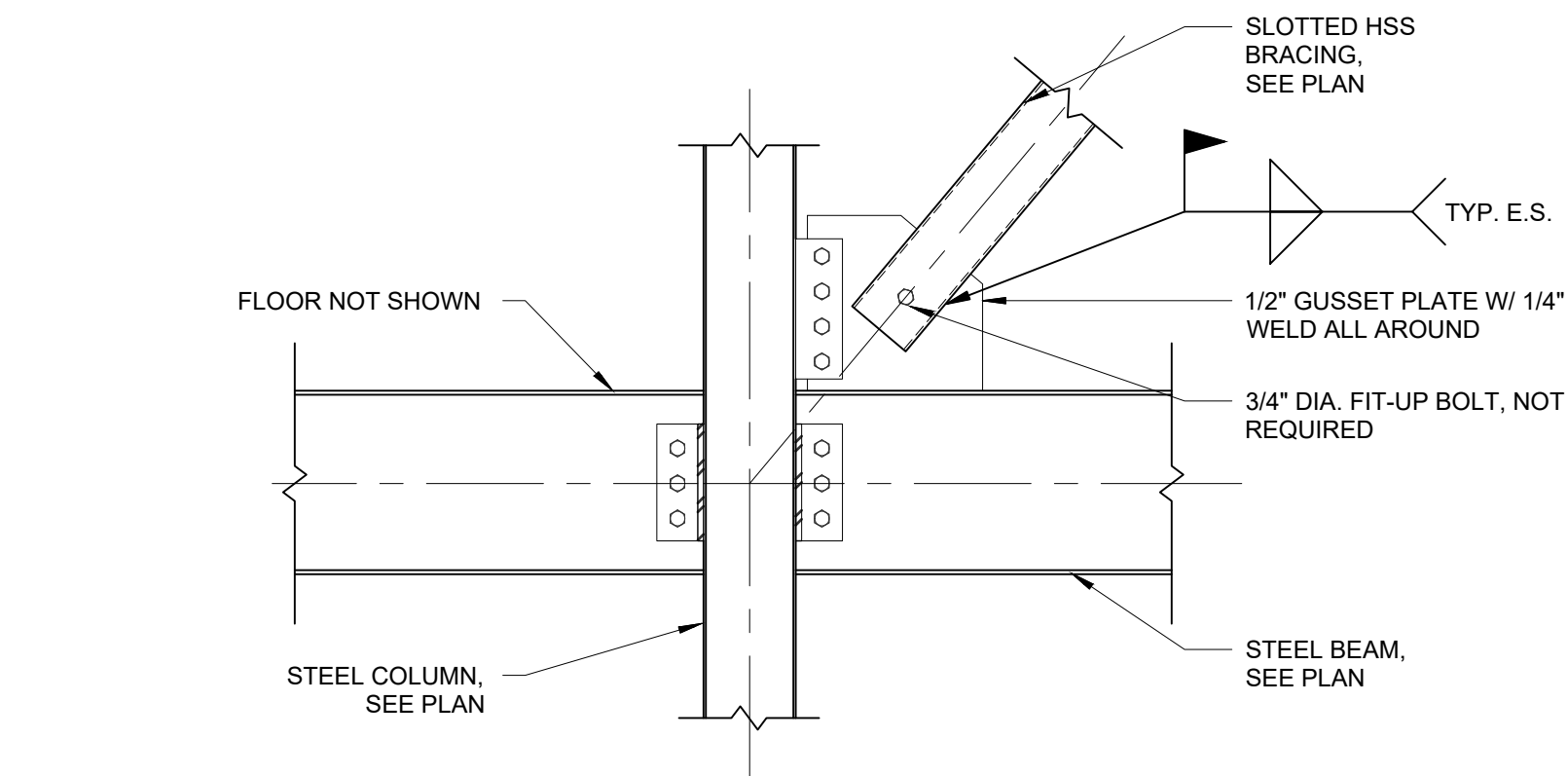


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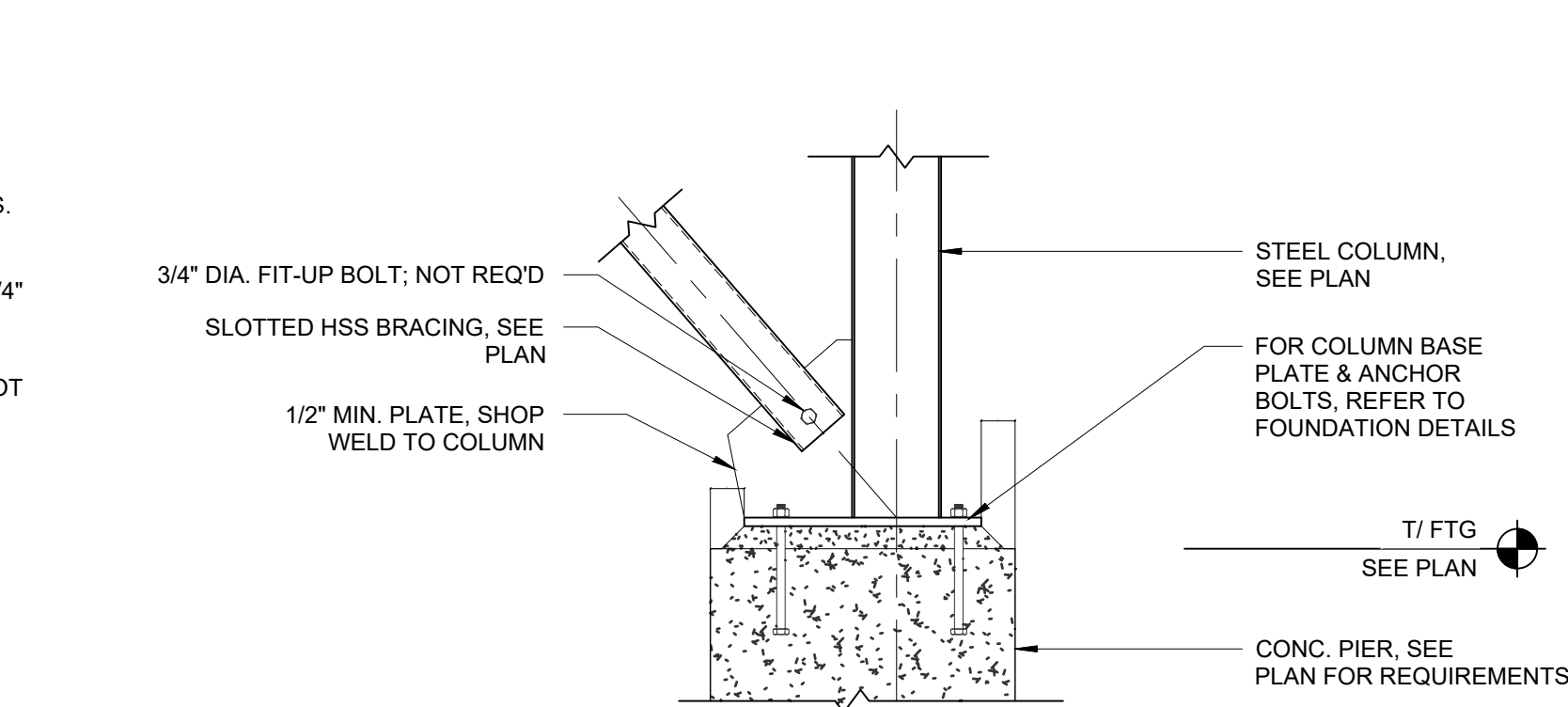
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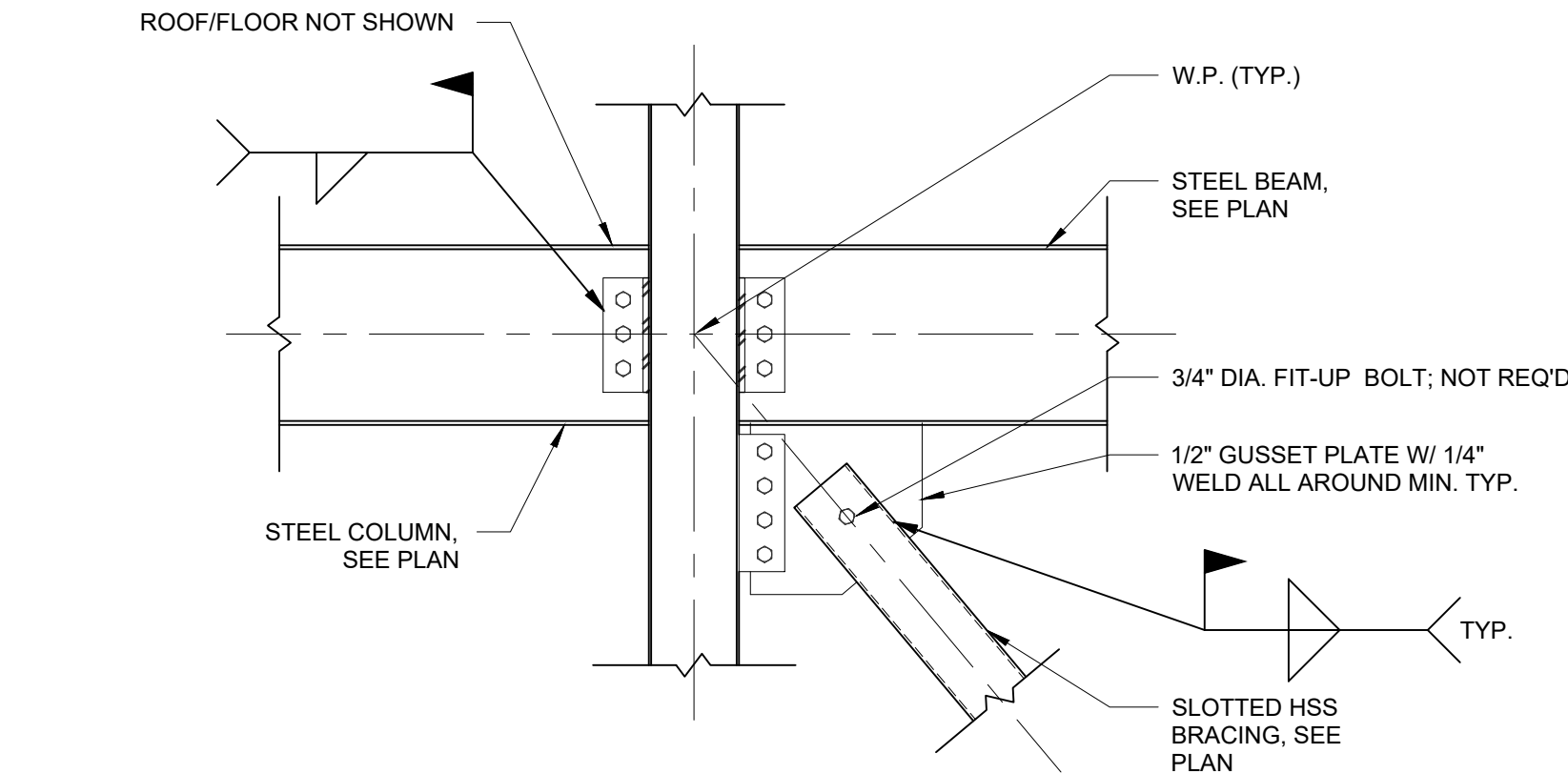
DRAWING TITLE :	
ROOF STRUCTURAL AND MASONRY DETAILS	
DWG NUMBER :	S-203
22 OF 60	



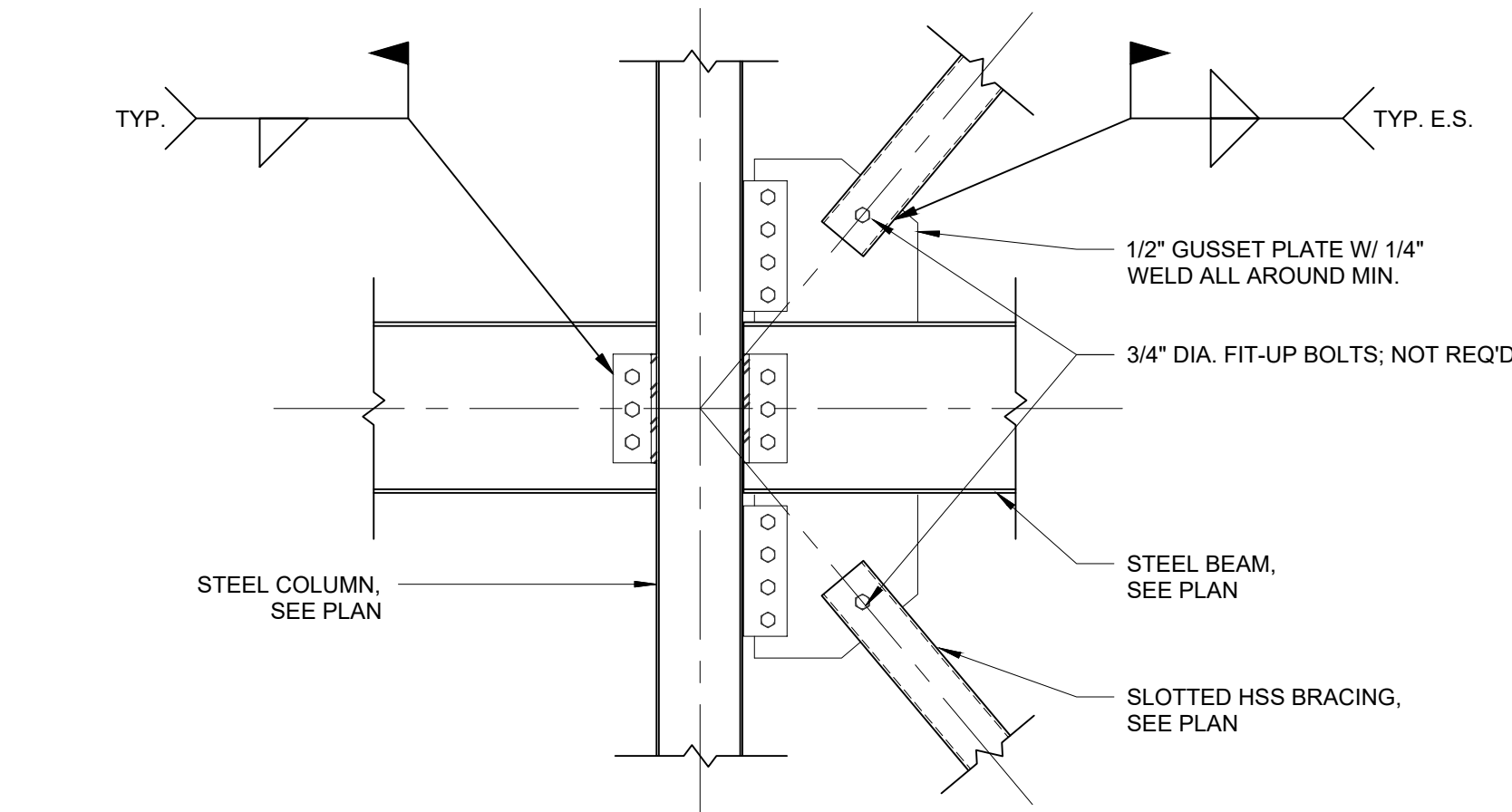
1
-
HSS BRACE ABOVE BEAM
3/4" = 1'-0"



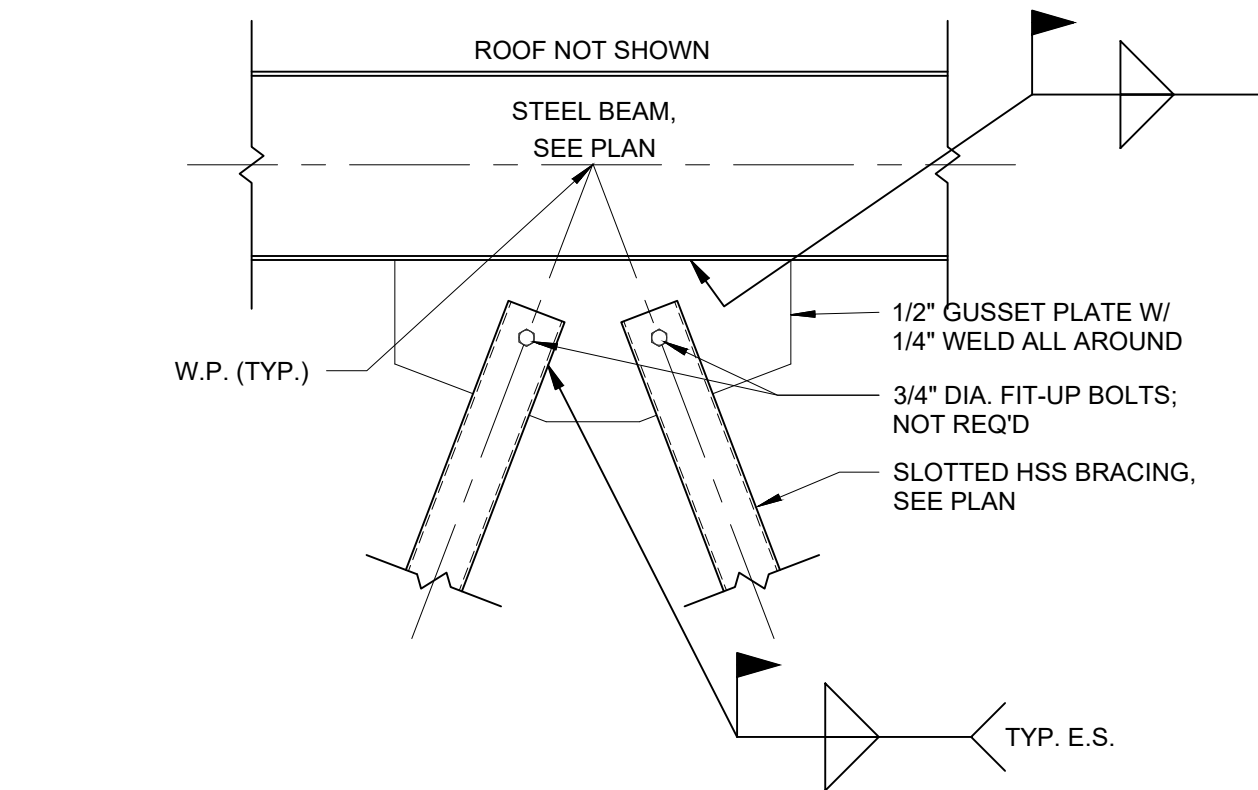
2
-
HSS BRACE AT BASE
3/4" = 1'-0"



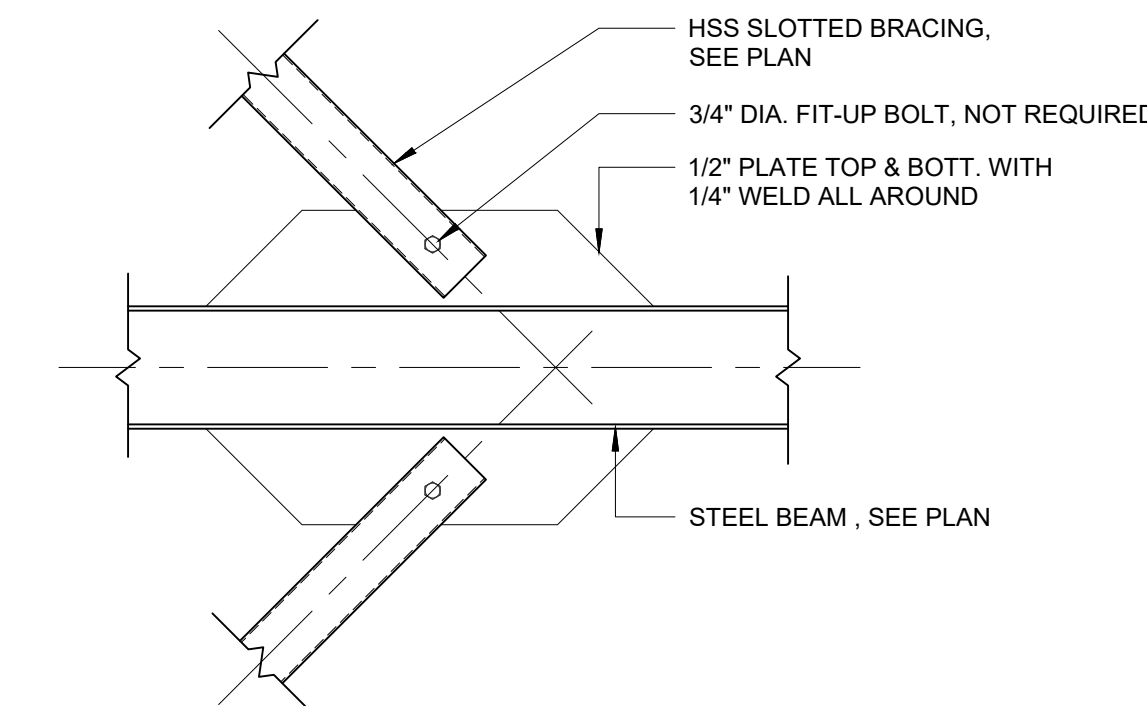
5
-
HSS BRACE BELOW BEAM
3/4" = 1'-0"



3
-
HSS BRACE AT BEAM END
3/4" = 1'-0"



4
-
HSS BRACE AT MID-SPAN
3/4" = 1'-0"



7
-
HSS BRACED FRAME AT MID-BEAM
3/4" = 1'-0"

ALL DIMENSIONS SHALL BE BE VERIFIED IN FIELD (V.I.F.).
COORDINATE LOCATION OF STEEL BEAMS TO MEET THE
EQUIPMENT SUPPORT REQUIREMENTS.

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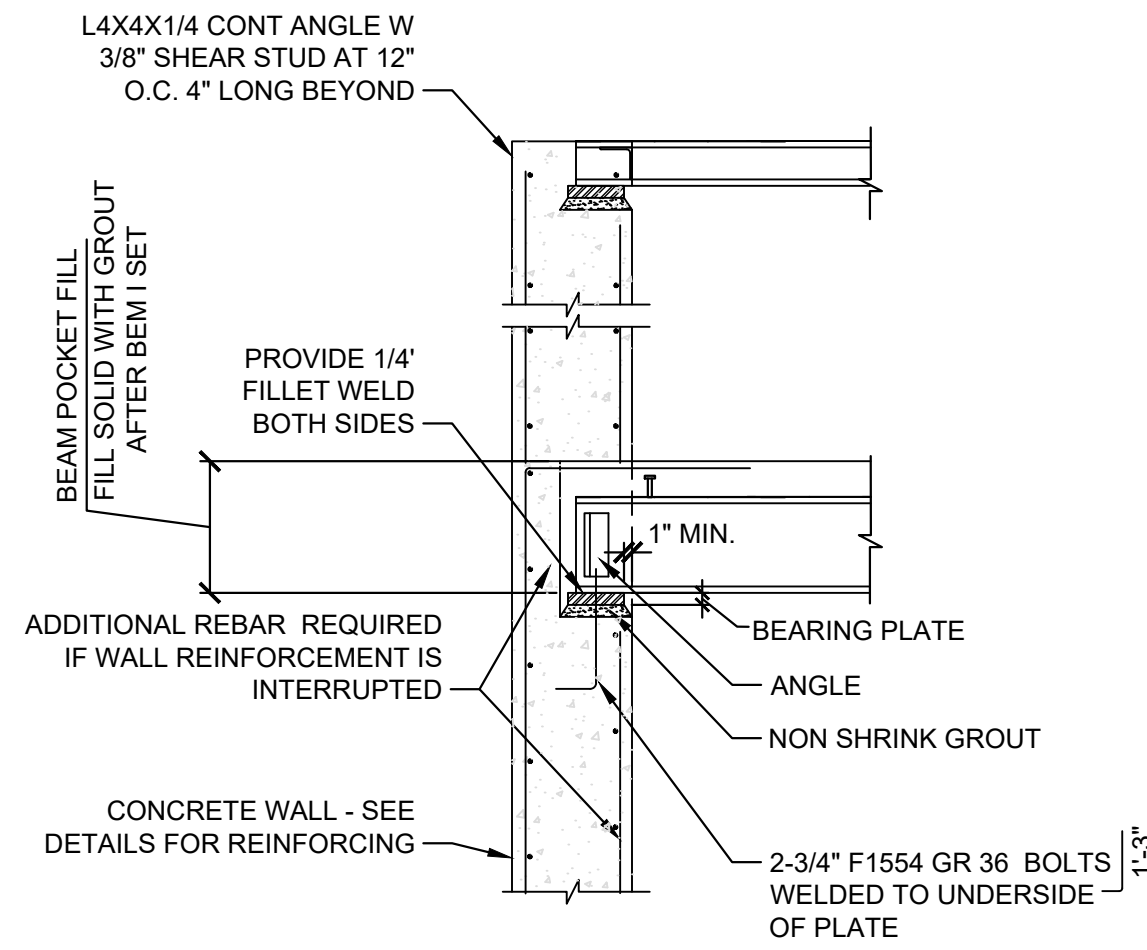
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BRACING DETAILS

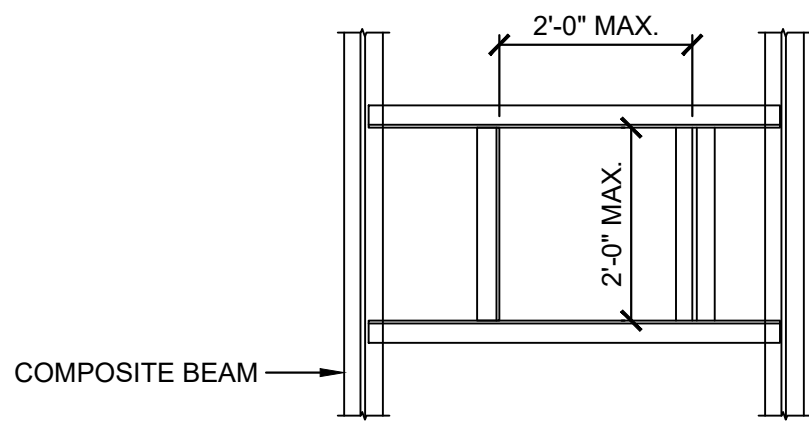
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S-204

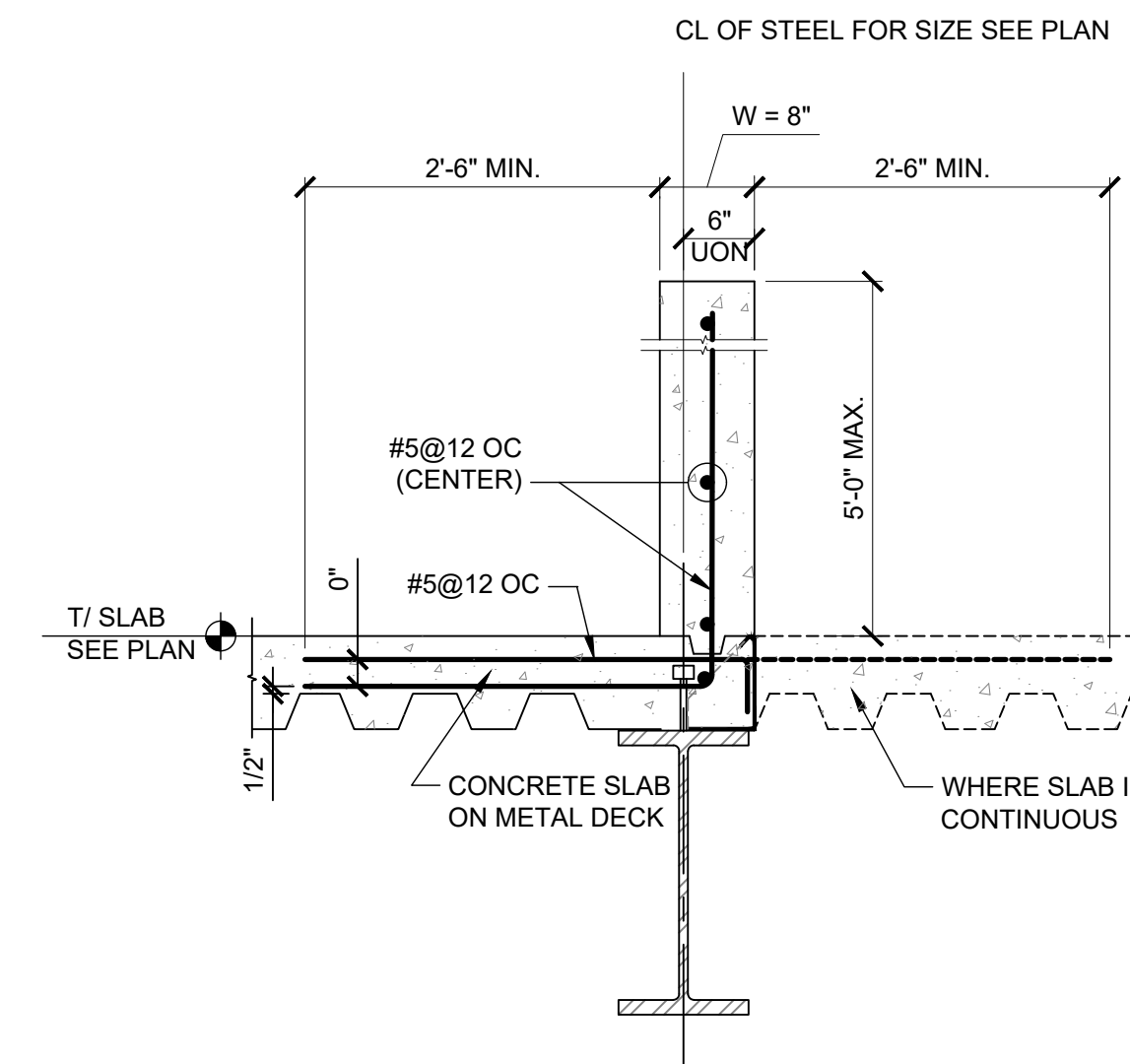
23 OF 60



1 WALL POCKET DETAIL
1/2" = 1'-0"

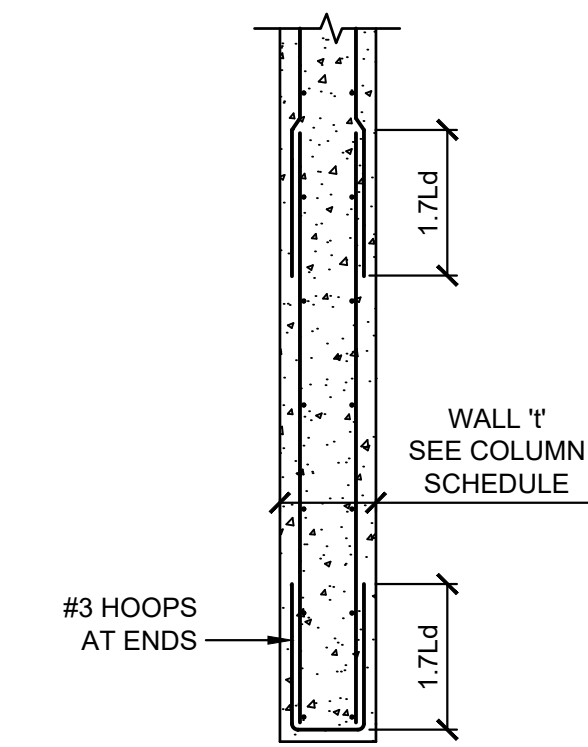


5 TYP. FRAMES AT OPENINGS
1/2" = 1'-0"



NOTES:
FOR CURB SIZE & LOCATION SEE
ARCHITECTURAL DRAWINGS

6 TYPICAL CONCRETE CURB
PARALLEL TO DECK
3/4" = 1'-0"

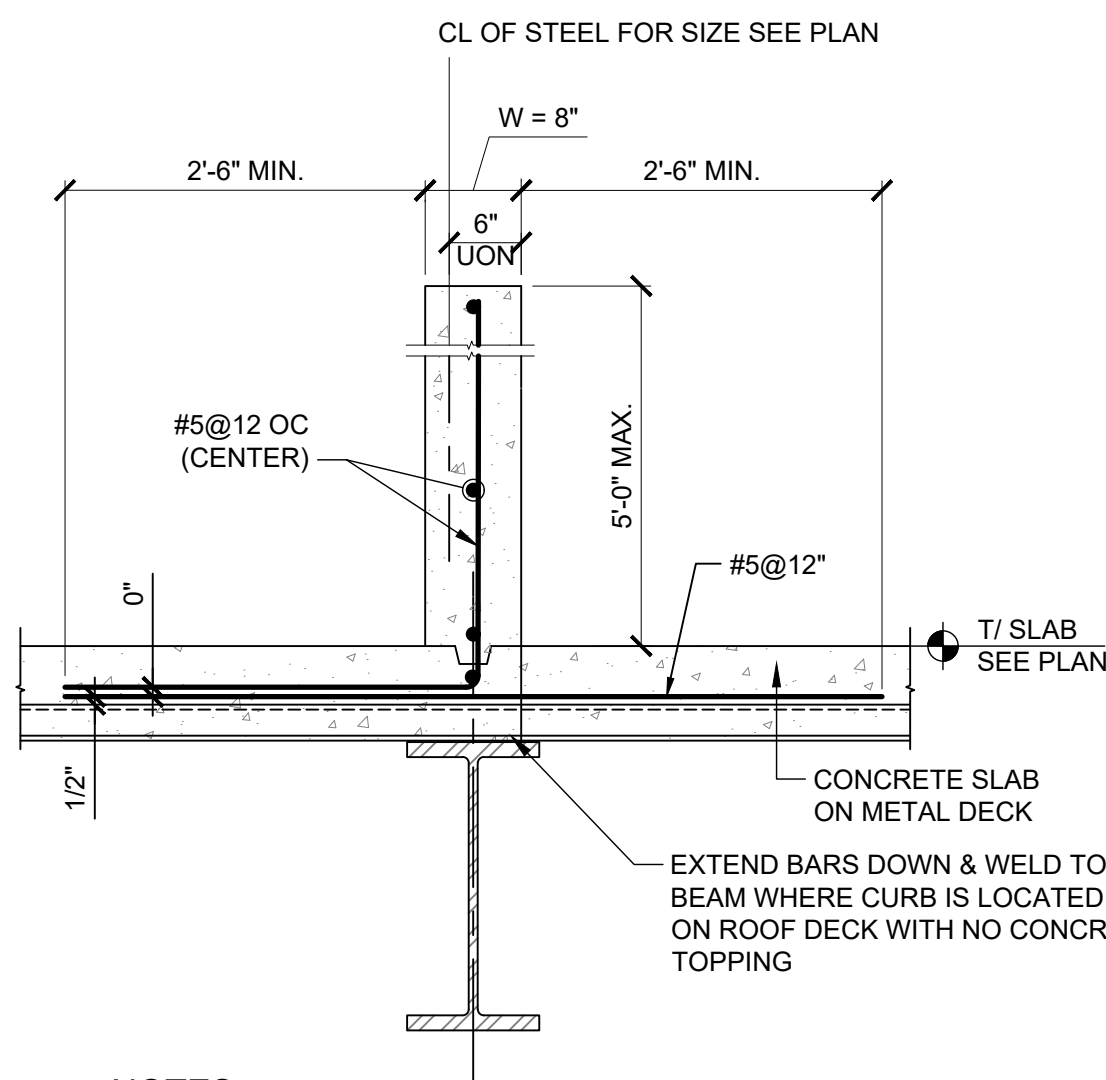


2 BEARING WALL
DETAIL & SCHEDULE
1/2" = 1'-0"

BEARING WALL REINFORCING SCHEDULE		
WALL 'I'	VERTICAL REINF.	HORIZONTAL REINF.
8"	#5@18	#3@11
9"	#5@18	#3@9
10"	#5@18	#4@16
12"	#5@18	#4@13

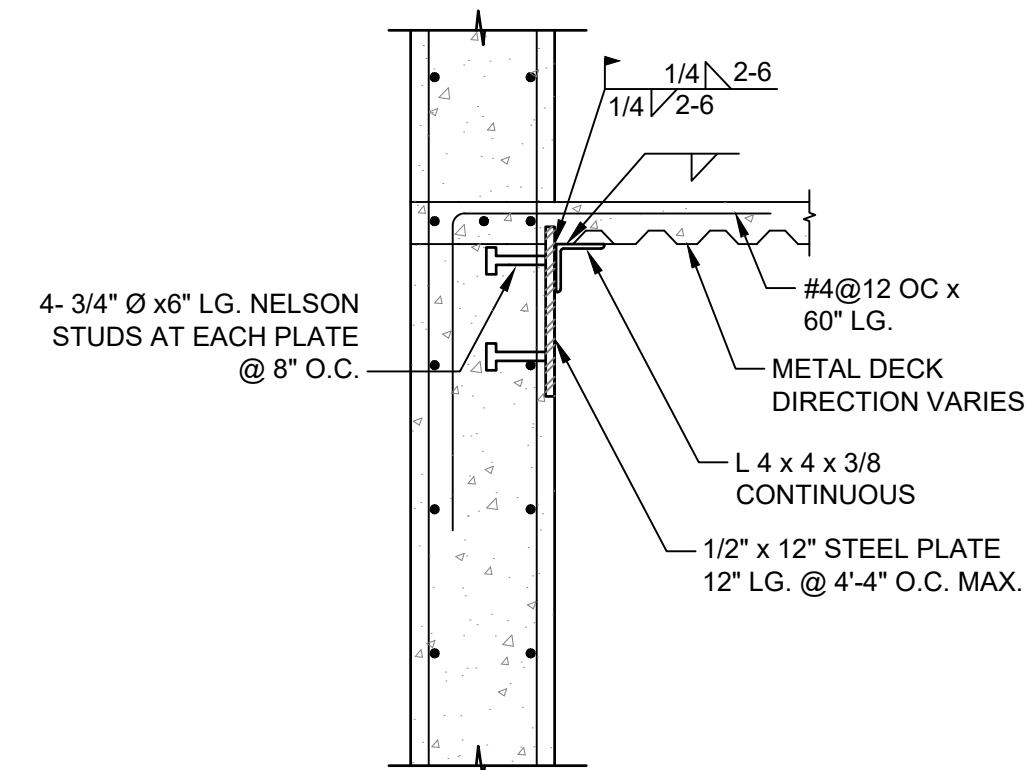
SEE PLAN FOR WALL REINFORCING AT ELEVATOR
AND STAIR.

BASE PLATE SCHEDULE		
BEAM	BASE PLATE	STIFFENER
W10	8X8X1"	NO STIFFENER
W12	8X8X1"	NO STIFFENER
W14	8X8X1"	NO STIFFENER
W16	8X8X1"	NO STIFFENER
W18	8X8X1"	NO STIFFENER
W24	10X8X1"	2-1/2" STIFFENER
W30	12X12X1"	2-1/2" STIFFENER NOTCH PLATE AS REQUIRED AT CORNER CONDITION



NOTES:
FOR CURB SIZE & LOCATION SEE
ARCHITECTURAL DRAWINGS

7 TYPICAL CONCRETE CURB
PERPENDICULAR TO DECK
3/4" = 1'-0"



3 TYPICAL METAL DECK SUPPORT
3/4" = 1'-0"

REINFORCEMENT ANCHORAGE SCHEDULE																	
TABULATED VALUES ARE GIVEN IN INCHES & ARE CALCULATED FOR = 60,000 P.S.I.																	
STONE CONCRETE																	
BAR SIZE	Fc	3000 P.S.I.				4000 P.S.I.				5000 P.S.I.				6000 P.S.I.			
		ld	ldt	ldh	ldc	ld	ldt	ldh		ld	ldt	ldh		ld	ldt	ldh	
#3	12	13	6/8	8	12	13	6/7										
#4	12	17	8/11	11	12	17	7/9	9	12	17	6/8	9	12	17	6/8	9	
#5	15	21	10/14	14	15	21	8/12	12	15	21	7/11	11	15	21	7/10	11	
#6	19	27	11/16	16	18	25	10/14	14	18	25	9/13	14	18	25	8/12	14	
#7	26	37	13/19	19	23	32	12/17	17	21	29	10/15	16	21	29	10/14	16	
#8	35	48	15/22	22	30	42	13/19	19	27	38	12/17	18	24	34	11/16	18	
#9	44	61	17/25	25	38	53	15/21	21	34	48	13/19	20	31	43	13/18	20	
#10	56	78	19/28	28	48	67	17/24	24	43	60	15/22	23	39	55	14/20	23	
#11	68	96	22/31	31	59	83	19/27	27	53	74	17/24	25	48	68	15/22	25	
NOTES	A & B	C	D	A & B	C	D	A & B	C	D	A & B	C	D	A & B	C	D		

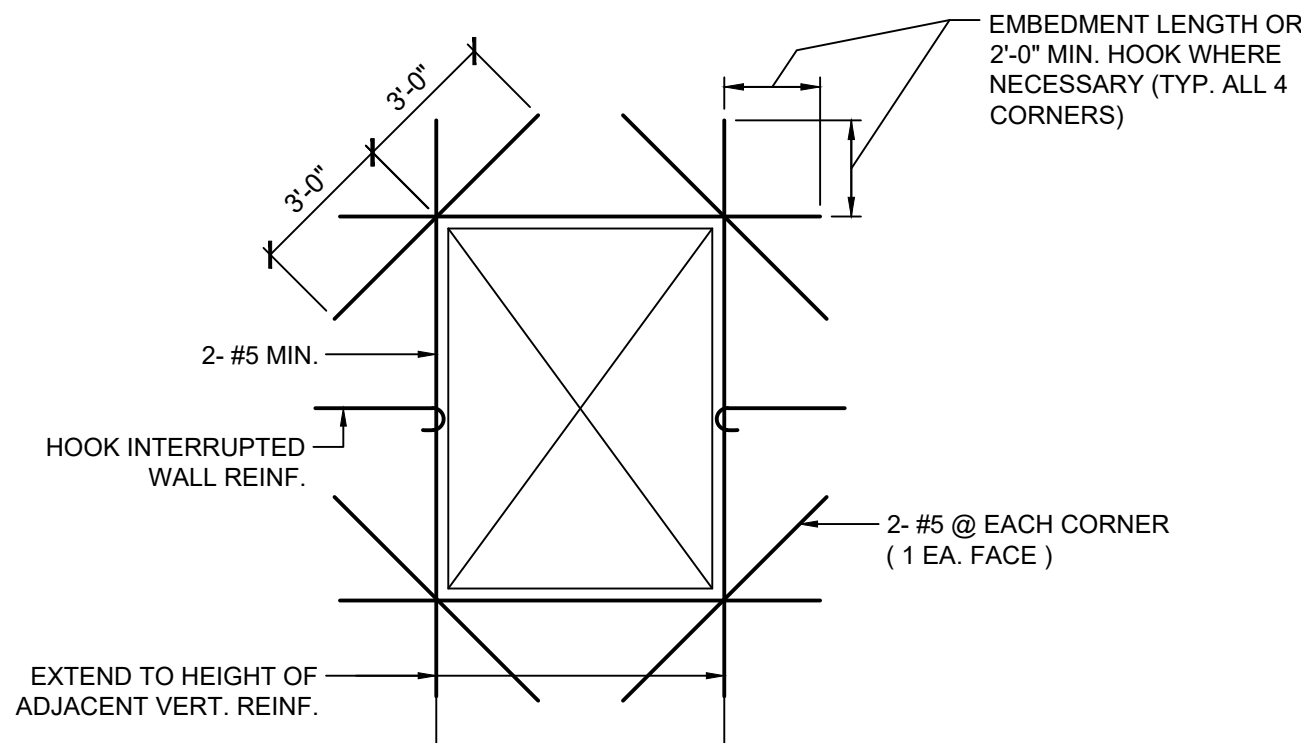
NOTE A : USE ld TABULATED VALUES WHEN BARS HAVE LESS THAN 12" OF CONCRETE
BELOW BOTTOM OF BARS (ie. FOR BOTTOM REINFORCING AND FOR TOP REINFORCING IN
SHALLOW BEAMS AND SLABS). USE ldt WHEN BARS HAVE MORE THAN 12" OF FRESHLY
PLACED CONCRETE BELOW BOTTOM OF BARS.

NOTE B : TABULATED VALUES MAY BE REDUCED BY 20% IF BARS ARE SPACED MORE
THAN 6" AND ARE AT LEAST 3" FROM PARALLEL CONCRETE EDGES.

NOTE C : LOWER VALUES OF DEVELOPMENT LENGTH OF HOOKED BARS, ldh, MAY BE
USED IF TAIL EXTENSION OF THE HOOK HAS AT LEAST 2" COVER AND CONCRETE COVER TO
SIDE OF HOOK IS AT LEAST 2 1/2" THICK. OTHERWISE USE HIGHER ldh VALUES TABULATED.

NOTE D : HOOKS SHALL NOT BE CONSIDERED EFFECTIVE IN REDUCED ldc VALUES (COMP.
RESSION DEVELOPMENT LENGTH).

GLOSSARY :
ld = DEVELOPMENT LENGTH (IN.) = LENGTH REQUIRED TO ANCHOR REINFORCING AT
EACH END.
ldt = DEVELOPMENT LENGTH (IN.) = LENGTH REQUIRED TO ANCHOR TOP BARS
HAVING MORE THAN 12" OF FRESH CONCRETE PLACED UNDER THEM.
ldh = DEVELOPMENT LENGTH (IN.) OF HOOKED BARS (TO PERIMETER OF HOOK).
ldc = DEVELOPMENT LENGTH (IN.) FOR BARS IN COMPRESSION.



TYPICAL WALL OPENING DETAILS

SCALE : N.T.S.

NOTES :
1. HOOK ALL INTERRUPTED CONTINUOUS BARS.
2. ONE HALF OF REINFORCING BAR AREA INTERRUPTED BY
OPENING SHALL BE PROVIDED EACH SIDE OF OPENINGS
ON THE SAME FACE, MIN. 2- #5 EACH SIDE.

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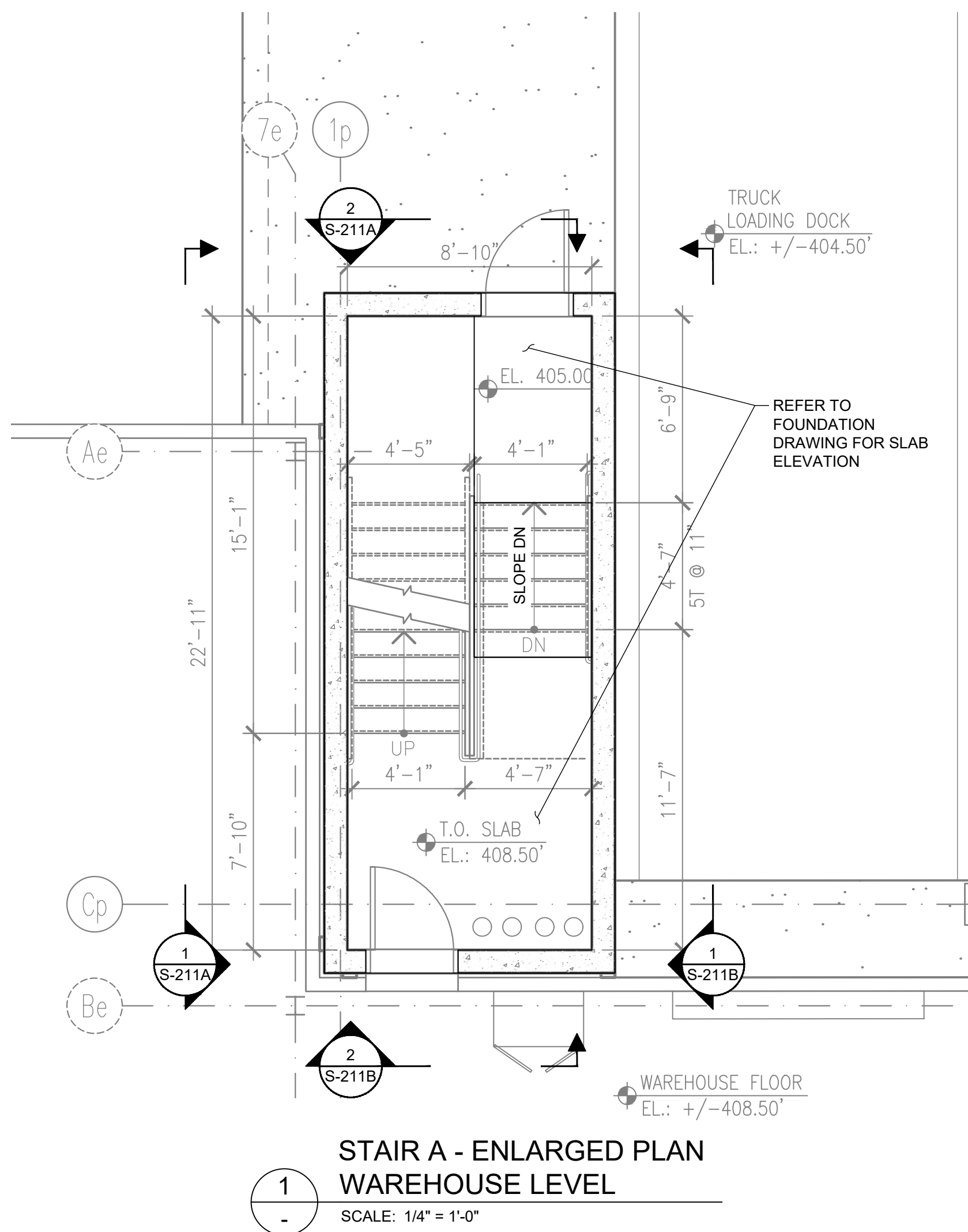
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CURB DETAILS

DWG NUMBER :

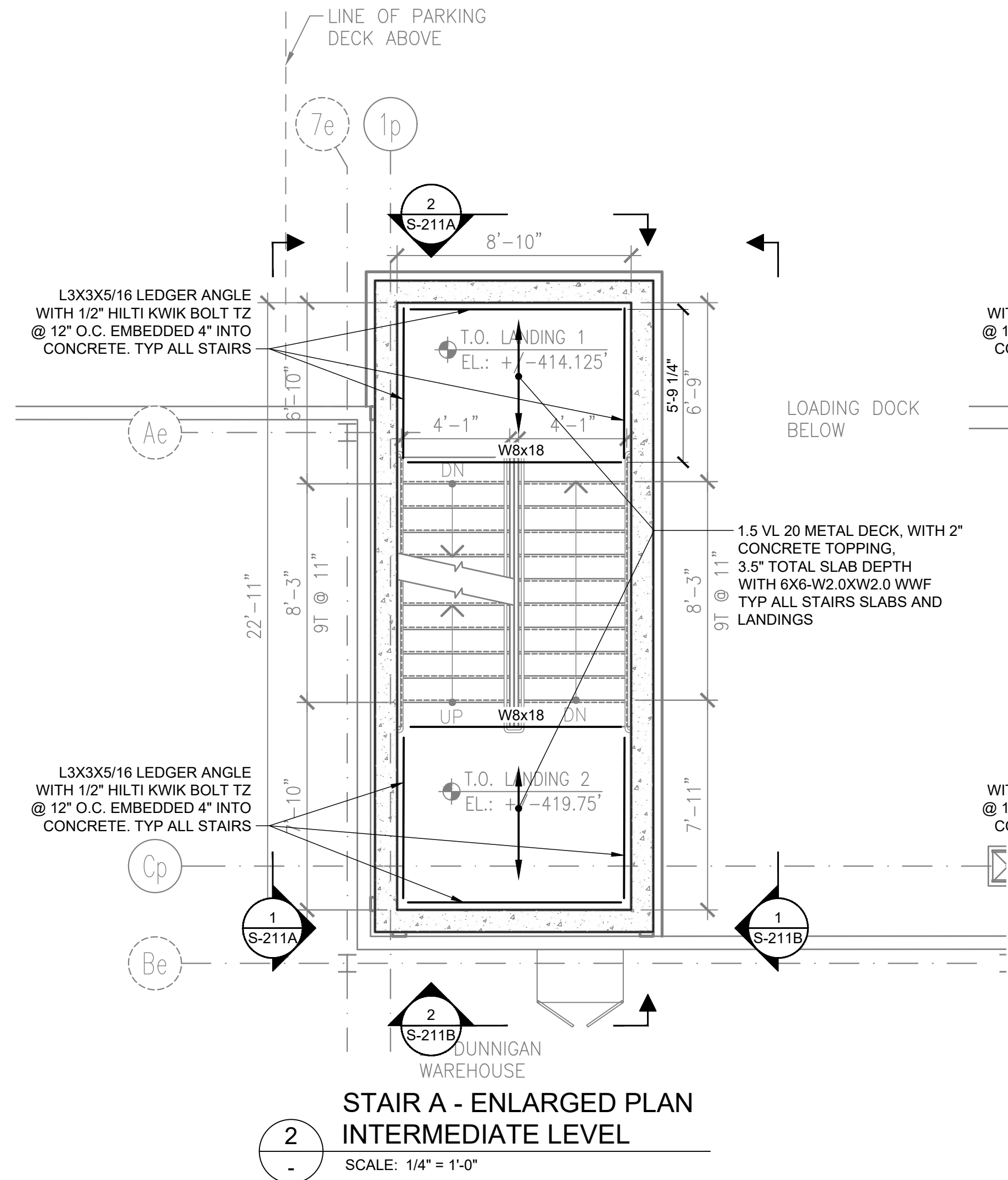
S-205

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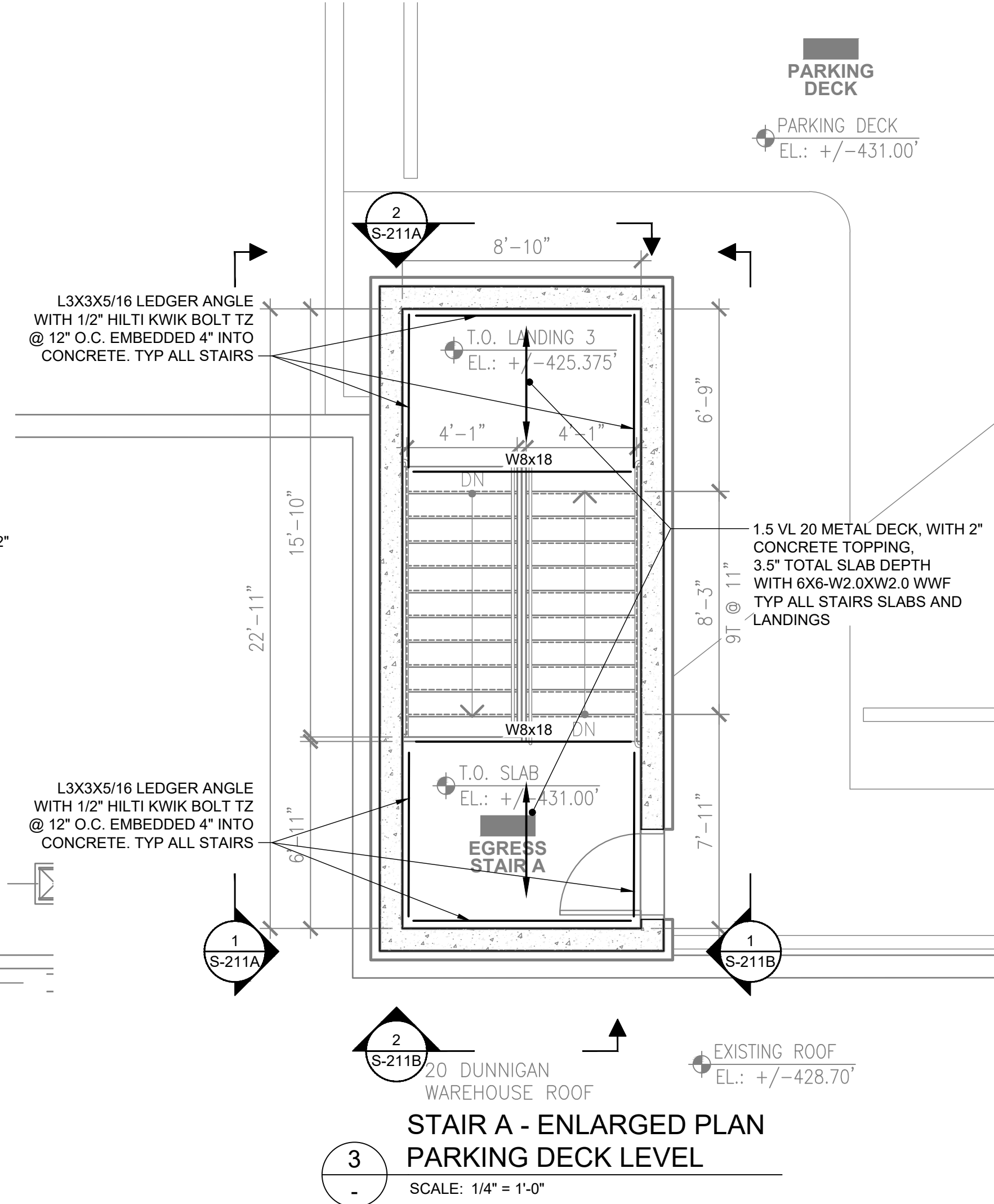
STAIR A - ENLARGED PLAN
WAREHOUSE LEVEL

SCALE: 1/4" = 1'-0"



STAIR A - ENLARGED PLAN
INTERMEDIATE LEVEL

SCALE: 1/4" = 1'-0"



STAIR A - ENLARGED PLAN
PARKING DECK LEVEL

SCALE: 1/4" = 1'-0"

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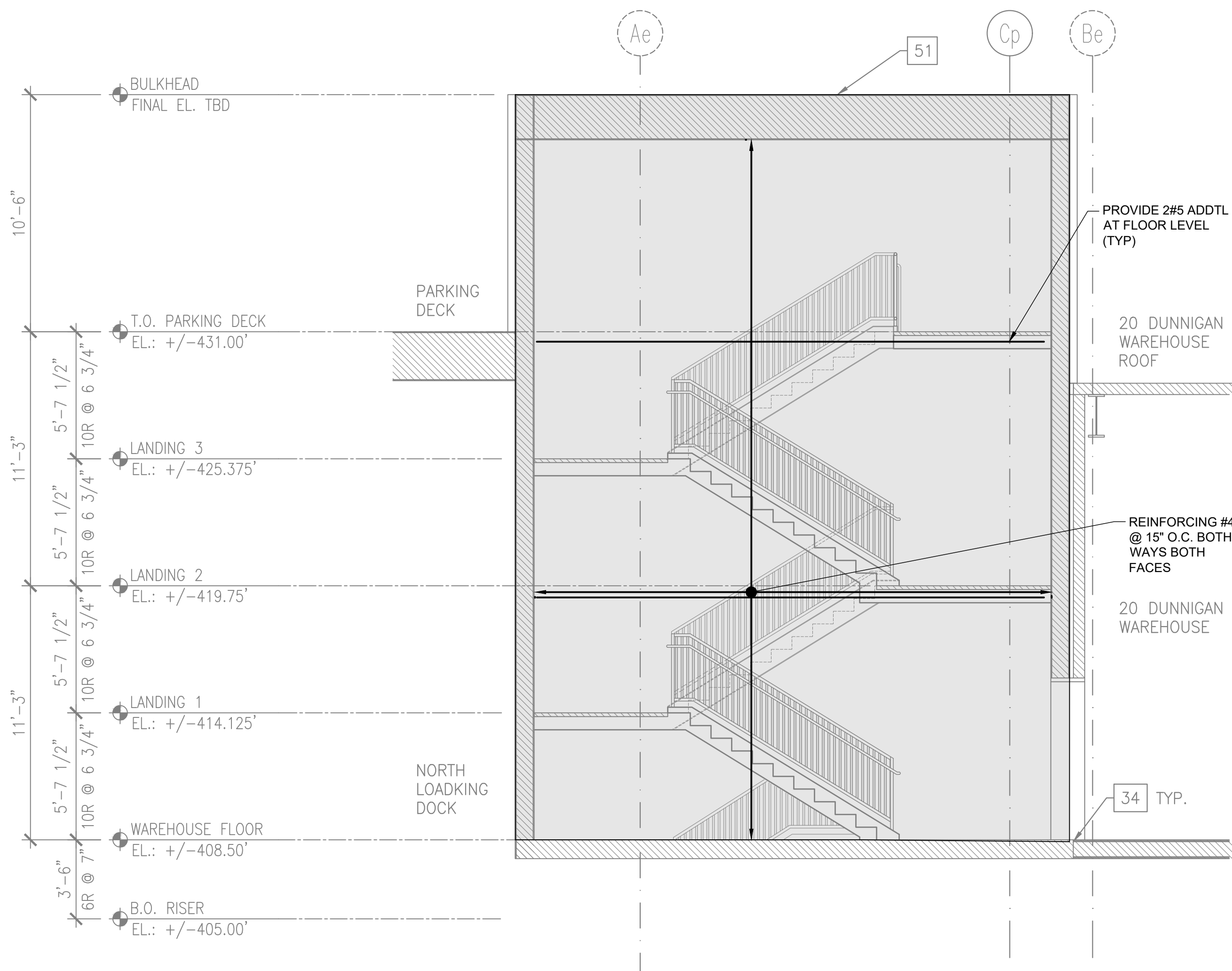
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STAIR A - PLANS

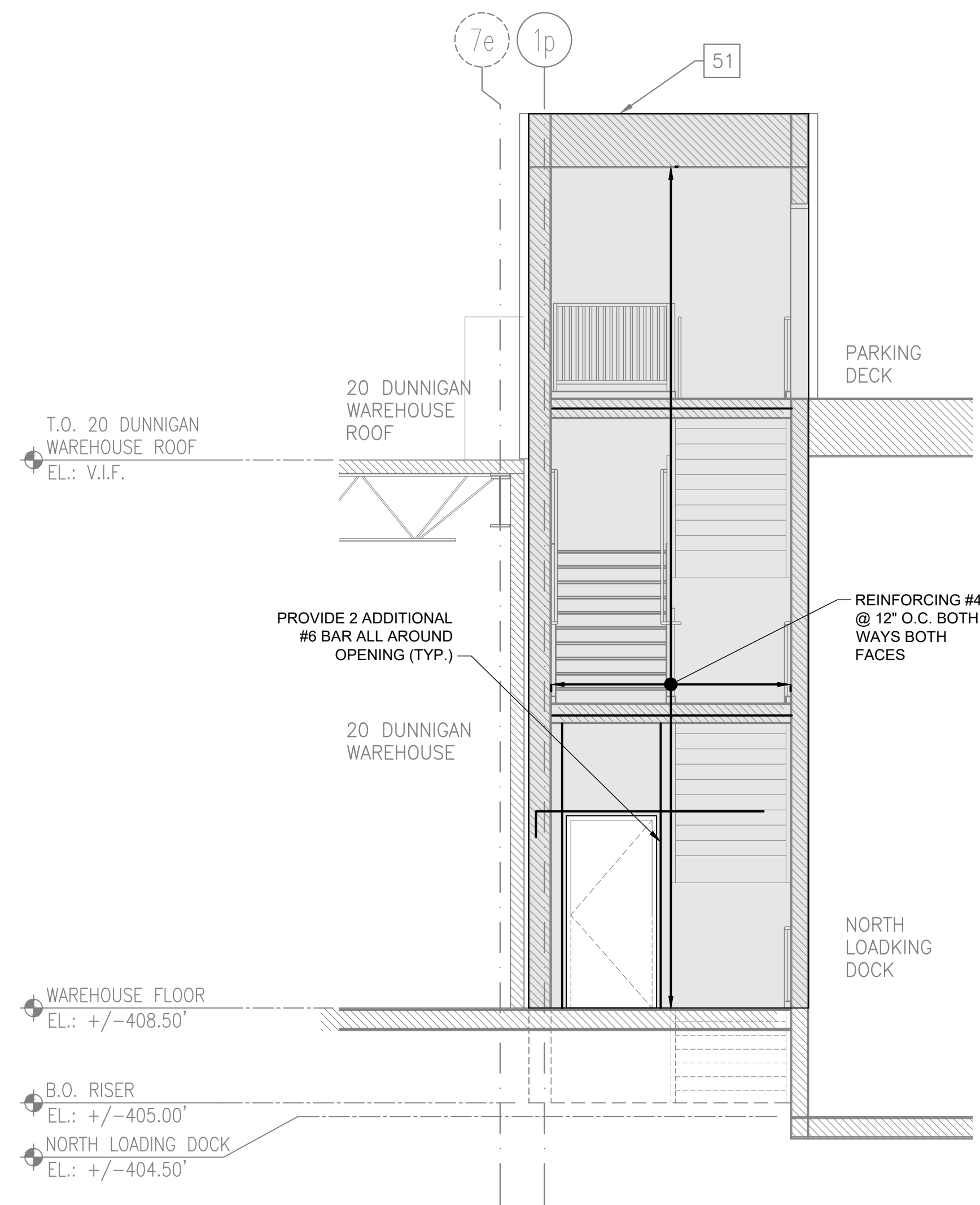
DWG NUMBER :

S-210

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1 STAIR A - ELEVATION (WEST)
210 SCALE: 1/4" = 1'-0"



2 STAIR A - ELEVATION (NORTH)
210 SCALE: 1/4" = 1'-0"

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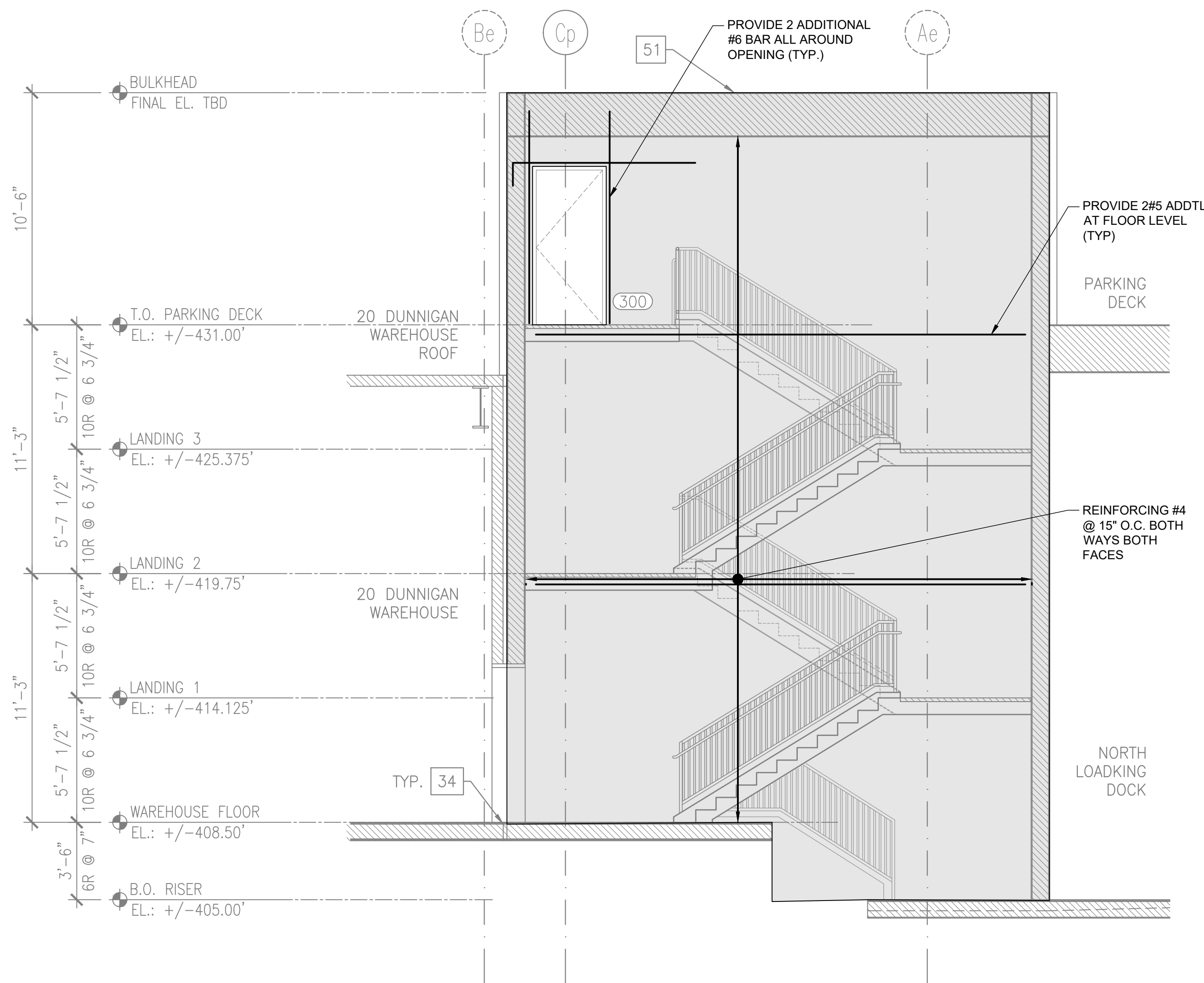
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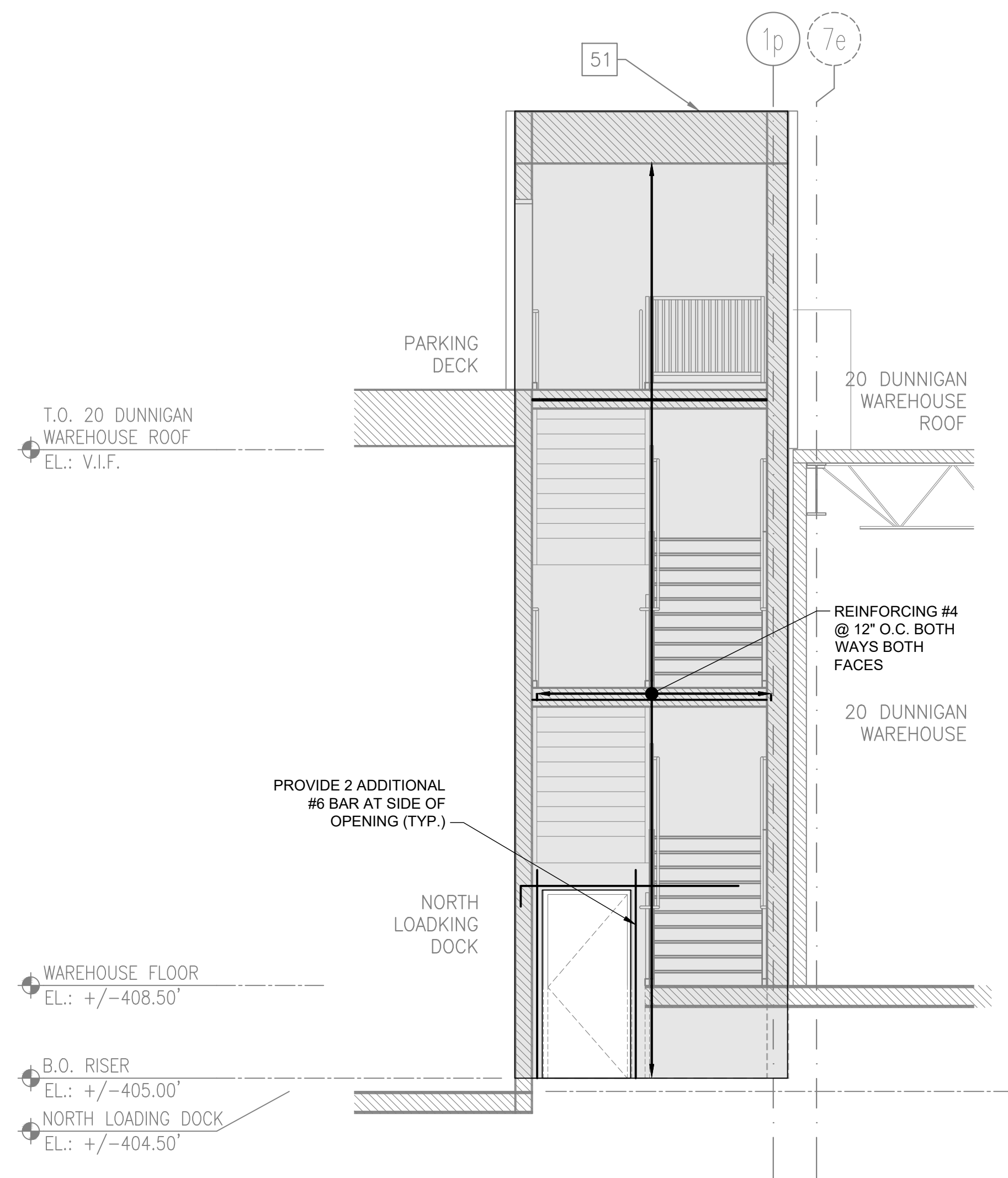
STAIR A - ELEVATIONS

DWG NUMBER :
S-211A

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1 STAIR A - ELEVATION (EAST)
210 SCALE: 1/4" = 1'-0"



2 STAIR A - ELEVATION (SOUTH)
210 SCALE: 1/4" = 1'-0"

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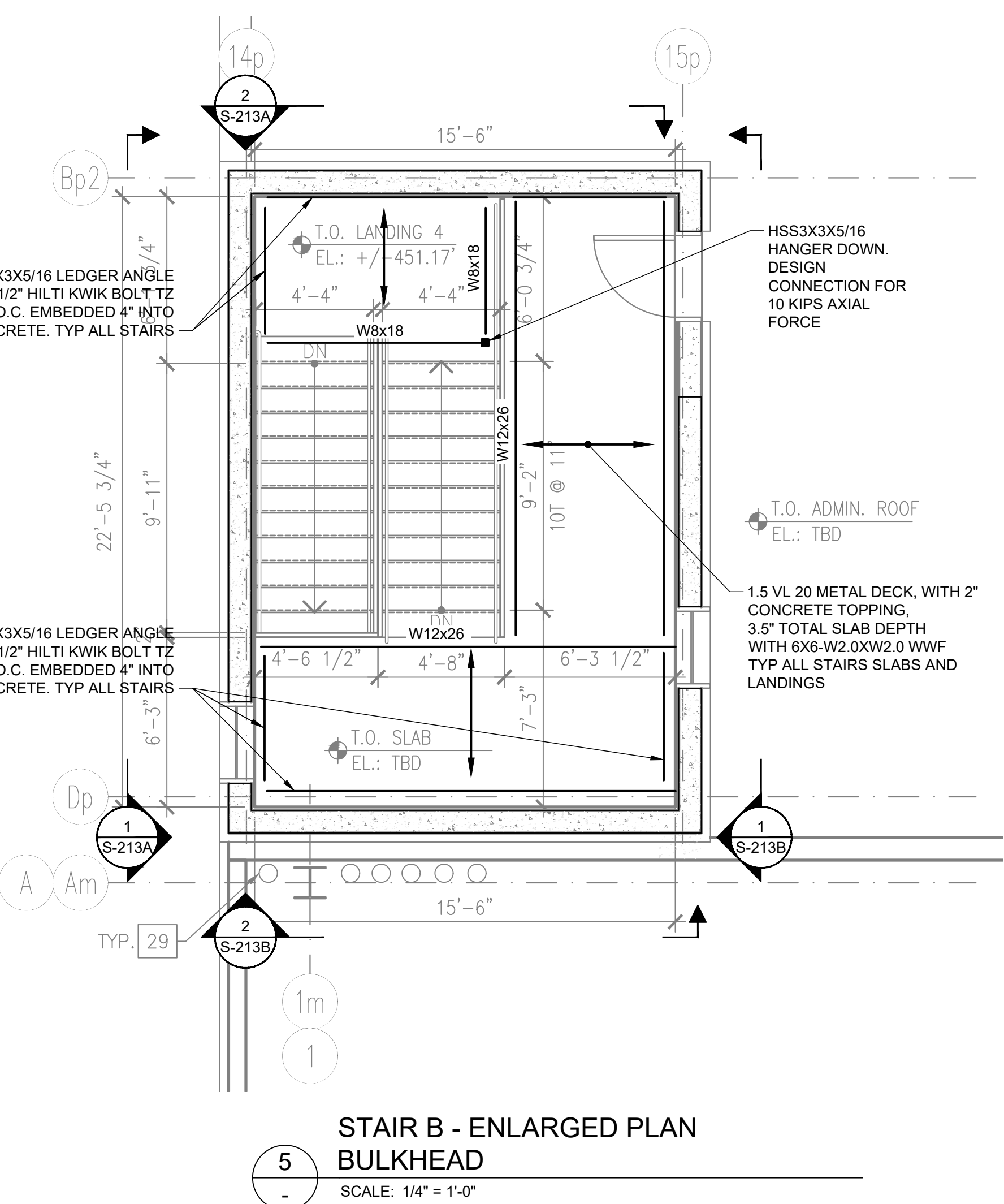
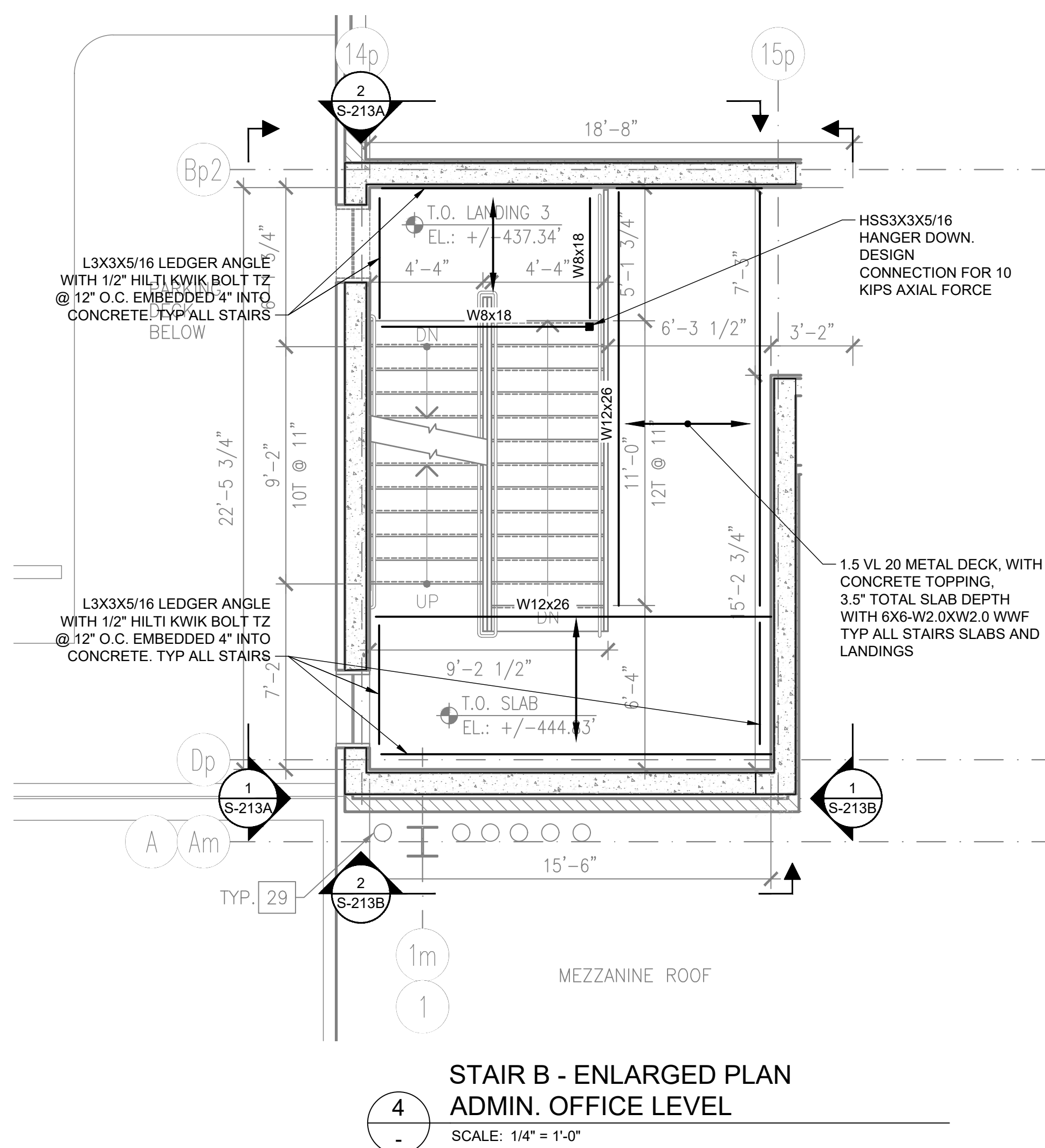
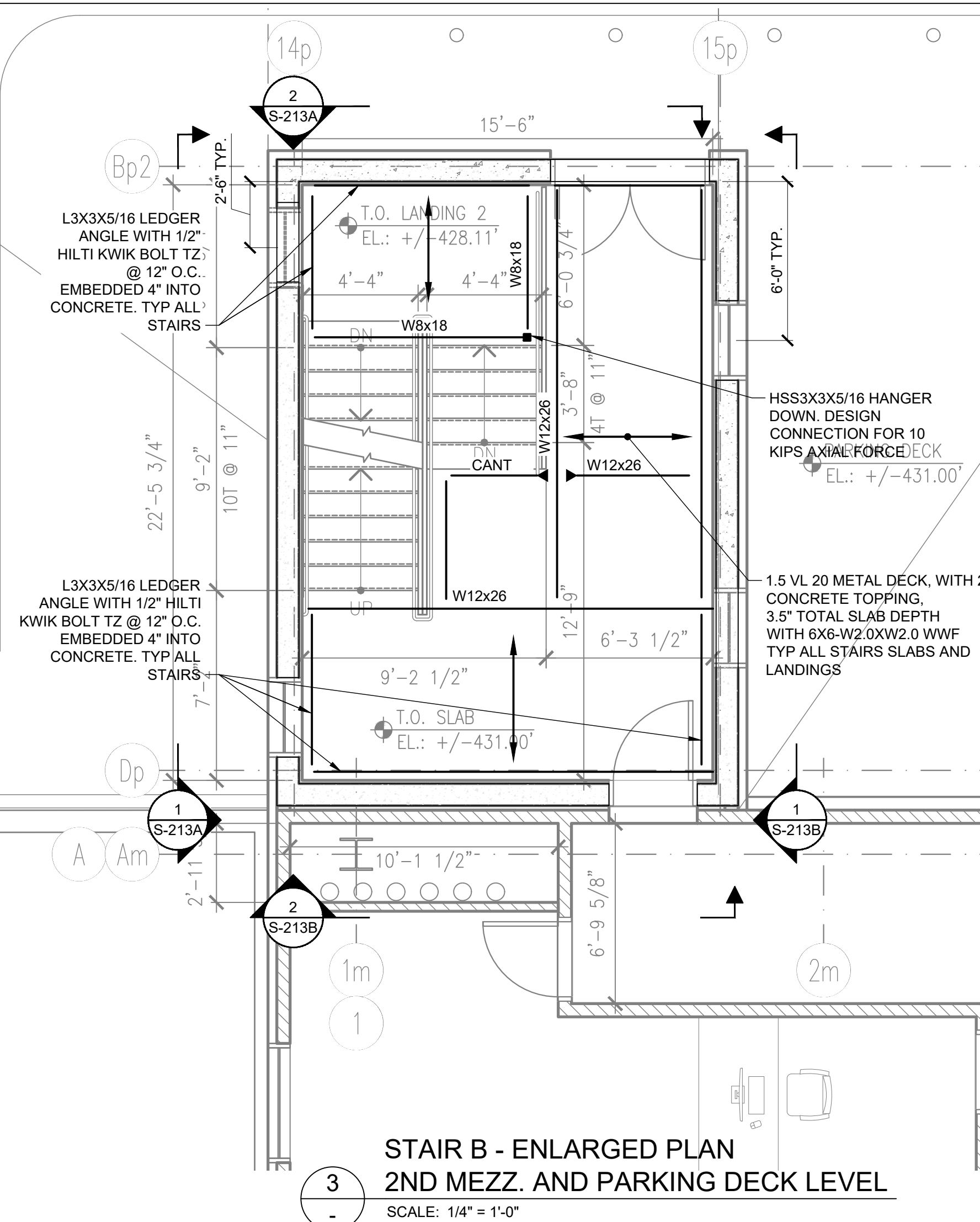
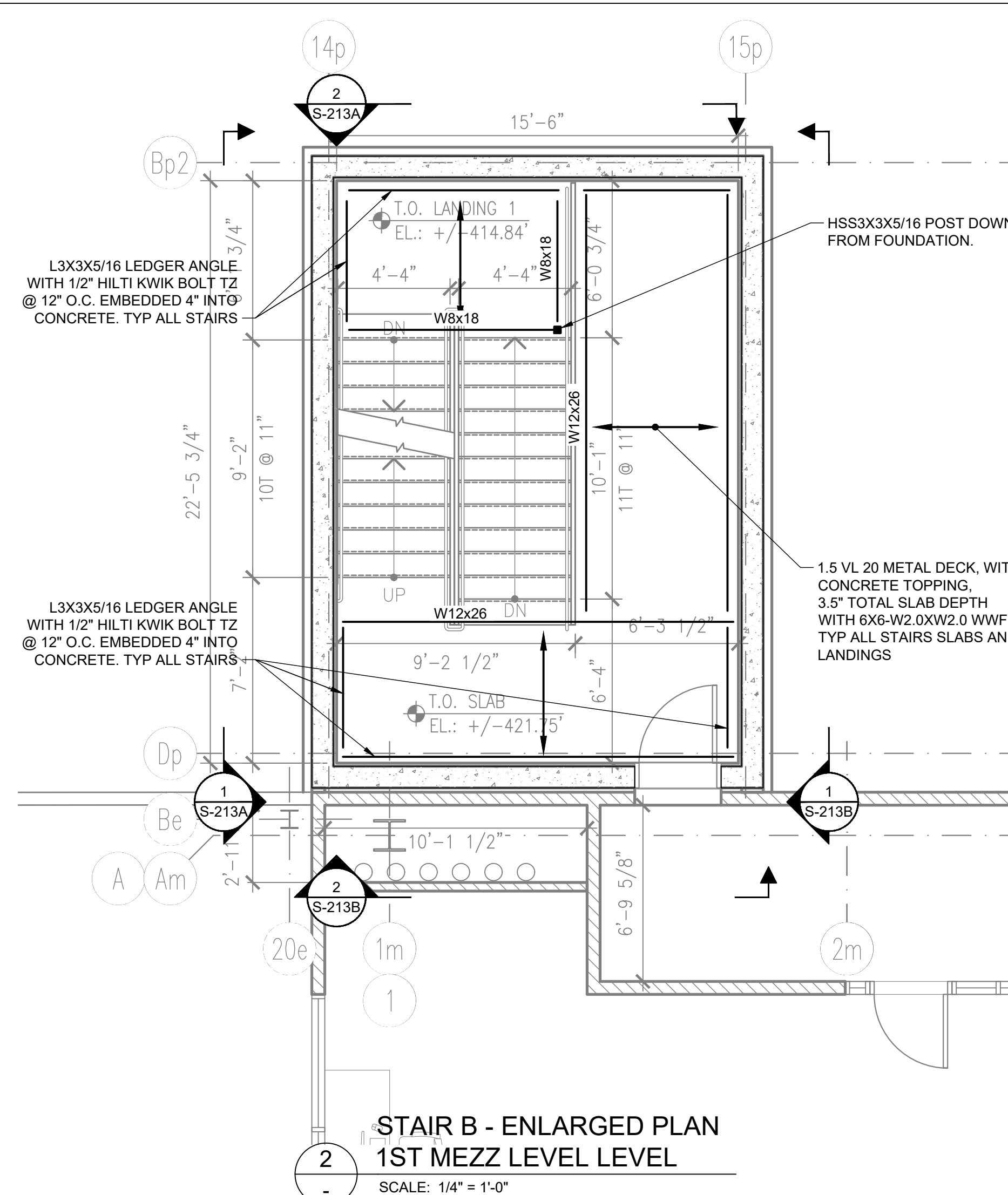
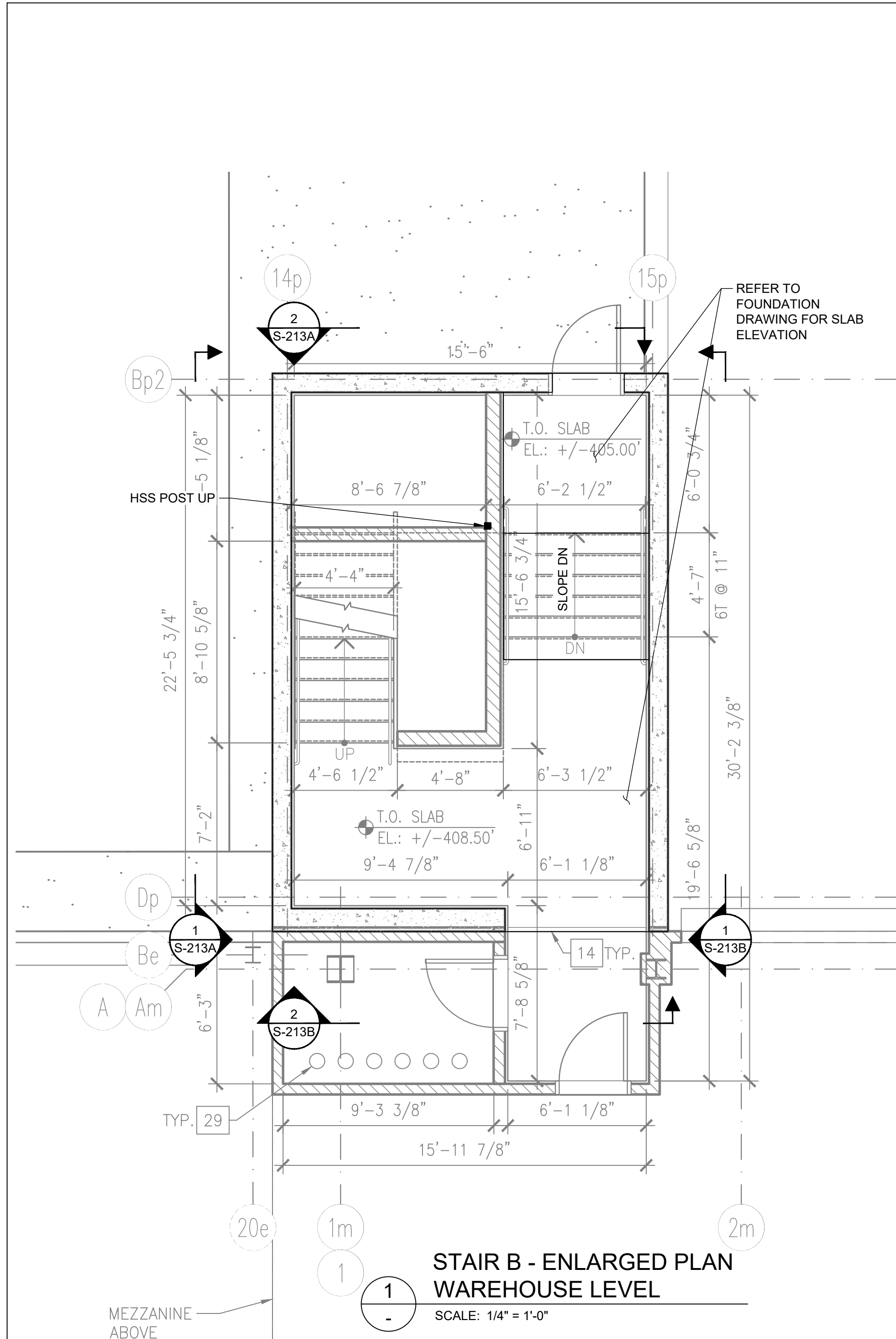
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STAIR A - ELEVATIONS

DWG NUMBER :
S-211B

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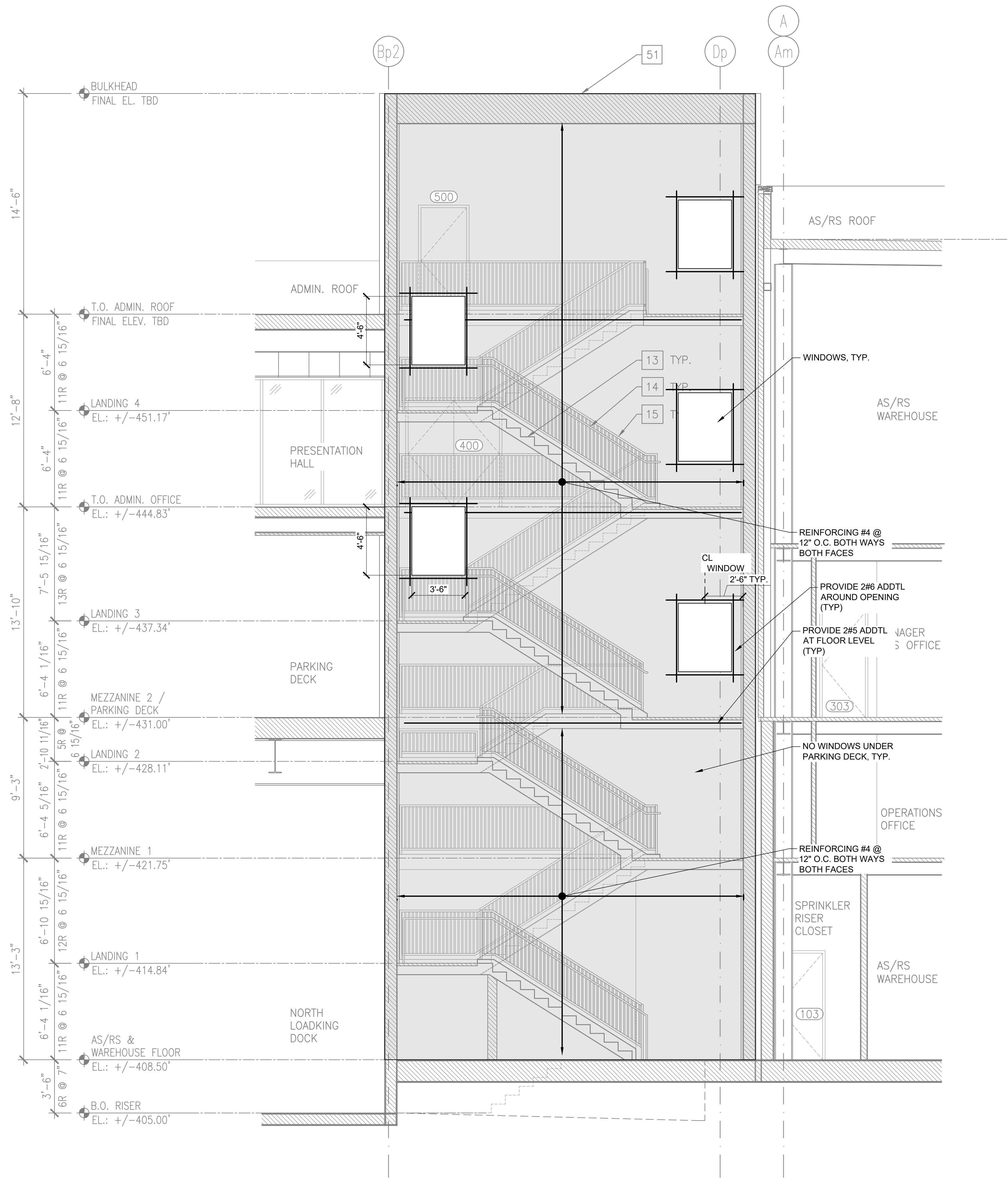
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STAIR B - PLANS

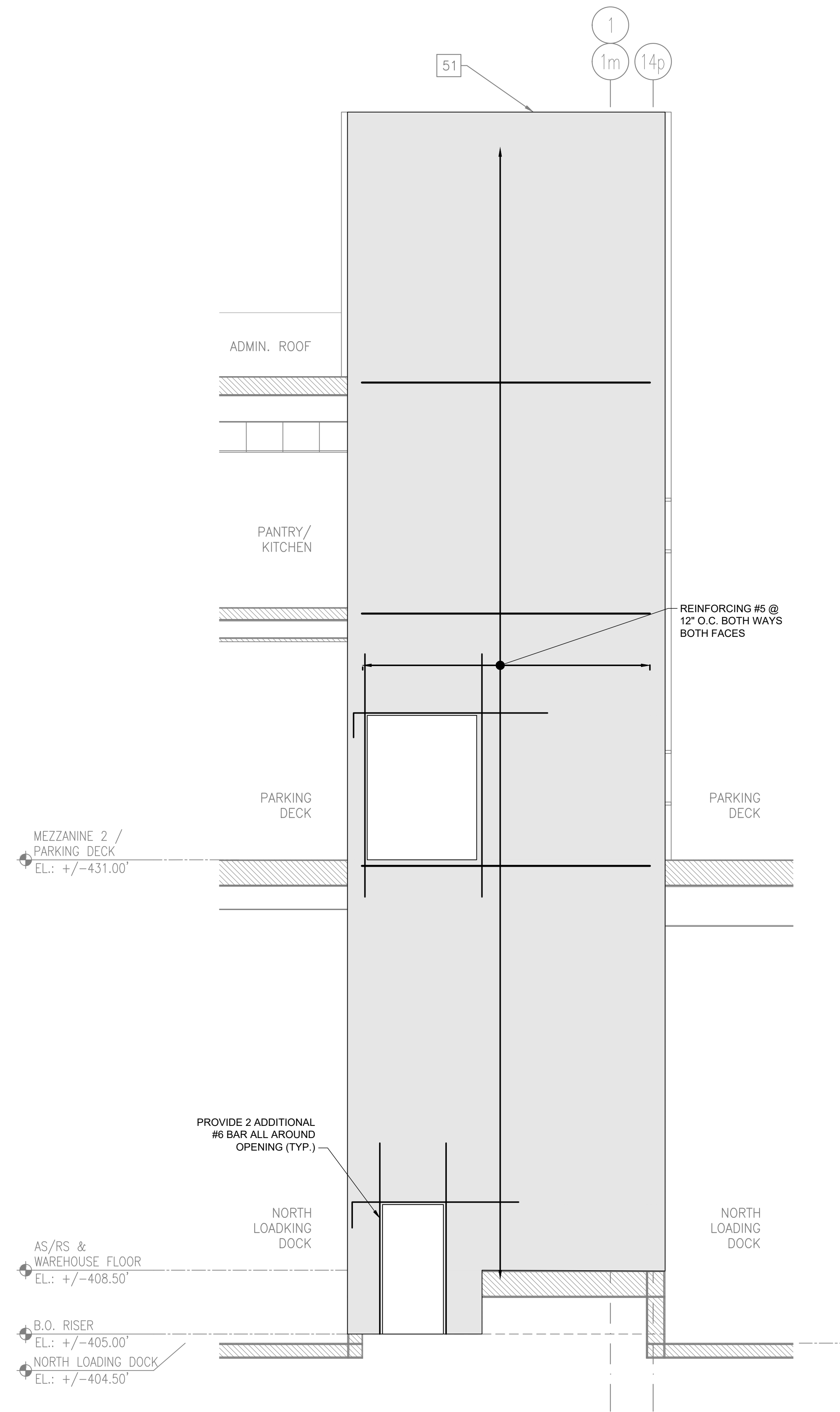
DWG NUMBER :

S-212

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1 STAIR B - ELEVATION (WEST)
212 SCALE: 1/4" = 1'-0"



2 STAIR B - ELEVATION (NORTH)
212 SCALE: 1/4" = 1'-0"

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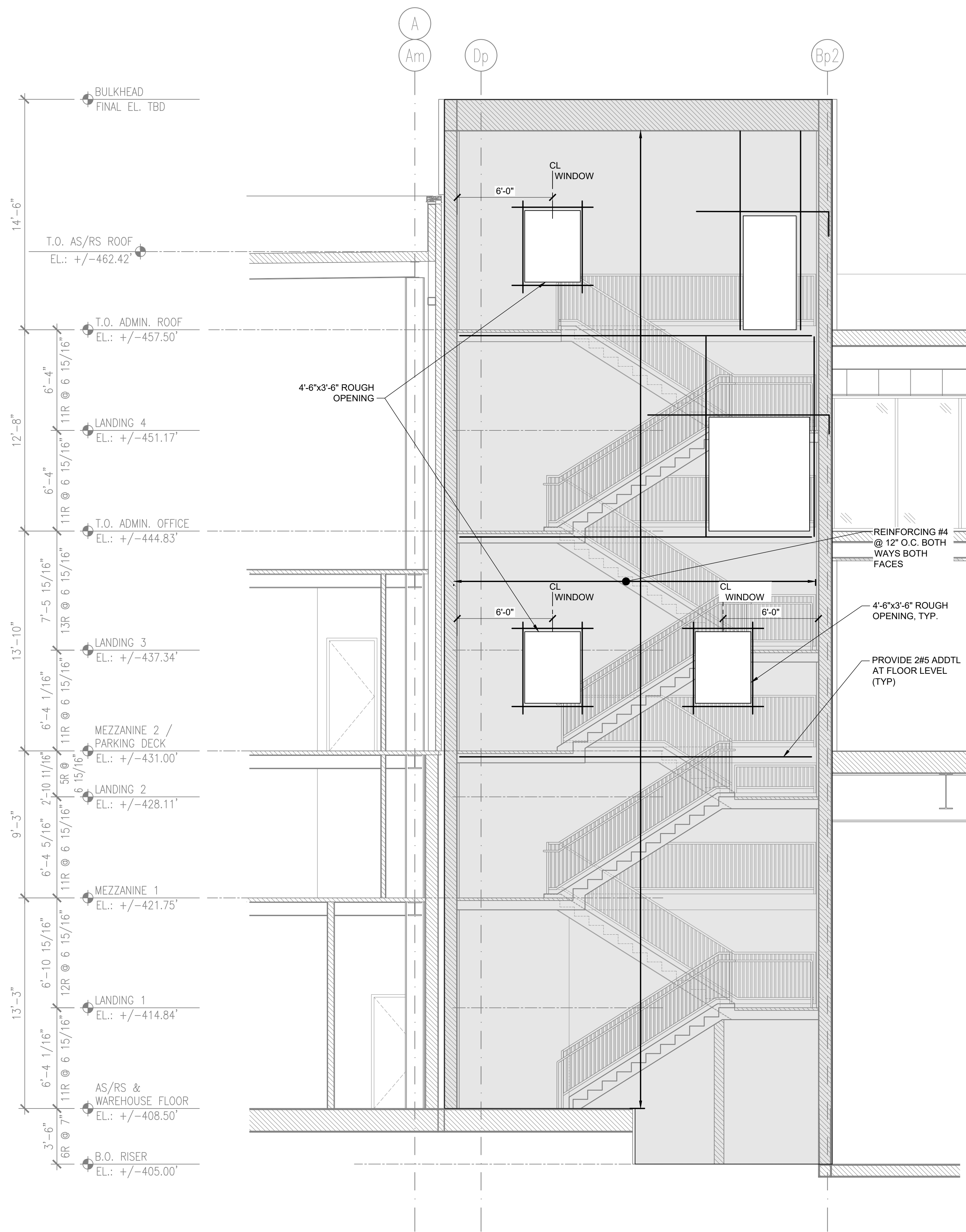
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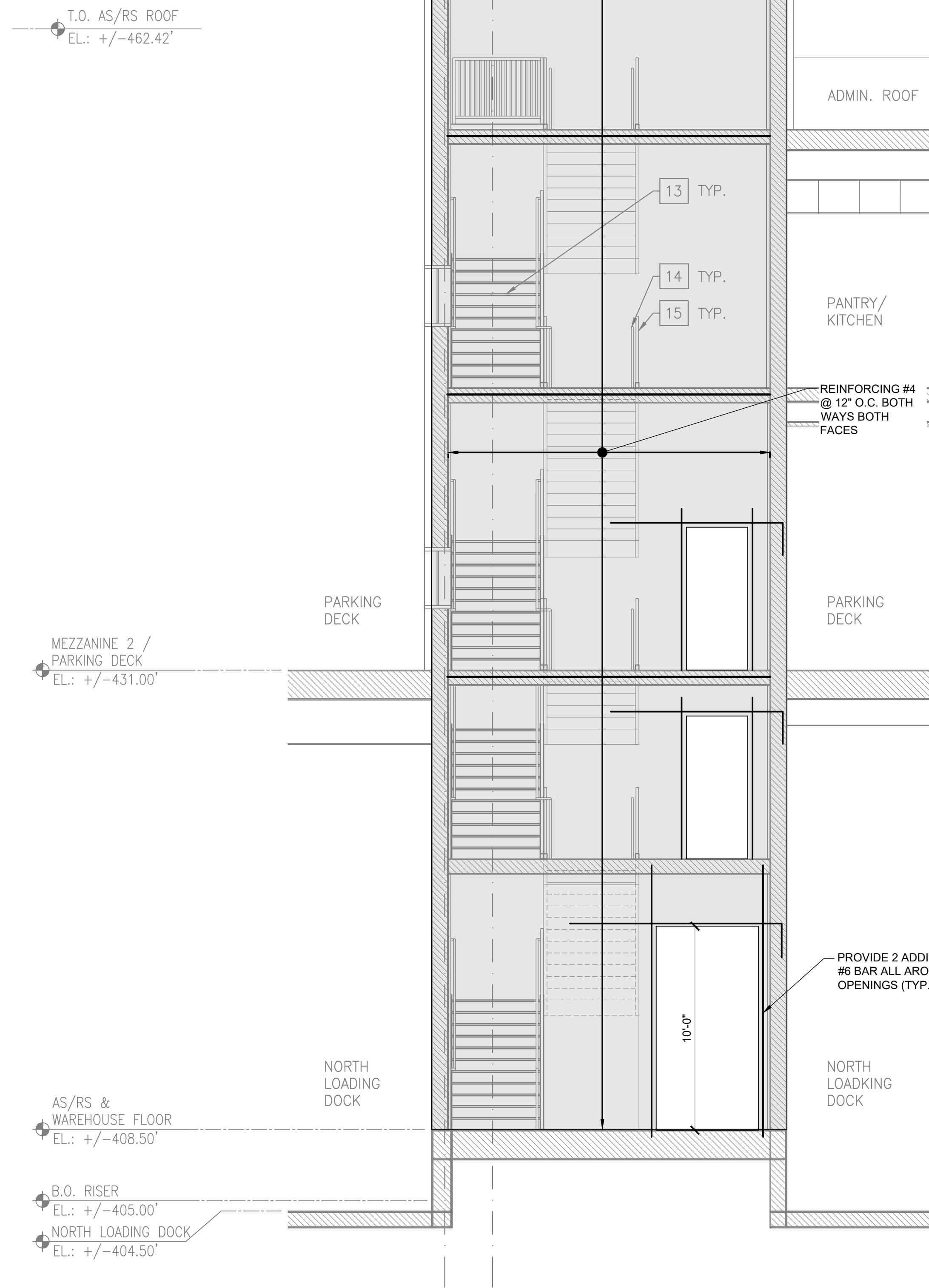
STAIR B - ELEVATIONS

DWG NUMBER :
S-213A

29 OF 60



1 STAIR B - ELEVATION (EAST)
212 SCALE: 1/4" = 1'-0"



2 STAIR B - ELEVATION (SOUTH)
212 SCALE: 1/4" = 1'-0"

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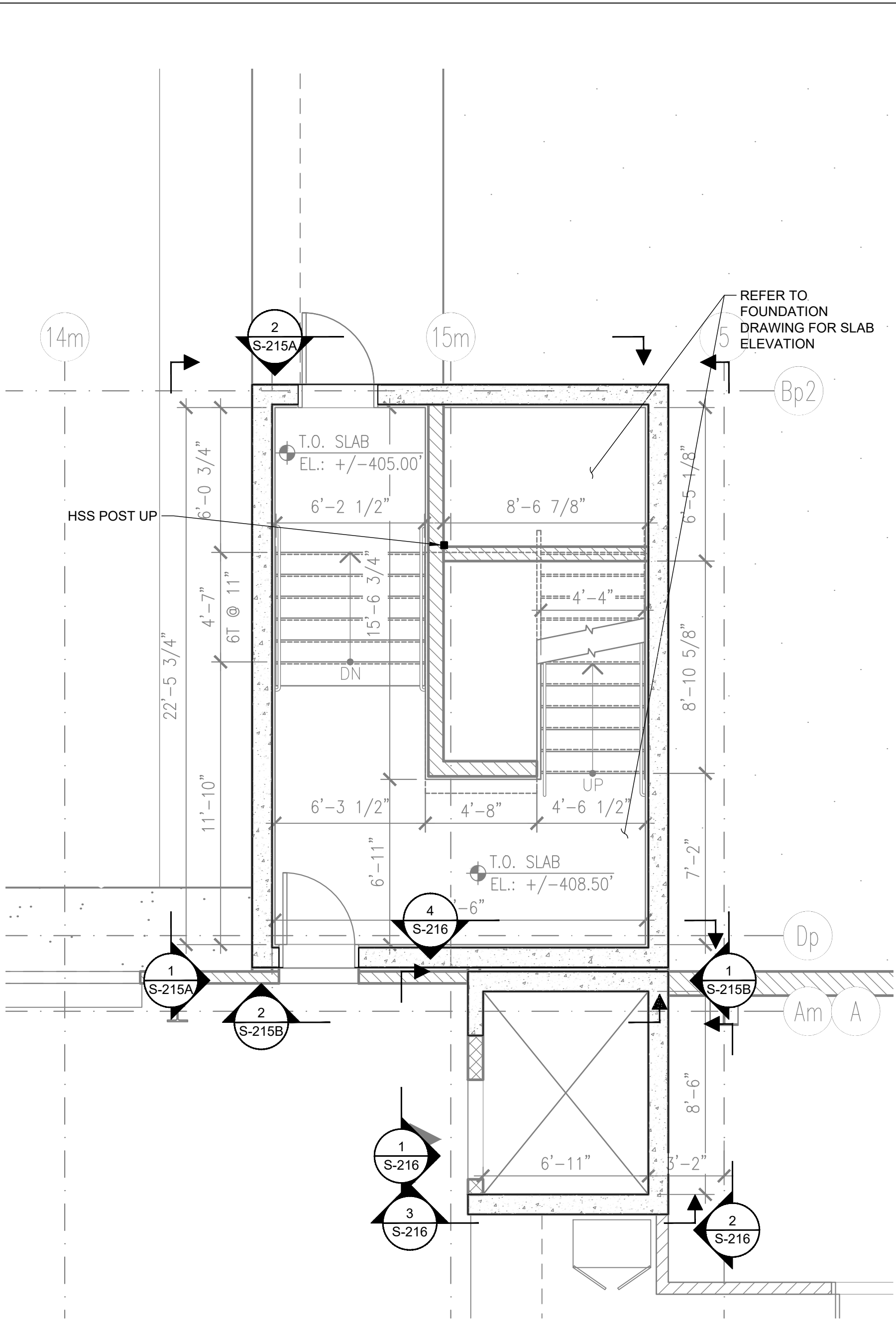
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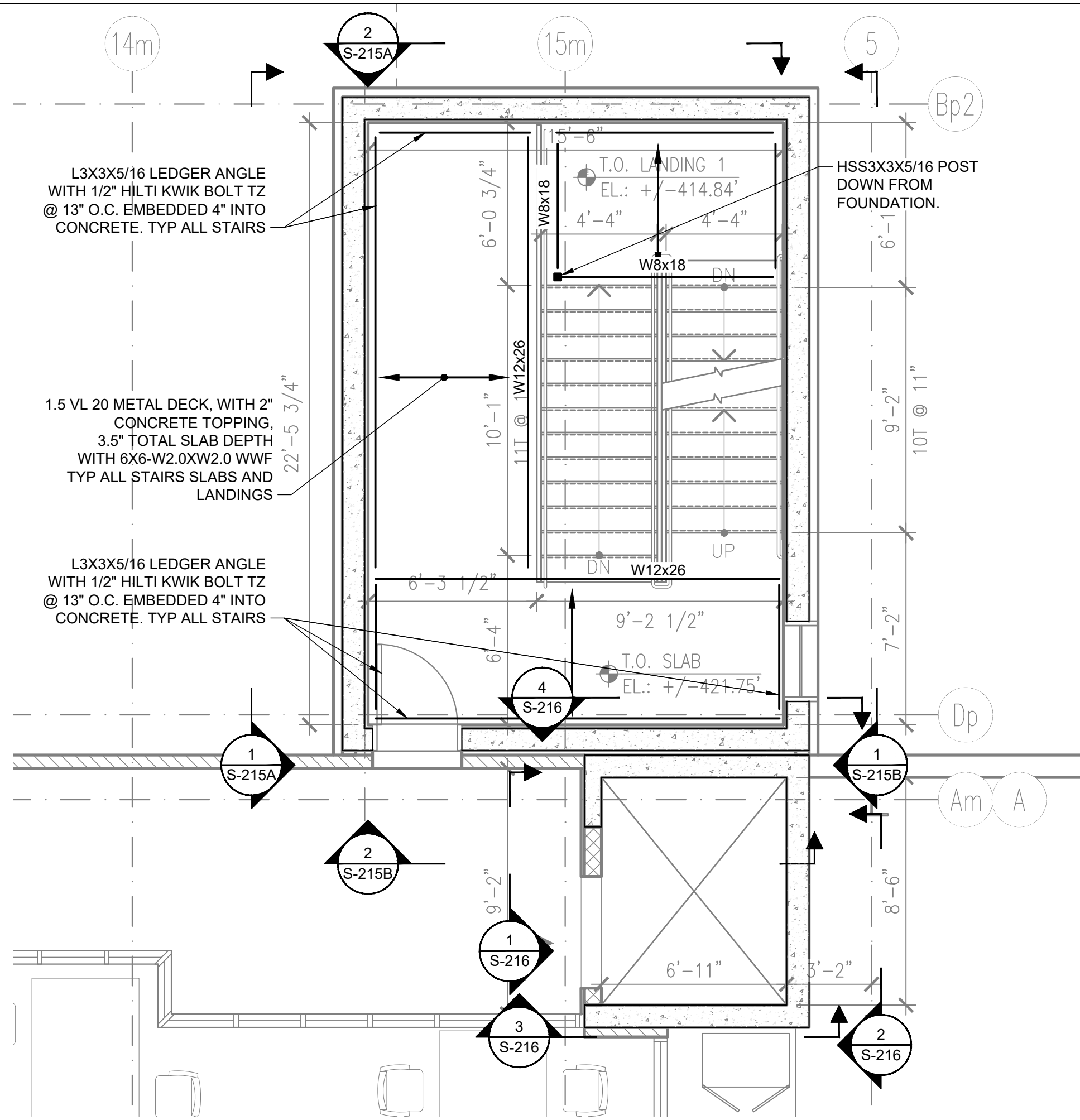
STAIR B - ELEVATIONS

DWG NUMBER :
S-213B

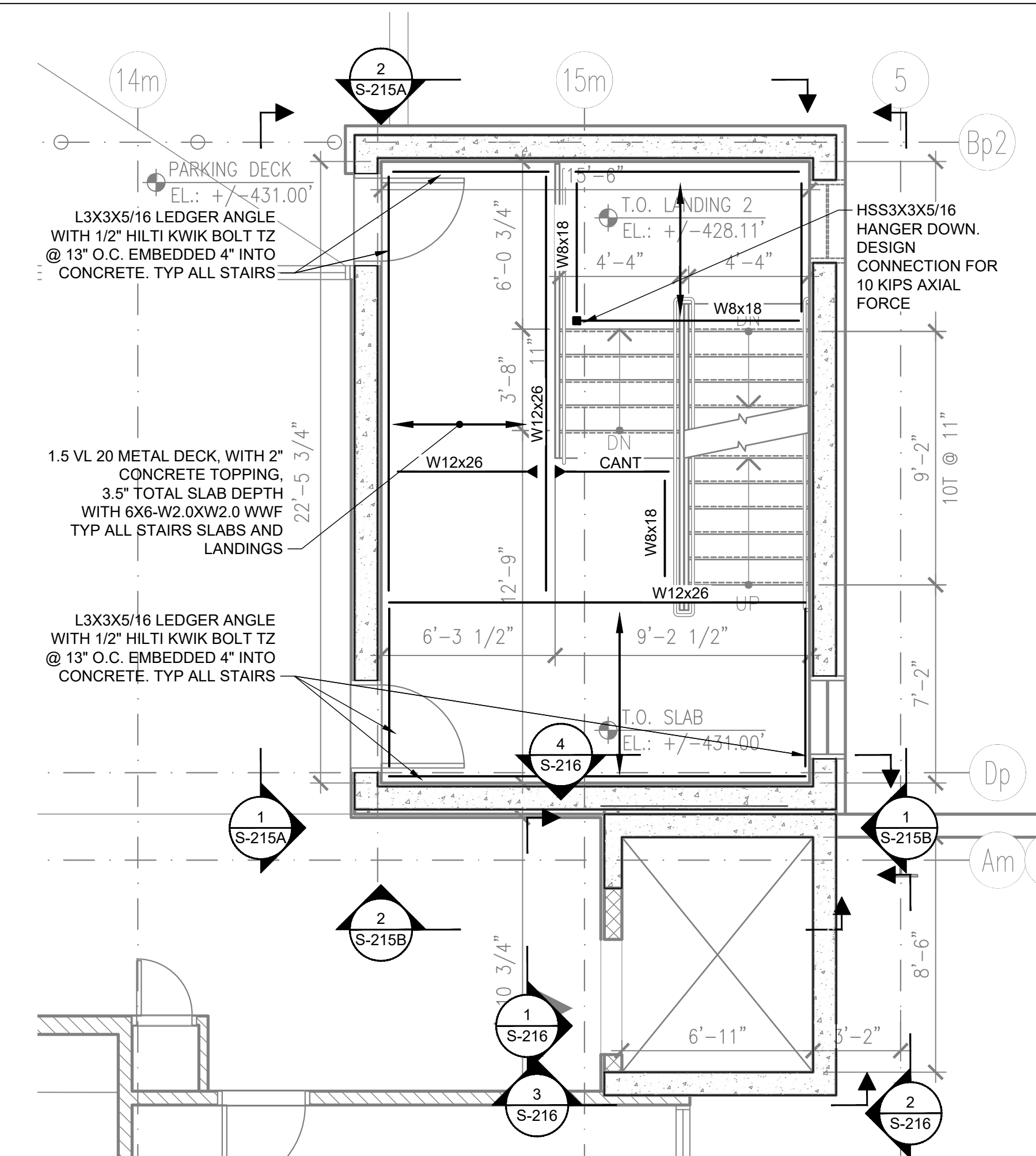
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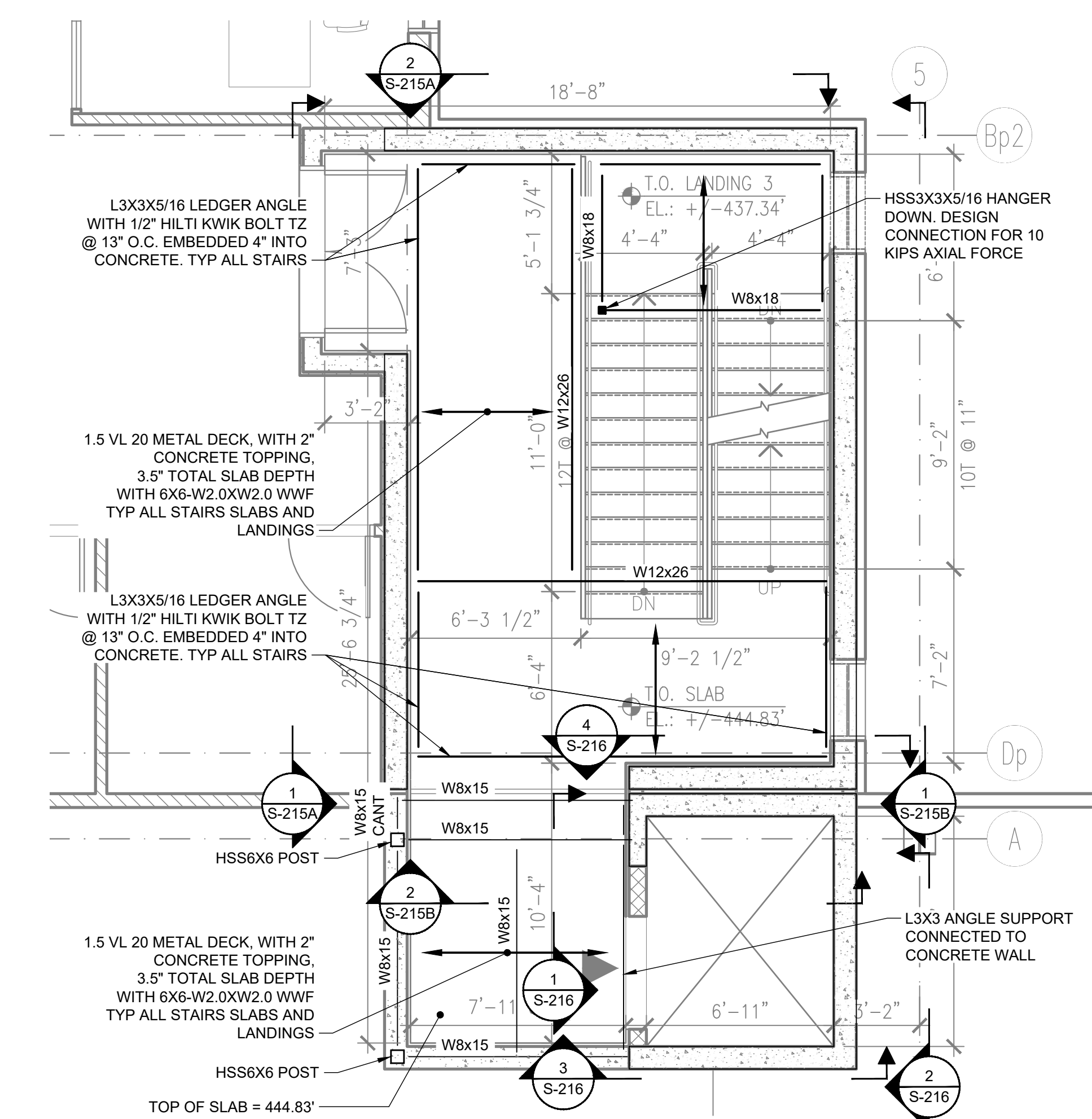
1 STAIR C - ENLARGED PLAN - WAREHOUSE LEVEL
SCALE: 1/4" = 1'-0"



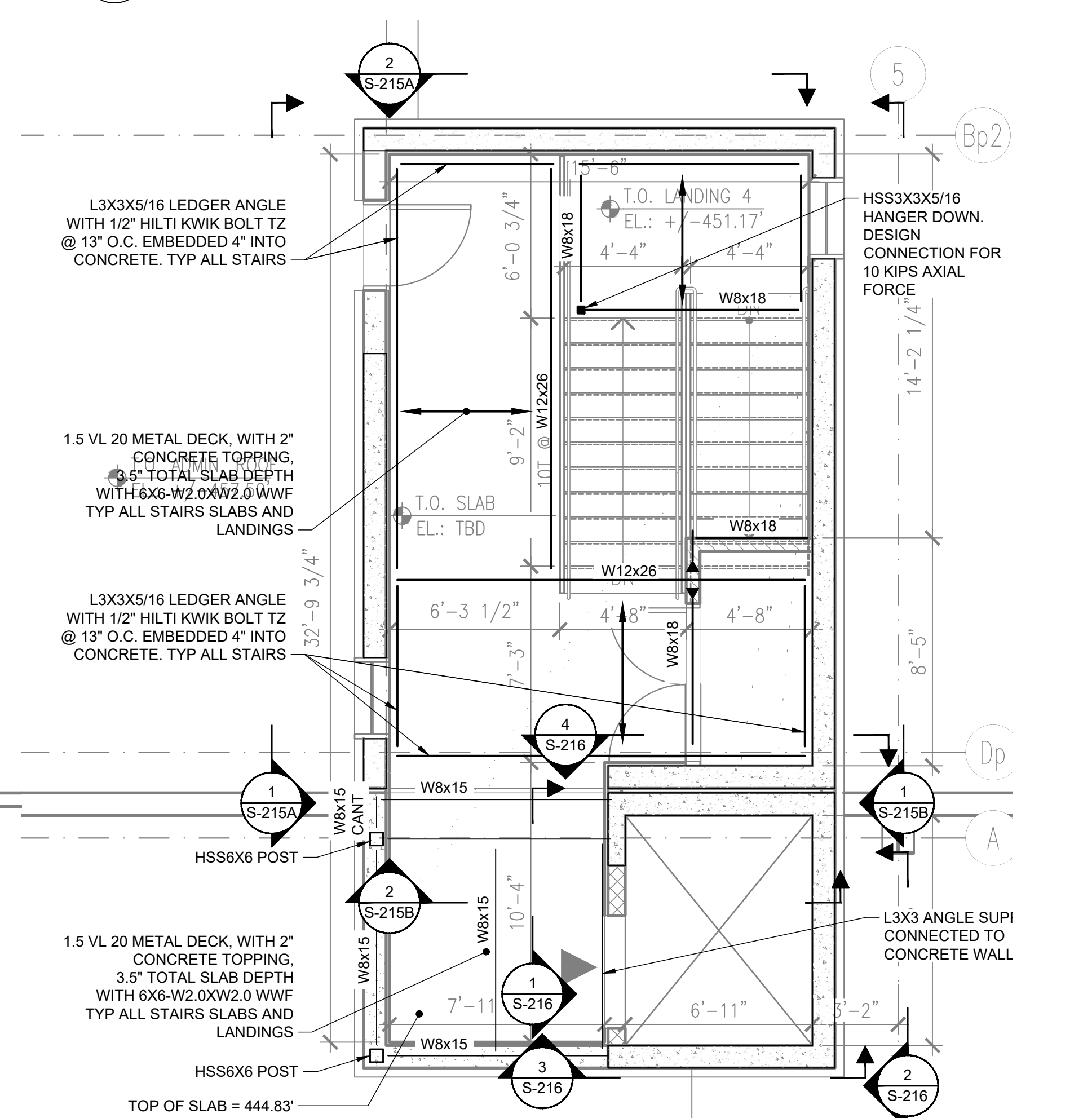
2 STAIR C - ENLARGED PLAN - 1ST MEZZ LEVEL
SCALE: 1/4" = 1'-0"



3 STAIR C - ENLARGED PLAN - 2ND MEZZ. AND PARKING DECK LEVEL
SCALE: 1/4" = 1'-0"



4 STAIR C - ENLARGED PLAN - ADMIN. OFFICE LEVEL
SCALE: 1/4" = 1'-0"



5 STAIR C - ENLARGED PLAN - BULKHEAD
SCALE: 1/4" = 1'-0"

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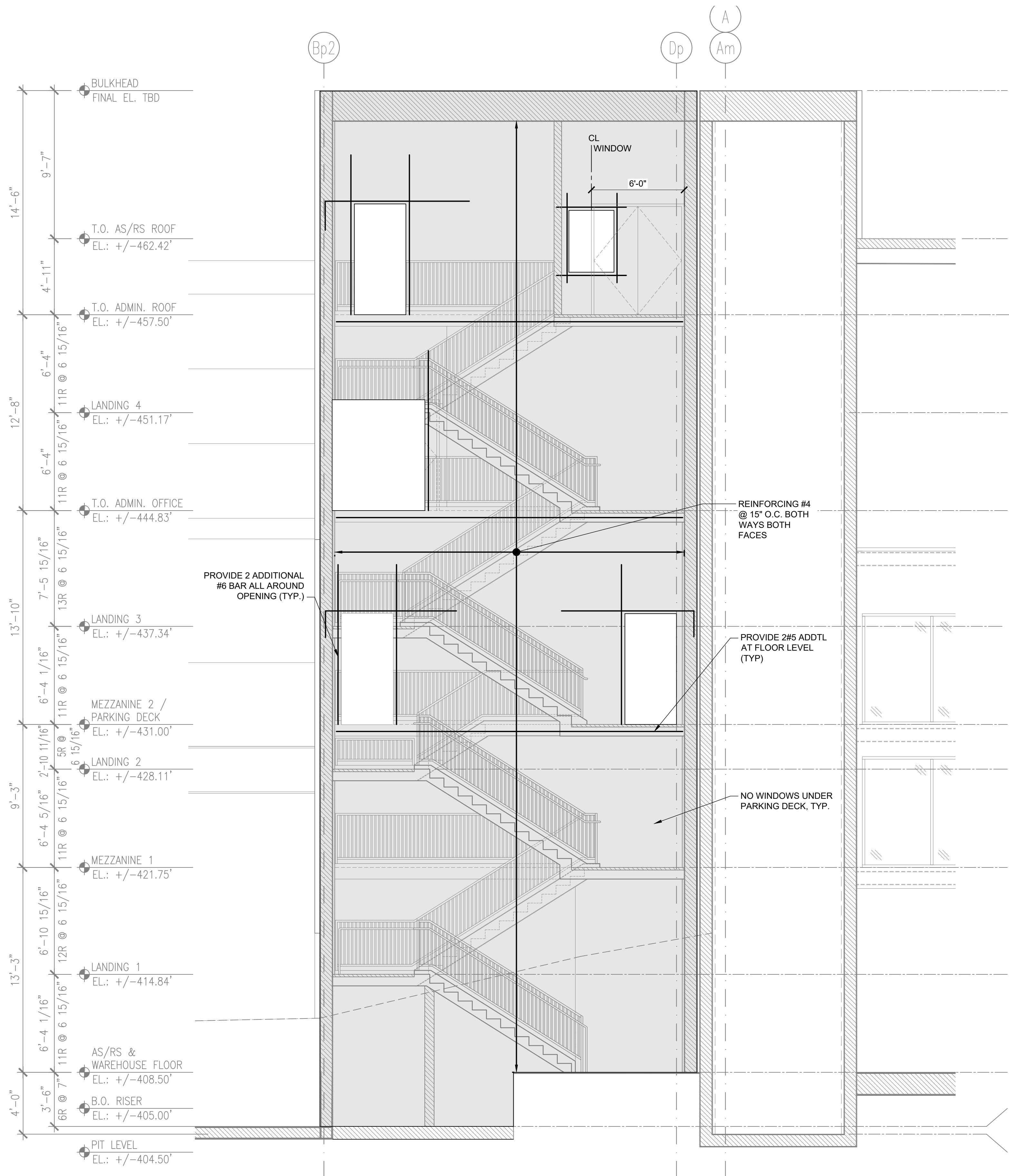
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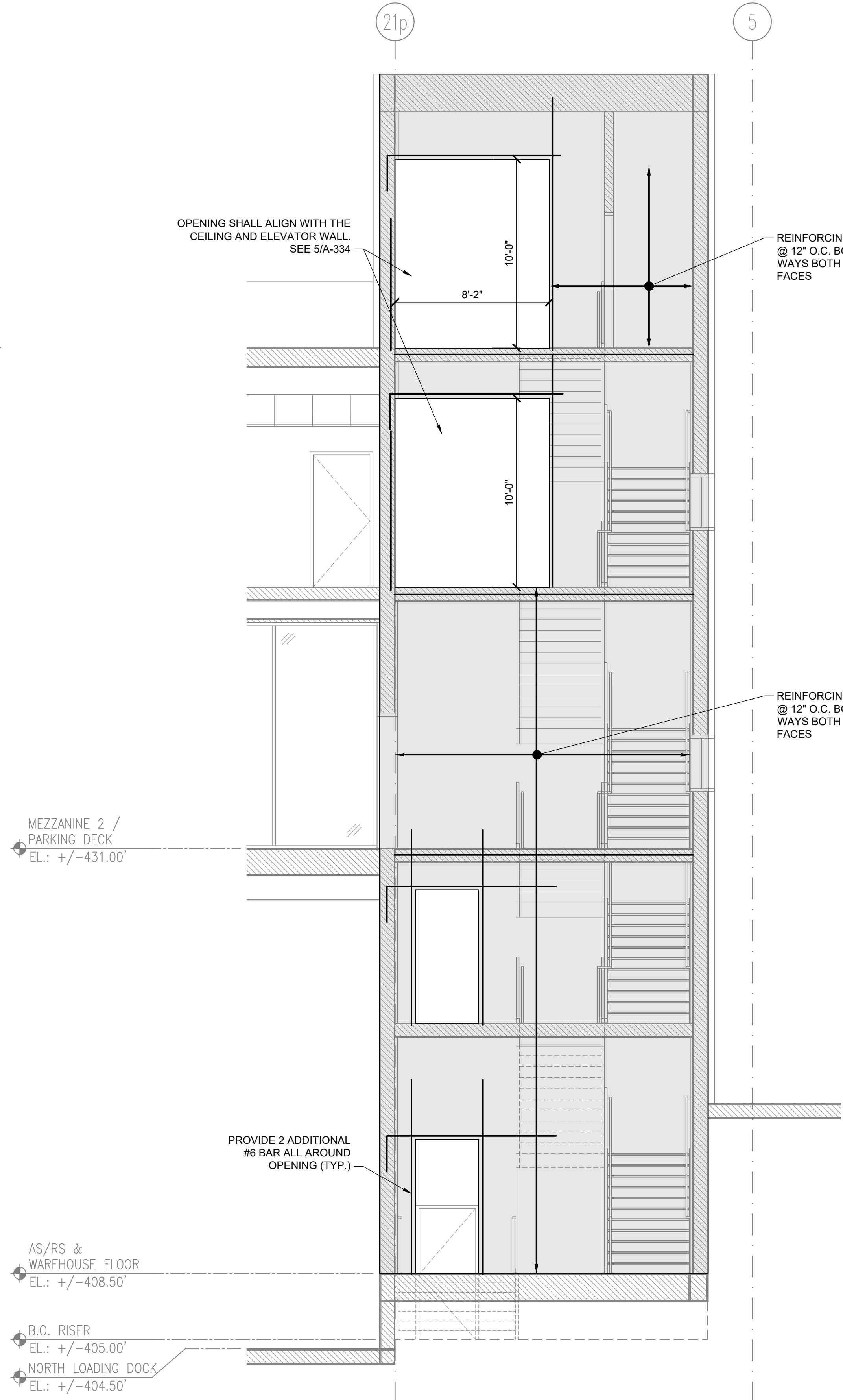
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STAIR C - PLANS
DWG NUMBER:
S-214
31 OF 60



1 STAIR C - ELEVATION (WEST)
214 SCALE: 1/4" = 1'-0"



2 STAIR C - ELEVATION (SOUTH)
214 SCALE: 1/4" = 1'-0"

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DRAWING TITLE :

STAIR C - ELEVATIONS

DWG NUMBER :
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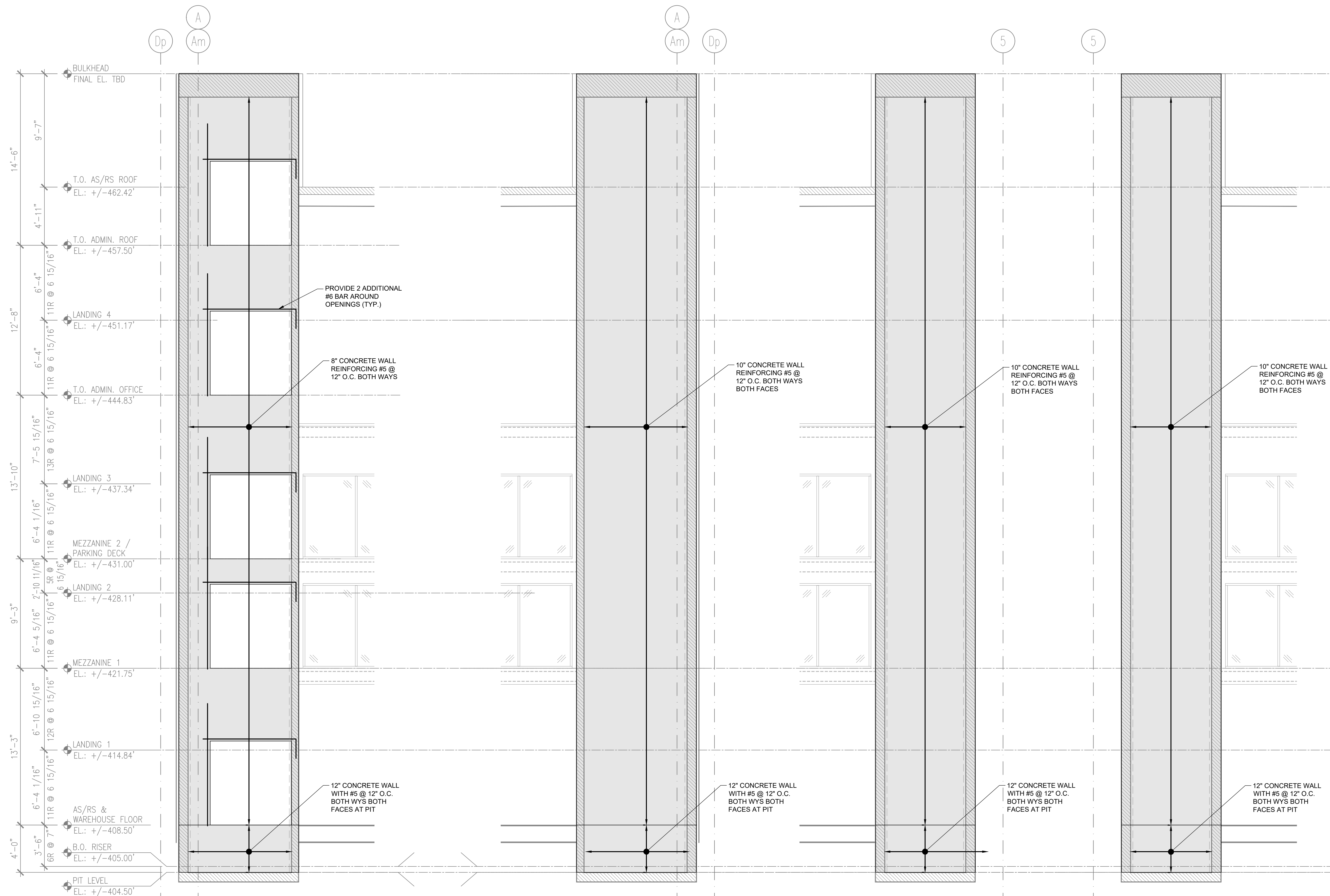
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ELEVATOR ELEVATIONS

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1
214

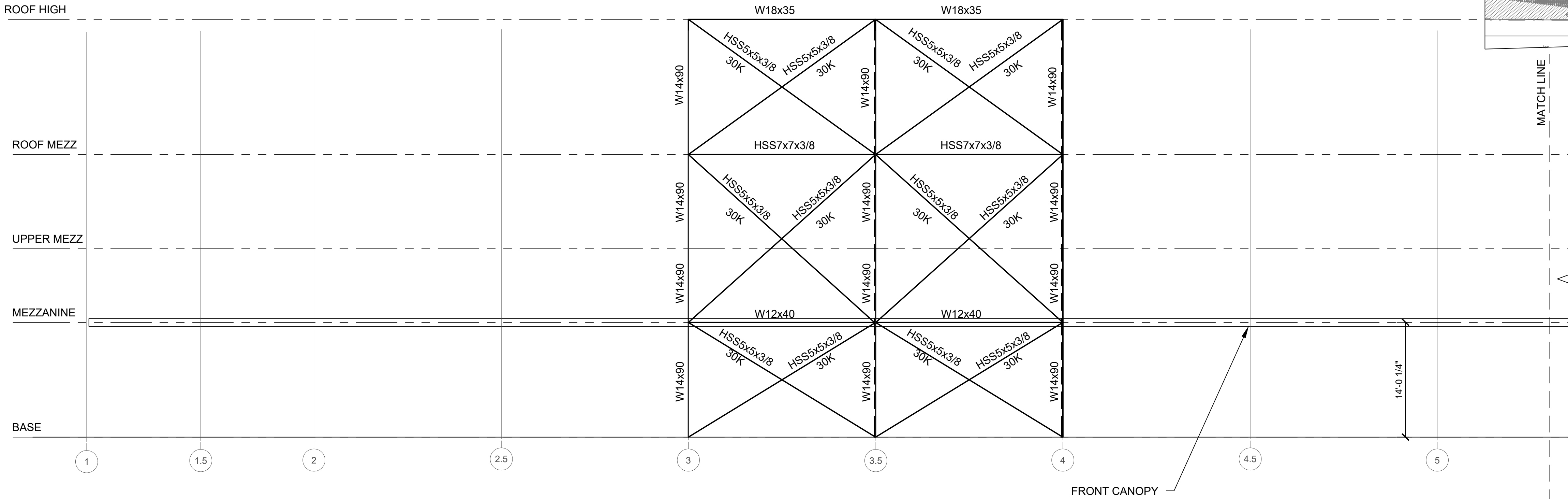
ELEVATOR - ELEVATION (WEST)

SCALE: 1/4" = 1'-0"

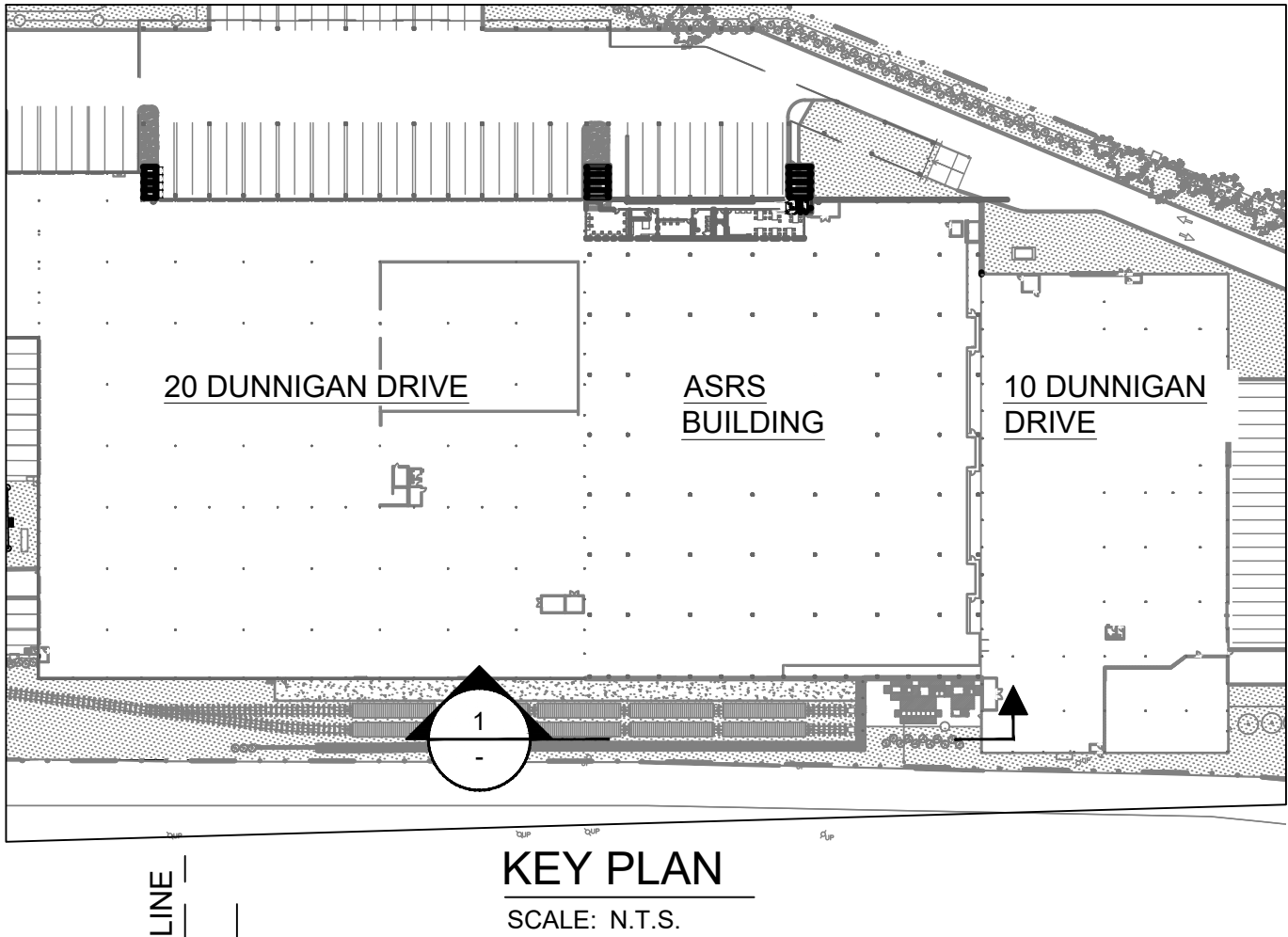
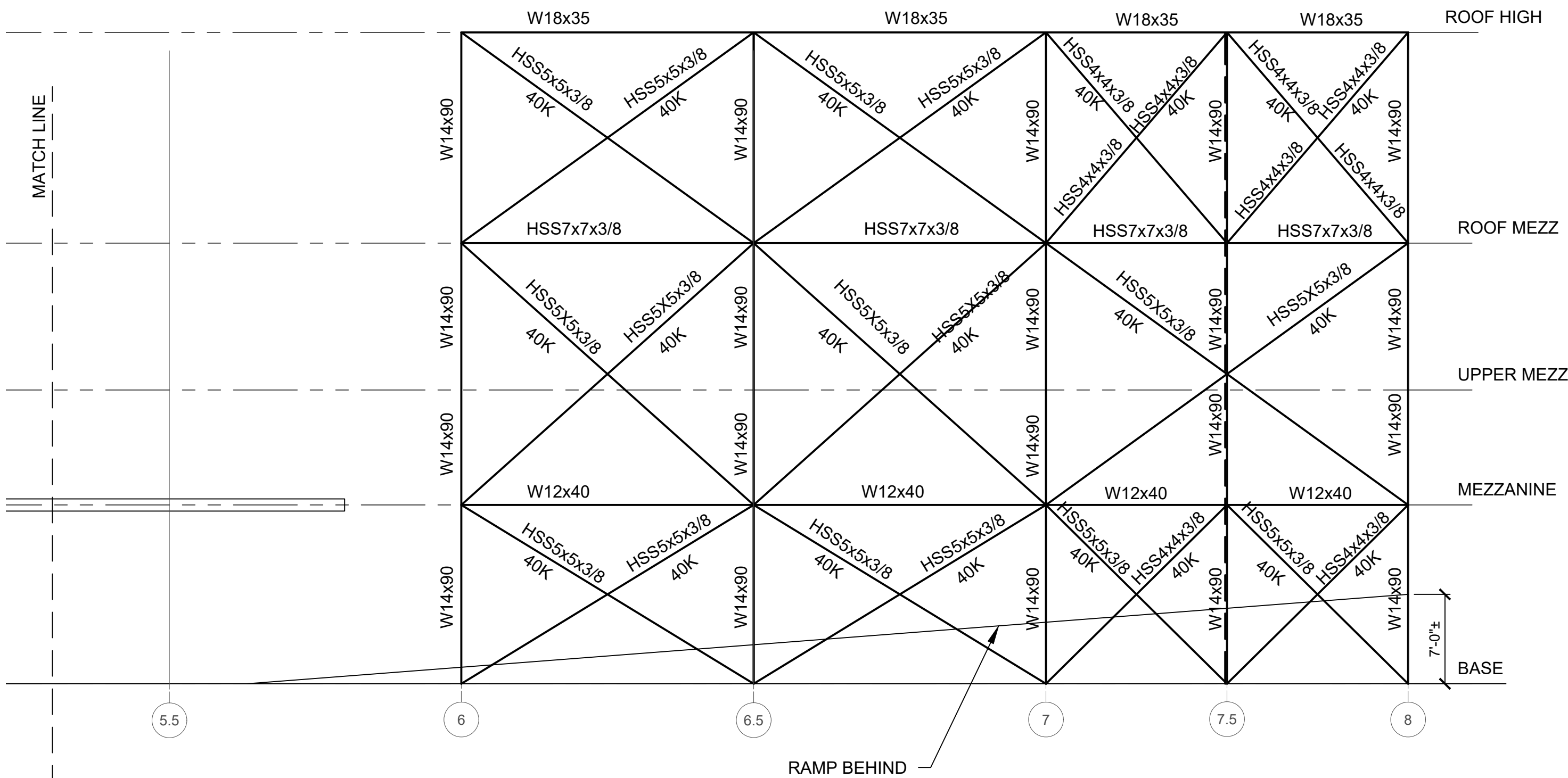
2 ELEVATOR - ELEVATION (EAST)
214 SCALE: 1/4" = 1'-0"

3 ELEVATOR - ELEVATION (SOUTH)
- SCALE: 1/4" = 1'-0"

4 ELEVATOR - ELEVATION (NORTH)
- SCALE: 1/4" = 1'-0"



1
-
ASRS BUILDING SOUTH ELEVATION
GRIDLINE I FRAME
SCALE: 1/8" = 1'-0"



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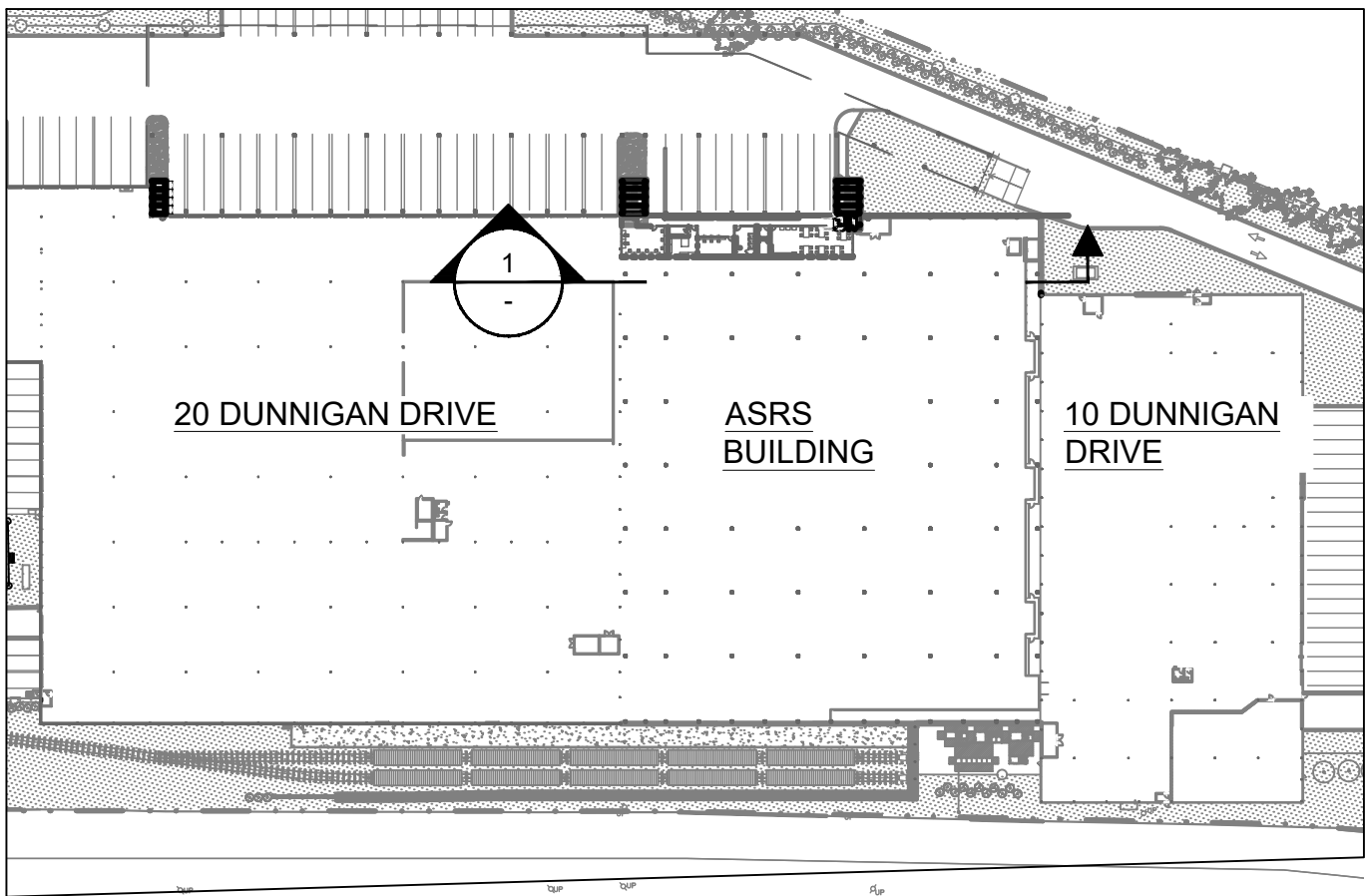
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APPROVED BY :	P. BRUNO
DATE :	9/10/2021
SCALE :	AS SHOWN

DRAWING TITLE :

LATERAL FRAMING
ELEVATIONS

DWG NUMBER :
S-301

36 OF 60



KEY PLAN
SCALE: N.T.S.

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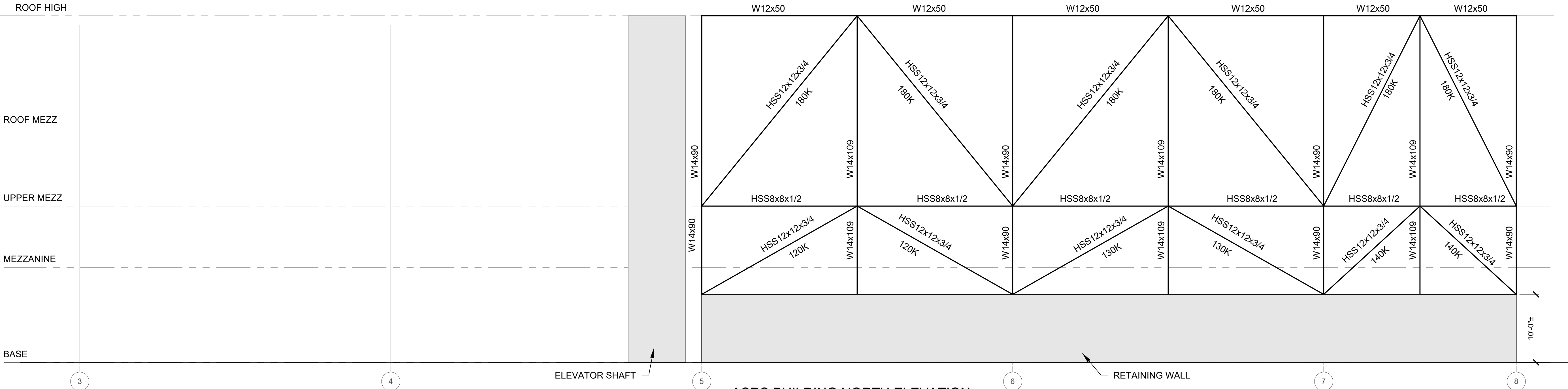
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ASRS BUILDING NORTH ELEVATION
GRIDLINE A FRAME

SCALE: 1/8" = 1'-0"

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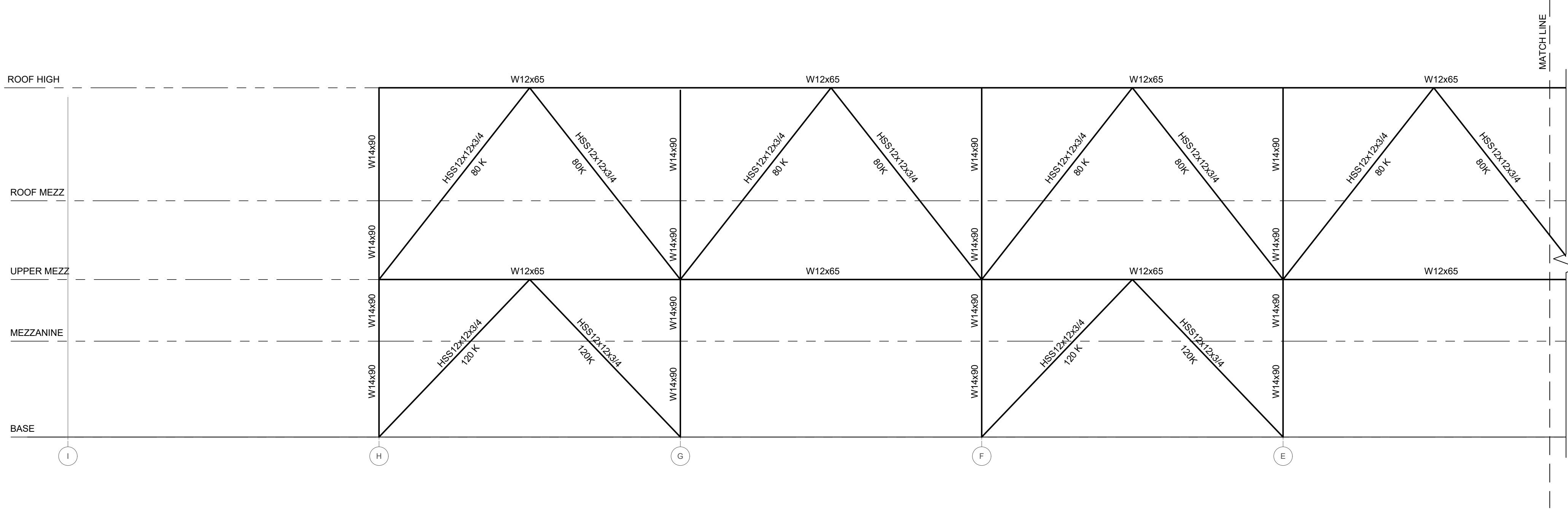
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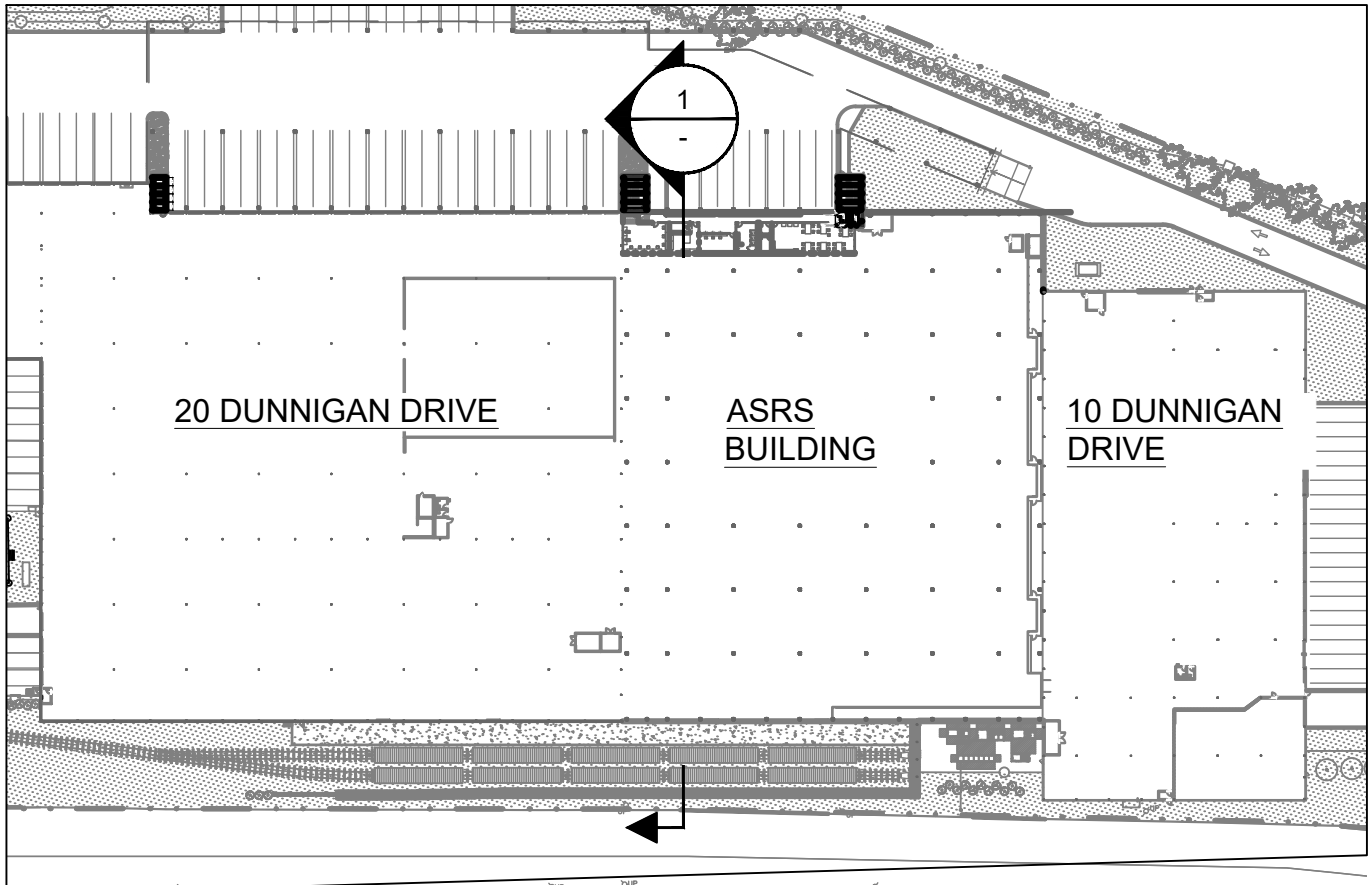
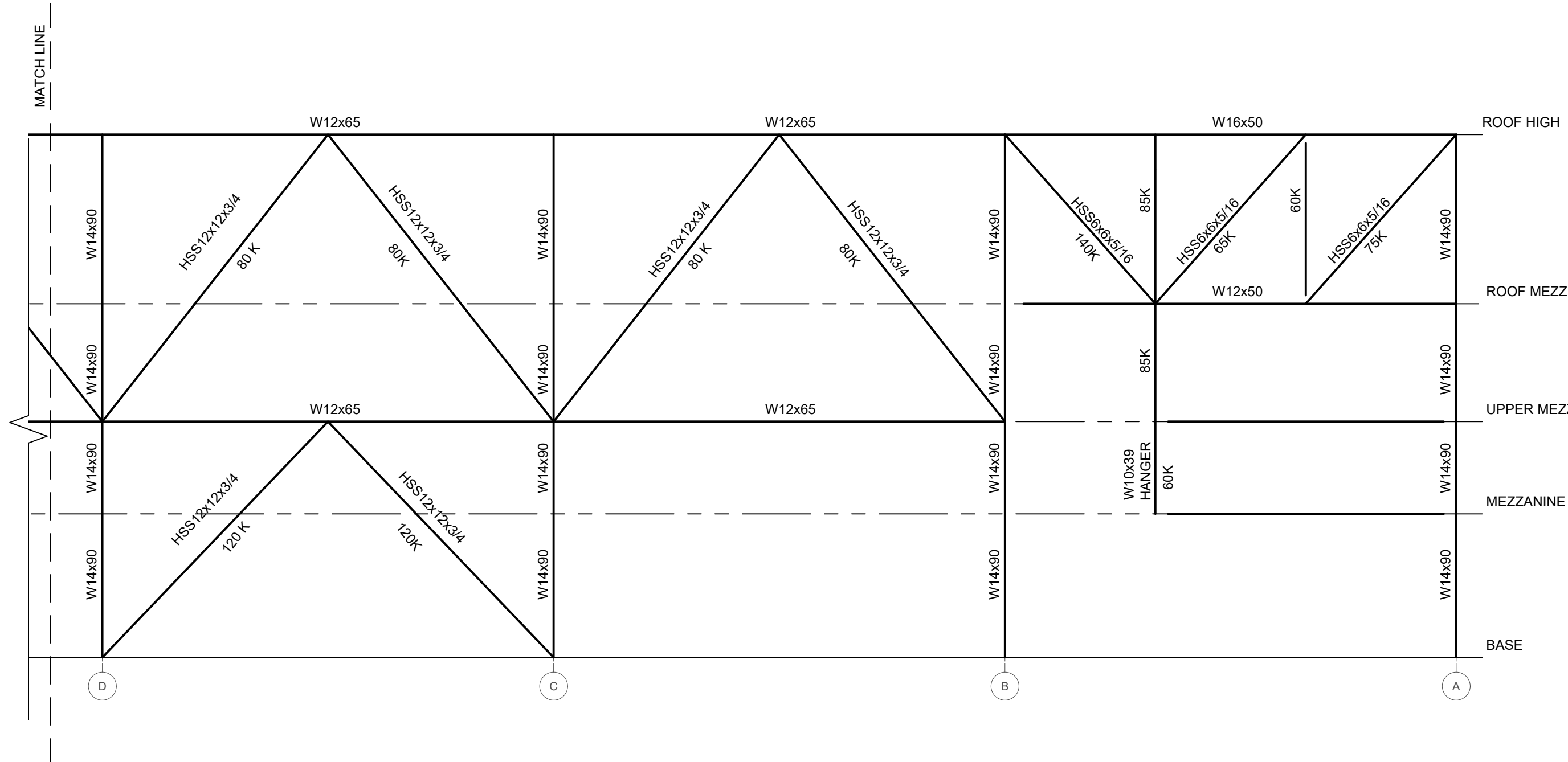
LATERAL FRAMING
ELEVATIONS

DWG NUMBER :

S-302



ASRS BUILDING WEST ELEVATION
GRIDLINE 1 FRAME
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.

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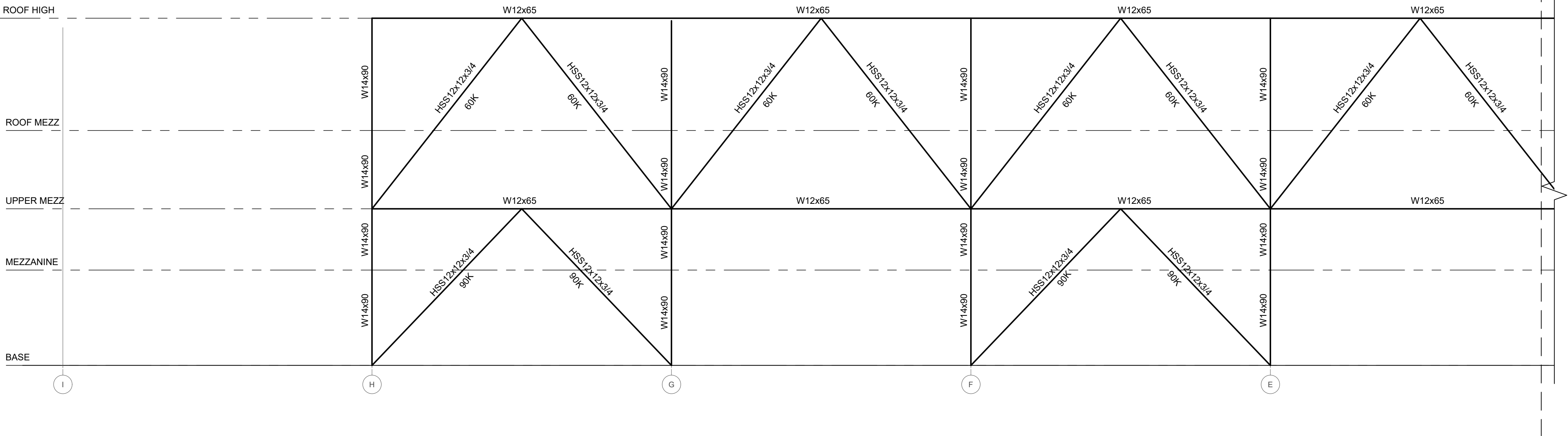
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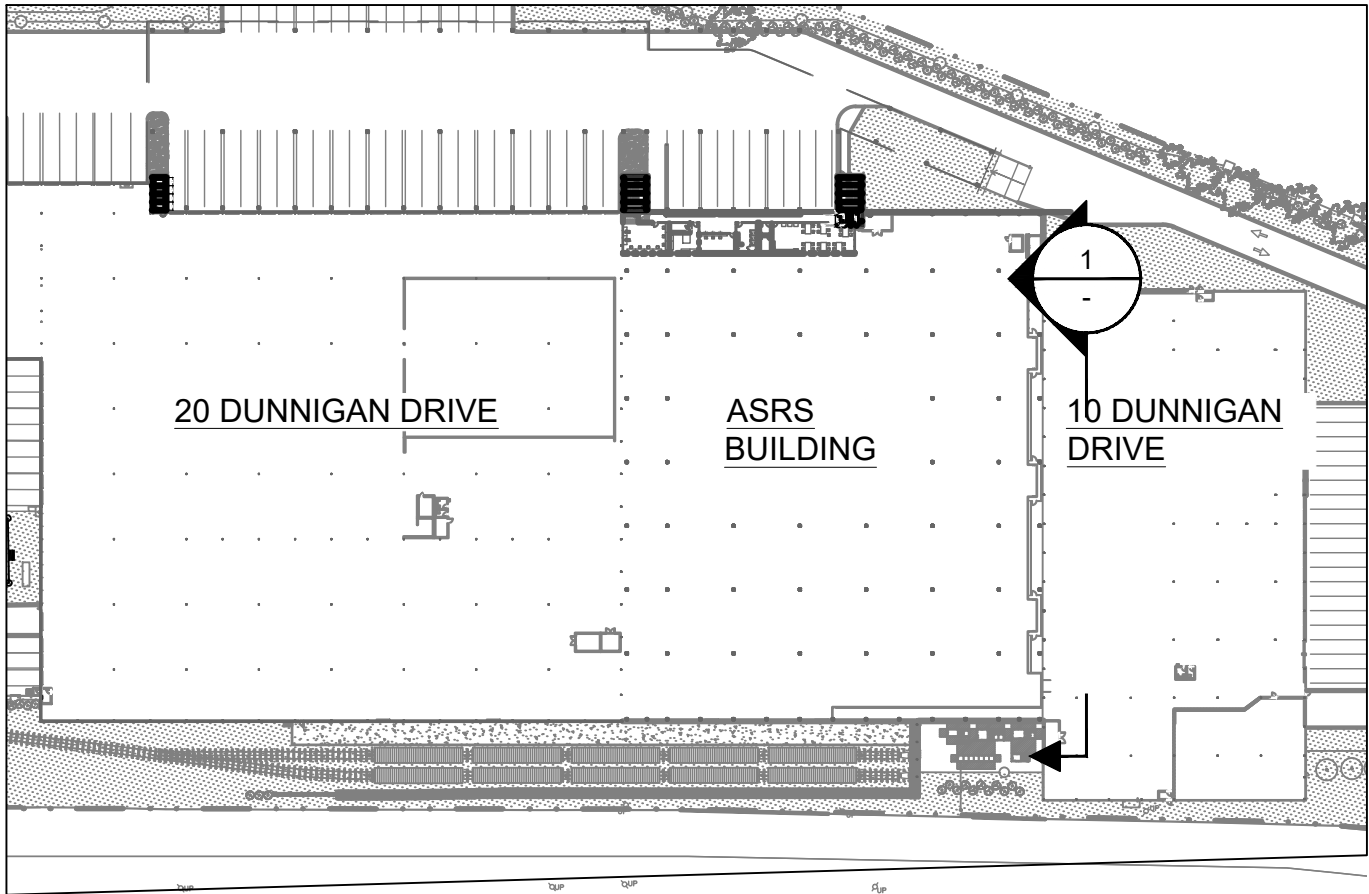
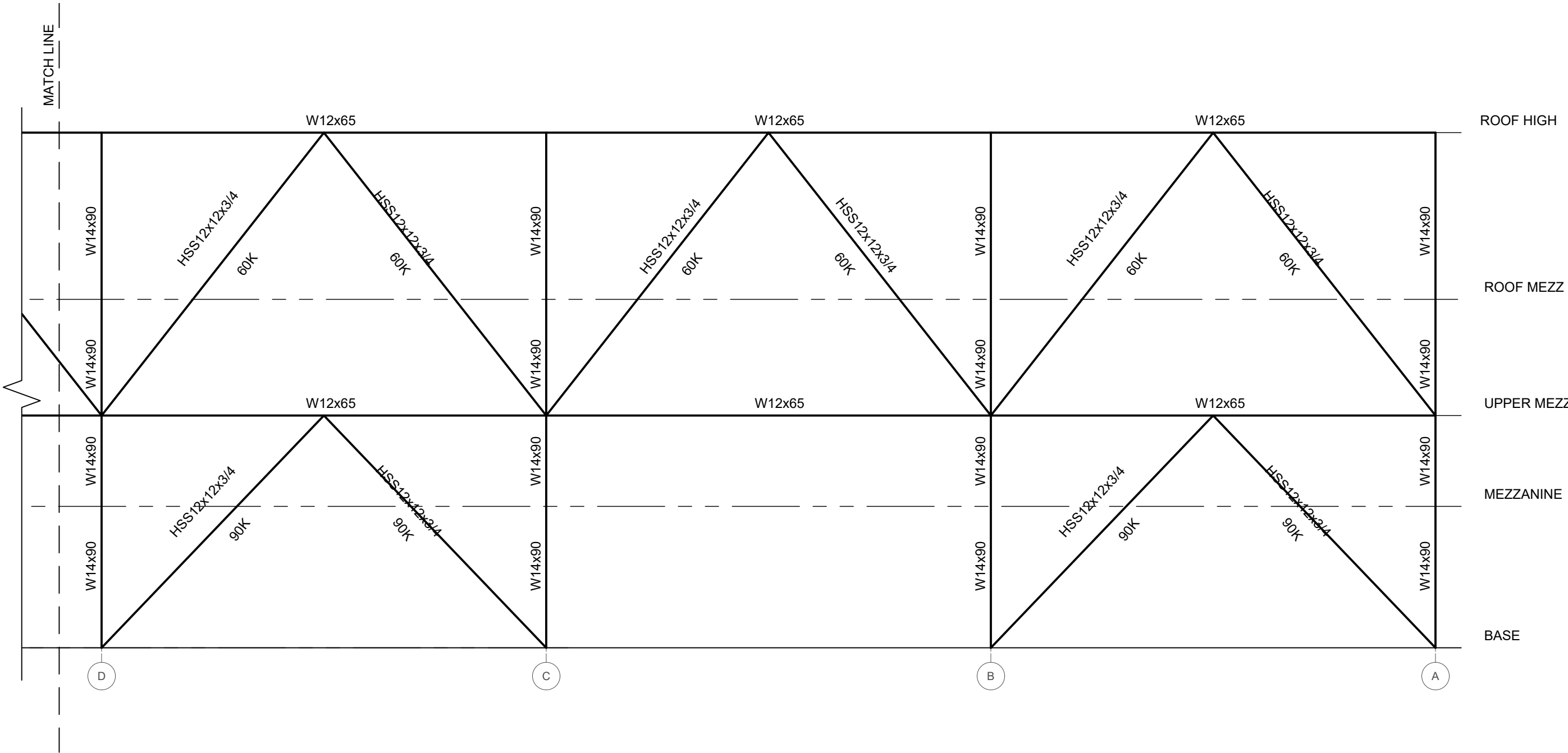
LATERAL FRAMING
ELEVATIONS

DWG NUMBER :
S-303

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ASRS BUILDING AT GRIDLINE 8
EAST ELEVATION
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.

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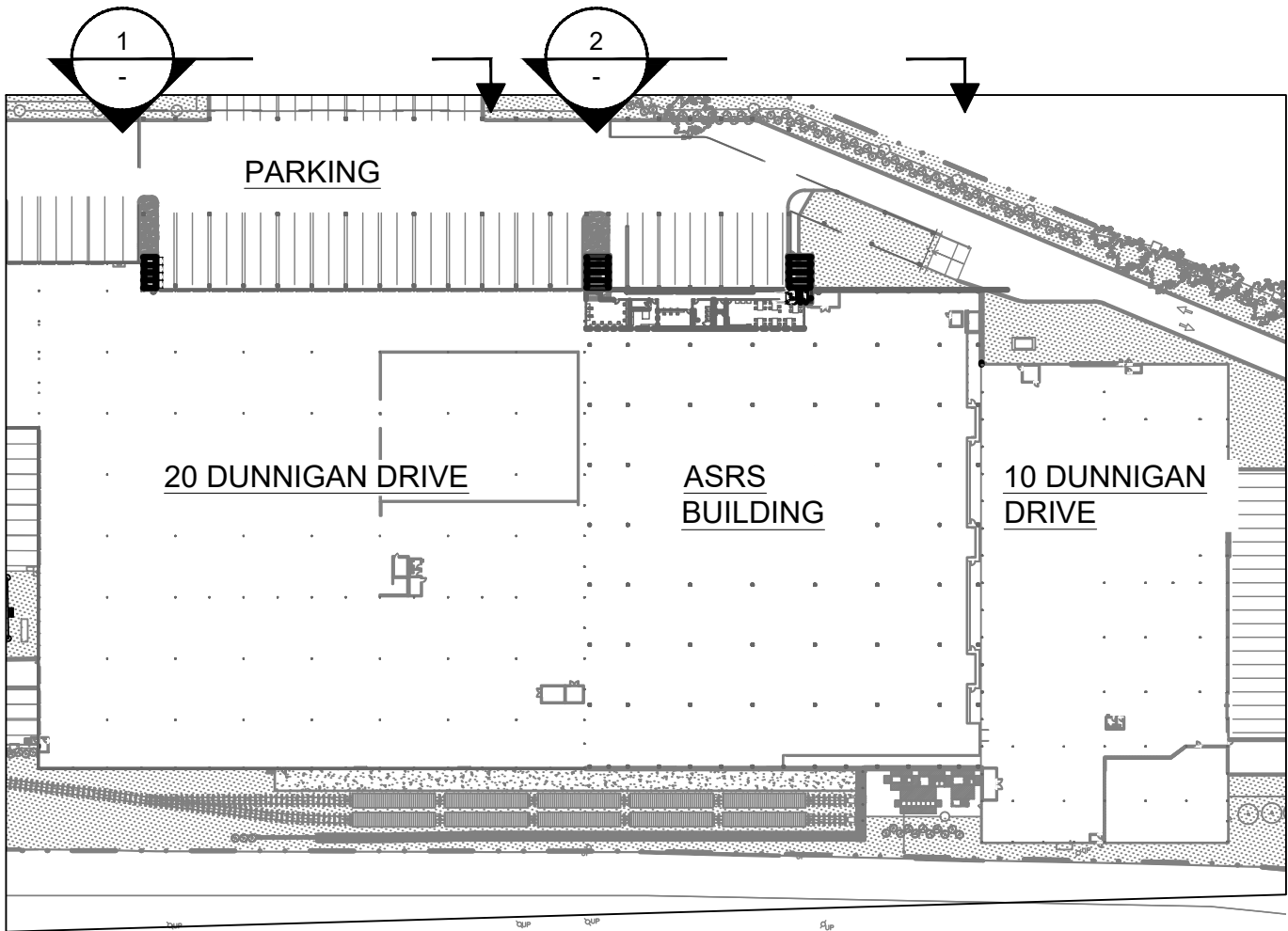
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DRAWING TITLE :

LATERAL FRAMING
ELEVATIONS

DWG NUMBER :

S-304



KEY PLAN
SCALE: N.T.S.

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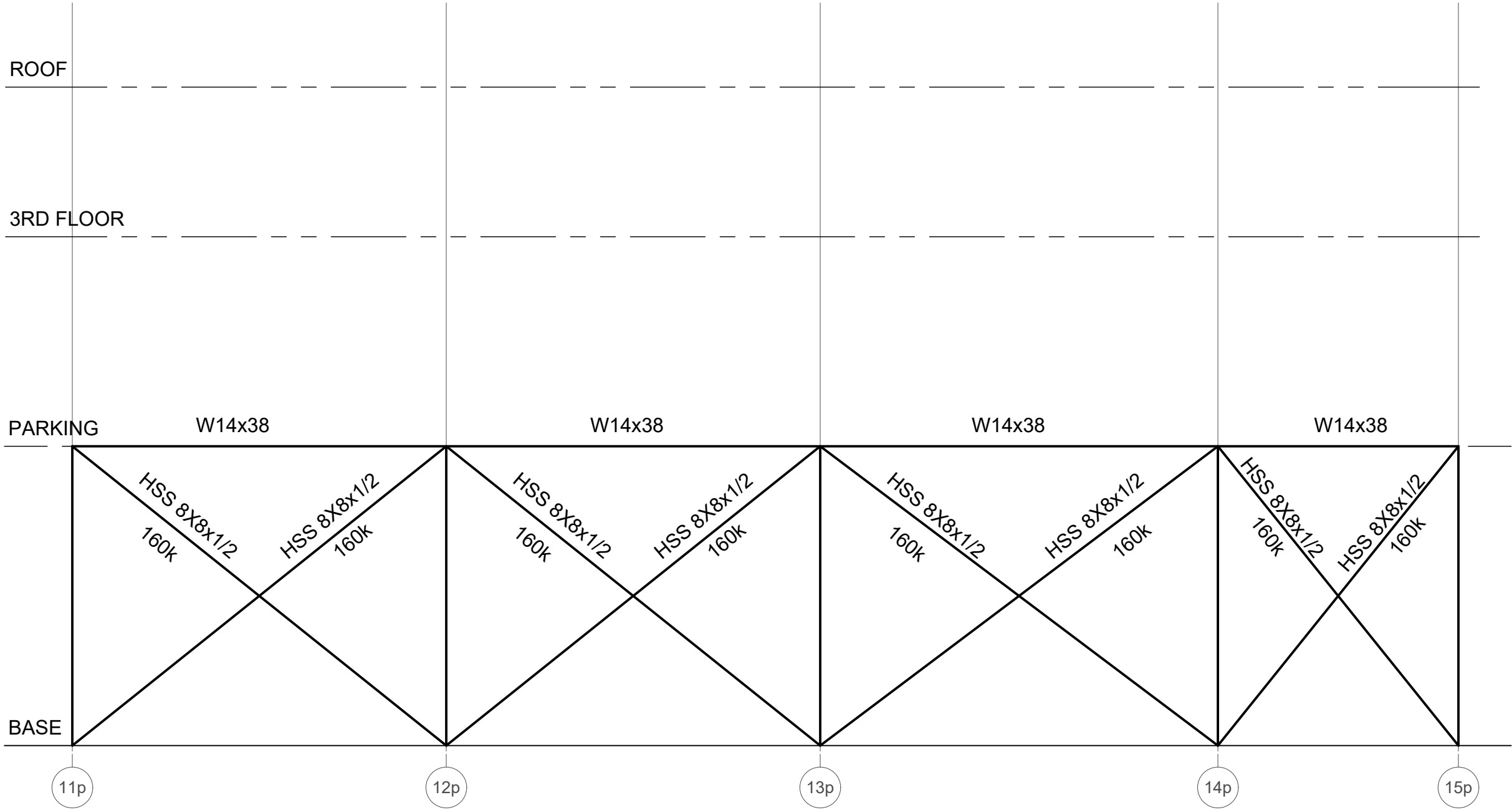
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ELEVATIONS

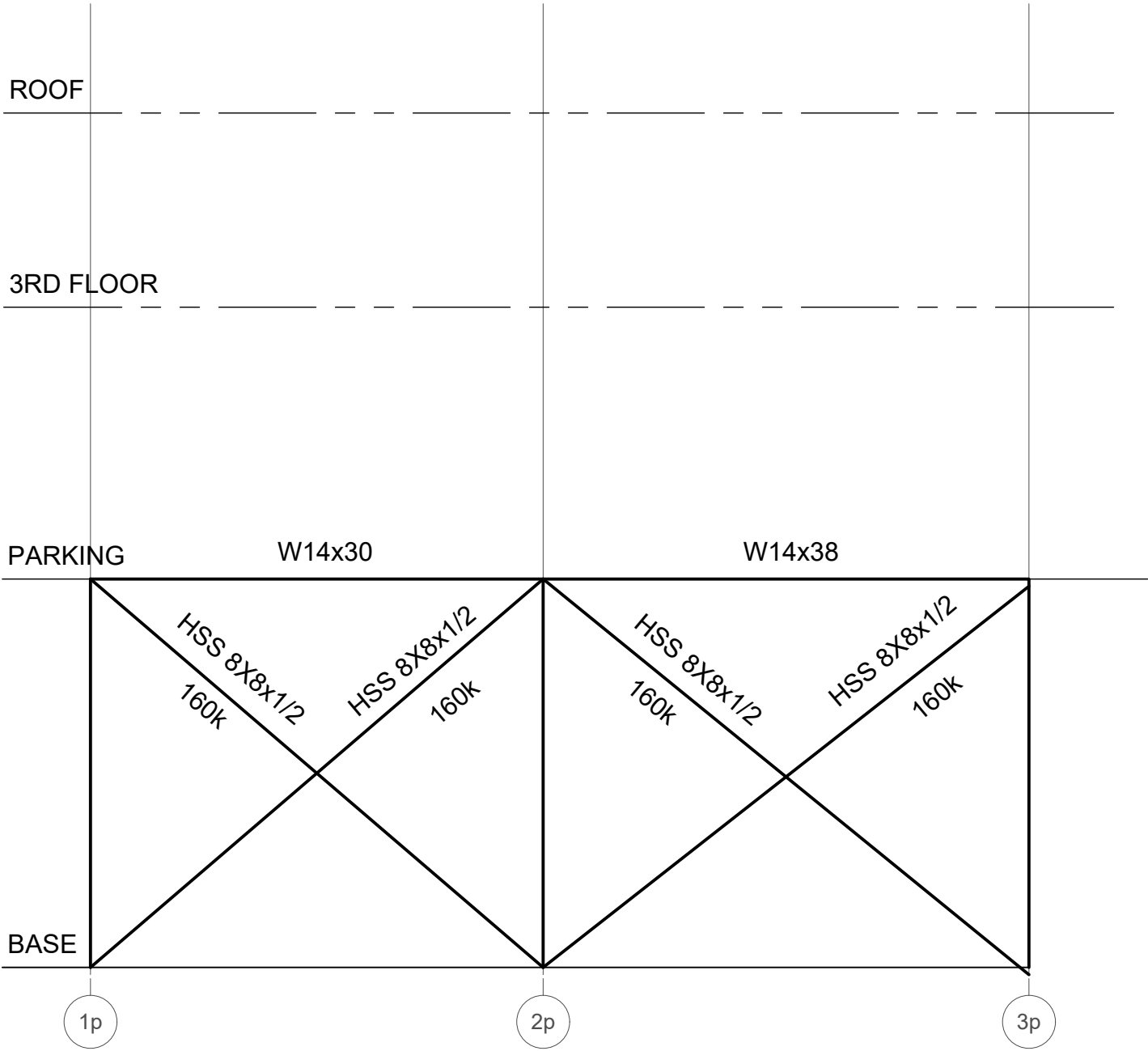
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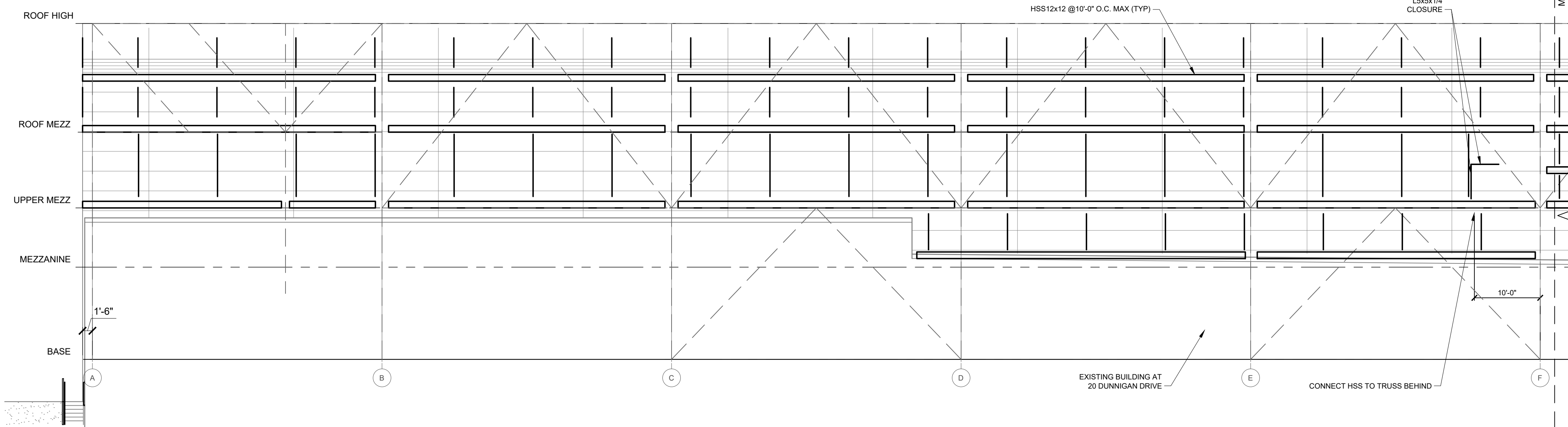
PARKING AREA NORTH ELEVATION
GRIDLINE Ap. EAST SIDE

1
-
SCALE: 1/8" = 1'-0"

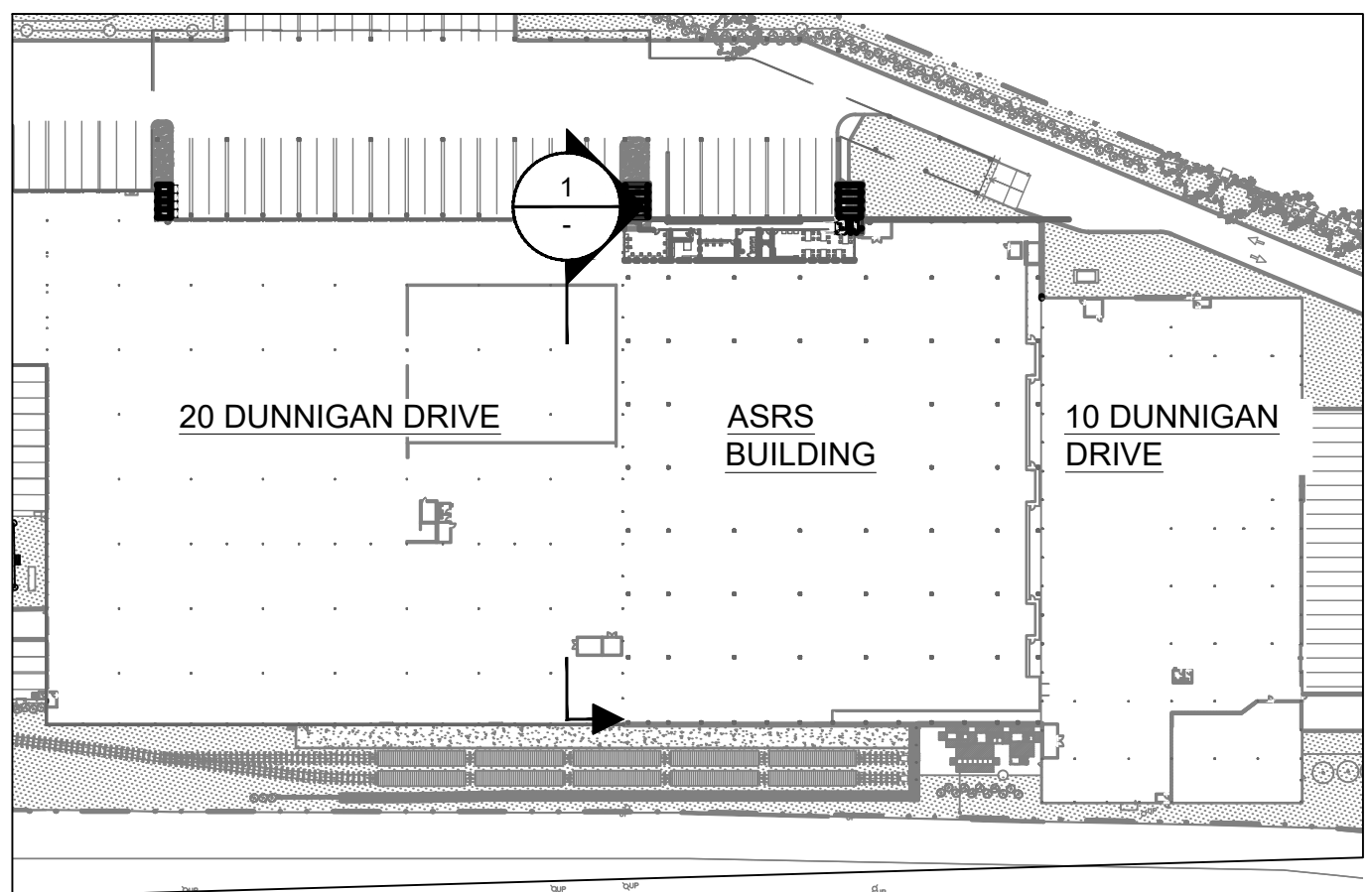


PARKING AREA NORTH ELEVATION
GRIDLINE Ap - WEST SIDE

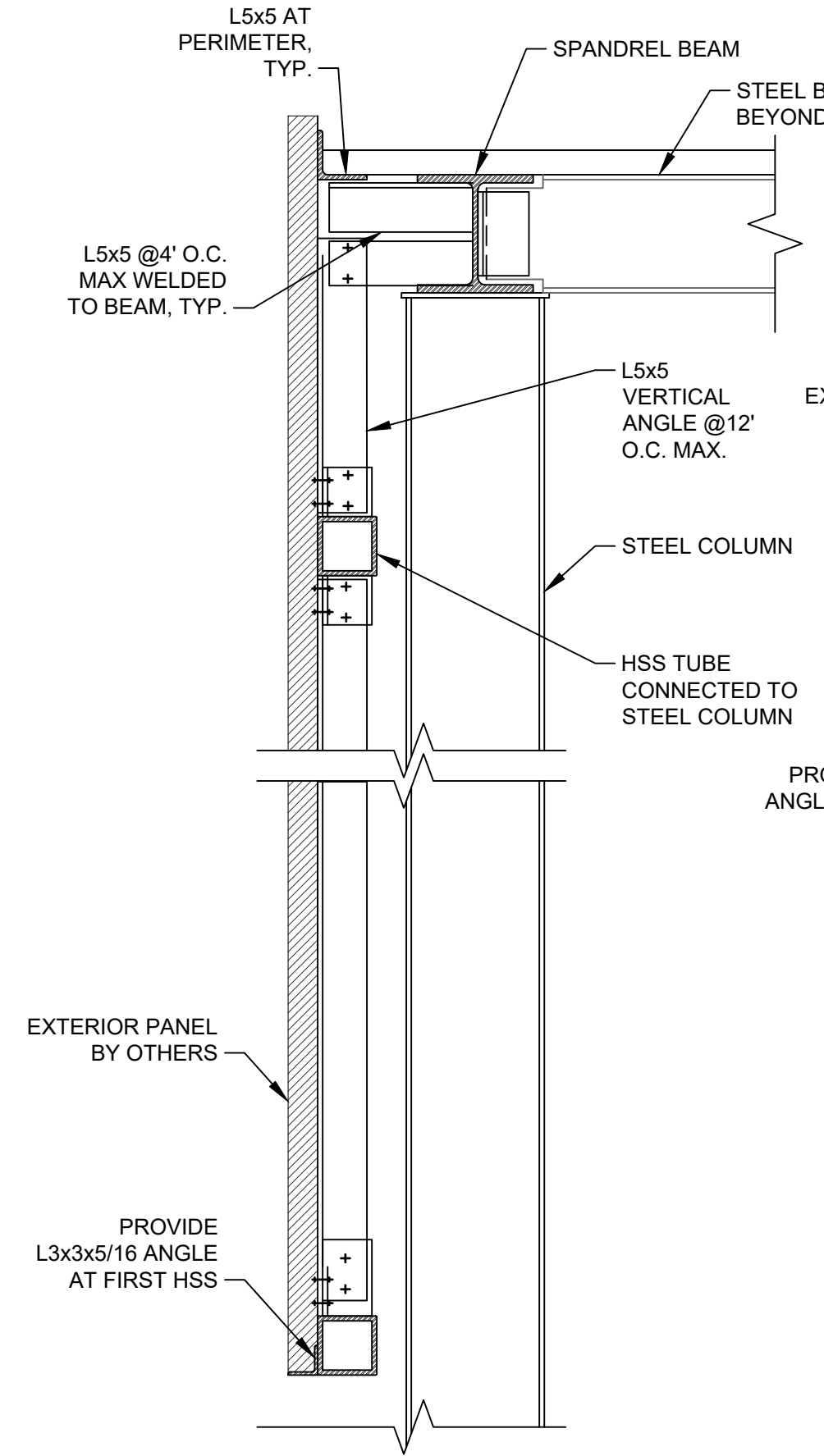
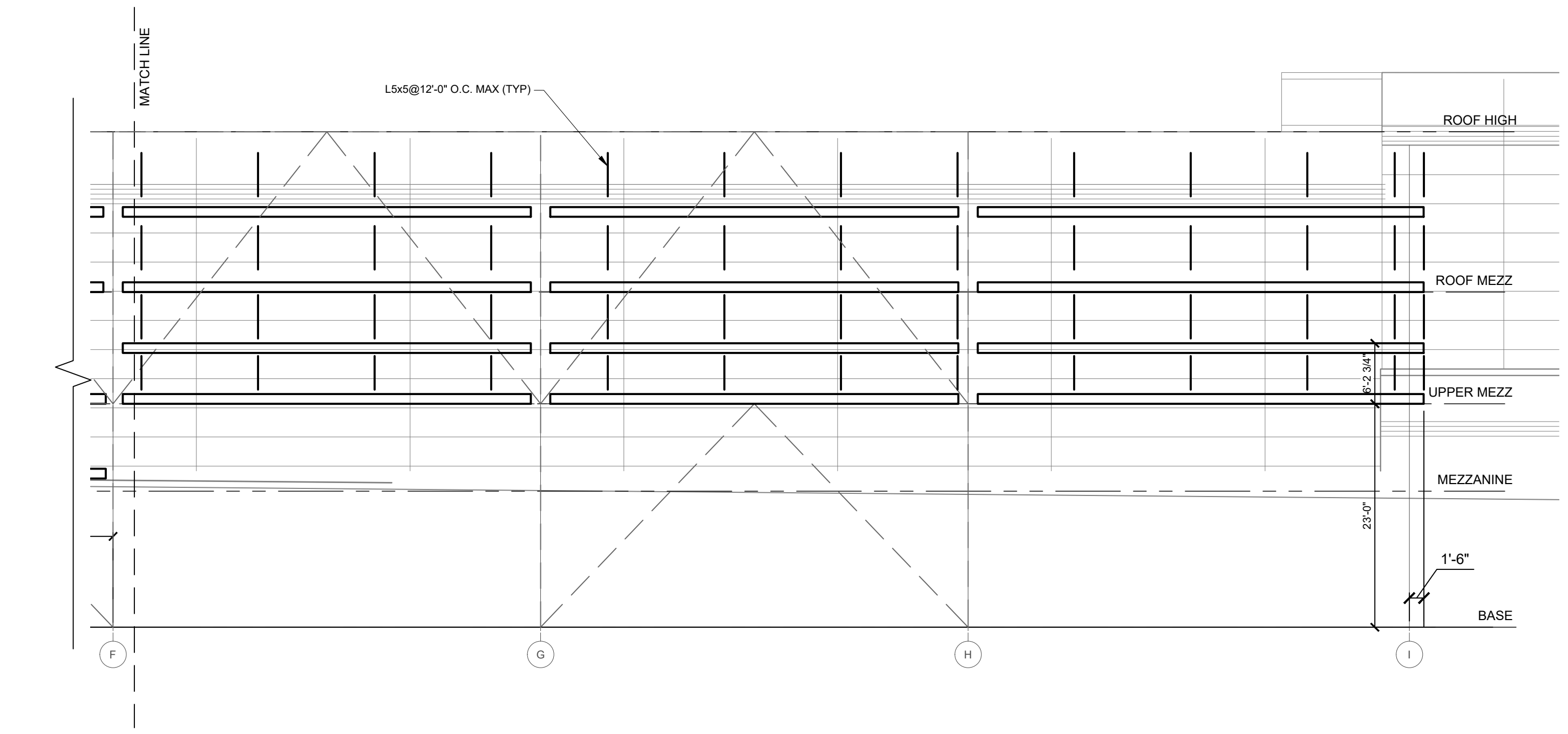
2
-
SCALE: 1/8" = 1'-0"



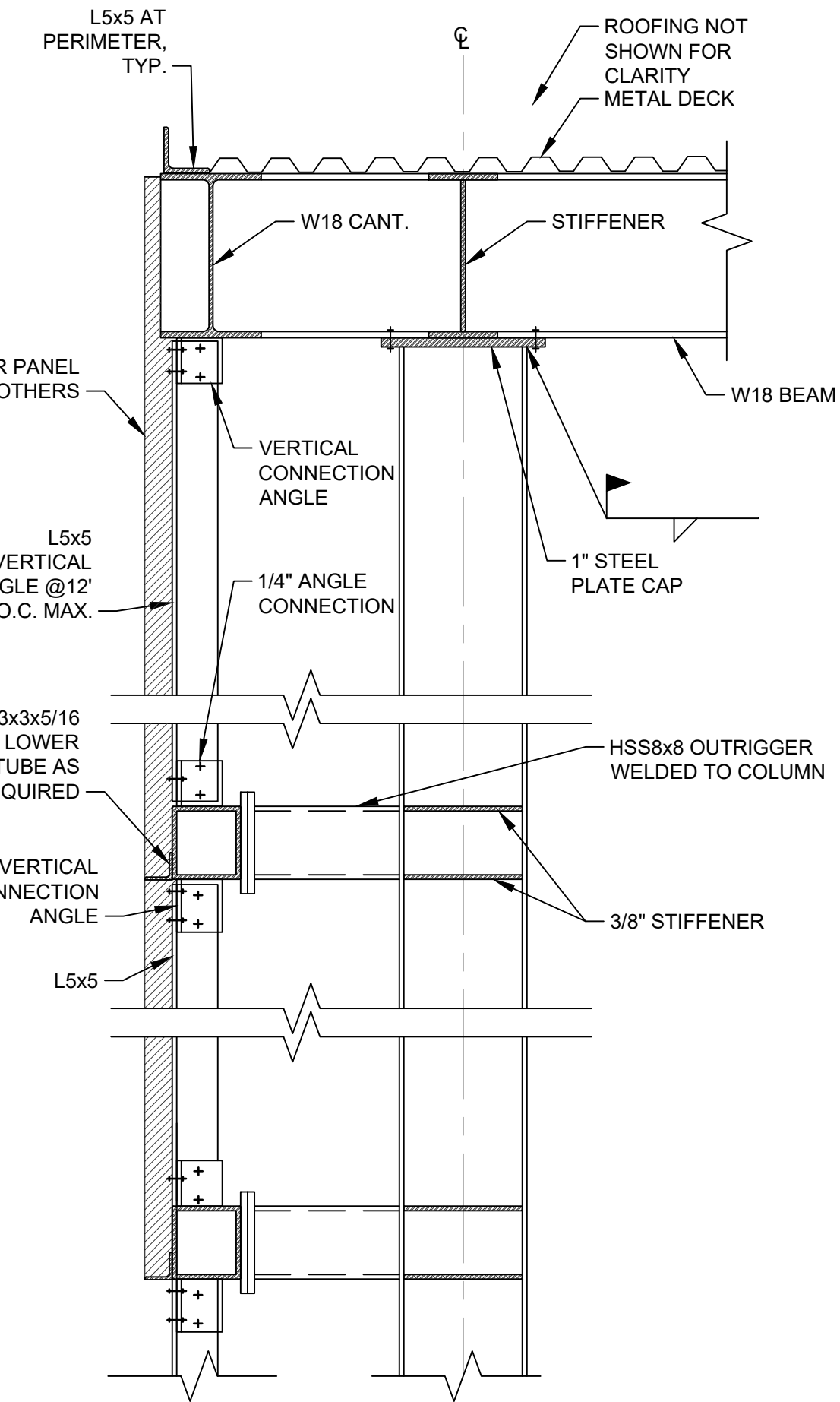
1
-
ASRS BUILDING WEST ELEVATION
EXTERIOR PANEL SUPPORTS
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



1
-
TYP. EXTERIOR SECTION
SCALE: 3/4" = 1'-0"



2
-
TYP. EXTERIOR SECTION AT EAST AND WEST WALL
SCALE: 3/4" = 1'-0"

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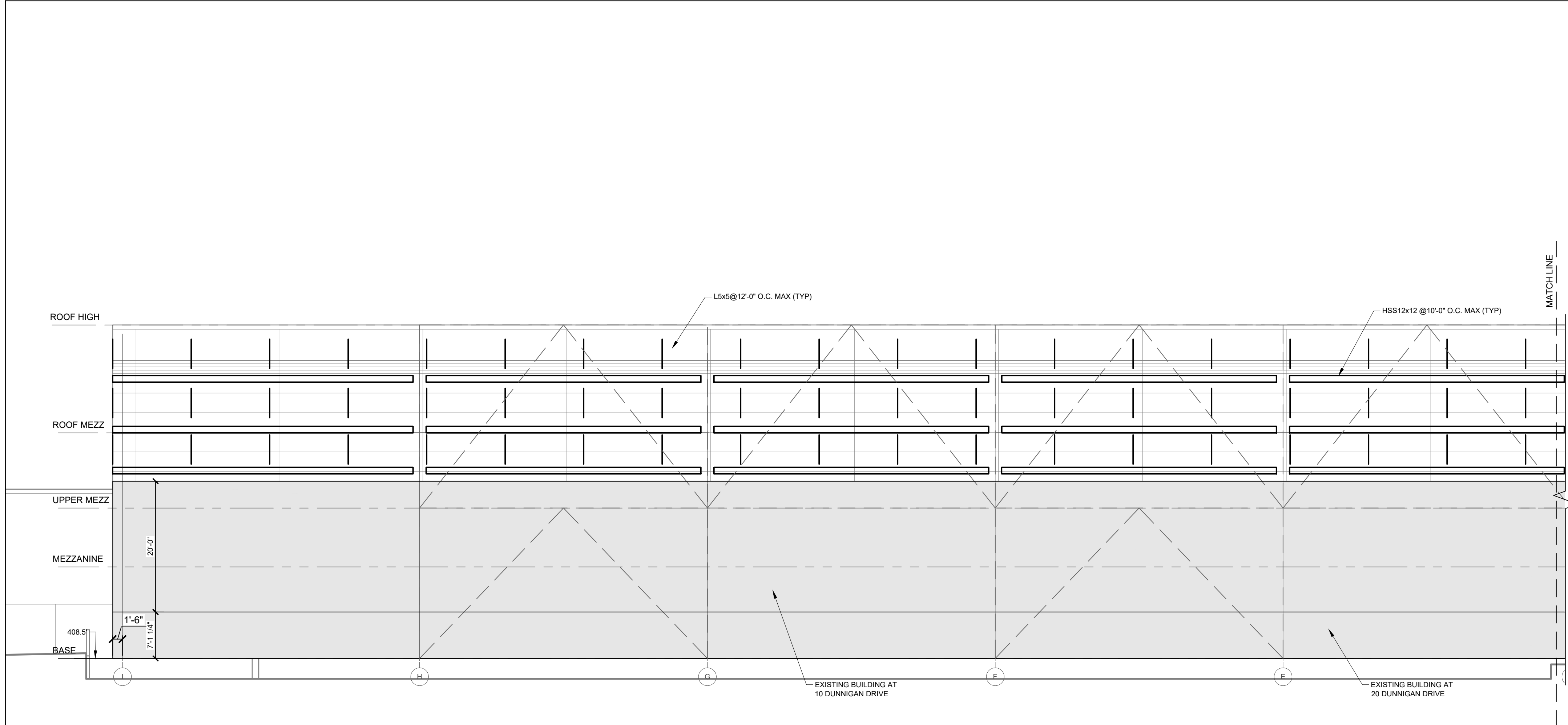
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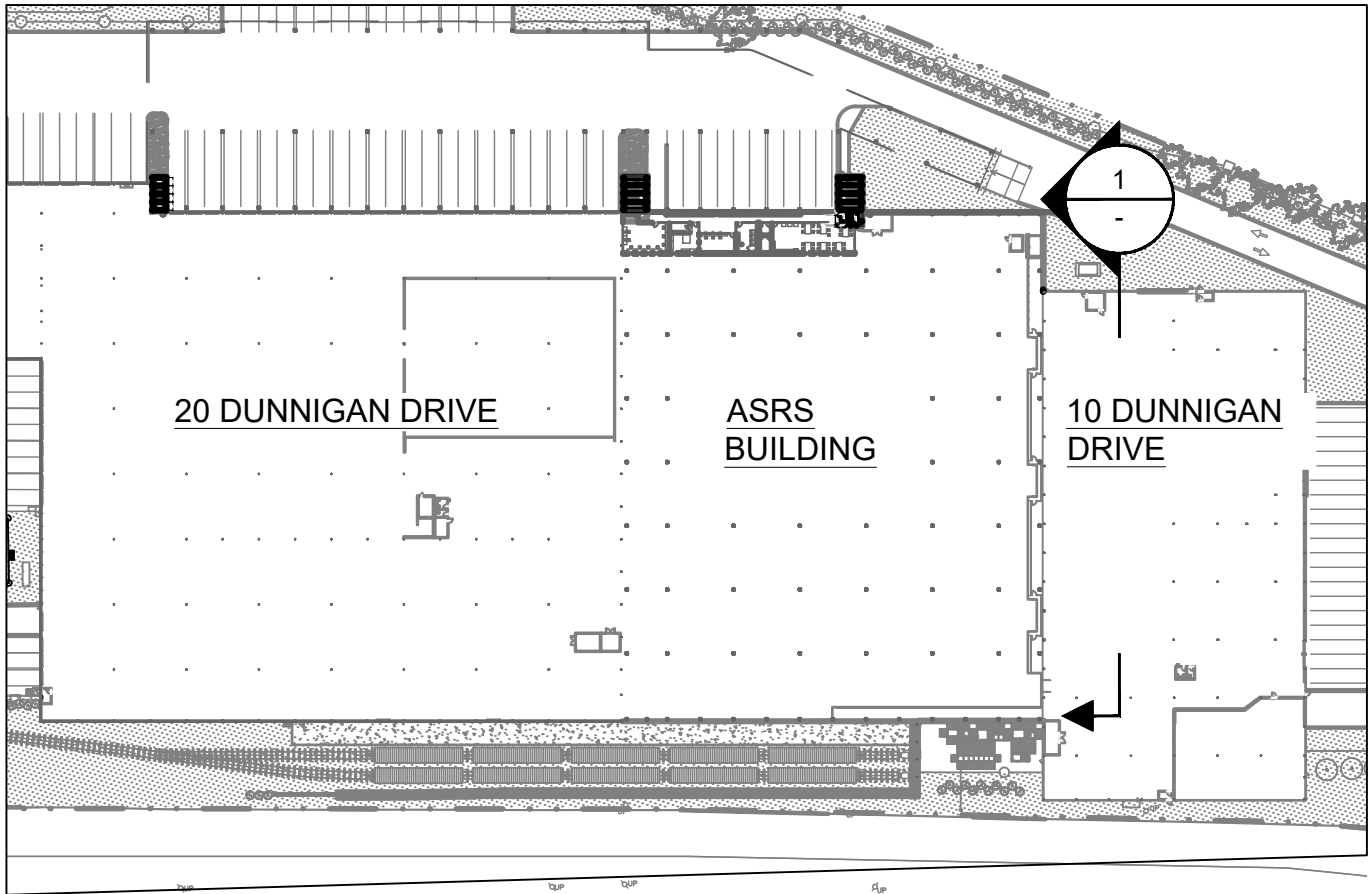
EXTERIOR PANEL
SUPPORTS

DWG NUMBER :

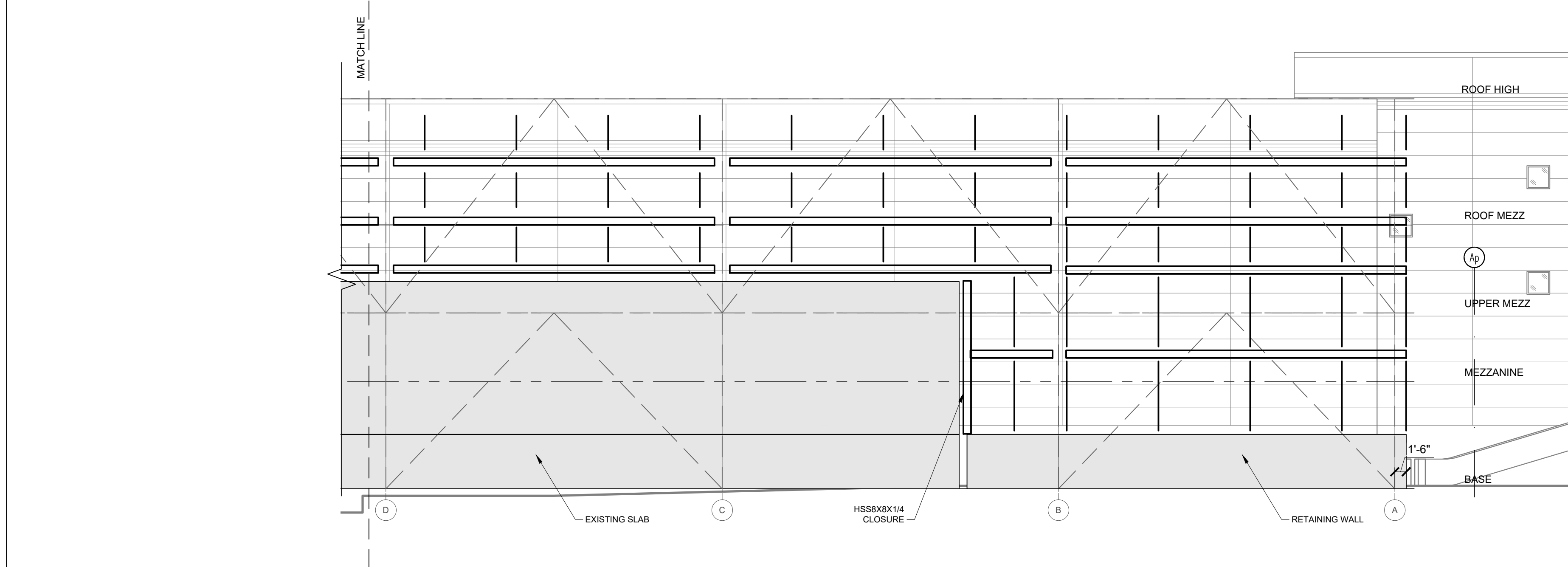
S-310



1
-
ASRS BUILDING EAST ELEVATION
EXTERIOR PANEL SUPPORTS
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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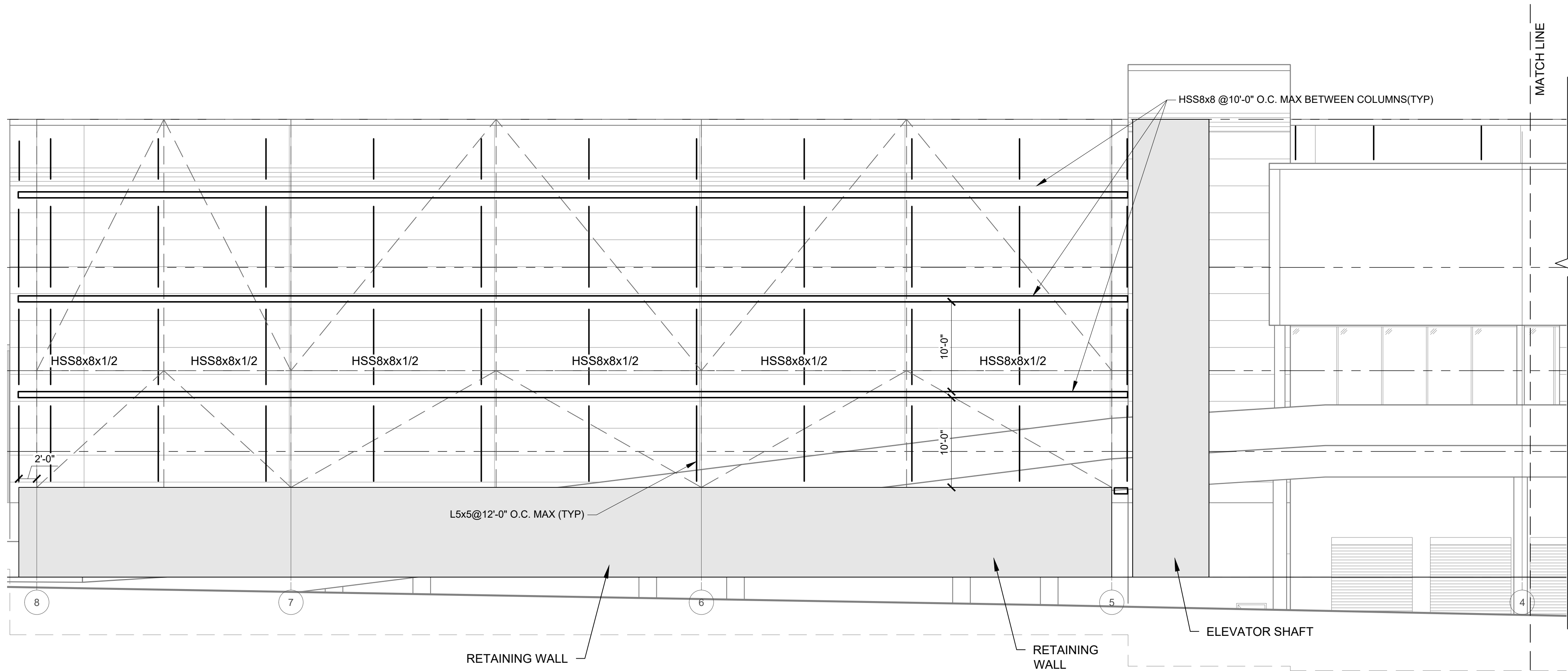
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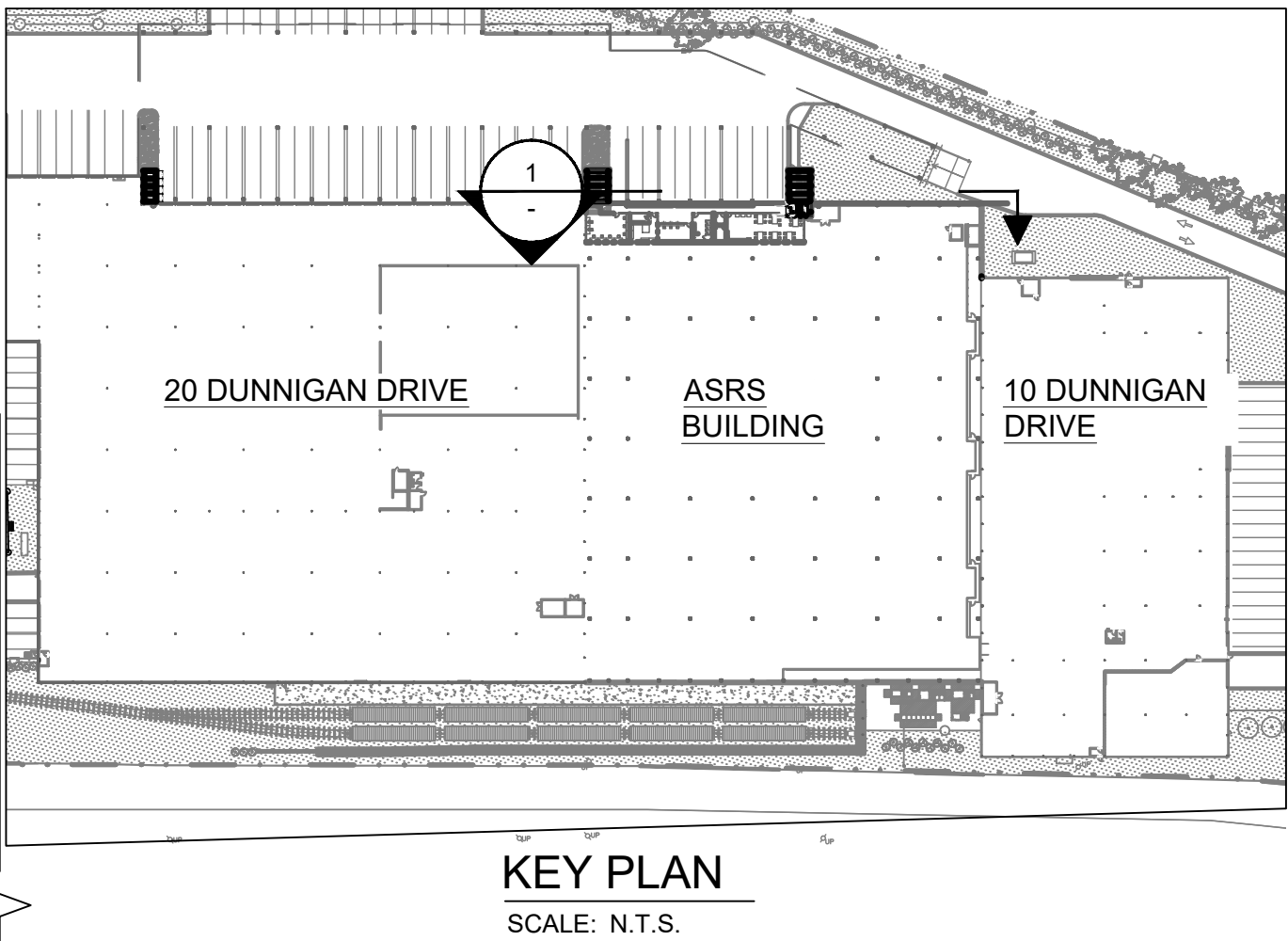
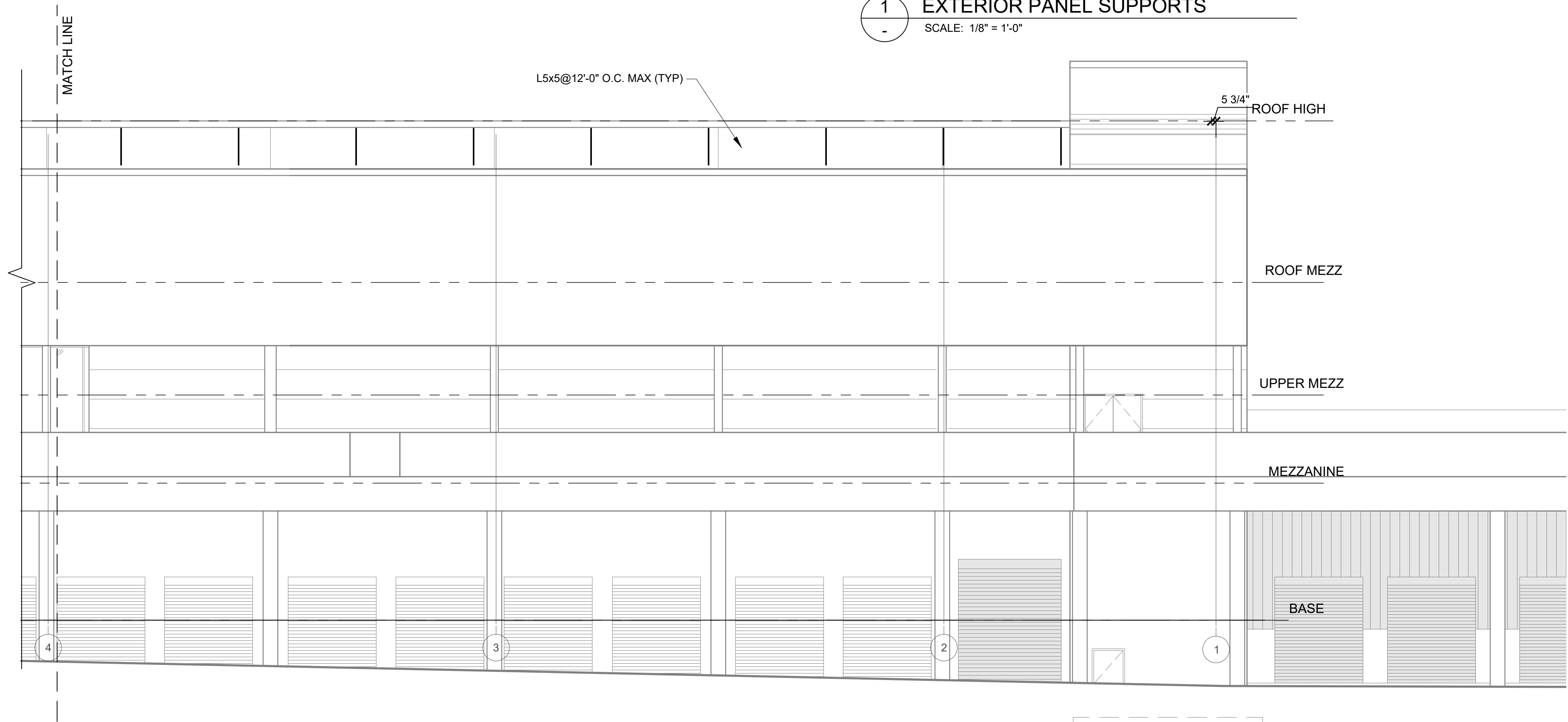
EXTERIOR PANEL
SUPPORTS

DWG NUMBER :
S-311

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ASRS BUILDING NORTH ELEVATION
EXTERIOR PANEL SUPPORTS
1
-
SCALE: 1/8" = 1'-0"



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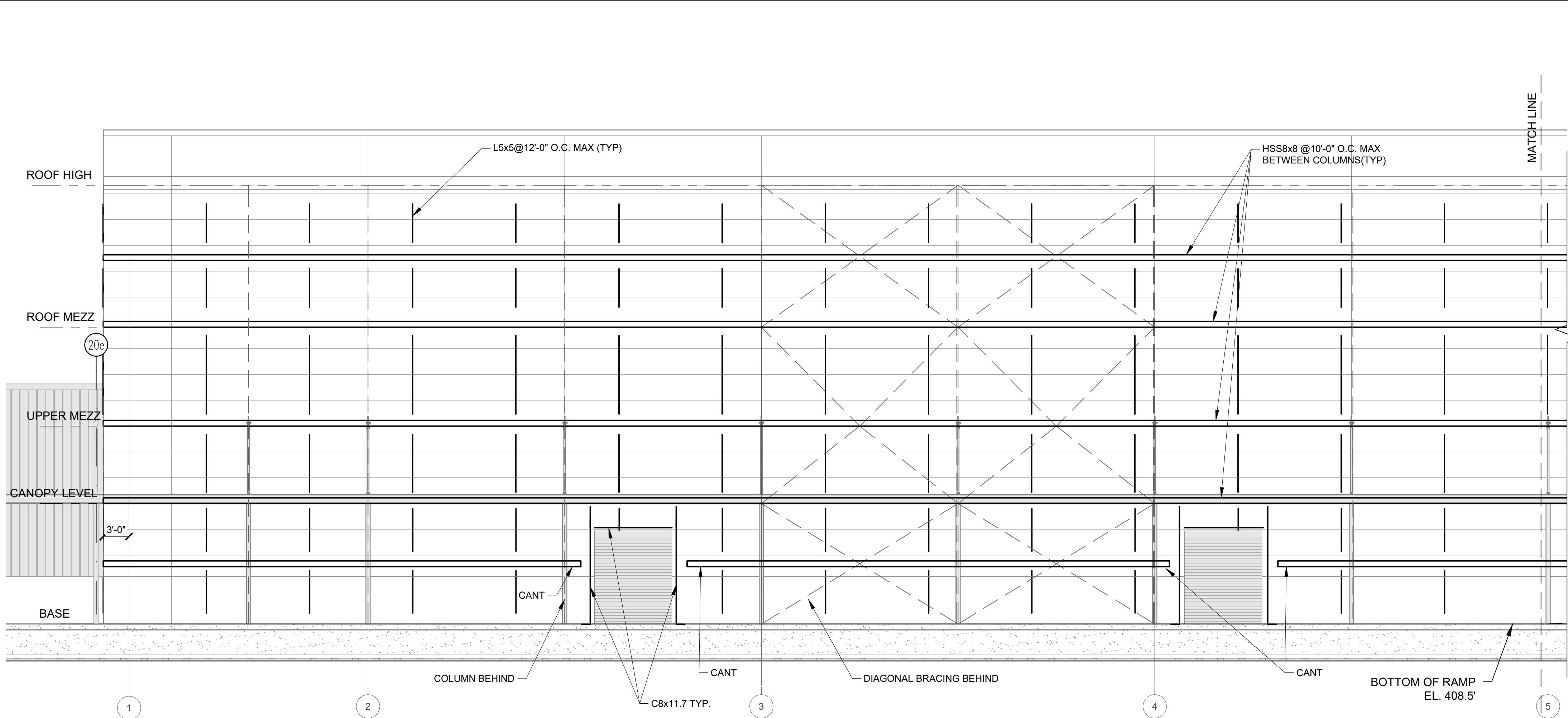
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EXTERIOR PANEL
SUPPORTS

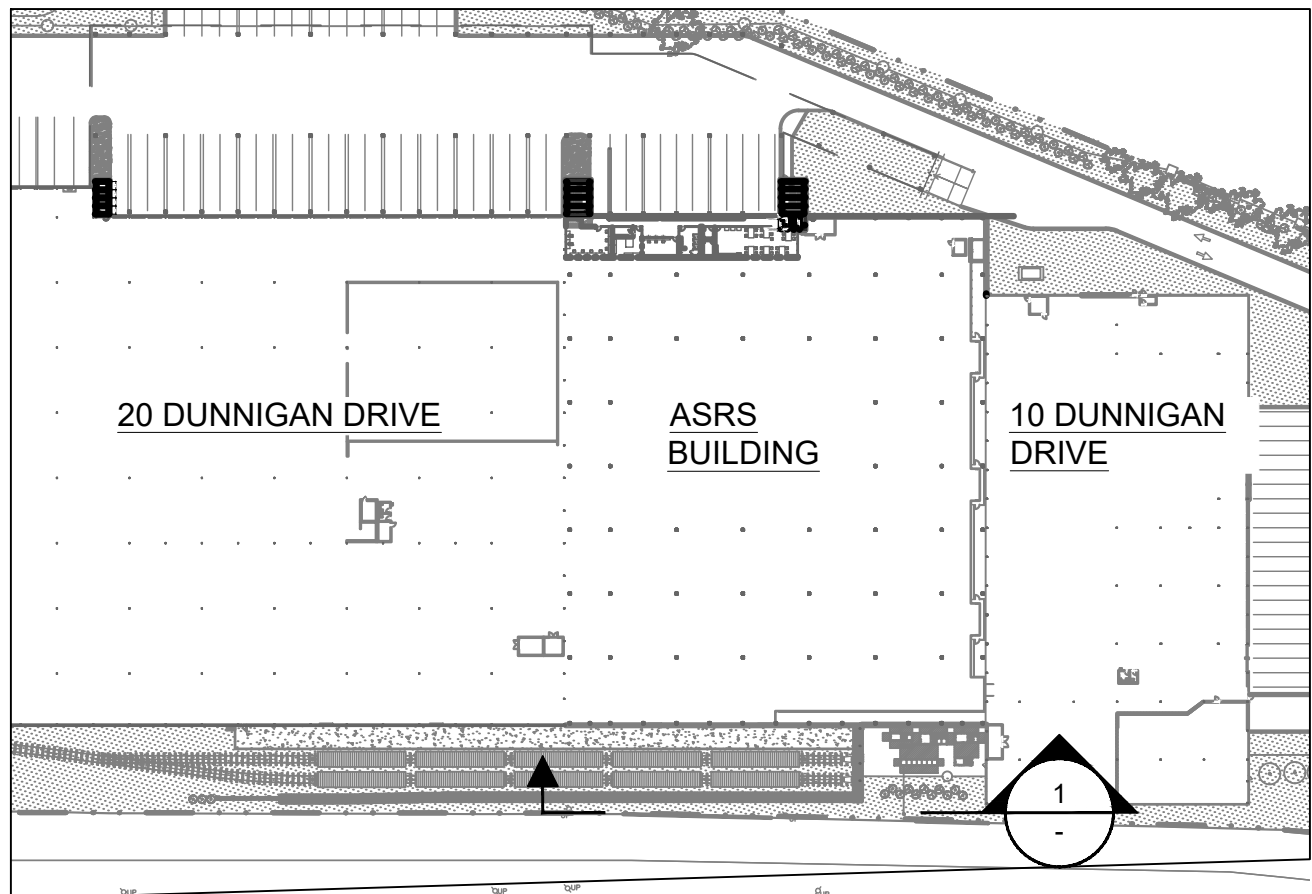
DWG NUMBER :

S-312



ASRS BUILDING SOUTH ELEVATION
EXTERIOR PANEL SUPPORTS

SCALE: 1/8" = 1'-0"



KEY PLAN

SCALE: N.T.S.

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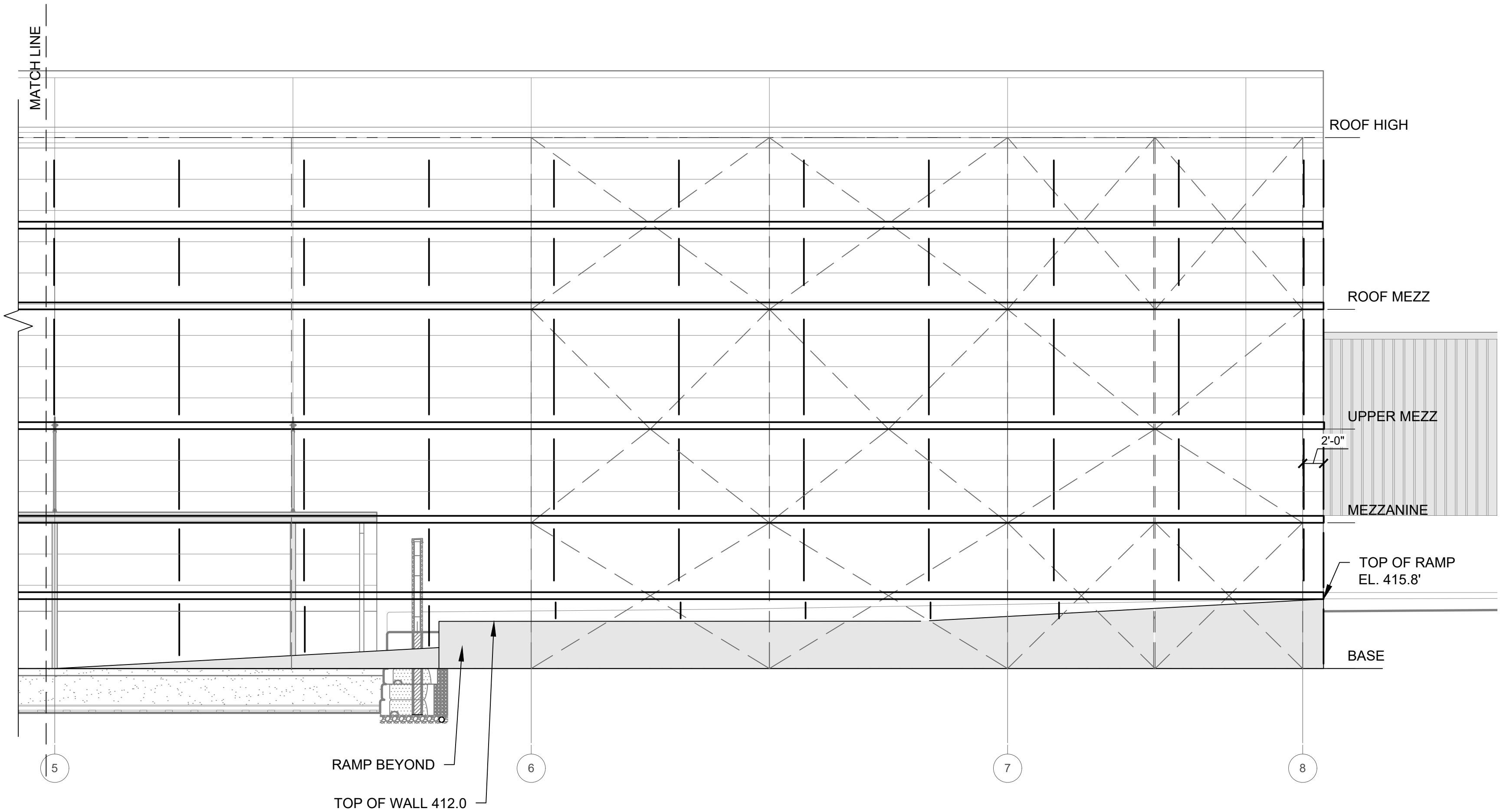
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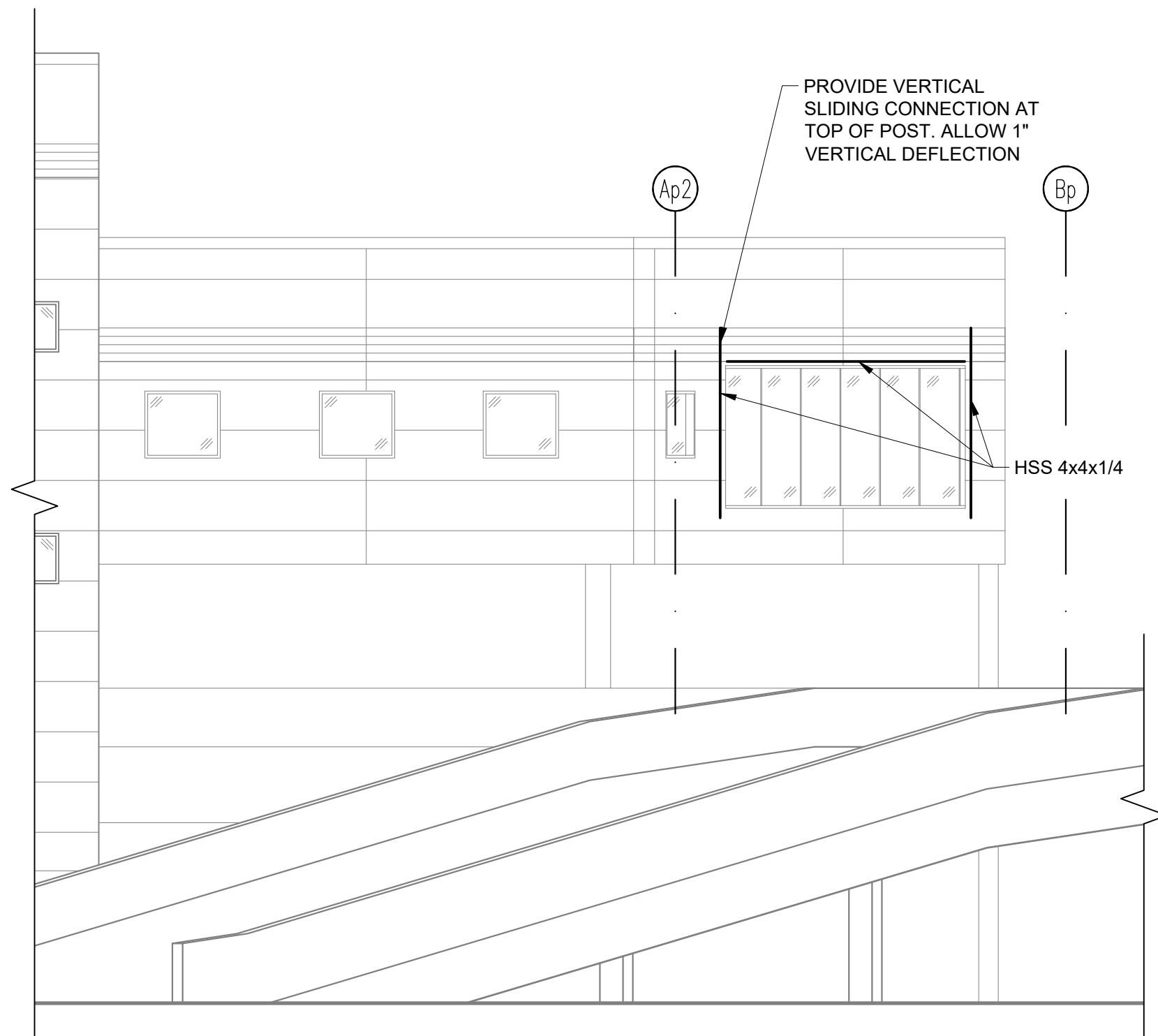
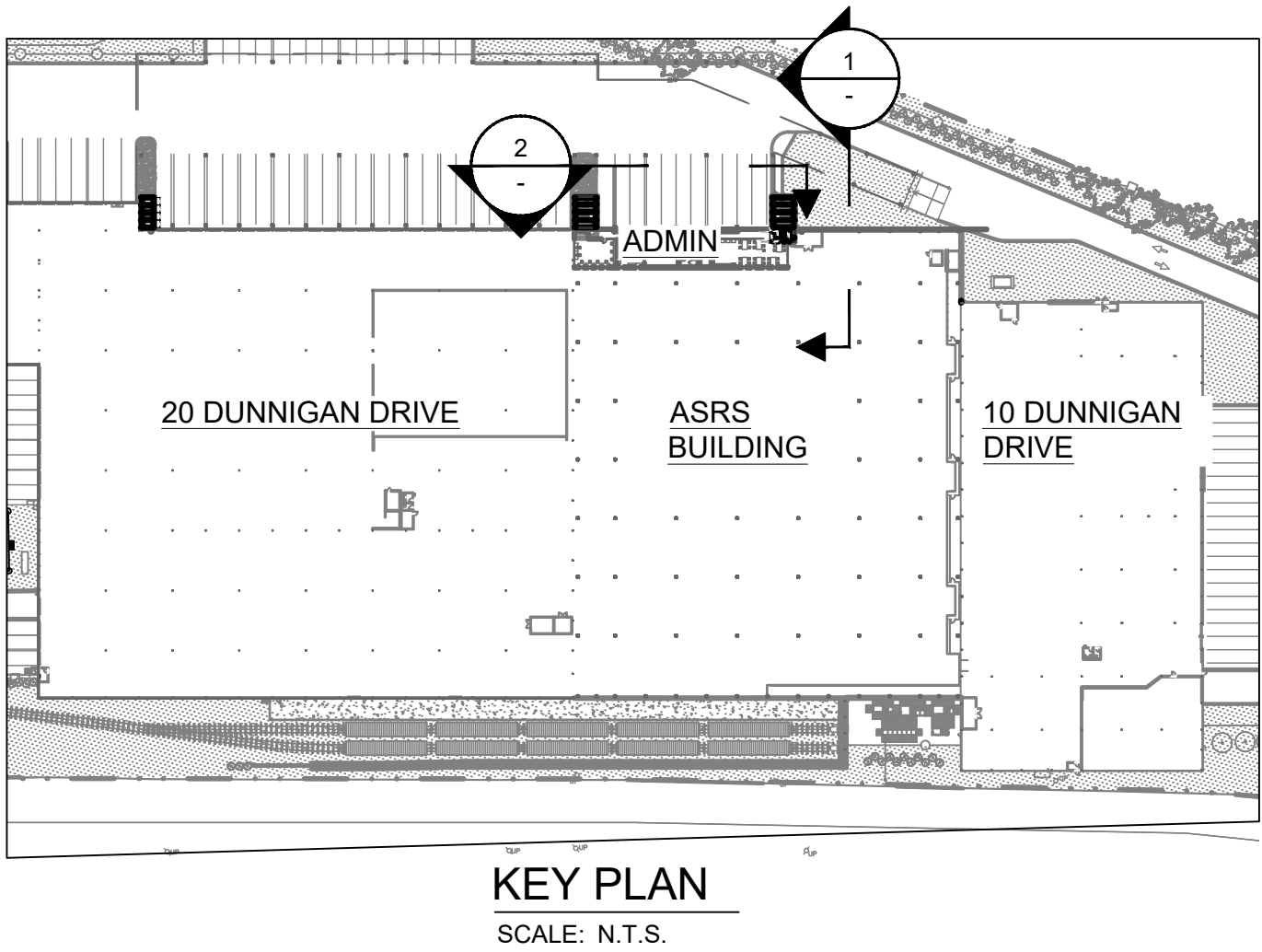
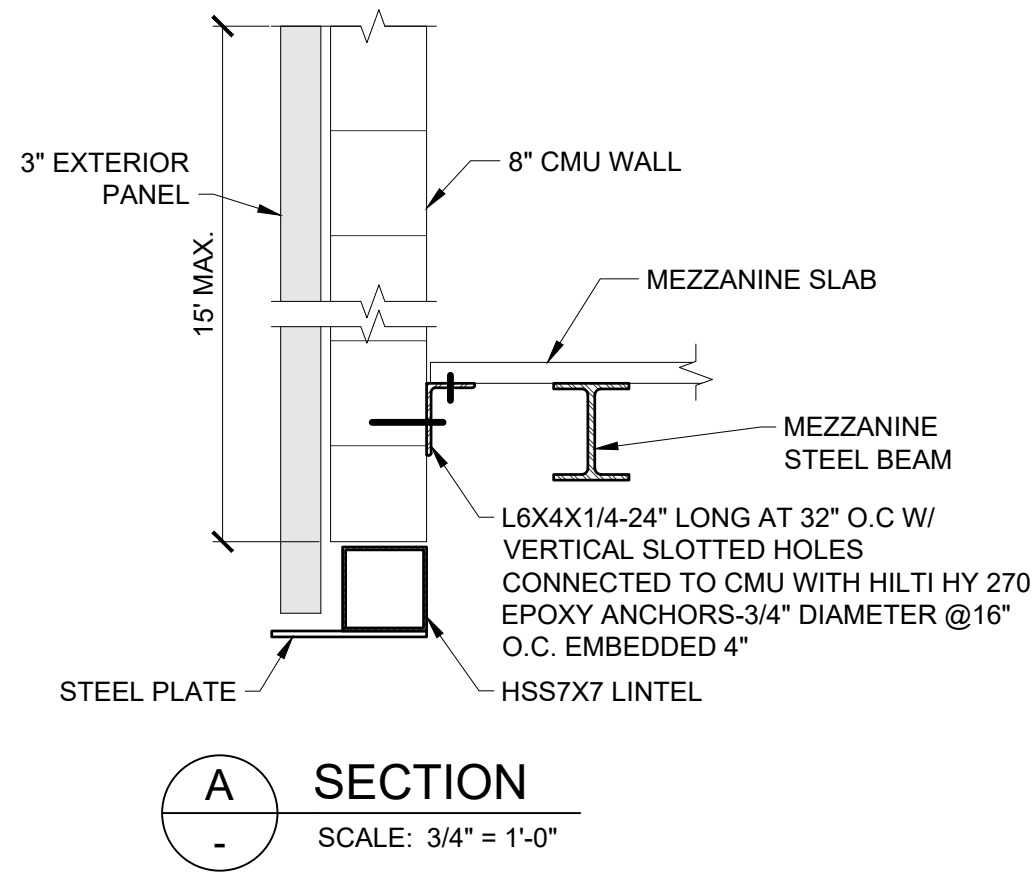
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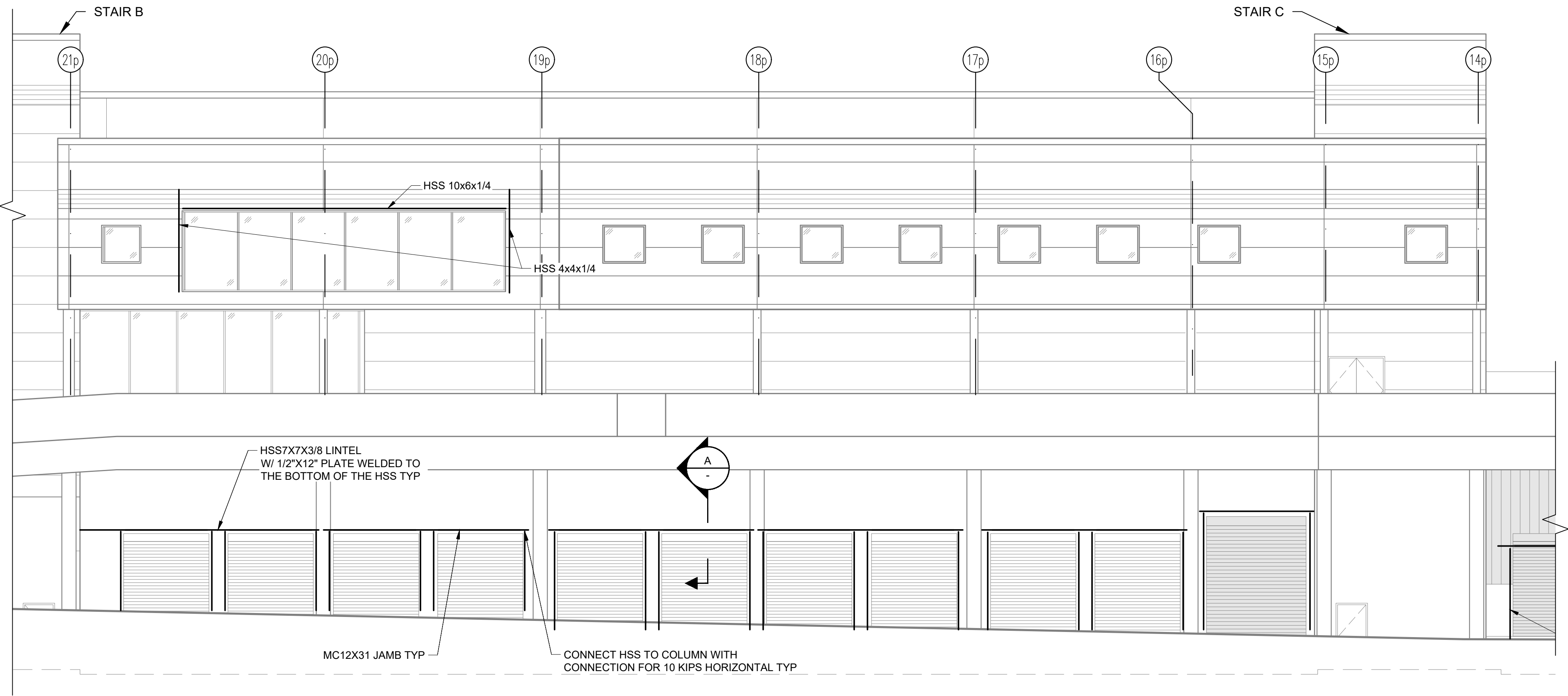
EXTERIOR PANEL
SUPPORTS

DWG NUMBER :

S-313



ADMIN BUILDING EAST ELEVATION
EXTERIOR PANEL SUPPORTS
SCALE: 1/8" = 1'-0"



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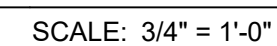
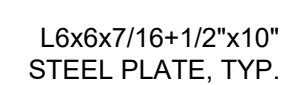
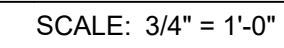
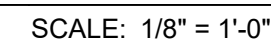
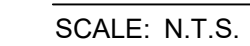
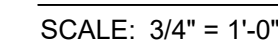
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EXTERIOR PANEL SUPPORTS

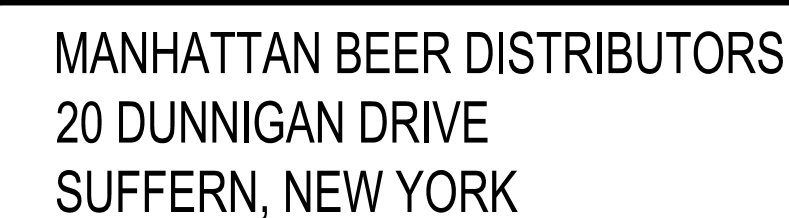
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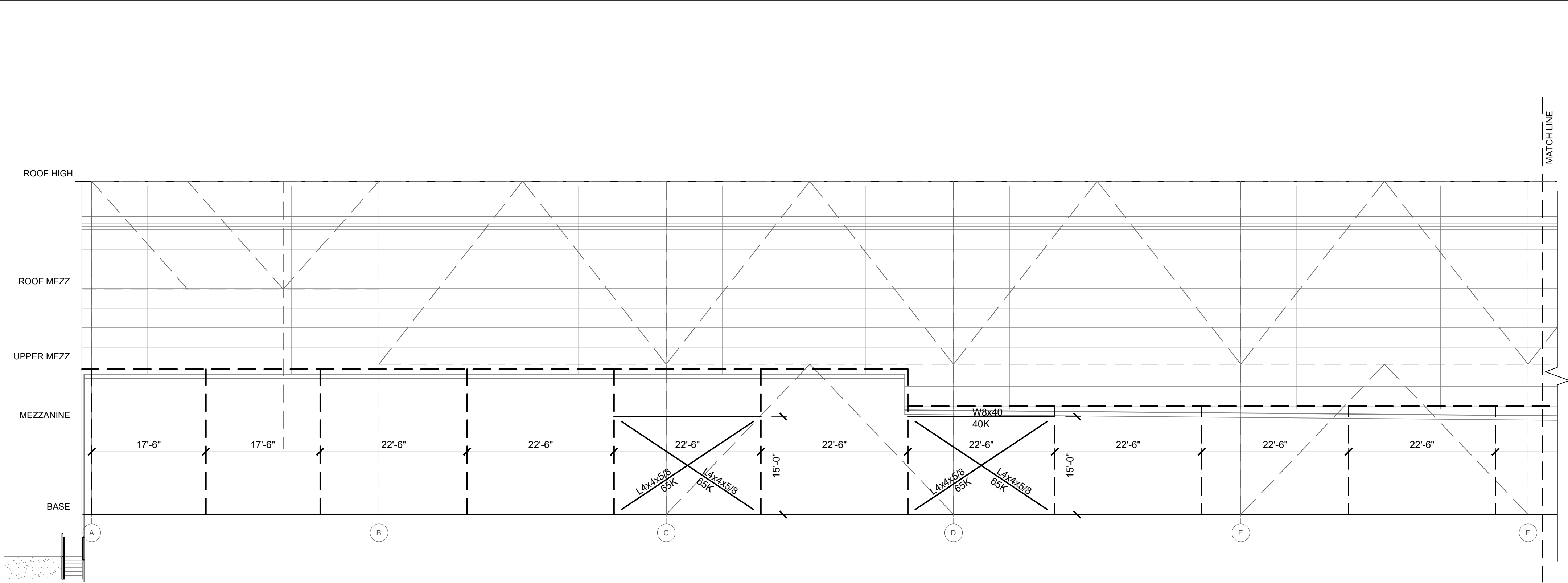


46 OF 60

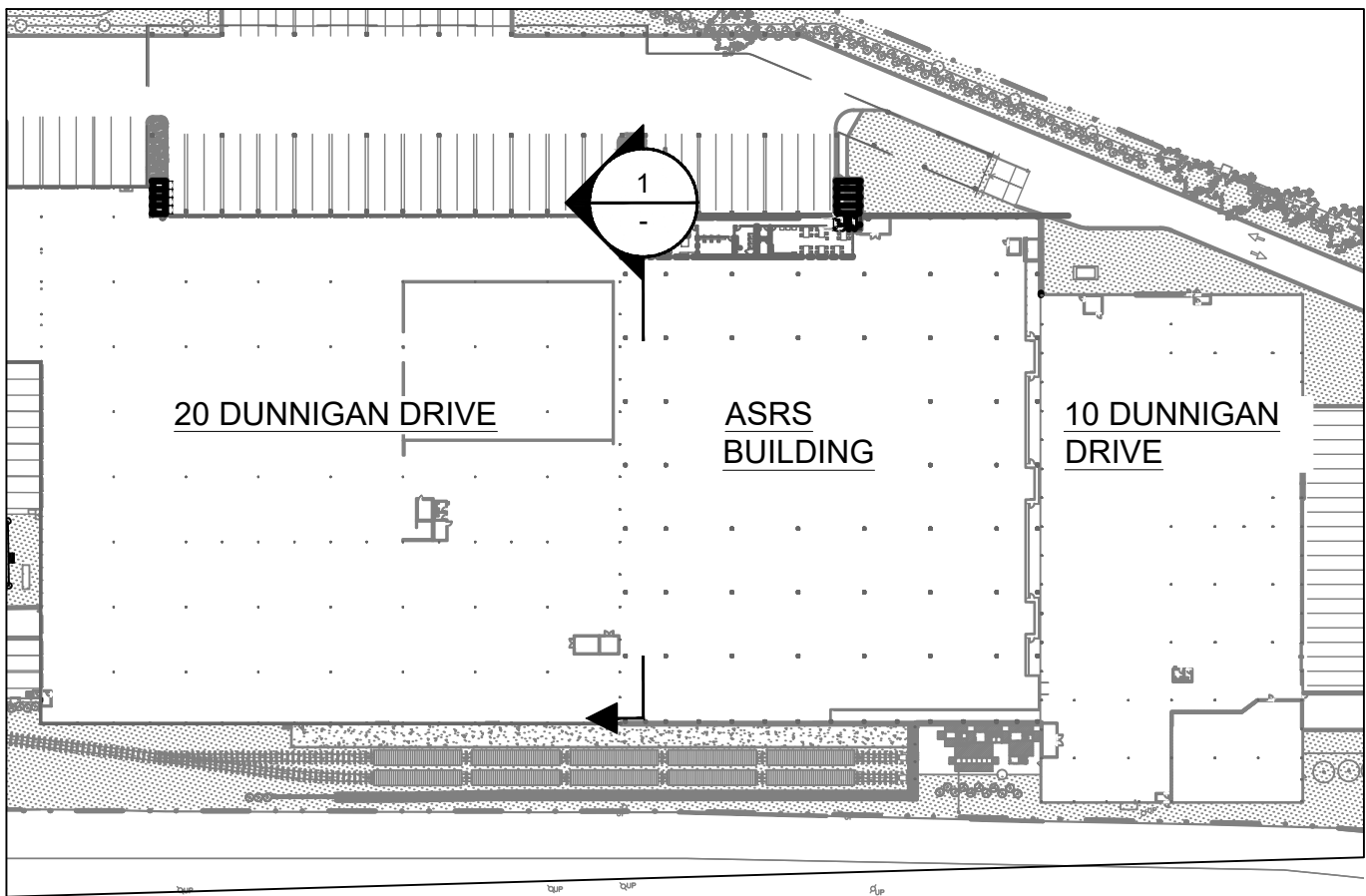


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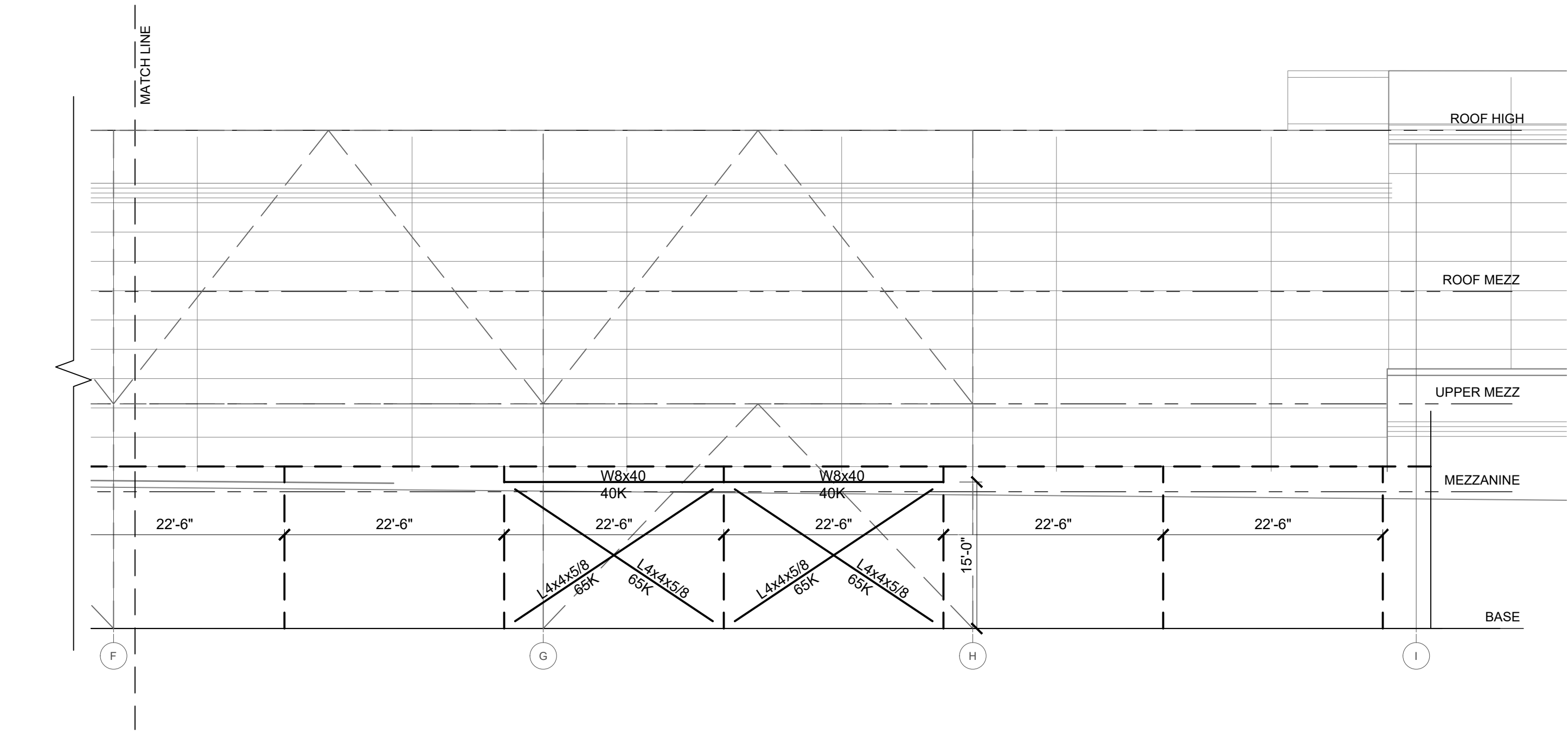
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ASRS BUILDING WEST ELEVATION
EXTERIOR PANEL SUPPORTS
1
SCALE: 3/32" = 1'-0"



KEY PLAN
SCALE: N.T.S.



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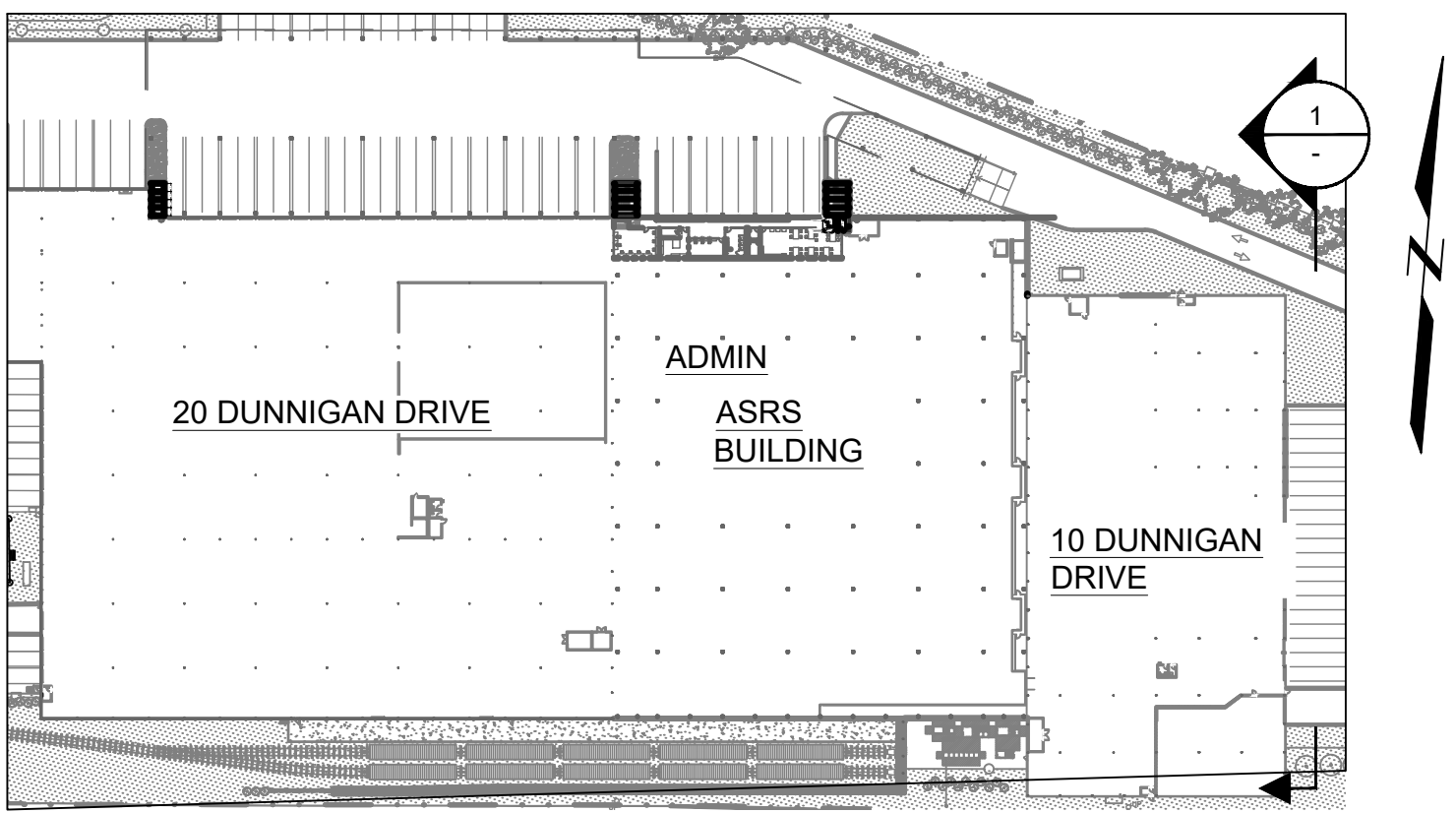
SCALE : AS SHOWN

DRAWING TITLE :

20 DUNNIGAN DRIVE BRACING
FRAME ELEVATION

DWG NUMBER :

S-322



KEY PLAN

SCALE: N.T.S.

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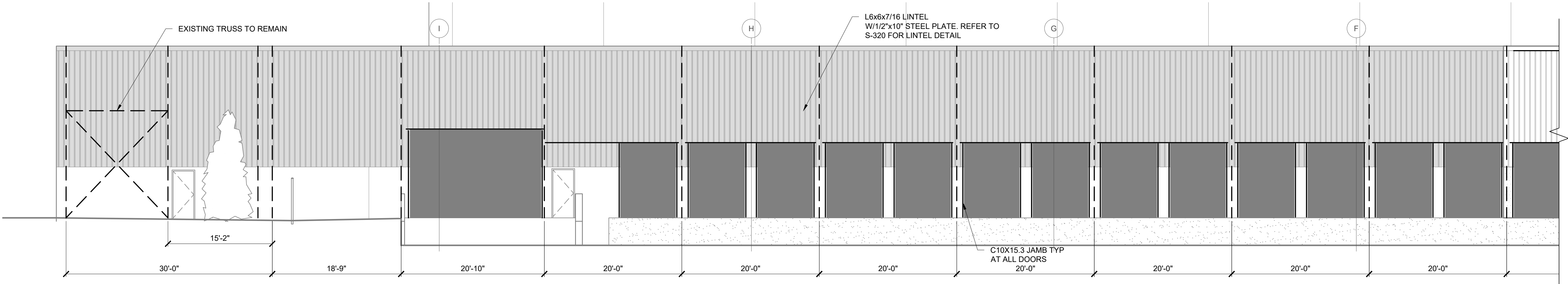
STRUCTURAL ENGINEER

GEI

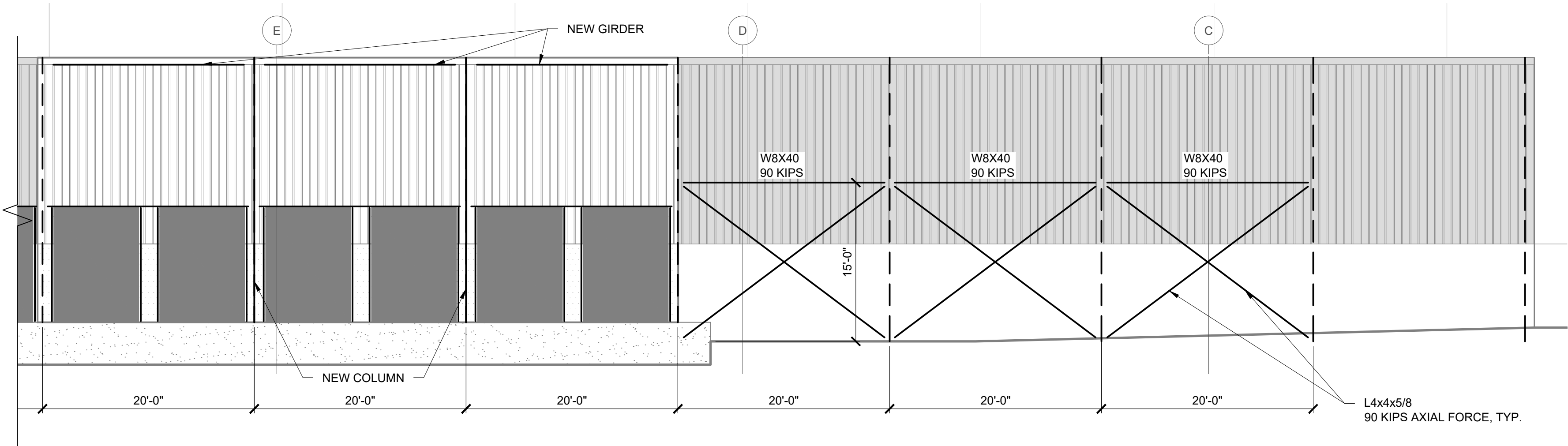
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1 EAST ELEVATION 10 DUNNIGAN DRIVE
SCALE: 1/8" = 1'-0"



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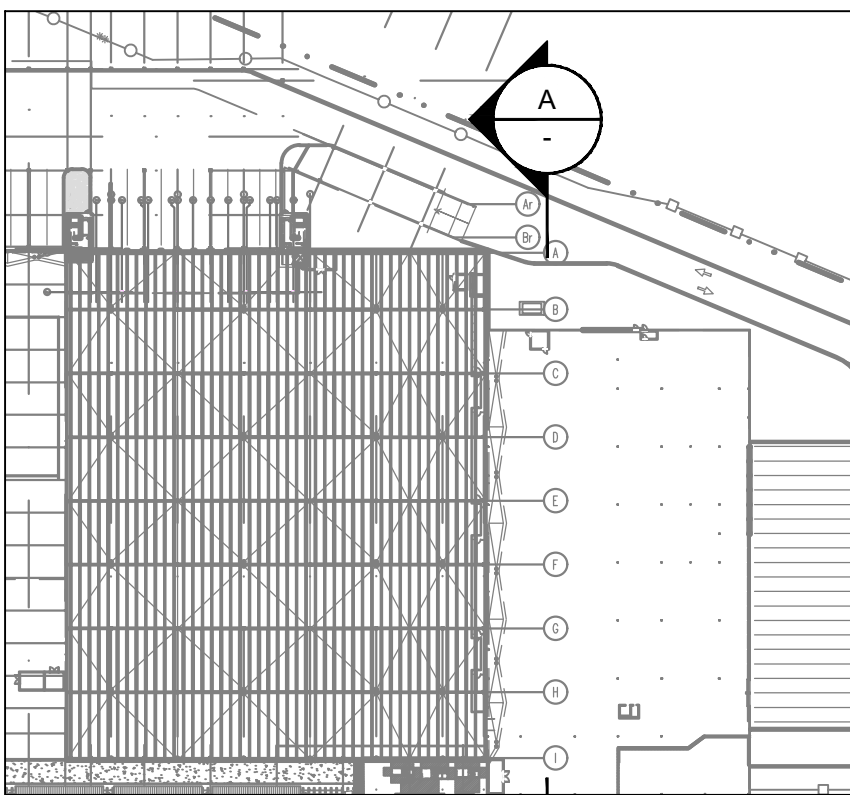
DRAWN BY :	D. CHECHURIN
CHECKED BY :	P. BRUNO
APPROVED BY :	P. BRUNO
DATE :	9/10/2021
SCALE :	AS SHOWN

DRAWING TITLE :

EAST ELEVATION 10
DUNNIGAN

DWG NUMBER :
S-323

49 OF 60



KEY PLAN
SCALE: N.T.S.

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DATE :	9/10/2021
SCALE :	AS SHOWN

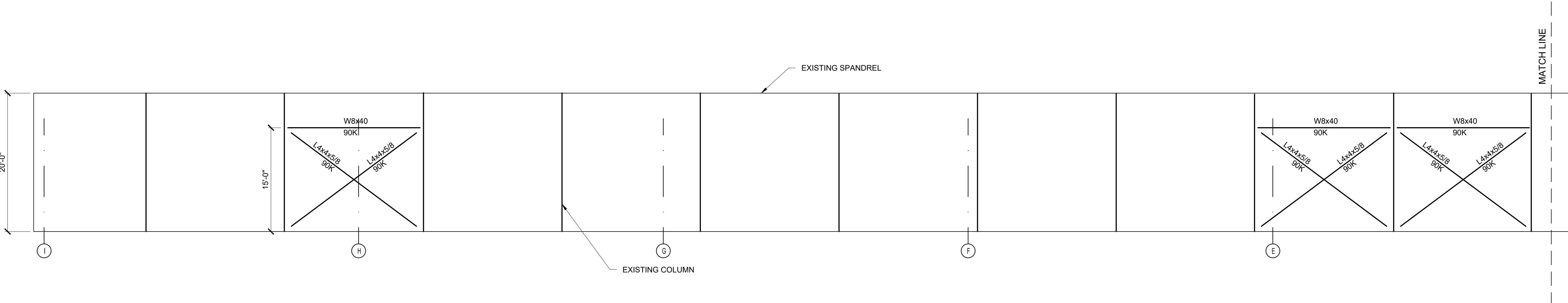
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BRACING ELEVATION AT
10 DUNNIGAN WEST WAL

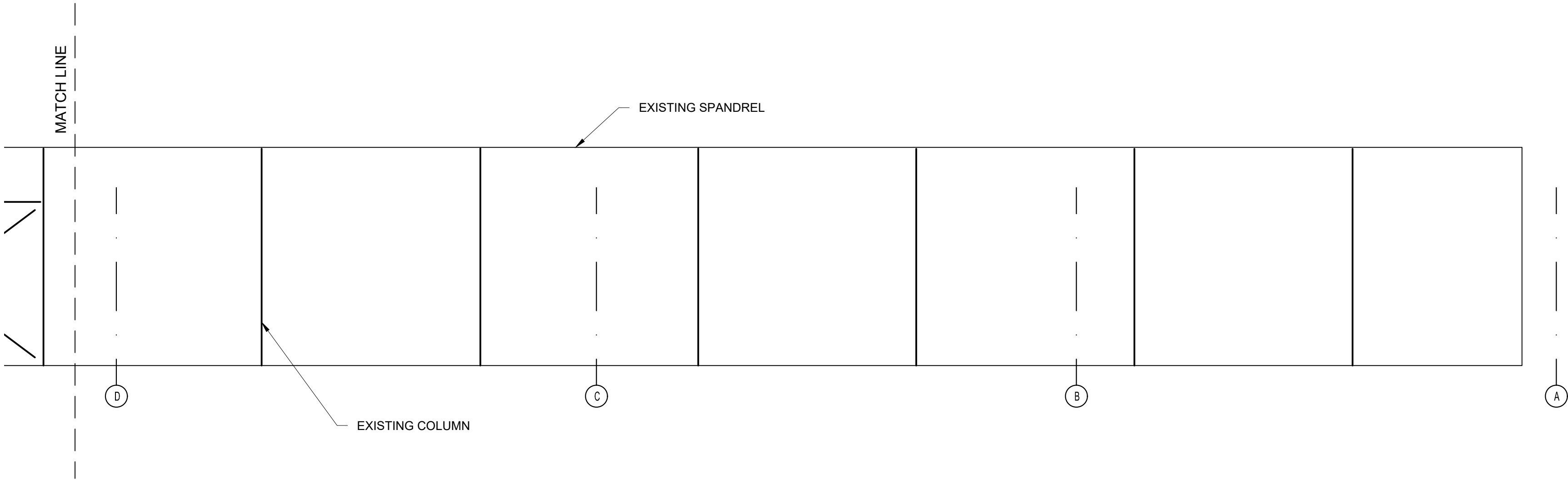
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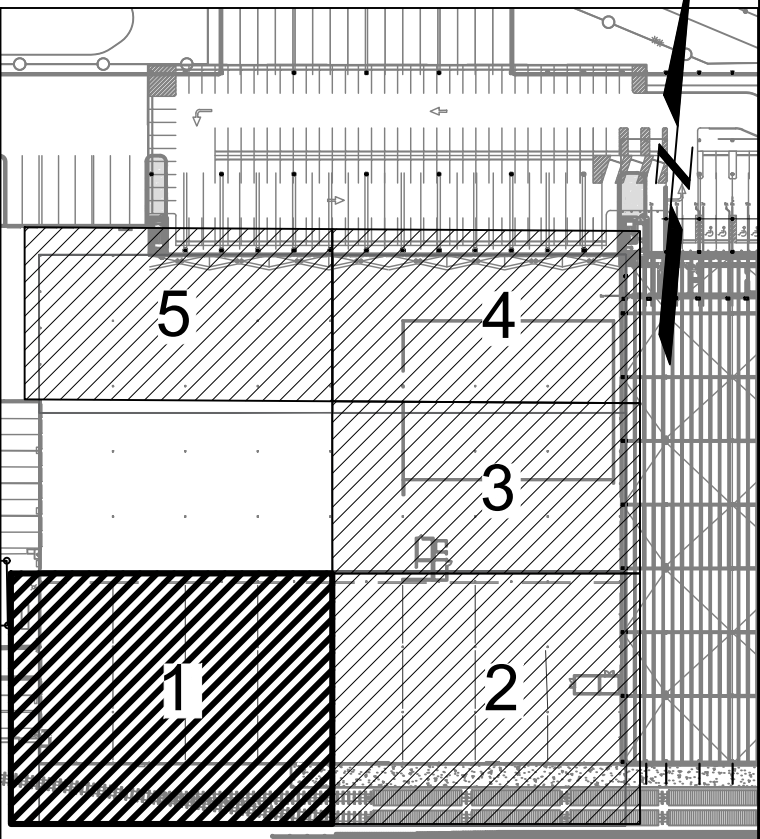
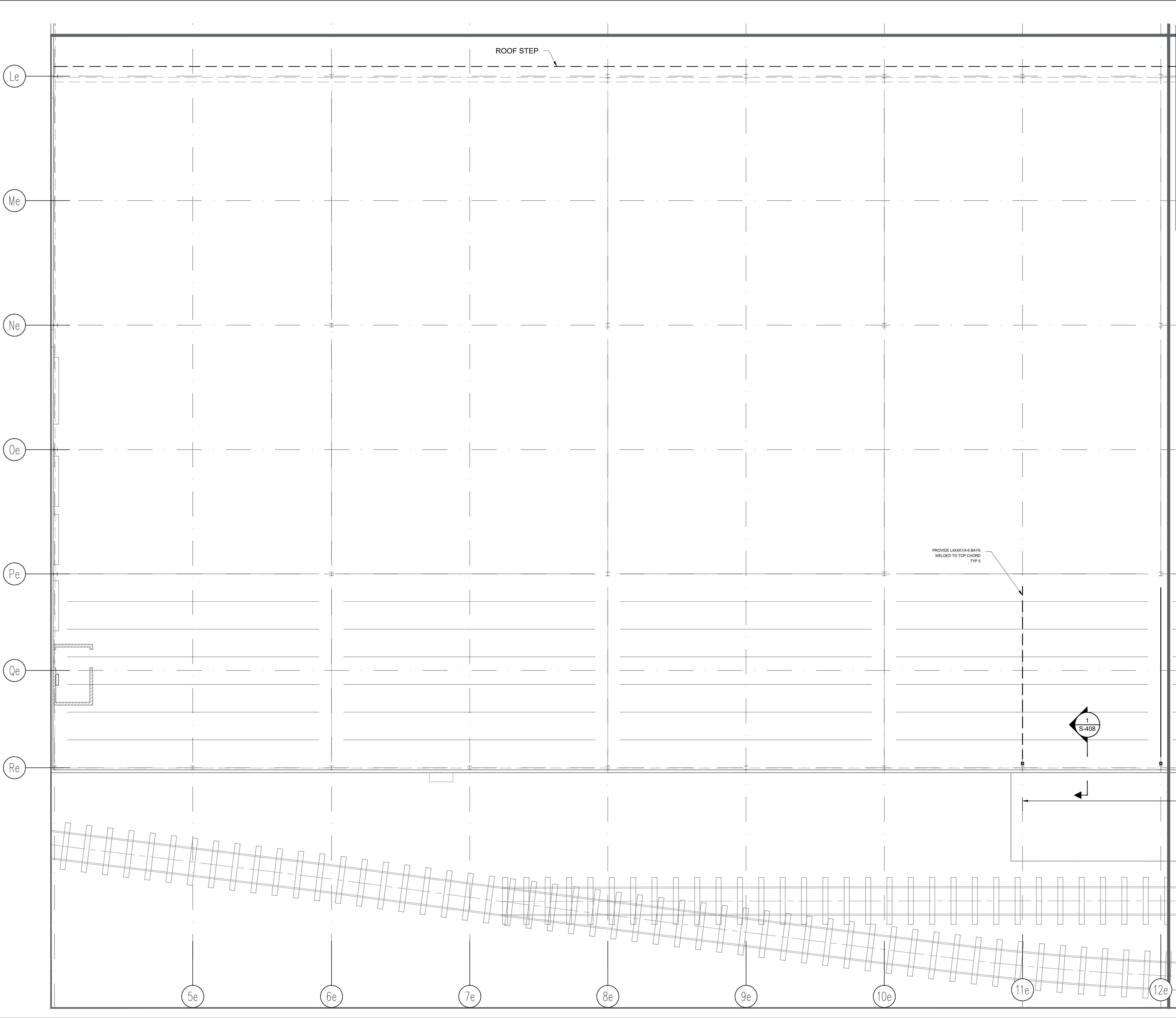
S-324

50 OF 60



BRACING ELEVATION AT 10
DUNNIGAN WEST WAL
SCALE: 1/8" = 1'-0"





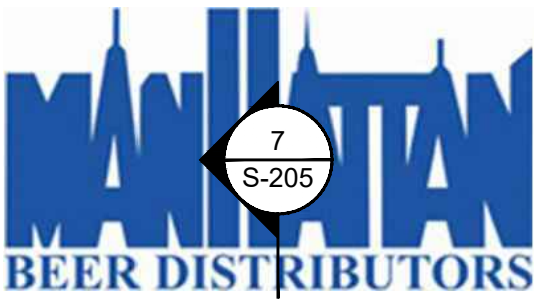
KEY PLAN
SCALE: N.T.S.

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SCALE :	AS SHOWN

DRAWING TITLE :

20 DUNNIGAN DRIVE ROOF
REINFORCING FRAMING PLAN

DWG NUMBER :

S-400

51 OF 60

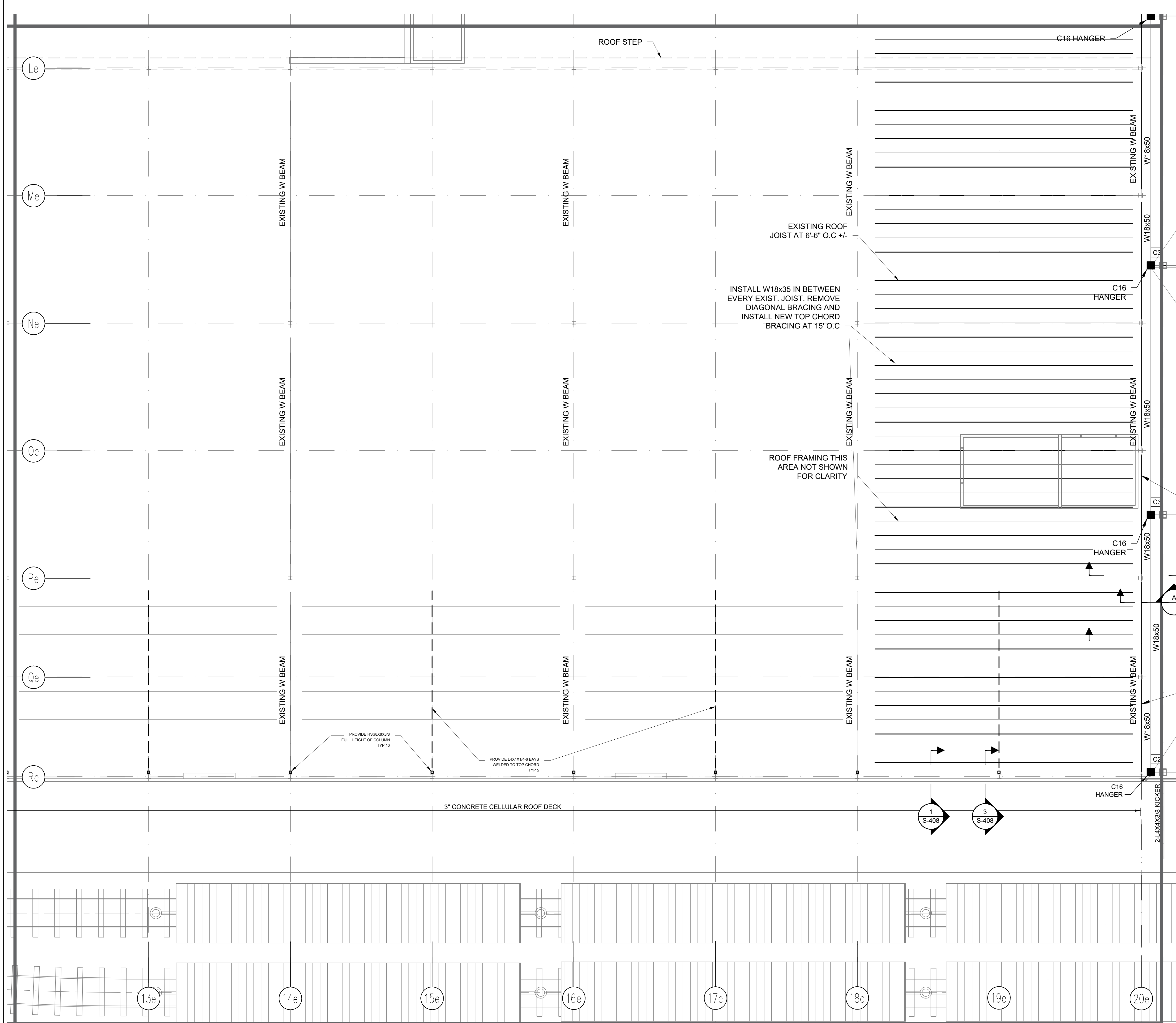
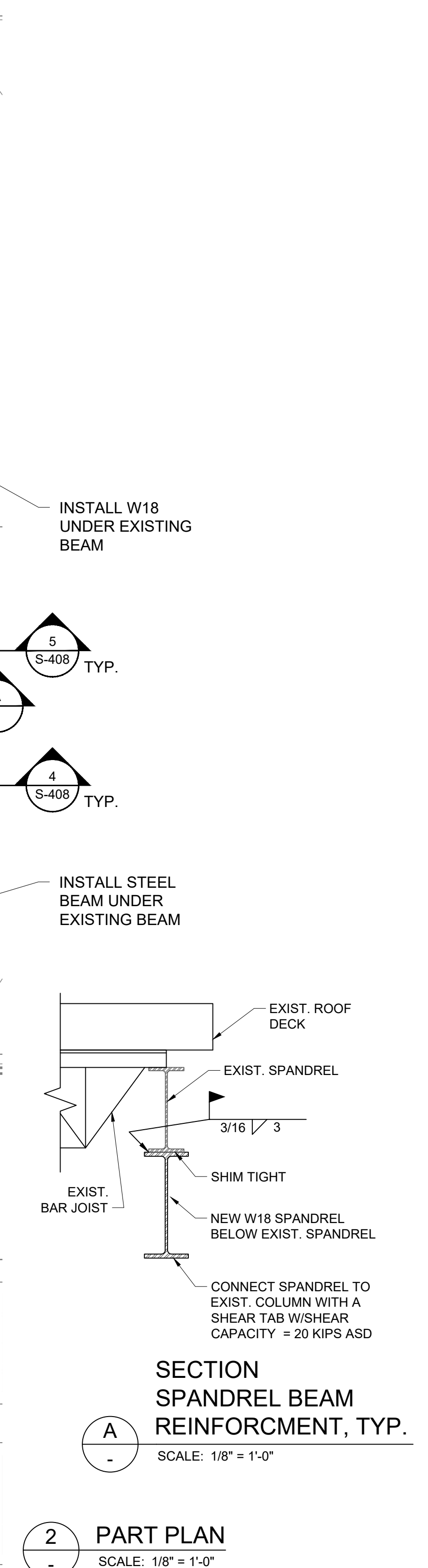
1 PART PLAN
SCALE: 1/8" = 1'-0"

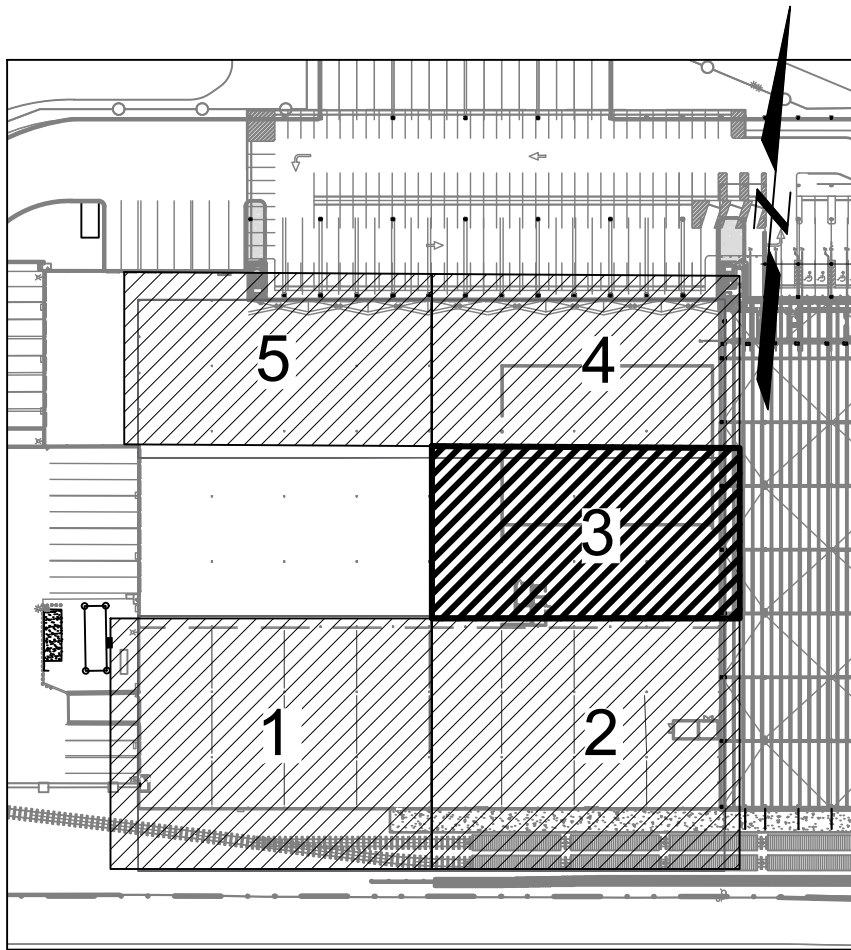
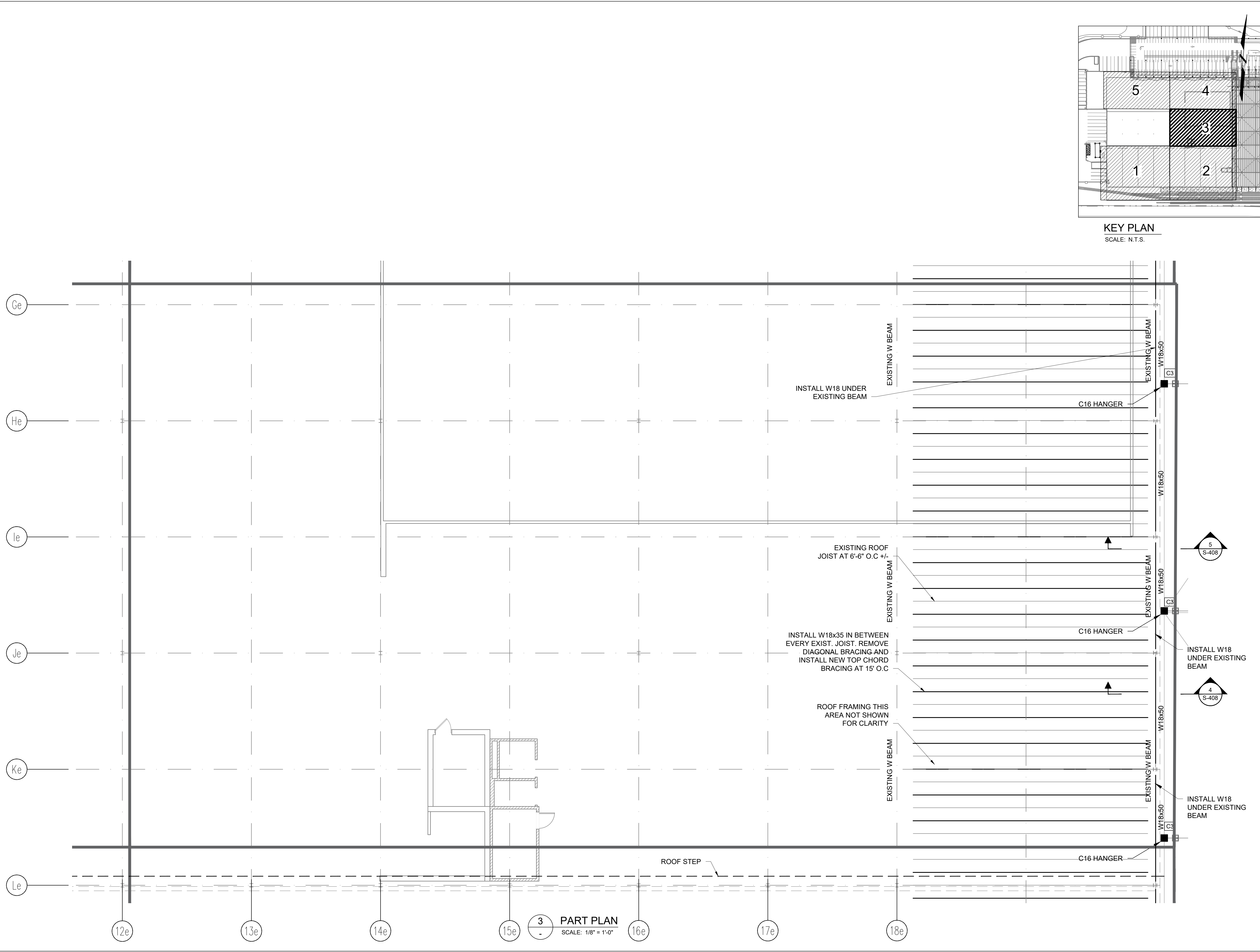
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SCALE : AS SHOWN

20 DUNNIGAN DRIVE ROOF
REINFORCING FRAMING PLAN

52 OF 60





KEY PLAN
SCALE: N.T.S.

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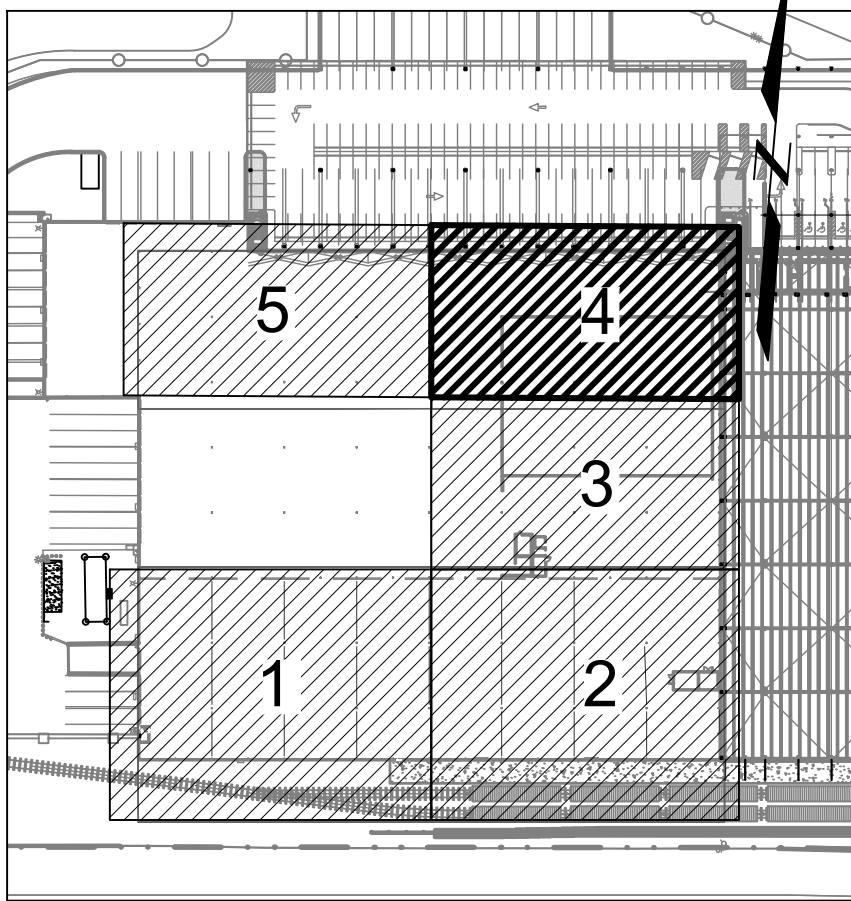
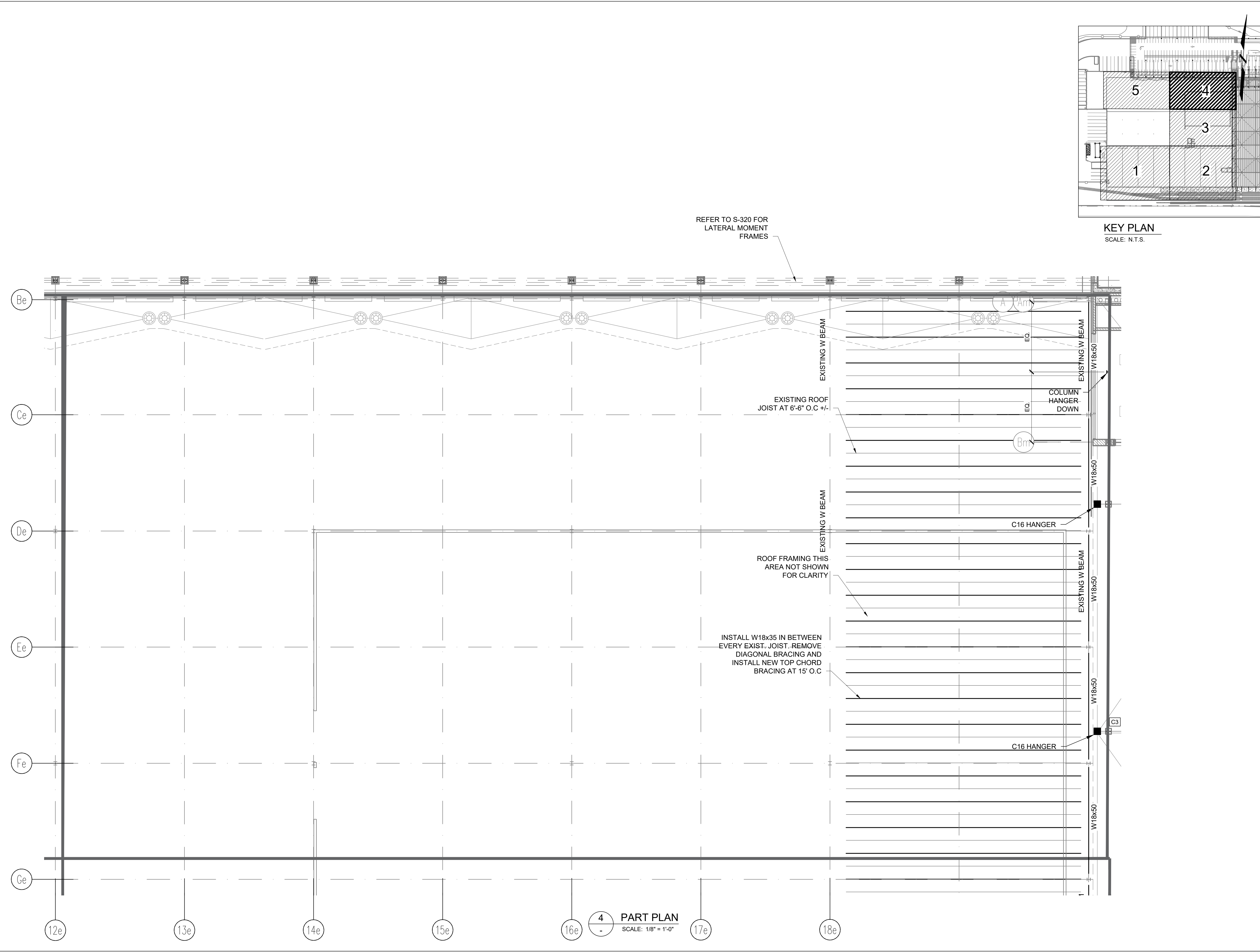
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DATE :	9/10/2021
SCALE :	AS SHOWN

20 DUNNIGAN DRIVE ROOF
REINFORCING FRAMING PLAN

DWG NUMBER :

S-402



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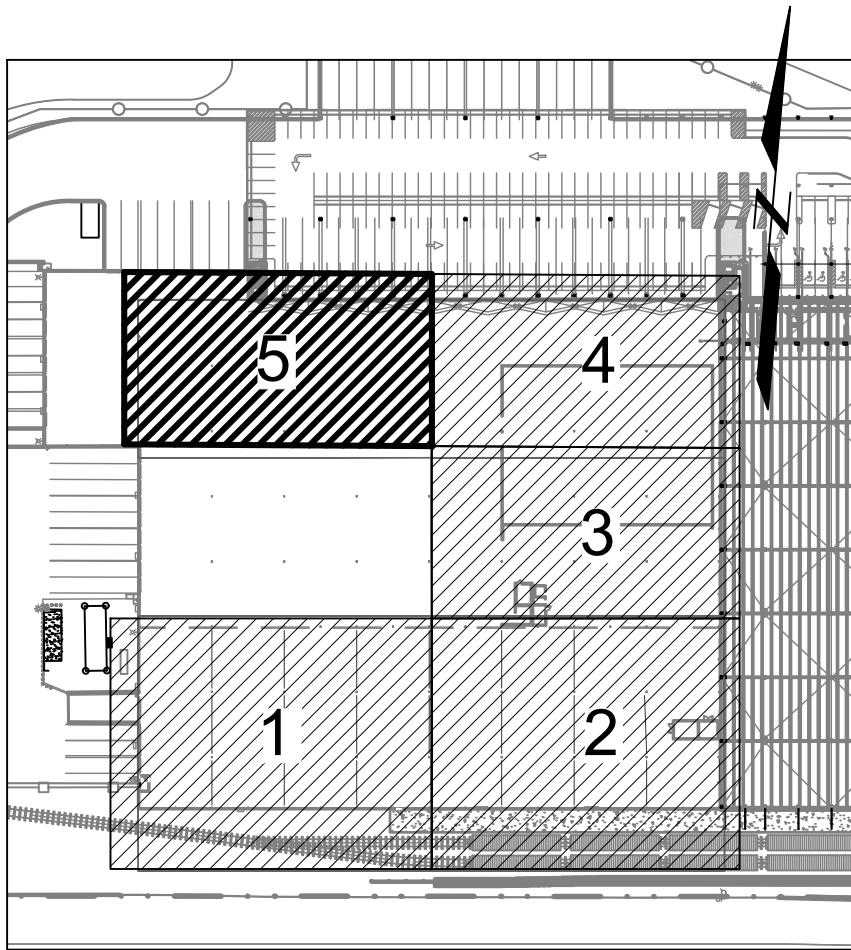
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SCALE :	AS SHOWN

DRAWING TITLE :

20 DUNNIGAN DRIVE ROOF
REINFORCING FRAMING PLAN

DWG NUMBER :
S-403
54 OF 60



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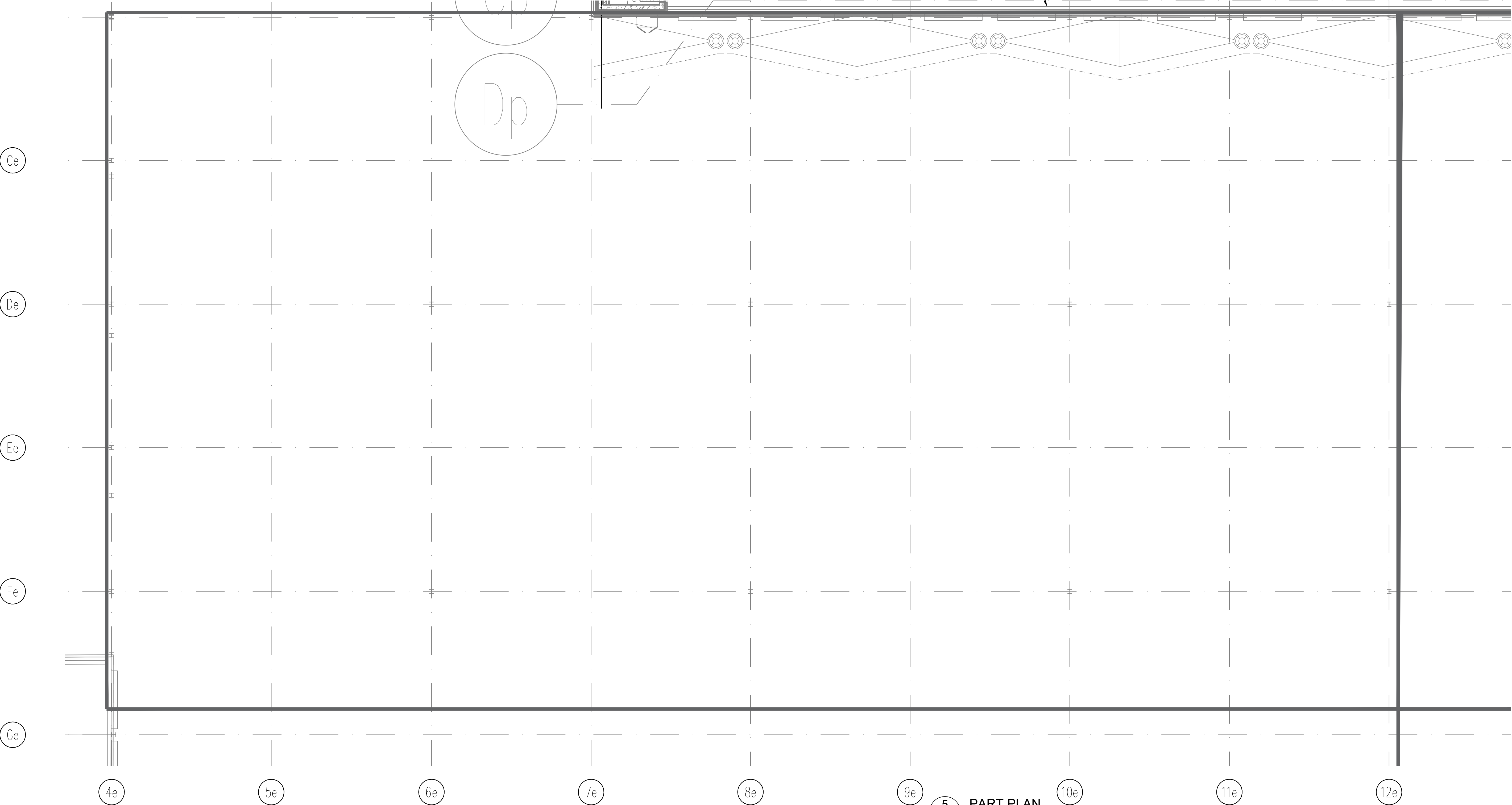
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KEY PLAN
SCALE: N.T.S.

REFER TO S-320 FOR
LATERAL MOMENT
FRAMES



5 PART PLAN
SCALE: 1/8" = 1'-0"

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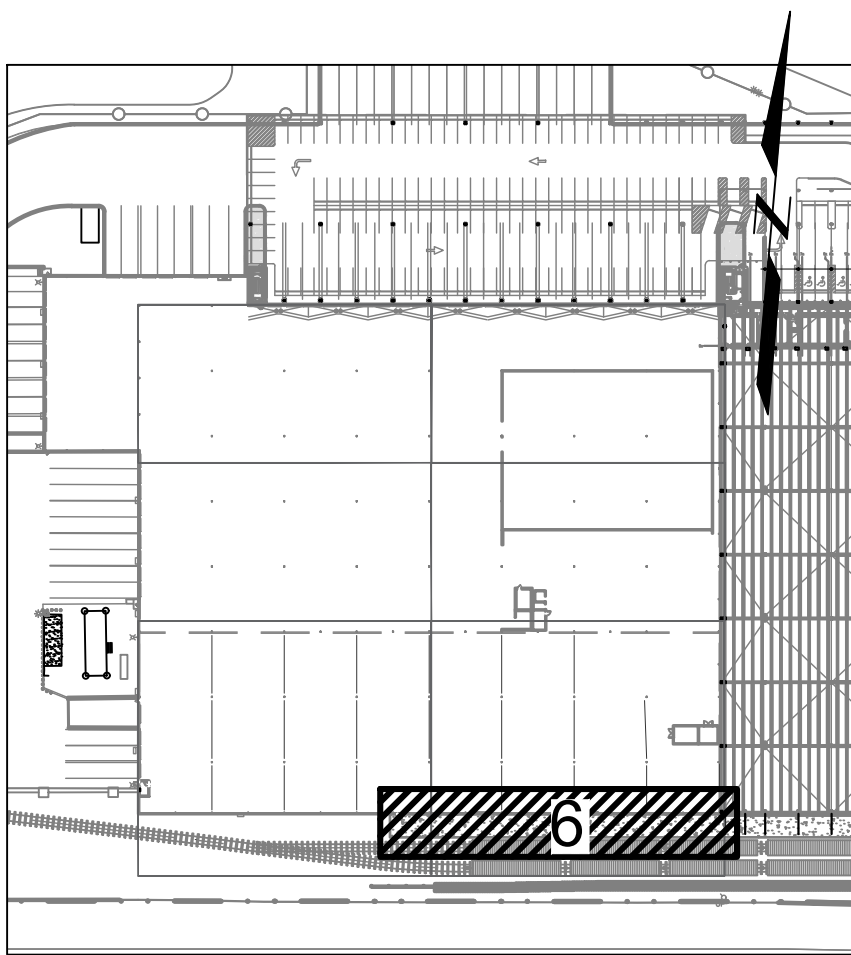
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SCALE :	AS SHOWN

DRAWING TITLE :

**20 DUNNIGAN DRIVE ROOF
REINFORCING FRAMING PLAN**

DWG NUMBER :
S-404
55 OF 60



KEY PLAN
SCALE: N.T.S.

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SCALE :	AS SHOWN

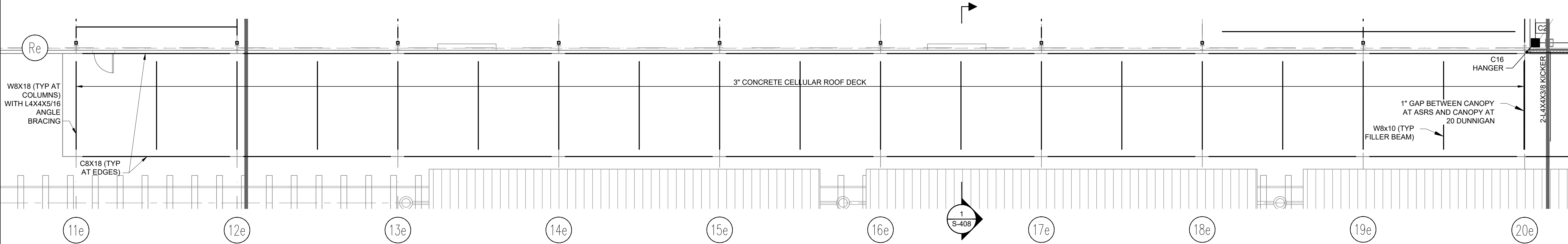
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PROPOSED CANOPY FRAMING
PLAN 20 DUNNIGAN DRIVE

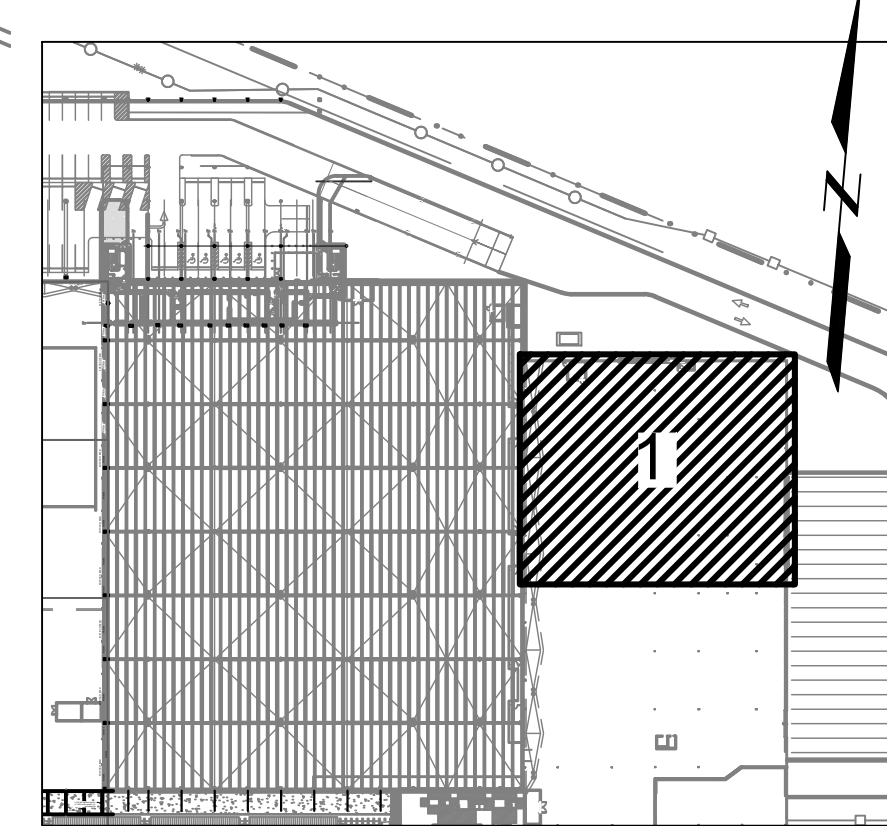
DWG NUMBER :

S-405

56 OF 60



6 PART PLAN
SCALE: 1/8" = 1'-0"



NEW DIAGONAL BRACING
REFER TO BUILDING
ELEVATION

— EXISTING ROOF JOISTS
TO REMAIN



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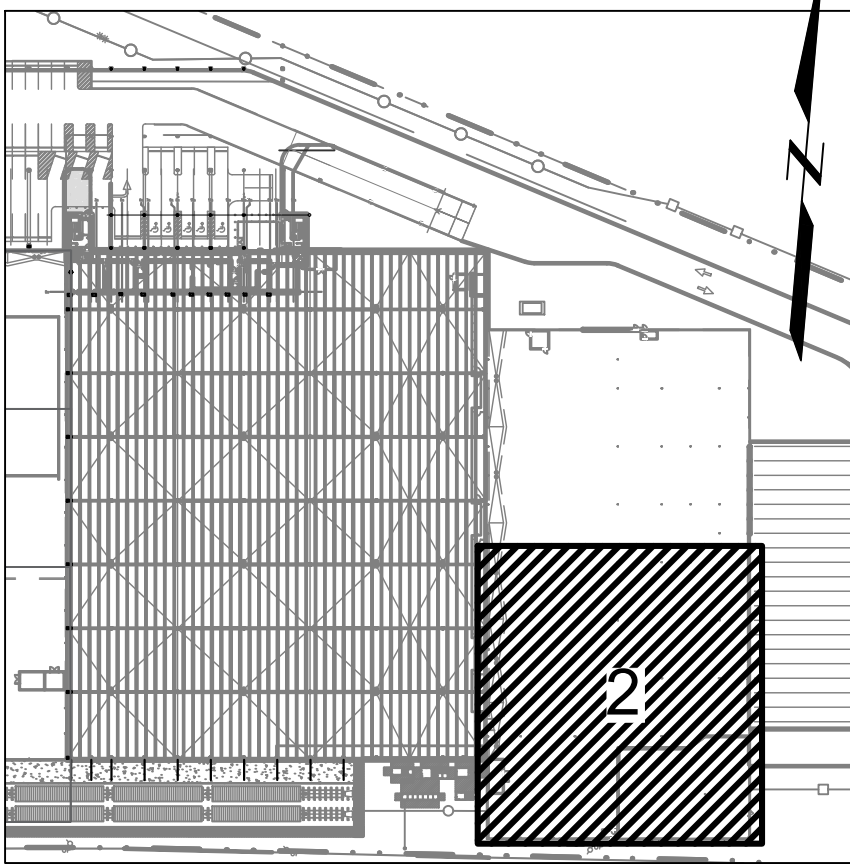
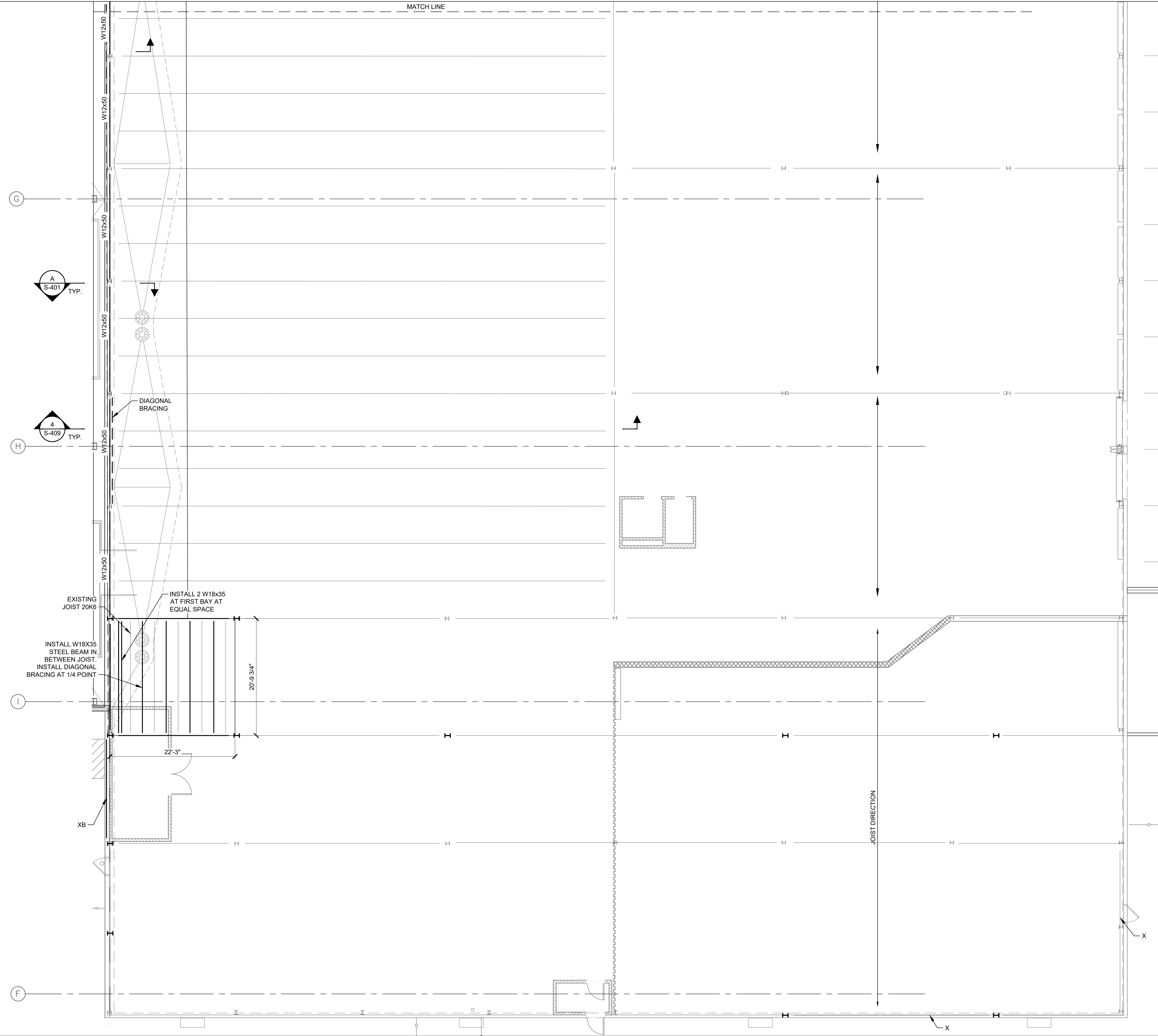
SCALE : AS SHOWN

DRAWING TITLE:

10 DUNNIGAN DRIVE
ROOF FRAMING PLAN

DWG NUMBER :
S-406

57 OF 60



KEY PLAN
SCALE: N.T.S.

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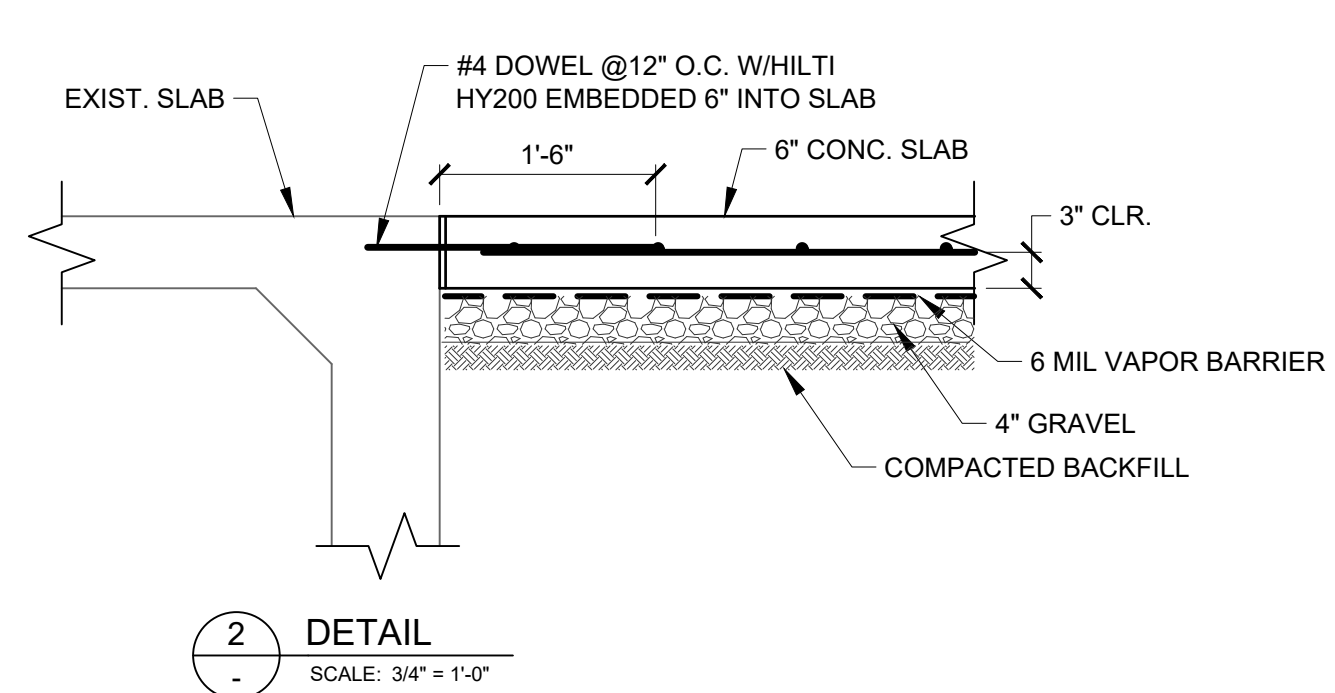
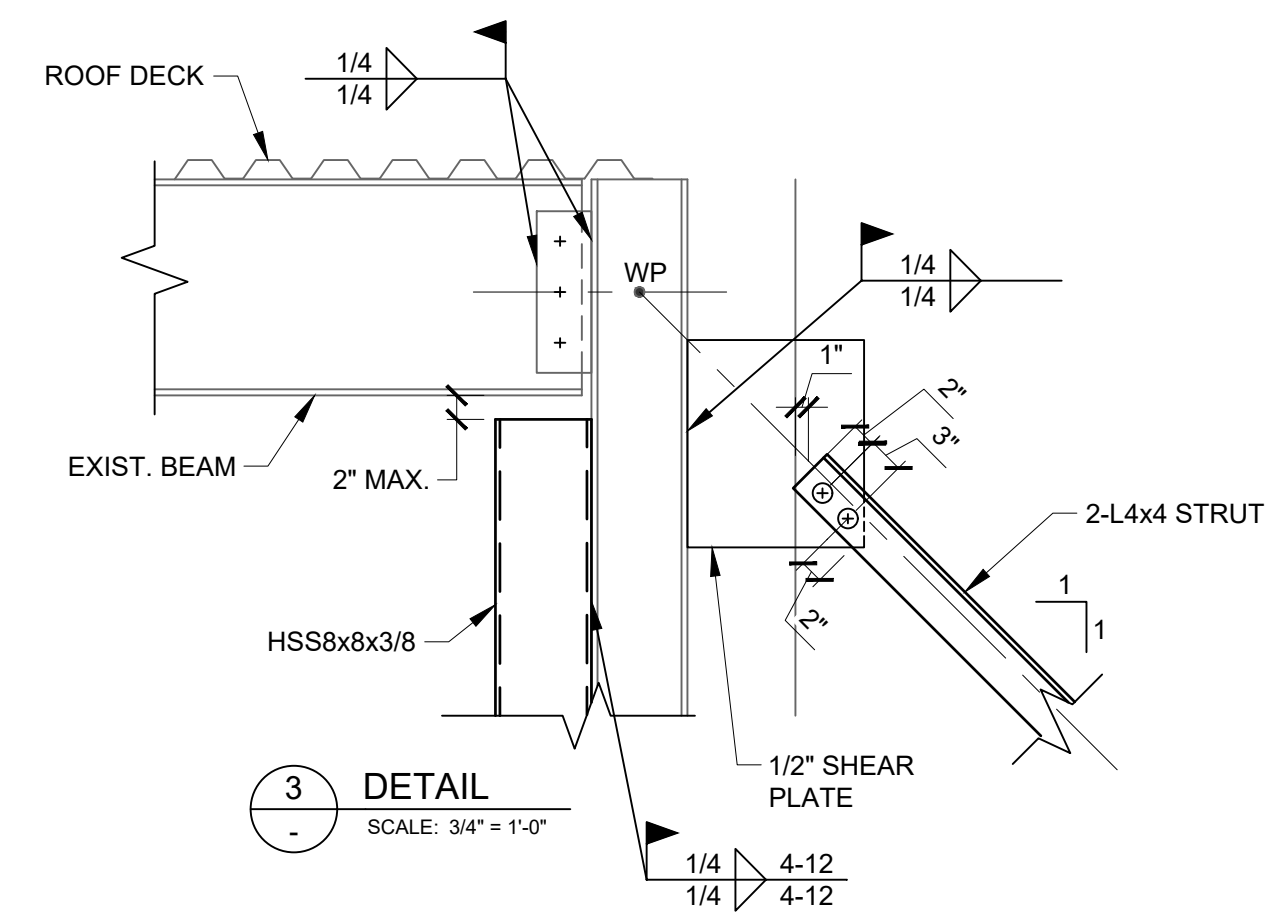
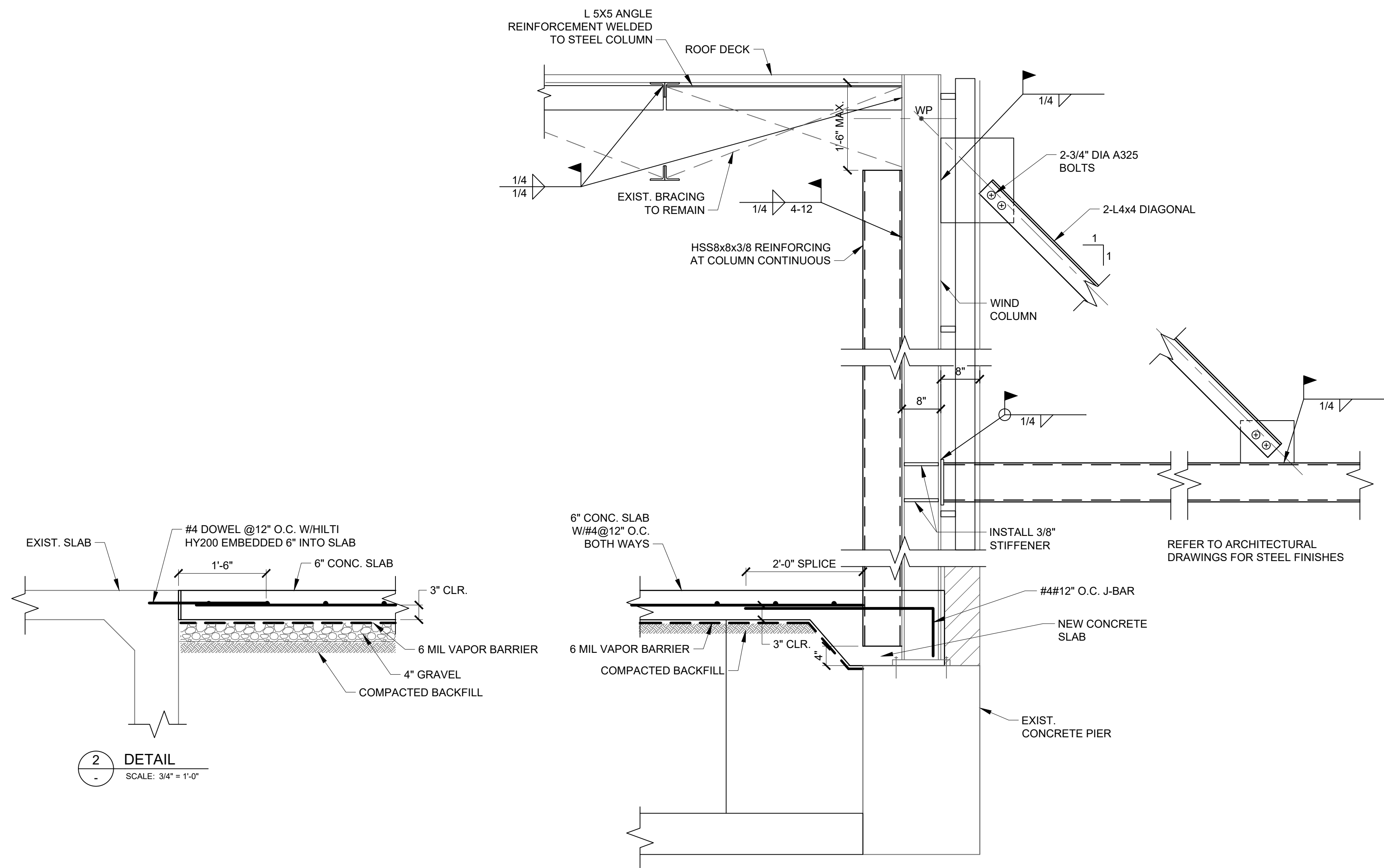
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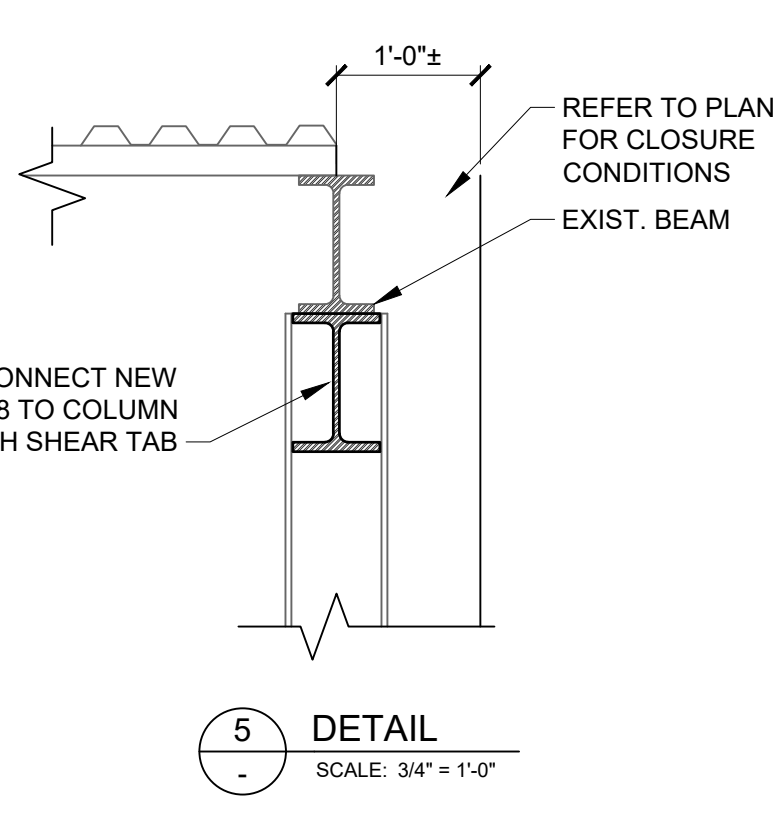
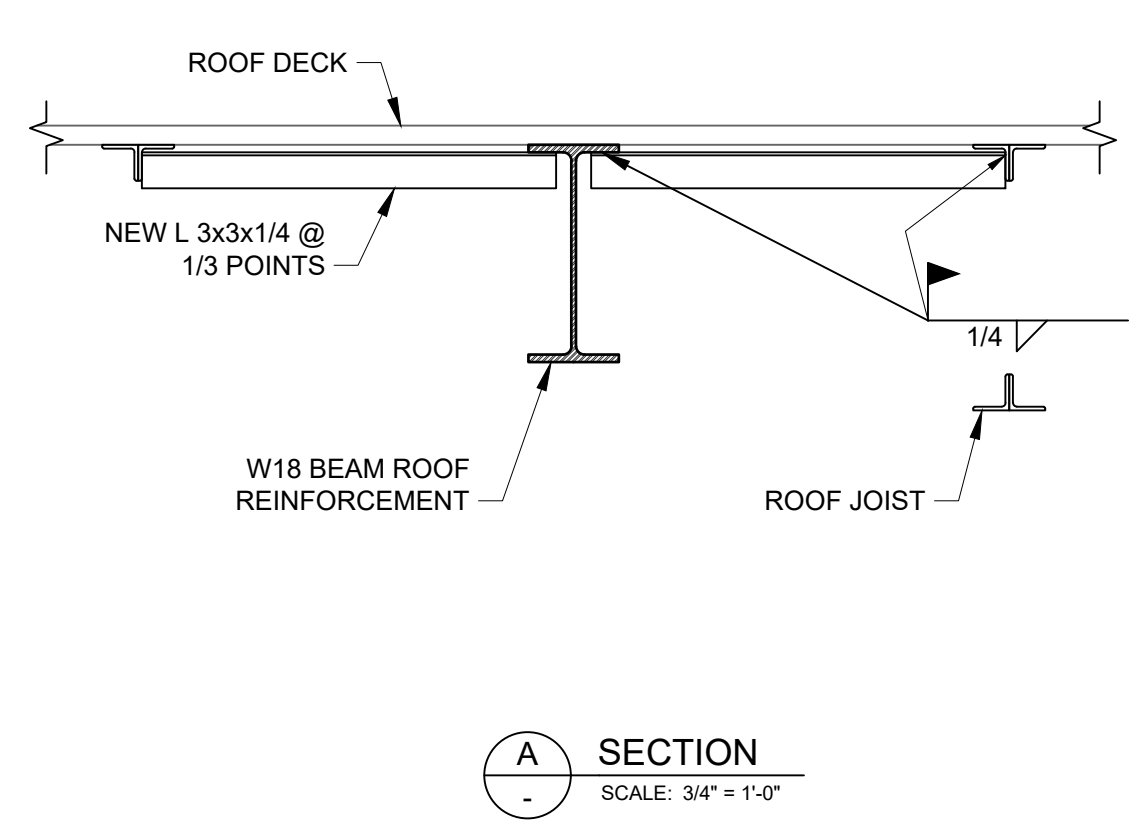
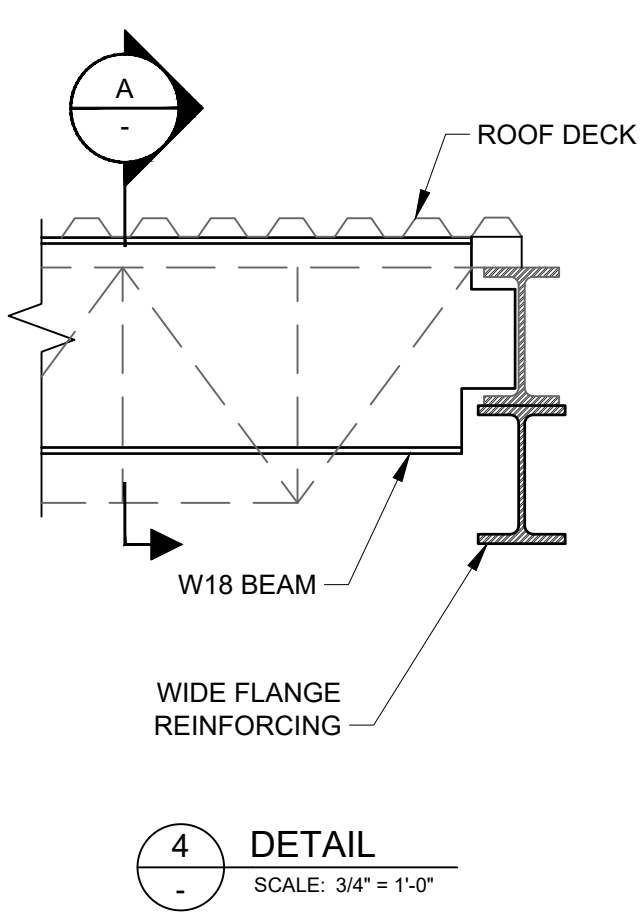
DRAWING TITLE :

10 DUNNIGAN DRIVE
ROOF FRAMING PLAN

DWG NUMBER :
S-407
58 OF 60



1
-
DETAIL
SCALE: 3/4" = 1'-0"



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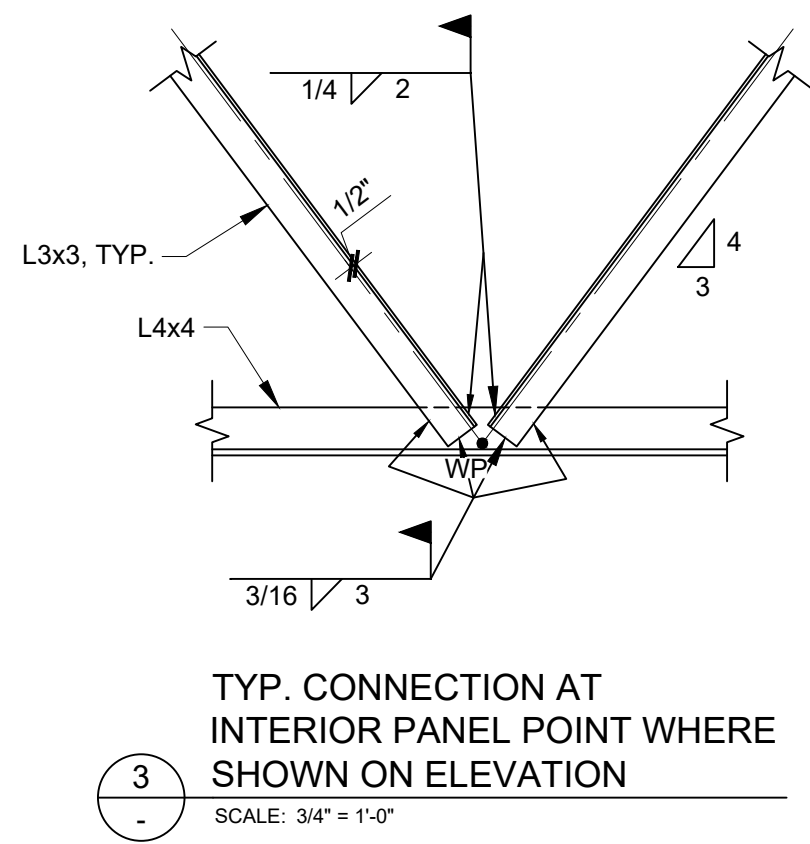
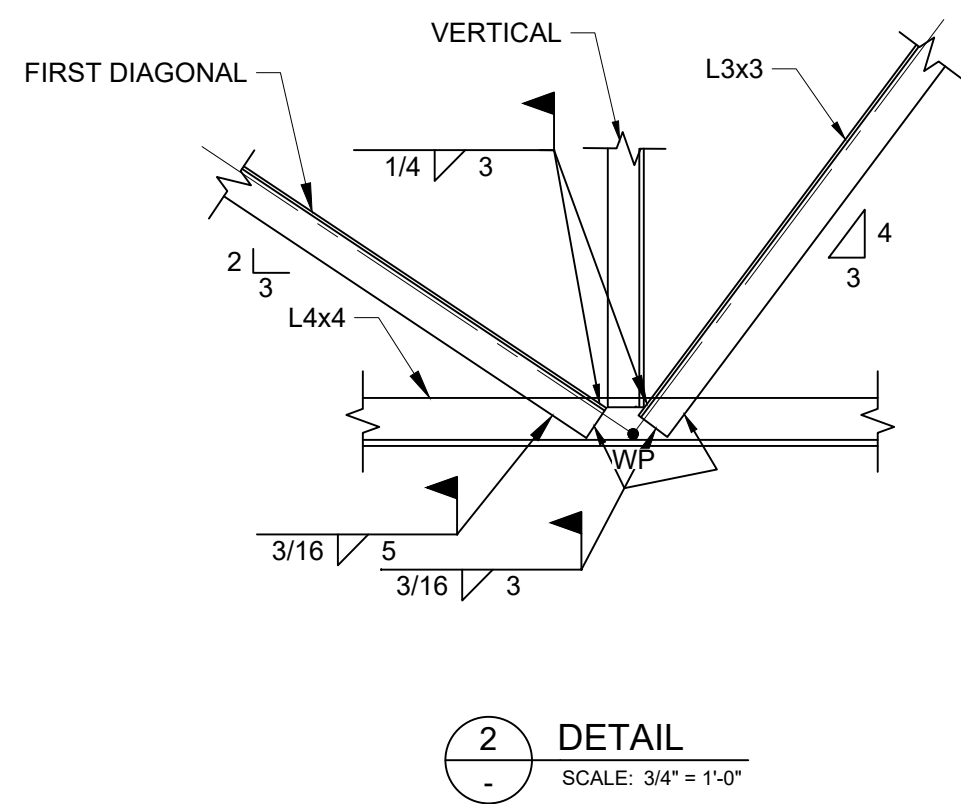
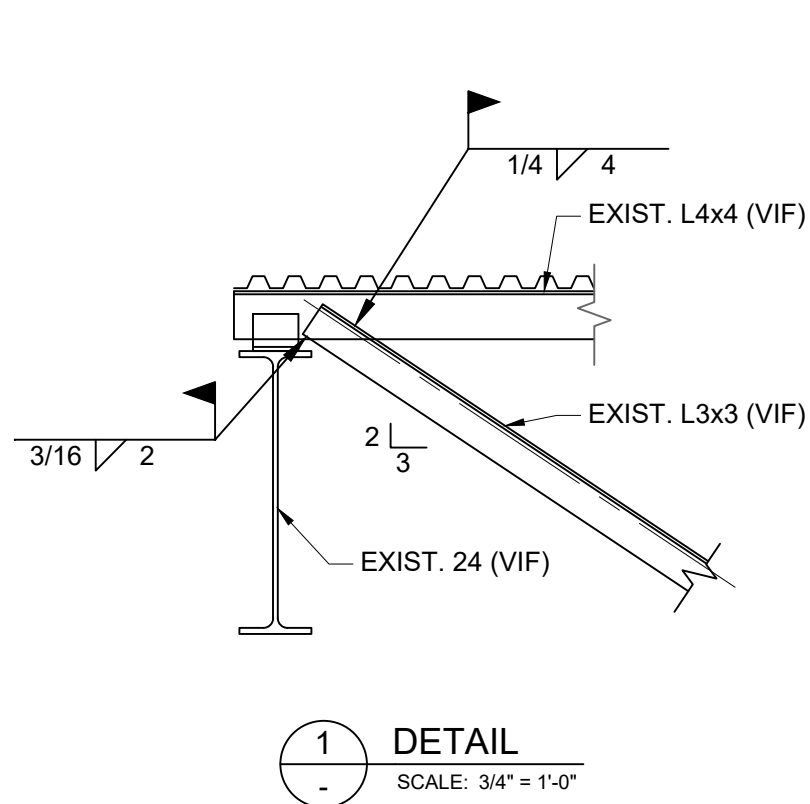
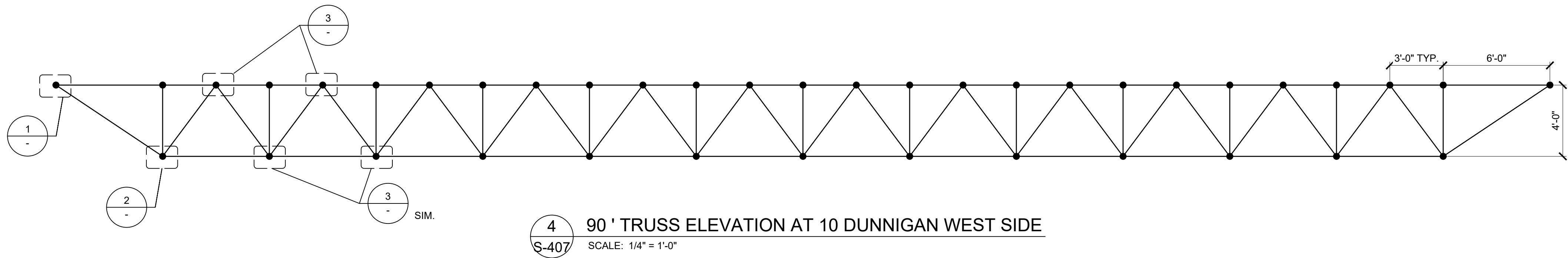
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SCALE :	AS SHOWN

DRAWING TITLE :

**20 DUNNIGAN DRIVE
DETAILS**

DWG NUMBER :
S-408

59 OF 60



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DRAWING TITLE :

20 DUNNIGAN DRIVE
DETAILS

DWG NUMBER :

S-409

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